



AIR VENTS / VACUUM RELIEF VALVES

Standard Kinetic Air/Vacuum Relief Valve -Mushroom Top

Air release occurs when air escape the system at startup and vacuum relief allows air to enter during shutdown. The air vent vacuum breakers are installed at the highest points in the drip field to keep soil from being sucked into the emitters due to back siphoning and back pressure. This is an absolute necessity with underground drip systems. They are also used for proper drainage of the supply and return manifolds. Use one on the high point of the supply manifold and one on the high point of the return manifold and any high points of the system.

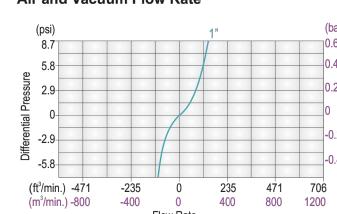
Features

Specification

Geoflow's kinetic air vacuum breakers have a twist off cap that is easy to take apart for cleaning. The large clear passageway allows lots of air to flow in and out easily. The elbow cap design is ideal for directing wastewater spray, directing spray downward. With the ball removed, these airvents can easily be used as a flush port. These can be used in freezing climates to capture warmer air that can be found below ground, in a sump during vacuum.

The Air Vacuum Breaker shall provide vacuum relief and noncontinuous air relief. Both the body and the ball shall be made of molded plastic. The ball shall be removable for easy cleaning. The air vacuum breaker shall be part number APVBK100M as supplied by Geoflow, Inc.

Air and Vacuum Flow Rate





Part No.	APVBK100M
Inlet	1"
Max Pressure	80 psi/185 ft.
Max Temp	140° F
Height	5.5"
Weight	1.2 oz.

Septic System Sizing

PACKED BED MEDIA WITH DRIP IRRIGATION ABSORPTION AREA

HYDRAULIC LOADING RATE = 0.70 GAL/DAY/SF MAX ABSORPTION DEPTH 8-10 INCHES LATERAL LENGTH = SEE LAYOUT SPACING FROM DRIP LINE TO DRIP LINE = 2 FT

DAILY WASTEWATER FLOW 17,750 GPD TOTAL SEE WASTEWATER DEMAND SIZING REPORT

17,750 GAL/DAY

17,750 GAL/DAY / 0.70 GAL/DAY/SF = 25,357 SF25,357 SF x .7 PACKED BED REDUCTION = 17,750 SF 17,750 SF/2 FT LINE SPACING = 8,875 L.F. LINE LENGTH8,875 LF/2 FT EMITTER SPACING = 4,438 EMITTERS

TANK SIZING TO PROVIDE 1.5 TIMES THE DAILY DESIGN FLOW 17,750 X 1.5 = 26,625 GALLONS

EFFLUENT TO TANK IS TO BE PUMPED. TANK SIZED TO BE INCREASED TO ALLOW ADDITIONAL TIME FOR SOLIDS TO SETTLE DUE TO THE EMULSIFICATION OF THE SEPTIC FLOWS FROM THE

6" MIN EARTH BACKFILL OVER PIPE

Detail-Absorption Field

SCALE: NONE

DRIPLINE TO FOLLOW EXISTING CONTOURS

3" SUPPLY/RETURN LINE 0.5" DRIPLINE (TYP.)

USE MINIMUM TANK SIZE OF 30,000 GALLONS

Absorption Field Pump Calculations

STATIC HEAD = 5.5'
RESIDUAL HEAD (DRIP LATERALS) = 20'

TDH = 25.5'FLOW = 150.9 GPMPROVIDE 2 ZONES PER FIELD TO REDUCE FLUSHING FLOW REQUIREMENT TO 75.46 GPM ACTUATE ZONES WITH A TWO-WAY MECHANICAL

DISTRIBUTION VALVE IN EACH FIELD

(OR APPROVED EQUIVALENT)

