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05/12/2023

THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT
COVER SHEET

Mark	Description	Date	By
A	Issued For Approval	04/18/2023	AML
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DATE 05/10/2023

PROJ. NO. 23002

DRAWING NO.

SS001

GENERAL NOTES:

- SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
- ALL WORK TO BE IN STRICT ACCORDANCE WITH THE IBC 2018, AISC, AND LOCAL ORDINANCES.
- ALL DIMENSIONS AND CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND ERECTION.
- SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
- SEE ARCHITECTURAL SHEETS FOR DECK BEARING ELEVATIONS, STRUCTURAL DETAILER SHALL DETERMINE ALL BEARING PLATE ELEVATIONS FROM ARCHITECTURAL DECK ELEVATIONS.
- SEE ARCHITECTURAL SHEETS FOR ADDITIONAL DIMENSIONS.
- SEE ARCHITECTURAL SHEETS FOR ACCESS HATCHES, DRAFT STOPS, ETC.
- SUBMIT SHOP DRAWINGS OF ALL STRUCTURAL STEEL, STEEL JOISTS, STEEL DECKING AND MISCELLANEOUS STEEL UPON REQUEST.
- SEE FRAMING PLANS FOR ADDITIONAL NOTES AND REQUIREMENTS.
- PROJECT NORTH AND NORTH ARE NOT ALWAYS THE SAME DIRECTION. PROJECT NORTH SHOWN IS FOR ORIENTATION OF STRUCTURAL STEEL ONLY.



DESIGN CRITERIA:

- WIND LOAD
 - BASIC WIND SPEED (3-SECOND GUST): 105 mph
 - EXPOSURE CATEGORY: C
 - IMPORTANCE FACTOR: 1.0
 - BUILDING ENCLOSURE CLASSIFICATION: ENCLOSED
 - INTERNAL PRESSURE COEFFICIENT: 0.18
 - COMPONENTS AND CLADDING: VARIES
- SNOW LOAD
 - GROUND SNOW LOAD (Pg): 57 psf
 - SLOPE FACTOR (Cs): 0.9
 - IMPORTANCE FACTOR: 0.8
 - THERMAL FACTOR (Ct): 1.0
 - FLAT ROOF SNOW LOAD (Pf): 27 psf
- SEISMIC LOAD
 - OCCUPANCY CATEGORY: I
 - IMPORTANCE FACTOR: 1.0
 - SITE CLASS: D DEFAULT
 - SEISMIC DESIGN CATEGORY: D
 - MAPPED SPECTRAL RESPONSE ACCELERATIONS
 - SHORT PERIOD (Ss): 0.917g
 - 1-SEC PERIOD (S1): 0.325g
 - DESIGN SPECTRAL RESPONSE COEFFICIENTS
 - SHORT PERIOD (Sds): 0.734g
 - 1-SEC PERIOD (Sd1): 0.325g
 - BASIC SEISMIC FORCE RESISTING SYSTEM AND RESPONSE MODIFICATION FACTOR
 - ORDINARY STEEL CONCENTRICALLY BRACED FRAMES: (E-W) R=3.25
 - N-S DIRECTION: ORDINARY STEEL MOMENT FRAMES: R=3.5
 - SEISMIC RESPONSE COEFFICIENT E-W (Cs): 0.226
 - SEISMIC RESPONSE COEFFICIENT N-S (Cs): 0.21
 - DESIGN BASE SHEAR (Vs = Cs x W): 12 KIPS
 - ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
- LIVE LOAD
 - ROOF LIVE LOAD (REDUCIBLE): 20 psf
 - SLABS ON GRADE: 125 psf
- SOILS
 - SITE CLASS: D DEFAULT
 - ALLOWABLE BEARING CAPACITY: 1500 psf
- RAIN LOAD
 - 15-MIN DURATION/100-YEAR RETURN, i: 4.8 in/hr
 - 60-MIN DURATION/100-YEAR RETURN, i: 2 in/hr
 - SUM OF HYDRAULIC AND STATIC HEAD, R: R<27 psf
- COLLATERAL LOAD:
 - ROOF COLLATERAL LOAD: 2 psf

ANCHOR ROD/BOLT NOTES:

- FOR GROUT BEARING, GENERAL CONTRACTOR IS TO PROVIDE AN ELEVATION NUT (PAINTED RED) AT EACH SET OF ANCHOR ROD/BOLTS. NUT IS TO BE SET AT THE PROPER ELEVATION (TOP OF NUT=BOTTOM OF BASE PLATE).
- GENERAL CONTRACTOR IS TO ASSURE LOCATION OF ALL ANCHOR ROD/BOLT PATTERNS TO BE WITHIN .375" AS CALLED FOR ON PLANS.
- THE CONTRACTOR/ERECTOR IS TO BRING ALL OTHER NUTS TO SAME ELEVATION AS THE GENERAL CONTRACTOR'S ELEVATION NUT.
- WHEN BASE PLATE BEARS ON THE FINISHED CONCRETE, GENERAL CONTRACTOR IS TO ASSURE THAT ANCHOR RODS/BOLTS AND CONCRETE BEARING IS WITHIN .375" FOR LOCATION AND ELEVATION.

WELDED NOTES:

- ALL WELDING SHALL BE EXECUTED BY A CERTIFIED WELDER.
- E-70XX ELECTRODES SHALL BE USED FOR ALL STRUCTURAL STEEL CONNECTIONS UNLESS OTHERWISE NOTED. E60XX ELECTRODES MAY BE USED FOR WELDING ROOF DECK AND FLOOR DECK.
- NO WELDING IS PERMITTED ON THE ANCHOR BOLTS WITHOUT PRIOR APPROVAL OF THE ENGINEER.

GENERAL STRUCTURAL NOTES:

- THE STRUCTURAL NOTES ARE THE PROJECT SPECIFICATIONS WHICH ARE PART OF THE CONSTRUCTION DOCUMENTS. SPECIFIC NOTES AND DETAILS IN THE DRAWINGS SHALL GOVERN OVER THE STRUCTURAL NOTES AND TYPICAL DETAILS. TYPICAL DETAILS SHALL APPLY WHERE SPECIFIC DETAILS ARE NOT SHOWN.
- THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE. IF ACTUAL CONDITIONS DIFFER FROM THOSE SHOWN ON CONTRACT DOCUMENTS, CONTRACTOR SHALL NOTIFY ENGINEER PRIOR TO FABRICATION OR CONSTRUCTION OF ANY AFFECTED ELEMENTS.
- OMISSIONS OR CONFLICTS FOUND IN THE CONSTRUCTION DOCUMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY WORK INVOLVED.
- DO NOT SCALE STRUCTURAL DRAWINGS. REFER TO ARCHITECT'S DRAWINGS FOR ALL DIMENSIONS.
- REVIEW OF CONSTRUCTION SUBMITTALS / SHOP DRAWINGS BY THE ENGINEER OF RECORD IS FOR GENERAL COMPLIANCE ONLY AND IS NOT INTENDED AS APPROVAL. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL PERTINENT INFORMATION AND ENSURING THAT DESIGN REQUIREMENTS ARE MET.
- THE CONTRACTOR SHALL VERIFY AND COORDINATE LOCATIONS AND SIZES OF ALL MECHANICAL OR OTHER EQUIPMENT BEFORE FABRICATING OR ERECTING EFFECTED STRUCTURAL ELEMENTS. LOCATIONS AND SIZES THAT DIFFER FROM THOSE INDICATED IN THE CONSTRUCTION DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT.
- THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST TO THE ENGINEER FOR APPROVAL BEFORE PROCEEDING WITH ANY CHANGES, MODIFICATIONS OR SUBSTITUTIONS AFFECTING ANY STRUCTURAL ELEMENTS.
- DURING AND AFTER CONSTRUCTION, THE DESIGN LOADS AS INDICATED IN THESE DOCUMENTS SHALL NOT BE EXCEEDED.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ADEQUATE TEMPORARY SHORING AND BRACING FOR ALL STRUCTURAL COMPONENTS UNTIL THE ENTIRE STRUCTURAL SYSTEM IS COMPLETED. DESIGN OF SUCH SHORING AND BRACING IS BY OTHERS.
- STRUCTURAL OBSERVATIONS SHALL BE CONDUCTED AS NEEDED BY A REPRESENTATIVE OF THE ENGINEER OF RECORD AND WILL CONSIST OF OBSERVING THE CONSTRUCTION OF CRITICAL STRUCTURAL ELEMENTS. THESE STRUCTURAL OBSERVATIONS SHALL NOT BE CONSTRUED AS SPECIAL INSPECTIONS OR APPROVAL OF CONSTRUCTION.
- ENGINEER SHALL NOT BE RESPONSIBLE FOR ACTIVITIES UNDER CONTROL OF THE CONTRACTOR SUCH AS CONSTRUCTION SITE SAFETY, MEANS, METHODS, AND SEQUENCING OF CONSTRUCTION.