Buoyancy Calculations

EMITTER DISCHARGE RATE = 1.16 GPH/EMITTER

TOTAL DOSE FLOW = 5,148 GPH (85.80 GPM)

 $0.74 \times 88 = 65.1 \text{ GPM} \text{ FLUSHING FLOW}$

AIR VACUUM RELIEF VALVE SEE DETAIL THIS SHEET

(TYP. OF 5)

ABSORPTION FIELD 1 & 2
TRANSPORT PIPE (3") = 34 LF

DRIP IRRIGATION LINE (.5") = 100 LF

OF EMITTERS = 4,438 EMITTERS

SUPPLY PIPE (3") = 90 LF

EMITTER SPACING = 2 FT

FLUSH VELOCITY = 1 FT/S

DOSAGE VOLUME = 436 GAL

FLOW = 150.9 GPM

IT IS POSSIBLE GROUNDWATER WILL BE ENCOUNTERED DURING THE SEPTIC TANK AND PACKED BED MEDIA TREATMENT EXCAVATION. DEPENDING ON THE CONDITIONS DISCOVERED DURING EXCAVATION, ANCHORING MAY BE REQUIRED FOR THE PACKED BED MEDIA TREATMENT UNITS. CONTACT THE MANUFACTURER/SUPPLIER FOR ANCHORING DETAILS AND DESIGN. DEPENDING ON THE SEPTIC TANK SELECTED ANCHORING MAY ALSO BE REQUIRED, SUCH AS FOR NON-CONCRETE TANKS. CONTACT THE ENGINEER OF RECORD FOR ADDITIONAL ANCHORING INFORMATION IF NEEDED.

Large Underground Wastewater System Notes

1. SYSTEM INSTALLATION TO COMPLY WITH WEBER-MORGAN HEALTH DEPARTMENT AND STATE ADMINISTRATIVE

2. INSTALL SEDIMENTATION AND EROSION CONTROL MEASURES. TEMPORARY DRAINAGE SWALES/BERMS MAY BE INSTALLED TO PROTECT THE SITE DURING RAINFALL EVENTS.

IT IS BEST TO MOUND SEVERAL INCHES OF SOIL OVER THE FINISH GRADE TO ALLOW FOR SETTLING. THIS

ALSO ENSURES THAT RUNOFF WATER IS DIVERTED AWAY FROM THE SYSTEM. AFTER THE SYSTEM IS COVERED, THE SITE SHOULD BE SEEDED OR SODDED TO PREVENT EROSION.

SYSTEMS SHALL BE MONITORED PERIODICALLY EVERY 6 MONTHS.

REPAIRS MUST BE MADE AS SOON AS POSSIBLE AFTER A MALFUNCTION IS DISCOVERED. NO TREES, CONCRETE, STRUCTURES, LIVESTOCK, OR ACTIVITY OVER THE SYSTEM, NOR ANY REPLACEMENT

SURFACE WATER TO BE DIRECTED AWAY FROM THE SYSTEM. SITE SHALL BE CLEARED OF VEGETATION AND SCARIFIED WITH FURROWS RESULTING FROM SCARIFICATION

BEING PERPENDICULAR TO SLOPE OF SITE. CHECK FOR PROPER SOIL MOISTURE PRIOR TO CONSTRUCTION. SOIL IS TOO WET IF IT FORMS INTO A

11. CUT OFF ALL GRASS, BRUSH AND TREES JUST ABOVE GROUND SURFACE AND REMOVE, BUT DO NOT REMOVE LARGE TREE STUMPS. RAKE OFF DEAD VEGETATION IF OVER AN INCH THICK.

12. TILL THE AREA TO A DEPTH OF 6 INCHES. THE TILLED AREA SHOULD BE AT LEAST THE TOTAL LENGTH AND WIDTH OF THE SYSTEM.

13. TILL WITH A MOLDBOARD PLOW, CHISEL PLOW, OR CHISEL TEETH MOUNTED ON A BACKHOE BUCKET.

NORMAL TEETH ON A BACKHOE ARE NOT SATISFACTORY AND SHOULD NOT BE USED. A ROTOTILLER SHOULD NOT BE USED.