STRUCTURAL STEEL:

- FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL OF STEEL CONSTRUCTION, FOURTEENTH EDITION.
- ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING (UNLESS NOTED OTHERWISE):
 - WIDE FLANGE SHAPES - ASTM A992 (Fy=50 KSI)
 - RECTANGULAR HSS - ASTM A500, GRADE B (Fy=46 KSI)
 - ROUND HSS - ASTM A500, GRADE B (Fy=42 KSI)
 - PIPE MEMBERS - ASTM A53, TYPE E OR S, GRADE B (Fy=35 KSI)
 - CHANNELS AND ANGLES - ASTM A36 (Fy=36 KSI)
 - PLATE - ASTM A36 (STANDARD)
 - PLATE - ASTM 572-50 (Fy=50 KSI)
 - ALL OTHER MEMBERS - ASTM A36 (Fy=36 KSI)
 - WELDS - E70XX ELECTRODES
 - ANCHOR BOLTS - ASTM F1554 GRADE 36 OR 55 OR 105; SEE PLANS FOR REQUIREMENT
 - HEADED STUDS - ASTM A307 (Fu=60 KSI)
- ALL BOLTS FOR STEEL TO STEEL CONNECTIONS TO BE 3/4" DIAMETER MINIMUM A325 HIGH STRENGTH BOLTS UNLESS NOTED OTHERWISE.
- ALL WELDS TO BE MADE BY A CERTIFIED WELDER.
- ALL WELDS AND BOLTING TO MEET APPROVAL OF SPECIAL INSPECTOR AS REQUIRED BY THE BUILDING OFFICIAL.
- ALL STEEL SHALL BE PROPERLY PRIMED EXCEPT AREAS THAT REQUIRE WELDING.
- ALL STEEL BEAMS USED AS GIRDERS SHALL HAVE WEB STIFFENERS ON EACH SIDE OF WEB AT BEARING ENDS AND AT CONCENTRATED LOADS AS REQUIRED BY THE ENGINEER.
- ANY MODIFICATION OF STRUCTURAL MEMBERS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS IS NOT PERMITTED WITHOUT PRIOR APPROVAL.
- ANY CONNECTIONS NOT DETAILED ON STRUCTURAL PLANS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO FABRICATION AND INSTALLATION.

STRUCTURAL CONNECTIONS:

- THE CONTRACTOR IS ULTIMATELY RESPONSIBLE TO PROVIDE ADEQUATE STRUCTURAL CONNECTIONS. CONNECTIONS MUST CARRY THE BEARING CAPACITY OF THE MEMBER AND ANY UPLIFT OR SEISMIC FORCES GENERATED IN THE MEMBER. SPECIAL CONSIDERATION SHALL BE GIVEN TO PREVENT CRUSHING OF THE MEMBER AT BEARING.
- THE CONTRACTOR SHALL STRICTLY ADHERE TO THE CONNECTION DETAILS SPECIFIED ON THE PLANS OR WITHIN THE CONSTRUCTION DOCUMENTS. PRIOR APPROVAL IS REQUIRED FOR ANY DEVIATION FROM THE CONSTRUCTION DOCUMENTS.
- SUBSTITUTION OF CONNECTIONS OTHER THAN THOSE SPECIFIED ON THE PLANS REQUIRES PRIOR APPROVAL. THE ENGINEER IS NOT RESPONSIBLE FOR CONNECTIONS NOT APPROVED PRIOR TO CONSTRUCTION OR INSTALLATION.
- IF CONNECTION DETAILS, APPROVED BY THE ENGINEER, HAVE NOT BEEN PROVIDED IN THE CONSTRUCTION DOCUMENTS, IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO SPECIFY AND PROVIDE ALL INFORMATION TO THE ENGINEER FOR ADDITIONAL ASSISTANCE.

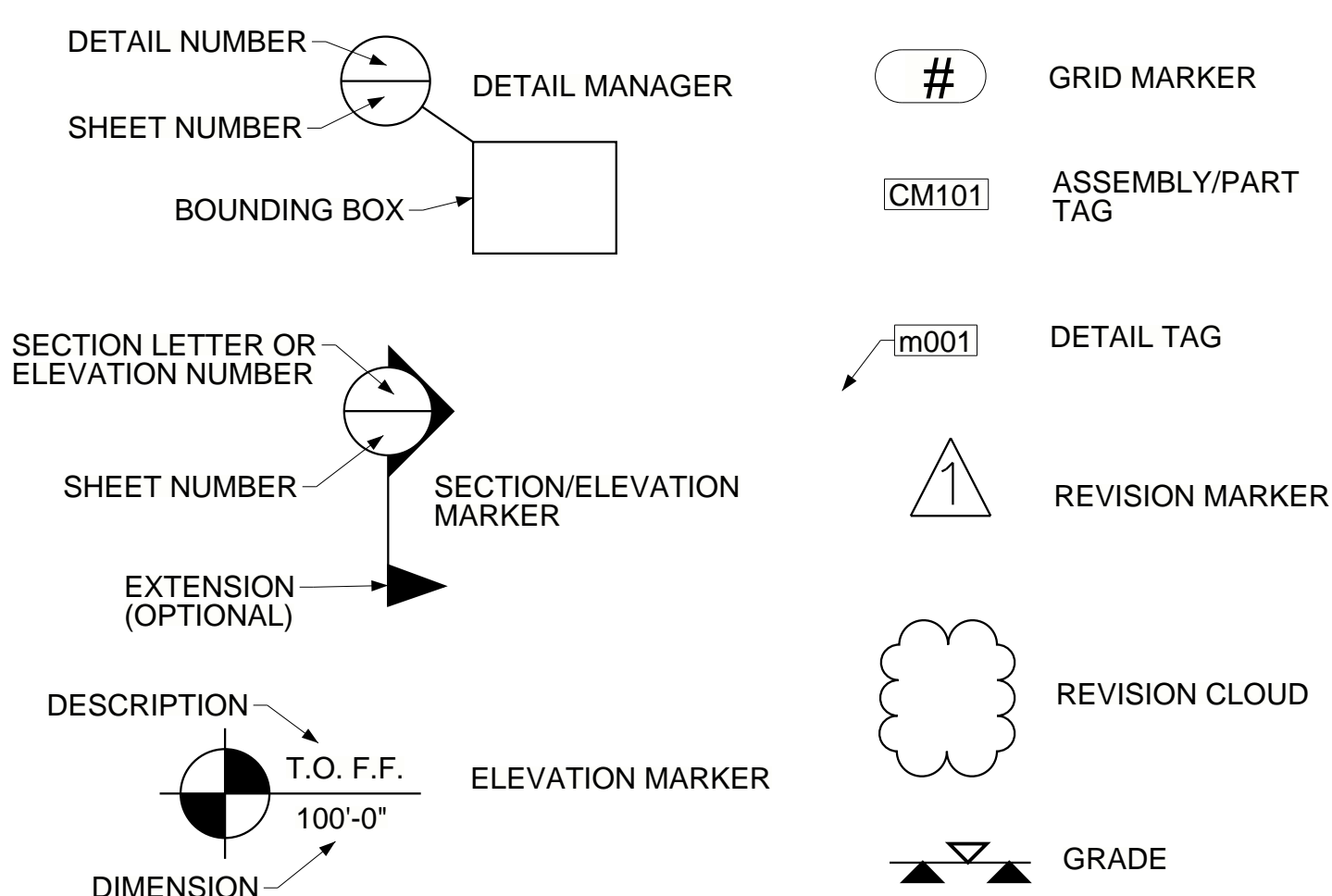
BOLTED CONNECTION NOTES:

- ALL HIGH STRENGTH CONNECTIONS ARE TO BE MADE USING A325 BOLTS. THREADS NEED TO NOT BE EXCLUDED FROM THE SHEAR PLANE UNLESS NOTED OTHERWISE BY DESIGNATION OF BOLT.
- BOLTS ARE TO BE INSTALLED AND TIGHTENED TO A SNUG TIGHT CONDITION. THIS CONDITION IS DEFINED AS THE TIGHTNESS THAT EXISTS WHEN ALL SURFACES ARE IN FIRM CONTACT AND IS USUALLY ATTAINED BY A FEW HITS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A WORKER USING AN ORDINARY SPUD WRENCH.
- HARDENED WASHERS TO BE USED ON OVERSIZED HOLES AND SLOTS.
- ALL BOLTS AND NUTS TO BE STORED IN A CLEAN DRY PLACE.