14. AVOID TRAFFIC ON TILLED AREA, ESPECIALLY BENEATH THE ABSORPTION AREA. 15. IF COMPACTION OR RUTS OCCUR DURING CONSTRUCTION, RE-TILL THE COMPACTED OR RUTTED AREA.

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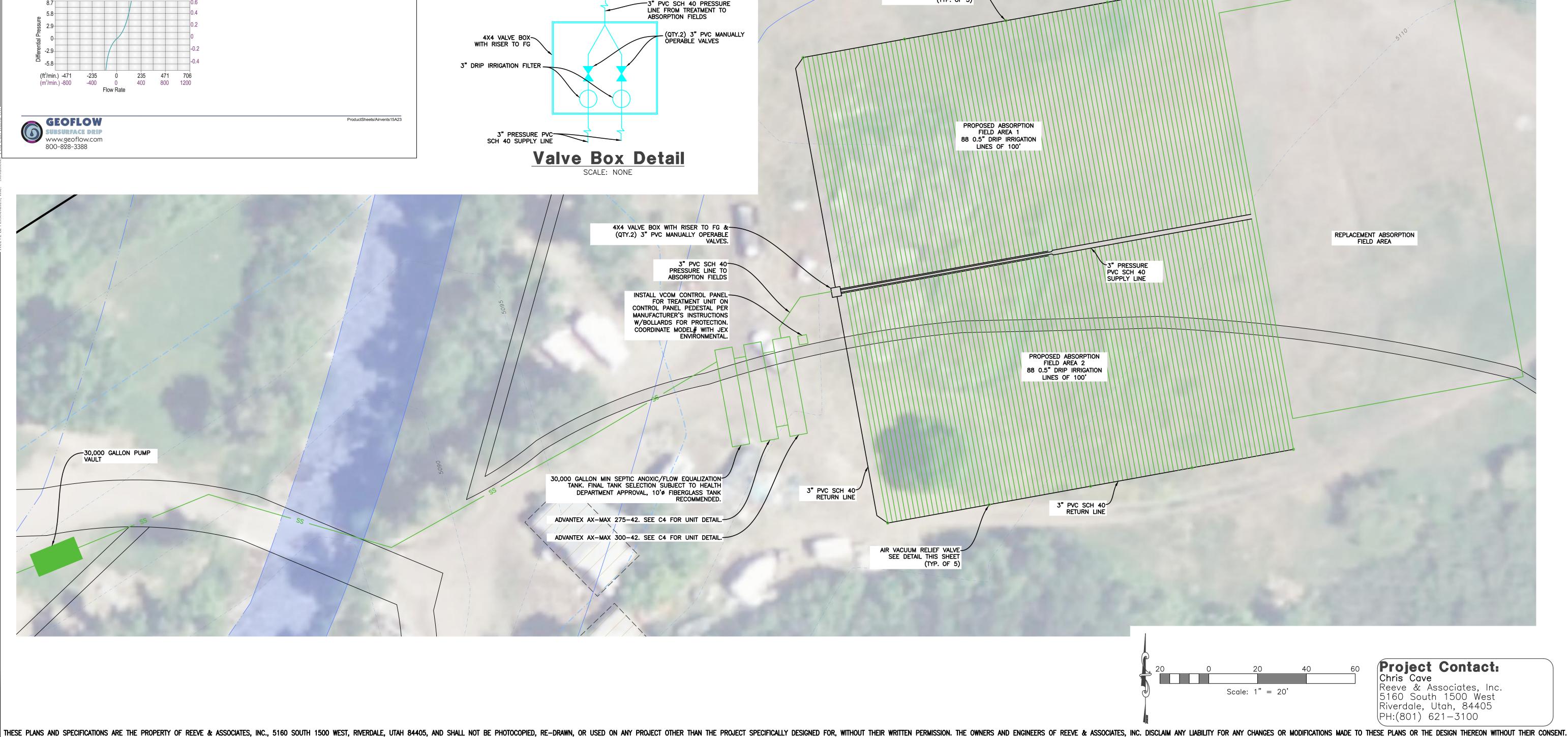
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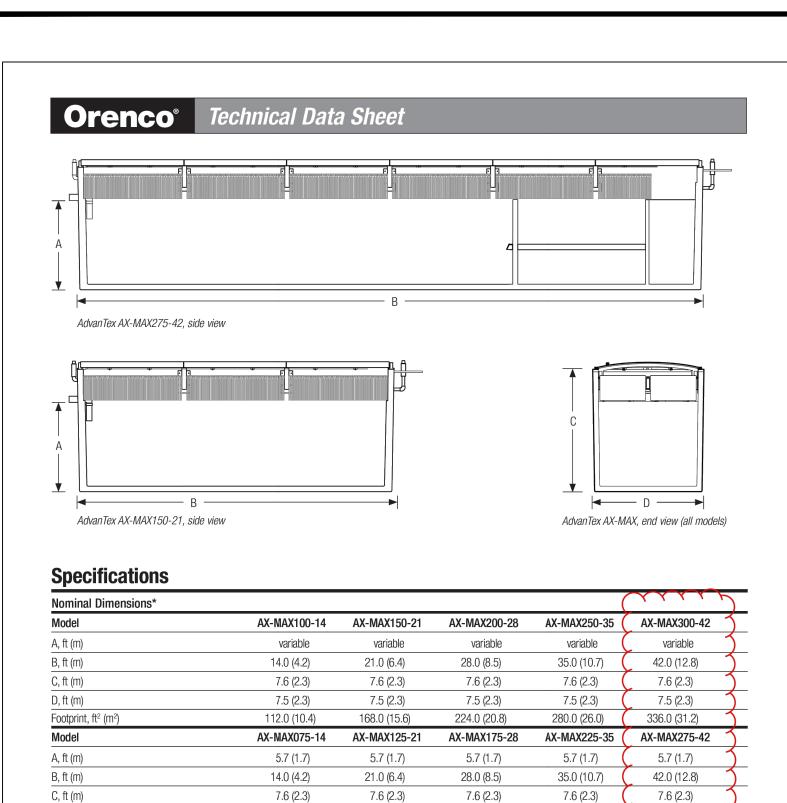
Project Info. Planner: Designer:

<u>Date: October 2023</u> Name:

Number: 8036-01

4 Total Sheets





7.5 (2.3)

168.0 (15.6)

7.5 (2.3)

224.0 (20.8)

Orenco Systems® Inc. , 814 Airway Ave., Sutherlin, OR 97479 USA • 800-348-9843 • 541-459-4449 • www.orenco.com

7.5 (2.3)

280.0 (26.0) (336.0 (31.2)

7.5 (2.3)

7.5 (2.3)

112.0 (10.4)

*See AdvanTex® AX-Max Treatment System drawings for exact dimensions and specific treatment configurations.

D, ft (m)

Footprint, ft² (m²)

NTD-ATX-MAX-1 Rev. 1.2, © 02/15 Page 2 of 2

	and is available as an Excel spreadsheet. It can be geoflow.com. To calculate the area required for your
1. the quantity of effluent to be disposed of (in gallons per day) and
2. the soil acceptance rate (i.e. gallons per day	per square foot).
Make a sketch of the dispersal area with contour	lines.
WORKSHEET 1 - DISPERSAL FIELD DESIGN FOR	SINGLE ZONE
Worksheet Dispersal Field	Formula
A. Quantity of effluent to be dispersed per day 17,750 gpd	
B. Soil type or hydraulic loading rate	Based on soil analysis
0.70 loading rate (gal/sq. ft./day)	Refer to State or Local regulations. If none, refer to Table 2 on page 10
C. Determine the total area required (17,750/.70)X.70 = 17,750 ft2 square ft	Divide gpd by loading rate. A/B
(Packed Bed Reduction)	
D. Choose the spacing between each WASTEFLOW line and each WASTEFLOW emitter	Standard spacing is 2 ft.
i) 2 ft. between WASTEFLOW lines	
ii) 2 ft. between WASTEFLOW emitters	