ABBREVIATIONS

L	ANGLE	GA	GAUGE	R	RADIUS
AB	ANCHOR BOLTS	GALV	GALVANIZED	RAF	RAFTER
ALT	ALTERNATE	GP	GUSSET PLATE	REF	REFERENCE
ARCH	ARCHITECT	GR	GRADE	REQ'D	REQUIRED
		GRD	GIRDER	RO	ROUGH OPENING
BM	BEAM	HB	HORIZONTAL BRIDGING	SL	STEEL (GIRT) LINE
BP	BASE PLATE	HED	HEADER	SCH	SCHEDULE
BRG	BEARING	HT	HEIGHT	SFRS	SEISMIC FORCE RESISTING
BLDG	BUILDING	HORZ	HORIZONTAL	SYSTEM	
BOF	BOTTOM OF FOOTING	HSA	HEADED STUD ANCHORS	SHT	SHEET
BTM	BOTTOM	ID	INSIDE DIAMETER	SIM	SIMILAR
BS	BOTH SIDES	IN	INCH	SL	STEEL LINE
		INSUL	INSULATION	SQ	SQUARE
CF	CONTINUOUS FOOTING	INT	INTERIOR	SSR	STANDING SEAM ROOF
CL	CENTERLINE	JB	JACK BEAM	STAG	STAGGERED
CMU	CONCRETE MASONRY UNIT	JBS	JOIST BEARING SEAT	STD	STANDARD
COL	COLUMN	JST	JOIST	STP	STIFFENER PLATE
CONC	CONCRETE	K	KIPS	STL	STEEL
CONC	CONSTRUCTION	KLF	KIPS PER LINEAR FOOT	TAB	TOP AND BOTTOM
CONT	CONTINUOUS	KSF	KIPS PER SQUARE FOOT	TBC	TENSION CONTROL BOLT
CNTR	CENTER	LBS	POUNDS	THDS	THREADS
		MAX	MAXIMUM	TOC	TOP OF CONCRETE
DB	DECK BEARING	MECH	MECHANICAL	TOD	TOP OF DECK
DBA	DEFORMED BAR ANCHOR	MF	MODULAR FRAME	TOF	TOP OF FOUNDATION
DET	DETAIL	MIN	MINIMUM	TOFF	TOP OF FINISHED FLOOR
		MISC	MISCELLANEOUS	TOS	TOP OF STEEL
DIM	DIMENSION	MTP	MOMENT TRANSFER PLATE	TOW	TOP OF WALL
DS	DOWN SPOUT	NIC	NOT IN CONTRACT	TS	TUBE STEEL
DWG	DRAWING	NS	NEAR SIDE	TYP	TYPICAL
		NTS	NOT TO SCALE	UB	UPLIFT BRACE
EHS	EXTRA HIGH STRENGTH	OAL	OVERALL LENGTH	UNO	UNLESS NOTED OTHERWISE
ELEV	ELEVATION	OC	ON CENTER		
EOD	EDGE OF DECK	OD	OUTSIDE DIAMETER		
EQUIP	EQUIPMENT	OHD	OVERHEAD DOOR		
EQ	EQUAL	OPNG	OPENING		
ES	EAVE STRUT	OPP	OPPOSITE		
EXP	EXPANSION	OW	OPEN WEB		
EXST	EXISTING	PL	PLATE		
EXT	EXTERIOR	PLF	POUNDS PER LINEAR FOOT		
		PSF	POUNDS PER SQUARE FOOT		
		PSI	POUNDS PER SQUARE INCH		
		PT	POINT		
		PURL	PURLIN		
FCA	FLANGE BRACE				
FCP	FRICTION CLIP ANGLE				
FDTN	FRICTION CLIP PLATE				
F	FOUNDATION				
FF	FINISHED FLOOR				
FL	FLOOR				
FO	FRAMED OPENING				
FS	FAR SIDE				
FT	FEET OF FOOT				
FW	FOUNDATION WALL				

SYMBOL LEGEND



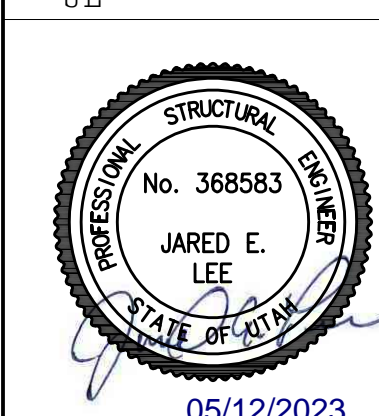
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THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EBDEN UT
GENERAL NOTES-SYMBOLS-ABBREVIATIONS

By	AML	AML			
Date	04/18/2023	05/10/2023			

Description	Issued For Approval	Issued For Permit			
	A	B			
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PROJ. NO. 23002
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OPEN WEB STEEL JOISTS AND GIRDERS

- CARE SHALL BE EXERCISED AT ALL TIMES TO AVOID DAMAGE THROUGH CARELESS HANDLING DURING UNLOADING, STORING AND ERECTING. DROPPING OF JOIST GIRDERS IS NOT PERMITTED.
- WHERE JOIST GIRDERS ARE UTILIZED AT COLUMNS, THE JOIST GIRDER SHALL BE FIELD-BOLTED AT THE COLUMN.
- BEFORE HOISTING CABLES ARE RELEASED AND BEFORE AN EMPLOYEE IS ALLOWED ON JOIST GIRDER OR JOIST, THE FOLLOWING CONDITIONS SHALL BE MET.
 - THE SEAT AT EACH END OF THE JOIST GIRDER OR JOIST IS ATTACHED IN ACCORDANCE WITH NOTE 4 OR 5, RESPECTFULLY.
 - WHERE STABILIZER PLATES ARE REQUIRED THE JOIST GIRDER BOTTOM CORD SHALL ENGAGE THE STABILIZER PLATE.
- GIRDER BEARING SEAT ATTACHMENTS:
 - MASONRY AND CONCRETE-ENDS OF JOIST GIRDERS RESTING ON STEEL BEARING PLATES ON MASONRY OR STRUCTURAL CONCRETE SHALL BE ATTACHED THERETO WITH A MINIMUM OF TWO 1/4 INCH FILLET WELDS 2 INCHES LONG, OR WITH TWO 3/4 INCH ASTM A307 BOLTS, OR EQUIVALENT.
 - STEEL-ENDS OF JOIST GIRDERS RESTING ON STEEL SUPPORTS SHALL BE ATTACHED THERETO WITH A MINIMUM OF TWO 1/8 INCH FILLET WELDS 2 INCHES LONG, OR WITH TWO 3/4 INCH ASTM A307 BOLTS, OR EQUIVALENT.
- JOIST BRIDGING SEAT ATTACHMENTS:
 - K SERIES JOISTS-
 - MASONRY AND CONCRETE-ENDS OF K-SERIES JOISTS RESTING ON STEEL BEARING PLATES ON MASONRY OR STRUCTURAL CONCRETE SHALL BE ATTACHED THERETO WITH A MINIMUM OF TWO 1/8 INCH FILLET WELDS 2 INCHES LONG, OR WITH TWO 1/2 INCH ASTM A307 BOLTS, OR EQUIVALENT AND SHALL BEAR A MINIMUM OF 2 1/2 INCHES.
 - STEEL-ENDS OF K-SERIES JOISTS RESTING ON STEEL SUPPORTS SHALL BE ATTACHED THERETO WITH A MINIMUM OF TWO 1/8 INCH FILLET WELDS 2 INCHES LONG, OR WITH TWO 1/2 INCH ASTM A307 BOLTS, OR EQUIVALENT AND SHALL BEAR A MINIMUM OF 2 1/2 INCHES.
 - LH AND DLH SERIES JOISTS-
 - MASONRY AND CONCRETE-ENDS OF LH AND DLH SERIES JOISTS RESTING ON STEEL BEARING PLATES ON MASONRY OR STRUCTURAL CONCRETE SHALL BE ATTACHED THERETO WITH A MINIMUM OF TWO 1/4 INCH FILLET WELDS 2 INCHES LONG, OR WITH TWO 1/2 INCH ASTM A307 BOLTS, OR EQUIVALENT AND SHALL BEAR A MINIMUM OF 4 INCHES.
 - STEEL-

JOIST SECTION NUMBER*	FILLET WELD	BEARING SEAT BOLTS FOR ERECTION	MINIMUM BEARING LENGTH
02 TO 06 INCL.	2-3/16" x 2"	2-3/4" A307	2 1/2"
07 TO 17 INCL.	2-1/4" x 2"	2-3/4" A307	4"
18 TO 25 INCL.	2-1/4" x 4"	2-3/4" A325	6"

* LAST TWO DIGITS OF JOIST DESIGNATION

- JOIST GIRDERS AND JOISTS SHALL NOT BE USED AS ANCHORAGE POINTS FOR A FALL ARREST SYSTEM UNLESS WRITTEN DIRECTION TO DO SO IS OBTAINED FROM A "QUALIFIED PERSON." SEE OSHA 29 CFR PART 1926 SAFETY STANDARDS FOR STEEL ERECTION.

STEEL DECK

- STEEL DECK SHALL BE OF THE PROFILE DEPTH AND THICKNESS AS INDICATED ON THE DRAWINGS.
- STEEL DECK SHALL HAVE A MINIMUM END BEARING OF 2 INCHES. END JOINTS SHALL BE LAPPED 2 INCHES MINIMUM.
- COMPOSITE AND NON-COMPOSITE STEEL DECKS MAY BE EITHER LAPPED OR BUTTED AT CONTRACTOR'S OPTION.
- STEEL DECK SHALL BE TRIPLE SPAN CONTINUOUS WHERE POSSIBLE. DO NOT LOCATE SINGLE SPANS AT EDGES OR CORNERS.
- WELDED ATTACHMENT OF STEEL DECK UNITS TO THE SUPPORTING MEMBERS SHALL CONFORM TO AWS D1.3. WELDING OF STEEL DECK SHALL BE PERFORMED BY CERTIFIED LIGHT GAGE STEEL WELDERS.
- ARC SPOT OR ARC SEAM (PUDDLE) WELDS SHALL HAVE AN EFFECTIVE FUSION AREA TO SUPPORTING MEMBERS, EQUIVALENT TO AT LEAST 3/8 INCH BY 1 INCH LONG OR 1/2 INCH DIAMETER AND IN NO CASE ANY WELD SPACING EXCEED 3 FEET.

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05/12/2023

THE GOOSE

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SS003

SPECIAL INSPECTION SCHEDULE

AREAS REQUIRING SPECIAL INSPECTION:	FREQUENCY		COMMENTS
	CONT.	PERIODIC	
FABRICATORS (IBC 1704.2.5)			
	X		IF FABRICATOR IS APPROVED, ON-SITE INSPECTION IS NOT REQUIRED BUT A CERTIFICATION OF COMPLETION MUST BE PROVIDED TO THE B.O. (IBC 1704.2.5.2)
SOILS (IBC 1705.6)			
VERIFY ADEQUATE MATERIALS BELOW FOOTINGS		X	PRIOR TO PLACEMENT OF CONCRETE
EXCAVATION EXTEND TO PROPER DEPTH AND MATERIALS		X	PRIOR TO PLACEMENT OF COMPACTED FILL OR CONCRETE
CLASSIFICATION AND TESTING OF FILL MATERIALS		X	CHECK CLASSIFICATION AND GRADATIONS AT EACH LIFT, BUT NOT LESS THAN ONCE FOR EACH 10,000 FT OF SURFACE AREA
VERIFY PROPER FILL MATERIALS, LIFT THICKNESSES AND IN-PLACE DENSITIES	X		PRIOR TO PLACEMENT OF CONCRETE
VERIFY PROPERLY PREPARED SITE AND SUBGRADE		X	PRIOR TO PLACEMENT OF CONCRETE
CONCRETE CONSTRUCTION (IBC 1705.3)			
REINFORCING STEEL PLACEMENT		X	VERIFY SIZE, CLEARANCES, SPLICES, AND PROPER TIES
EMBEDDED BOLTS OR PLATES	X		
VERIFY REQUIRED DESIGN MIX		X	VERIFY MIX DESIGN MEETS STRENGTH AND EXPOSURE REQUIREMENTS LISTED ON APPROVED PLANS
CONCRETE PLACEMENT/SAMPLING	X		INCLUDES SAMPLING FOR AIR, SLUMP, STRENGTH AND TEMPERATURE TECHNIQUES
INSPECT FORMWORK		X	VERIFY SHAPE, LOCATION AND MEMBER DIMENSIONS
POST-INSTALLED ANCHORS	X		IN ACCORDANCE WITH APPROVED ICC-ES REPORT, PERIODIC INSPECTIONS ALLOWED IF STATED IN EX' REPORT
COLD-FORMED STEEL CONSTRUCTION (IBC 1705.11.3)			
COMPONENTS OF WIND-AND SEISMIC-FORCE RESISTING SYSTEMS		X	VERIFY PROPER SCREW ATTACHMENT, BOLTING AND ANCHORING OF SHEAR WALLS, BRACES AND HOLD-DOWNS HAVING A FASTENER SPACING LESS THAN OR EQUAL TO 4" O.C.
OTHER THAN STRUCTURAL CONSTRUCTION (IBC 1705.2.2)			
<i>STEEL ROOF & FLOOR DECK:</i>			
MATERIAL VERIFICATION OF STEEL DECK		X	IDENTIFICATION MARKINGS PER APPLICABLE ASTM STANDARD
ROOF AND DECK WELDS		X	VERIFY THAT THE WELDS CONFORM TO AWS D1.3
<i>WELDING OF REINFORCING STEEL:</i>			
VERIFICATION OF WELDABILITY (EXCEPT A706 BAR)		X	VERIFY MATERIAL IS ABLE TO CONFORM TO AWS D1.4
STRUCTURAL STEEL CONSTRUCTION (IBC 1705.2, 1705.11, 1705.12)			
<i>PRIOR TO WELDING (TABLE N5.4-1, AISC 360-10):</i>			
VERIFY WELDING PROCEDURES	X		
MATERIAL IDENTIFICATION		X	VERIFY TYPE AND GRADE OF MATERIAL
WELDER IDENTIFICATION		X	VERIFY THERE IS A SYSTEM IN PLACE TO IDENTIFY THE WELDER WHO HAS WELDED A JOINT OR MEMBER
FIT-UP GROOVE WELDS		X	VERIFY JOINT PREPARATION, DIMENSIONS, CLEANLINESS, TACKING AND BACKING
ACCESS HOLES		X	VERIFY CONFIGURATION AND FINISH
FIT-UP WELDS		X	VERIFY ALIGNMENT, GAPS AT ROT, CLEANLINESS OF STEEL SURFACES, TACK WELD QUALITY AND LOCATION
<i>DURING WELDING (TABLE N5.4-2, AISC 360-10):</i>			
USE OF QUALIFIED INSPECTORS		X	VERIFY THAT WELDERS ARE APPROPRIATELY QUALIFIED
CONTROL AND HANDLING OF WELDING CONSUMABLES		X	VERIFY PACKAGING AND EXPOSURE CONTROL
CRACKED TACK WELDS		X	VERIFY WELDING IS NOT OVER CRACKED TACK WELD
ENVIRONMENTAL CONDITIONS		X	VERIFY WIND SPEED IS WITHIN LIMITS AS WELL AS PRECIPITATION AND TEMPERATURE
WPS FOLLOWED		X	VERIFY ITEMS SUCH AS WELDING EQUIPMENT SETTINGS, TRAVEL SPEED, WELDING MATERIALS, SHIELDING GAS, TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED, AND PROPER POSITION.
WELDING TECHNIQUES		X	VERIFY INTERPASS AND FINAL CLEANING EACH PASS IS WITHIN PROFILE LIMITATIONS, AND QUALITY OF EACH PASS

SPECIAL INSPECTION SCHEDULE (CONTINUED)