WORKSHEET:

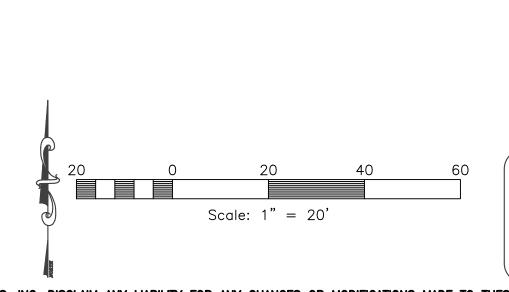
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u)_____ji. E. How many linear feet of dripline in the total area? 8,875 ft. (Area / 2) for 2ft. line spacing. C/2.0 or (Area / 1) for 1 ft. line spacing.. C/1.0 or (Area / 1.5) for 1.5ft line spacing. C/1.5F. Calculate the number of emitters (Linear ft. of dripline /2) for 2 ft emitter spacing. E/2 or <u>4,438</u> emitters (Linear ft. of dripline / 1) for 1 ft emitter spacing. E/1 or (Linear ft. of dripline /1.5) for 1.5 ft emitter spacing E/1.5G. Choose pressure compensating or Classic dripline See page 4 and Appendix 1 for details ____X ___ WASTEFLOW Classic dripline or ____ WASTEFLOW PC ½ gph dripline _____ WASTEFLOW PC 1 gph dripline GEOFLOW Inc. 800-828-3388/415-927-6000 www.geoflow.com

Sketch a layout of the WASTEFLOW lines in the dispersal plot to make sure that the maximum lateral length of each WASTEFLOW line is not exceeded. Count number of laterals for use in flushing velocity below.	See Maximum Length of Run table in Appendix 1.
H. Determine dripfield pressure	Standard pressure is 20 psi. WASTEFLOW Classic systems need between 15 and 45 psi (34.7 and 104 ft.) at the start of the dripfield. WASTEFLOW PC systems need between 10 and 45 psi (23.1 ft. to 104 ft.) at the start of the dripfield.
I. Determine feet of head required at dripfield 46.20 ft. of head	Multiply pressure by 2.31 to get head required. $H \times 2.31$
J. What is the flow rate per emitter?	See WASTEFLOW flow rates in Appendix 1.
K. Determine total dose flow for the area 5,148 gph 85.80 gpm	Number of emitters multiplied by the emitter flow rate at the design pressure. Gph = No of emitters (F) × gph per emitter (J) Gpm = gph/60
L. Count dripline laterals in the zone 88 laterals	1 lateral = connection form supply line to return line regardles of the number of loops
 M. Determine additional flow required to flush the zone 1 ft/s flush velocity gph 65.1 gpm 	Number of dripline laterals (L.) multiplied by flush velocity multiplier: Flush V elocity Multiplier 1/2 ft/sec. 0.37 1 ft/sec 0.74 2 ft/sec 1.48
N. Total Flow required to flush zone PROVIDE 2 ZONES PER FIELD, 150.9/2 = 75.46 GPM FLSUH PER ZONE	M+K in gpm

PSI = Ft. of head divided by 2.31 DAILY DOSE: DRIPLINE LATERAL VOLUME: 0.99 GAL/LATERAL DOSAGE: (0.99 GAL/LATERAL)*(88 LATERALS)*(5) = 436 GAL

GEOFLOW Inc. 800-828-3388/415-927-6000 <u>www.geoflow.com</u>



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4 Total Sheets