AREAS REQUIRING SPECIAL INSPECTION:	FREQUENCY		COMMENTS
	CONT.	PERIODIC	
STRUCTURAL STEEL CONSTRUCTION (CONTINUED)			
<i>AFTER WELDING (TABLE N5.4-3, AISC 360-10):</i>			
WELDS CLEANED		X	VERIFY THAT WELDS HAVE BEEN PROPERLY CLEANED
SIZE, LENGTH AND LOCATION OF WELDS	X		
WELDS MEET VISUAL ACCEPTANCE CRITERIA	X		
ARC STRIKES	X		
K-AREA	X		
BACKING & WELDING TABS REMOVED	X		
REPAIR ACTIVITIES	X		
DOCUMENT ACCEPTANCE/REJECTION OF WELD	X		
<i>NONDESTRUCTIVE TESTING (TABLE N5.5, AISC 360-10):</i>			
CJP WELDS (RISK CAT. II)		X	ULTRASONIC TESTING SHALL BE PERFORMED ON 10% OF CJP GROOVE WELDS IN BUTT, T- AND CORNER JOINTS SUBJECT TO TRANSVERSELY APPLIED TENSION LOADING IN MATERIALS $\frac{3}{16}$ INCH THICK OR GREATER. TESTING RATE MUST BE INCREASED IF >5% OF WELDS HAVE BEEN UNACCEPTABLE DEFECTS.
ACCESS HOLES (FLANGE >2")	X		
WELDED JOINTS SUBJECT TO FATIGUE	X		
<i>OTHER STEEL INSPECTIONS (TABLE N5.7, AISC 360-10; TABLES J8-1 AND J10-1, AISC 341-10):</i>			
STRUCTURAL STEEL DETAILS		X	ALL FABRICATED STEEL AND THEIR CONNECTIONS SHALL BE INSPECTED TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN AND IN THE APPROVED PLANS
ANCHOR RODS/EMBEDS SUPPORTING STRUCTURAL STEEL		X	SHALL BE ON THE PREMISES DURING THE PLACEMENT OF ANCHOR RODS/ EMBEDMENTS. VERIFY DIAMETER, GRADE, TYPE, AND LENGTH OF ELEMENT AND THE EXTENT OF DEPTH OF EMBEDMENT PRIOR TO PLACEMENT OF CONCRETE.
REDUCED BEAM SECTIONS (RBS)		X	VERIFY CONTOUR AND FINISH AS WELL AS DIMENSIONAL TOLERANCES (SEE TABLE J8-1 OR AISC 341)
PROTECTED ZONES		X	VERIFY THAT NO HOLES OR UNAPPROVED ATTACHMENTS ARE MADE WITHIN THE PROTECTED ZONE (SEE TABLE J8-1 OR AISC 341)

SPECIAL INSPECTIONS NOTES:

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. SPECIAL INSPECTION NOTES ARE CONSTRUCTION DOCUMENTS THAT SHALL BE INCLUDED WITH THE STRUCTURAL PLANS AND PROJECT SPECIFICATIONS.
3. SPECIAL INSPECTION SHALL BE PROVIDED BY OWNER OR OWNER AGENT ACCORDING TO IBC SECTION 17. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR SHALL SEND REPORTS TO THE OWNER, THE BUILDING OFFICIAL, THE ARCHITECT, THE ENGINEER AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING THAT THE SPECIAL INSPECTION WORK WAS, TO THE BEST OF HIS KNOWLEDGE, IN CONFORMANCE WITH THE PLANS, SPECIFICATIONS AND APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC.
4. SPECIAL INSPECTIONS SHALL BE IN ACCORDANCE WITH THE IBC AND AS DIRECTED BY THE BUILDING OFFICIAL. THE OWNER SHALL BE RESPONSIBLE FOR EMPLOYING SPECIAL INSPECTORS WHO MEET THE QUALIFICATIONS STATED IN THE IBC.
5. SPECIAL INSPECTIONS FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360 AND THE QUALITY ASSURANCE REQUIREMENTS OF THE AISC 341. TESTING FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341. SPECIAL INSPECTION FOR STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH SECTION 1705.2.2 OF THE IBC.
6. SPECIAL INSPECTIONS REQUIRED BY SECTION 1705 ARE NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATION OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

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Drawn by:
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Checked by:
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05/12/2023

THE GOOSE

2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT

SPECIAL INSPECTION SCHEDULE

Mark	Description	Date	By	
			AML	AML
A	Issued For Approval	04/19/2023		
B	Issued For Permit	05/10/2023		

DATE 05/10/2023

PROJ. NO. 23002

DRAWING NO.

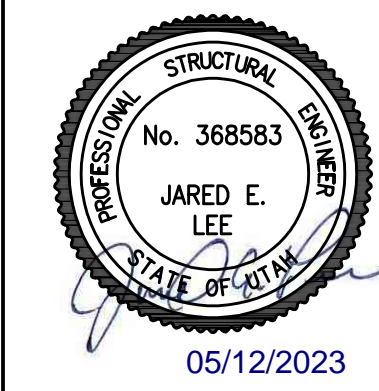
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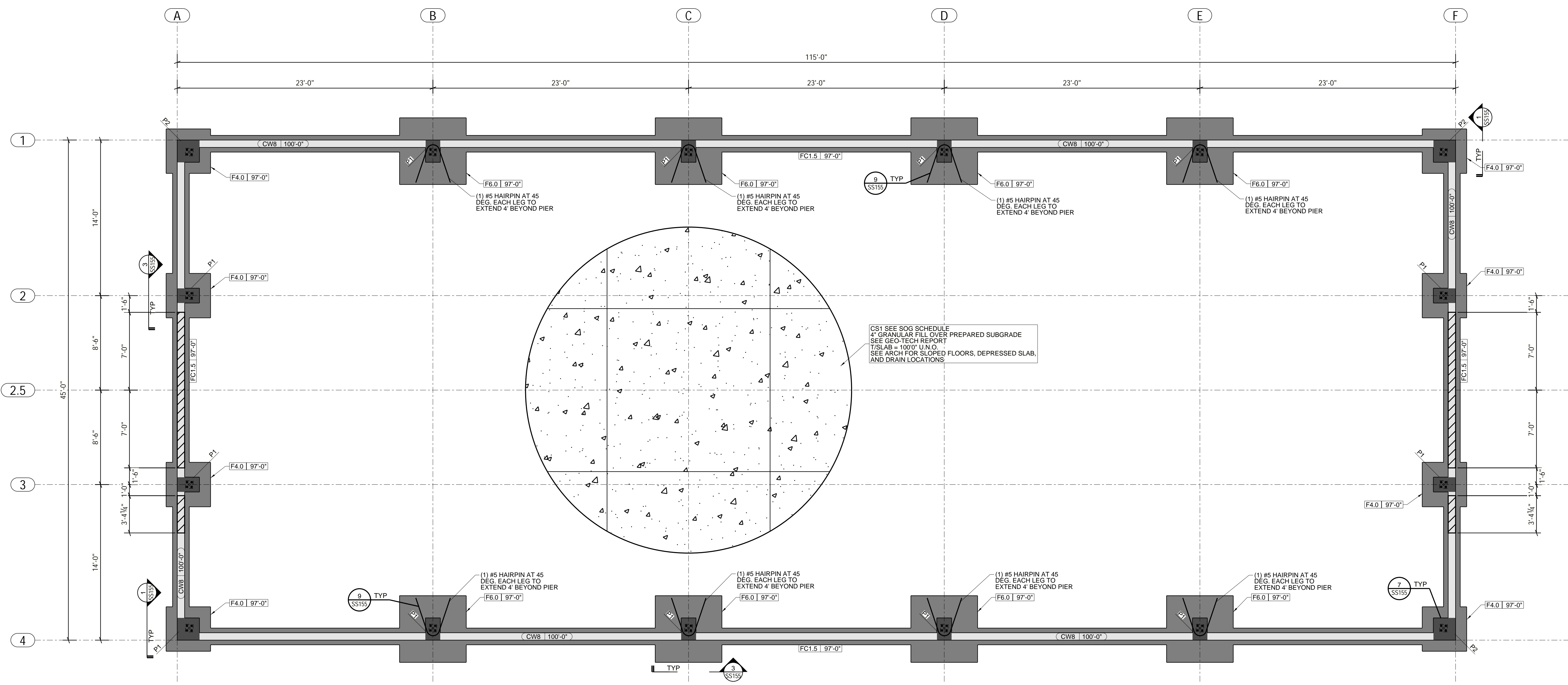


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THE GOOSE
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SUNNYFIELD FARM EDEN, UT
FOUNDATION PLAN



FOOTING AND FOUNDATION PLAN NOTES:

- COORDINATE LOCATION OF DEPRESSED SLAB, SLOPED SLABS, AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- SEE ARCHITECTURAL DRAWINGS AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOOR, SIDEWALKS, ETC.
- SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO COLUMN AND SLAB EDGES.
- SEE ARCHITECTURAL/STRUCTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.
- SEE ARCHITECTURAL DRAWING FOR CONTROL JOINT LOCATIONS.
- CONTRACTOR TO BE RESPONSIBLE FOR DAMAGE TO SLAB FROM HEAVY LOADING DURING CONSTRUCTION.
- ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE SOILS ENGINEER OF RECORD PRIOR TO THE PLACEMENT OF ANY REINFORCING STEEL OR CONCRETE. A WRITTEN REPORT PREPARED BY THE SOILS ENGINEER OF RECORD STATING THE RESULTS OF THE INSPECTION SHALL BE PROVIDED TO THE CITY INSPECTOR PRIOR TO ANY FOUNDATION INSPECTION. EXCAVATIONS SHALL BE MADE IN COMPLIANCE WITH CALIFORNIA REGULATIONS.
- THE CIVIL ENGINEER SHALL PROVIDE A LINE AND GRADE CERTIFICATION TO THE CITY INSPECTOR PRIOR TO ANY FOUNDATION INSPECTION. THIS CERTIFICATION SHALL STATE THE THE STRUCTURE HAS BEEN LOCATED ACCORDING TO THE CITY APPROVED GRADING PLANS.
- A FINAL COMPACTION REPORT AND SOILS ENGINEER INSPECTION REPORT MUST BE SUBMITTED TO THE BUILDING INSPECTOR PRIOR TO THE FOUNDATION INSPECTION AND THE CITY INSPECTOR MUST INSPECT EXCAVATIONS BEFORE PLACEMENT OF ANY CONCRETE.
- PRIOR TO THE FOUNDATION INSPECTION, THE SOILS ENGINEER SHALL NOTIFY THE BUILDING OFFICIAL IN WRITING THAT: THE BUILDING PAD WAS PREPARED IN ACCORDANCE WITH THE SOILS REPORT, UTILITY TRENCHES HAVE BEEN PROPERLY BACKFILLED AND COMPACTED, EXCAVATIONS HAVE BEEN MADE TO COMPLY WITH THE RECOMMENDATIONS OF BEARING CAPACITY AND SPECIAL SOIL CHARACTERISTICS.
- SEE SCHEDULES ON SHEETS (SS153-SS154) FOR:
*FOOTINGS
*CONCRETE WALLS
*CONCRETE PIERS
*CONCRETE SLABS
*GRADE BEAMS
*ANCHOR ROD AND BASE PLATES
*REINFORCING LAP AND SPLICE LENGTHS
- SEE TYPICAL FOOTING AND FOUNDATION DETAILS ON SHEET (SS155)
- ALL ANCHOR RODS TO BE F1554, GRADE 36 U.N.O.
- FINISH FLOOR SLAB ELEVATION 100'-0"
- SEE SHEET (SS201) FOR PLACEMENT OF ANCHOR RODS.
- SEE DETAIL S/SS155 FOR ALL PIPING AT FOUNDATIONS.
- BLOCK OUT SLAB FOR ALL PIERS AND COLUMNS. PLACE BLOCK OUT AT 4' MIN FROM EDGE OF PIER/COLUMN BASE PLATE.

MARKS AND SYMBOL LEGEND

- SECTION MARK
- SHEET NUMBER
- FOOTING DESIGNATION
- TOF ELEVATION
- DEPRESSED FND WALL, POUR SLAB OVER
- CONCRETE WALL
- STEEL COLUMN PER SUPPLIER
- CONCRETE PIER, SEE SCHEDULE
- CONTINUOUS FOOTING, SEE SCHEDULE
- SPOT FOOTING, SEE SCHEDULE
- PAD FOOTING, SEE SCHEDULE
- FOOTING STEP, SEE SCHEDULE
- FOOTING WALL, SEE SCHEDULE

Mark	By	Date	Description
A	AML	04/18/2023	Issued For Approval
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DRAWING NO. SS101			

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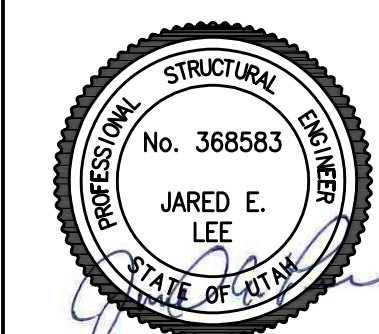
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THE GOOSE

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FOUNDATION NOTES

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DATE 05/10/2023

PROJ. NO. 23002

DRAWING NO.

SS151

GENERAL CONCRETE NOTES:

- CONCRETE CONSTRUCTION SHALL CONFORM TO AMERICAN CONCRETE INSTITUTE, ACI 318.
- ALL FOOTINGS SHALL BE CENTERED UNDER WALLS, COLUMNS, PILASTERS, ETC. U.N.O. ON THE PLANS.
- ALL FOOTING ELEVATIONS SHOWN ARE TO TOP OF FOOTING.
- CONCRETE SHALL BE NORMAL WEIGHT AND SHALL HAVE THE FOLLOWING 28-DAY COMPRESSIVE STRENGTHS:
 - 4,000 PSI - FOOTINGS, INTERIOR SLABS ON GRADE, SUSPENDED SLABS ON METAL DECK.
 - 4,000 PSI - COLUMNS, FOUNDATION AND RETAINING WALLS, EXTERIOR SLABS ON GRADE CURBS, AND GUTTERS.
 - 3,000 PSI - SLABS ON GRADE
 - 3,000 PSI - ALL OTHER CONCRETE
- USE ASTM TYPE I OR TYPE IA CEMENT
- CONCRETE PLACEMENT SHALL BE IN ACCORDANCE WITH ACI 305 HOT WEATHER CONCRETING & ACI 306 COLD WEATHER CONCRETING.
- WATER USED IN MIXING CONCRETE SHALL BE CLEAN AND FREE FROM INJURIOUS AMOUNTS OF OILS, ACIDS, ALKALIS, SALTS, ORGANIC MATERIALS OR OTHER SUBSTANCES THAT ARE DELETERIOUS TO CONCRETE OR STEEL REINFORCEMENTS.
- ALL INSERTS, ANCHOR BOLTS, PLATES, AND OTHER ITEMS TO BE CAST IN THE CONCRETE SHALL BE HOT-DIPPED GALVANIZED ACCORDING TO ASTM A153 UNLESS OTHERWISE NOTED.
- REINFORCING BARS, ANCHOR BOLTS, INSERTS, AND OTHER ITEMS TO BE CAST IN THE CONCRETE SHALL BE SECURED IN POSITION PRIOR TO PLACEMENT OF CONCRETE.
- ALL CONCRETE SHALL BE CONSOLIDATED BY MECHANICAL VIBRATORS.
- CONDUITS, PIPES, AND SLEEVES EMBEDDED WITHIN A SLAB OR WALL (OTHER THAN THOSE MERELY PASSING THROUGH) SHALL SATISFY THE FOLLOWING:
 - NO LARGER IN OUTSIDE DIMENSIONS THAN 1/3 THE OVERALL THICKNESS OF THE SLAB, OR WALL IN WHICH THEY ARE EMBEDDED
 - CONDUITS, PIPES, AND SLEEVES SHALL NOT BE PLACED THROUGH OR EMBEDDED IN A BEAM UNLESS SPECIFICALLY DETAILED
 - SPACED NO CLOSER THAN 3 DIAMETERS OR WIDTHS ON CENTER.
 - PLACED IN THE MIDDLE 1/3 OF SLAB OR WALL THICKNESS
- REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, ETC. TO BE CAST IN TO CONCRETE, AND FOR EXTENT AND LOCATION OF DEPRESSIONS, CURBS, RAMPS, ETC.
- CONSTRUCTION JOINTS SHALL BE MADE AND LOCATED SO AS TO NOT IMPAIR THE STRENGTH OF THE STRUCTURE. PROVIDE 2 X 4 (SHAPED) KEYWAY IN ALL VERTICAL AND HORIZONTAL JOINTS UNLESS NOTED OR DETAILED OTHERWISE.
- ALL STEEL REINFORCING SHALL BE CONTINUOUS THROUGH COLD JOINTS UNLESS NOTED OTHERWISE.
- NON-SHRINK GROUT SHALL BE A PREMIXED NON-METALLIC FORMULA. CAPABLE OF DEVELOPING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI IN 1 DAY AND 5,000 PSI IN 28 DAYS.
- LEAVE FRAMEWORK FOR OTHER STRUCTURAL ELEMENTS THAT SUPPORT WEIGHT OF CONCRETE IN PLACE UNTIL CONCRETE HAS ACHIEVED IT'S 28 DAY DESIGN COMPRESSIVE STRENGTH.

FOUNDATION NOTES:

- EXCEPT WHERE NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS, THE RECOMMENDATIONS OF THE ABOVE MENTIONED GEOTECHNICAL REPORT SHALL BE FOLLOWED, WHICH INCLUDE BUT ARE NOT LIMITED TO. SOIL CORRECTIONS / OVER-EXCAVATIONS, SUBGRADE PREPARATIONS, FILL REQUIREMENTS, AND COMPACTION REQUIREMENTS.
- CONTRACTOR SHALL PROVIDE FOR DE-WATERING OF EXCAVATION FROM SURFACE WATER, GROUND WATER OR SEEPAGE.
- EXCAVATIONS FOR ANY PURPOSE SHALL NOT REMOVE LATERAL SUPPORT FROM ANY FOOTING OR FOUNDATION WITHOUT FIRST UNDERPINNING OR PROTECTING THE FOOTING OR FOUNDATION AGAINST SETTLEMENT OR LATERAL SUPPORT.
- CONTRACTOR SHALL BRACE OR PROTECT ALL WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHING FLOORS ARE COMPLETELY IN PLACE AND HAVE ATTAINED THEIR FULL DESIGN STRENGTH.
- ALL FOOTINGS SHALL HAVE VERTICAL FACES FORMED WITH STANDARD FORMING MATERIALS (WOOD, METAL, ETC.) UNLESS NOTED OTHERWISE.
- TOP OF FOOTING ELEVATIONS SHOWN ON THE FOOTING AND FOUNDATION PLAN ARE BASED ON PRELIMINARY GRADING INFORMATION AND MUST BE VERIFIED PRIOR TO CONSTRUCTION.

REINFORCING STEEL NOTES:

- ALL REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.
- ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, SHALL BE SUPPLIED IN FLAT SHEETS, AND SHALL HAVE A MINIMUM SIDE LAP OF 8 INCHES.
- ADEQUATELY TIE AND SUPPORT ALL REINFORCING STEEL AS SPECIFIED BY ACI 315 TO MAINTAIN EXACT REQUIRED POSITION.
- CAST DOWELS IN FORMING FOR CONCRETE WALLS AND COLUMNS ABOVE DOWELS TO BE SAME QUANTITY, SIZE AND SPACING AS THE VERTICAL WALL AND COLUMN REINFORCING. DOWELS ARE TO PROJECT FROM FOOTING TO PROVIDE THE REQUIRED LAP SPlicing REQUIREMENTS. DOWELS INTO FOOTINGS SHALL TERMINATE WITH A SPlicing HOOK, AND SHALL EXTEND TO WITHIN 4 INCHES OF THE BOTTOM OF THE FOOTING BUT NEED NOT EXTEND MORE A DEVELOPMENT LENGTH INTO THE FOOTING.
- CLEAR CONCRETE COVER FOR REINFORCING BARS SHALL BE AS FOLLOWS, UNLESS OTHERWISE NOTED:
 - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 INCHES
 - CONCRETE FORMED AND EXPOSED TO EARTH OR WEATHER:
 - NO. 6 AND LARGER : 2 INCHES
 - NO. 5 AND SMALLER: 1.5 INCHES
 - CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
 - WALLS, SLABS, JOISTS:
 - NO. 11 BAR AND SMALLER: 0.75 INCHES
 - PEDESTALS, COLUMNS, BEAMS:
 - PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS: 1.5 INCHES
 - CONCRETE TILT-UP PANELS CAST AGAINST A RIGID HORIZONTAL SURFACE SUCH AS CONCRETE SLAB EXPOSED TO THE WEATHER:
 - NO. 8 AND SMALLER: 1 INCH
 - NO. 9 AND LARGER: 2 INCHES
- DO NOT WELD REINFORCING EXCEPT AS NOTED ON PLANS. WHERE WELDING REINFORCEMENT IN REQUIRED, USE ASTM A708.
- CLEAR DISTANCE BETWEEN THE SURFACE OF A BAR AND ANY SURFACE OF A MASONRY UNIT SHALL NOT BE LESS THAN 12/ INCH, UNLESS NOTED OTHERWISE.
- REINFORCING STEEL SHALL BE SPliced PER TYPICAL DETAILS AND SCHEDULES, UNLESS NOTED OTHERWISE.
- MECHANICAL SPLICE CONNECTORS SHALL DEVELOP IN TENSION 125 % OF THE SPECIFIED MINIMUM YIELD STRENGTH OF REINFORCEING BARS.

POST INSTALLED ANCHORS

GENERAL POST INSTALLED ANCHORS

- DO NOT INSTALL ADHESIVE ANCHORS IN CONCRETE IF LESS THAN 21 DAYS OLD; DO NOT INSTALL MECHANICAL ANCHORS, SCREW ANCHORS OR POWDER ACTUATED ANCHORS IN CONCRETE LESS THAN 7 DAYS OLD. CONTRACTOR MUST OBTAIN WRITTEN APPROVAL FROM THE ENGINEER TO INSTALL PRIOR TO CONCRETE CURING PERIODS. DO NOT APPLY FULL LOAD TO ANCHORS UNTIL CONCRETE HAS REACHED 28 DAY COMPRESSIVE STRENGTH.
- ANCHORS AND ADHESIVES SPECIFIED IN DETAILS SHALL BE PROVIDED BY CONTRACTOR.
- FOLLOW THE MANUFACTURE'S RECOMMENDATIONS AND CERTIFICATION TESTING REPORTS FOR ANCHOR INSTALLATION. SEE SPECIFIC ANCHORS BELOW FOR MORE INFORMATION.
- NO ANCHOR SHALL BE INSTALLED WITHIN 1.5 ANCHOR ROD DIAMETERS OF AN ABANDONED HOLE THAT HAS NOT BEAN FILLED WITH NON-SHRINK GROUT; INCREASE DISTANCE TO 3 ANCHORS ROD DIAMETERS WHEN THE ABANDONED HALE HAS NOT BEEN FILLED.

ADHESIVE ANCHORS

- ADHESIVES SHALL BE STANDARD STRENGTH OR HIGH STRENGTH, ACCORDING TO DETAILS PROVIDED. SEE BELOW FOR ACCEPTABLE ADHESIVES.
 - STANDARD ADHESIVES FOR ANCHORS IN CONCRETE INCLUDES:
 - SET-XP (ICC-ES ESR-2508) BY SIMPSON STRONG-TIE
 - PURE 50+ (ICC-ES ESR-3576) BY DEWALT
 - AC100+ GOLD (ICC-ES ESR-2582) BY DEWALT
 - HIT-RE 100 (ICC-ES ESR-3829) BY HILTI
 - HIGH STRENGTH ADHESIVES FOR ANCHORS IN CONCRETE INCLUDES:
 - SET-3G (ICC-ES ESR-4057) BY SIMPSON STRING-TIE
 - PURE 110+ (ICC-ES ESR-3298) BY DEWALT
 - AC200+ (ICC-ESESR-4027) BY DEWALT
 - HIT-RE 500-V3 (ICC-ES ESR-3814) BY HILTI
 - HIT-HY 200 (ICC-ES ESR-3187) BY HILTI
- FOR ANCHORS IN GROUTED MASONRY, ADHESIVE SHALL BE THE FOLLOWING: SET-XP (IAPMO UES ER-265) BY SIMPSON STRONG-TIE, AT-XP (IAPMO UES ER-281) BY SIMPSON STRONG-TIE, AC100+ (ICC-ES ESR-3200) BY DEWALT, HIT-HY-200-A (ICC-ES ESR-3963) BY HILTI, HIT-HY-200-R (ICC-ES ESR-3963) BY HILTI, OR CIA GEL (ICC-ES ESR-1702) BY USP.
- ADHESIVE SHALL BE WITHIN THE MANUFACTURES RECOMMENDED LIFE TIME AND PRIOR TO EXPIRATION DATE. DO NOT USE ADHESIVE THAT HAS NOT BEEN STORED PER MANUFACTURE'S RECOMMENDATIONS OR MAY HAVE EXPERIENCED FREEZE THAW CYCLES OR EXTREME HEAT.
- DO NOT INSTALL ADHESIVE ANCHOR IN WET OR DAMP HOLE UNLESS PRODUCT IS APPROVED FOR SUCH APPLICATIONS WITHOUT STRENGTH REDUCTIONS. DO NOT INSTALL ADHESIVE ANCHORS IF CONCRETE TEMPERATURE IN BELOW 50 DEGREES F UNLESS ADHESIVE IS APPROVED FOR LOWER TEMPERATURE WITHOUT STRENGTH REDUCTION.
- FOLLOW ALL THE MANUFACTURE'S RECOMMENDATIONS AND CERTIFICATION TESTING REPORTS REGARDING HOLE CLEANING PRIOR TO INSTALLATION OF ANCHOR. ALL HOLES SHALL BE DRILLED WITH ANSI STANDARD BITS DESIGNED FOR CONCRETE.

MECHANICAL ANCHORS

- CONCRETE ANCHORS SHALL BE: KWIK BOLT TZ (ICC-ES ESR-1917) BY HILTI, STRONG BOLT 2 (ICC-ES ESR-3037) BY SIMPSON STRONG TIE, OR POWER-STUD+ (ICC-ES ESR-2502) BY DEWALT.
- GROUTED MASONRY ANCHORS SHALL BE: KWIK BOLT 3 (ICC-ES ESR-1385) BY HILTI, WEDGE-ALL (ICC-ES ESR-1396) BY SIMPSON STRONG TIE, STRONG BOLT 2 (IAPMO-UES ER-240) BY SIMPSON STRONG TIE, OR POWER-STUD+ SD1 (ICC-ES ESR-2966) BY DEWALT.

SCREW ANCHORS

- CONCRETE SCREW ANCHORS SHALL BE: TITEN HD (ICC-ES ESR-2713) BY SIMPSON STRONG TIE, SCREW BOLT+ (ICC-ES ESR-3889) BY DEWALT OR KWIK HUS-EZ (ICC-ES ESR-3027) BY HILTI.
- GROUTED MASONRY SCREW ANCHORS SHALL BE: TITAN HD (ICC-ES ESR-1056) BY SIMPSON STRONG TIE, SCREW BOLT+ (ICC-ES ESR-4042) BY DEWALT OR KWIK HUS-EZ (ICC-ES ESR3056) BY HILTI.

POWDER ACTUATED FASTENERS

- FASTENERS DRIVEN INTO STEEL (EXCEPT AT METAL DECK) SHALL BE: X-U P8 TH UNIVERSAL KNURLED SHANK FASTENER (ICC-ES ESR-2269) BY HILTI, PDPA (ICC-ES ESR-2138) BY SIMPSON STEONG TIE OR 8MM HEAD SPORAL CSI DRIVE PIN (ICC-ES ESR-2024) BY DEWALT.

SPECIAL INSPECTIONS NOTES:

- SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
- SPECIAL INSPECTION NOTES ARE CONSTRUCTION DOCUMENTS THAT SHALL BE INCLUDED WITH THE STRUCTURAL PLANS AND PROJECT SPECIFICATIONS.
- SPECIAL INSPECTION SHALL BE PROVIDED BY OWNER OR OWNER AGENT ACCORDING TO IBC SECTION 17. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR SHALL SEND REPORTS TO THE OWNER, THE BUILDING OFFICIAL, THE ARCHITECT, THE ENGINEER AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING THAT THE SPECIAL INSPECTION WORK WAS, TO THE BEST OF HIS KNOWLEDGE, IN CONFORMANCE WITH THE PLANS, SPECIFICATIONS AND APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC.
- SPECIAL INSPECTIONS SHALL BE IN ACCORDANCE WITH THE IBC AND AS DIRECTED BY THE BUILDING OFFICIAL. THE OWNER SHALL BE RESPONSIBLE FOR EMPLOYING SPECIAL INSPECTORS WHO MEET THE QUALIFICATIONS STATED IN THE IBC.
- SPECIAL INSPECTIONS FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360 AND THE QUALITY ASSURANCE REQUIREMENTS OF THE AISC 341. TESTING FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341. SPECIAL INSPECTION FOR STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH SECTION 1705.2.2 OF THE IBCS.
- SPECIAL INSPECTIONS REQUIRED BY SECTION 1705 ARE NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATION OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

REQUIRED SPECIAL INSPECTION AND TESTS OF CONCRETE CONSTRUCTION TABLE 1705.3

TYPE	FREQUENCY OF INSPECTION		REFERENCE FOR CRITERIA	
	CONTINUOUS	PERIODIC	REFERENCED STANDARD	CBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	-	X	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706; b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND c. INSPECT ALL OTHER WELDS	X	X	AWS D1.4, ACI 318: 26.6.4	-
3. INSPECT ANCHORS CAST IN CONCRETE	-	X	ACI 318: 17.8.2	-
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS: a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a	X	X	ACI 318: 17.8.2.4 ACI 318: 17.8.2.4	-
5. VERIFY USE OF REQUIRED DESIGN MIX.	-	X	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH APPLICATION TECHNIQUES, TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	-	ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	-	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	X	ACI 318: 26.5.3-26.5.5	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR: a. APPLICATION OF PRESTRESSING FORCES; AND b. GROUTING OF BONDED PRESTRESSING TENDONS, AND STRUCTURAL SLABS	X	-	ACI 318: 26.10	-
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS	-	X	ACI 318: 26.9	-
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS FOR S ₁ INCH = 25.4MM.	-	X	ACI 318: 26.11.2	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	-	X	ACI 318: 26.11.1.2(b)	-

- a. WHERE APPLICABLE, SEE SECTION 1705.12, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE.
b. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318, OR OTHER QUALIFICATION PROCEDURES. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.

BASIS OF DESIGN:

1. GOVERNING BUILDING CODE ----- IBC 2018

FOUNDATION

- ALLOWABLE BEARING PRESSURE:
 - CONTINUOUS FOOTINGS:-----1500 PSF
 - ISOLATED FOOTINGS:-----1500 PSF
- LATERAL LOAD RESISTANCE:
 - COEFFICIENT OF FRICTION:-----0.40
 - ALLOWABLE PASSIVE PRESSURE:-----200 PSF PER FOOT
- MINIMUM FOOTING REQUIREMENTS:
 - CONTINUOUS FOOTING WIDTH: 12 INCHES MINIMUM
 - ISOLATED FOOTING WIDTH: 24 INCHES MINIMUM
 - MINIMUM EMBEDMENT DEPTH OF 18 INCHES
- STRUCTURAL FILL SHALL BE COMPACTED TO AT LEAST 90 PERCENT OF THE MAXIMUM DRY DENSITY PER ASTM D-1557 AND SHALL BE PREPARED PER GEOTECHNICAL REPORT.
- ALL WATER DRAINING OFF ROOF AND SURROUNDING AREAS SHALL BE DIRECTED AWAY FROM FOUNDATIONS.
- ALL EXCAVATIONS SHALL BE INSPECTED BY A REPRESENTATIVE OF THE GEOTECHNICAL CONSULTANT PRIOR TO PLACEMENT OF ANY CONCRETE.
- EXCAVATIONS SHALL BE TRIMMED NEAT, LEVEL AND SQUARE.
- COORDINATE LOCATION OF SLAB DEPRESSION, SLOPED SLABS, AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- THE ENGINEER HAS NO RESPONSIBILITY FOR THE DESIGN OF TEMPORARY SHORING, SCAFFOLDING, FORMING, UNDERPINNING, ETC., UNLESS IT IS SPECIFICALLY DETAILED WITHIN THESE PLANS.
- ALL BACKFILL SHOULD BE COMPACTED TO A MINIMUM OF 90 PERCENT RELATIVE COMPACTION. DO NOT BACKFILL WALLS BEFORE DESIGN STRENGTH IS REACHED. WALLS SHALL BE SUPPORTED FROM LATERAL LOADS UNTIL DESIGN STRENGTH IS REACHED AND SUPPORTING MEMBERS OR IN PLACE.
- COORDINATE PLACEMENT OF ANCHORS AND CAST PLATES WITH STEEL DRAWINGS.

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PREPARED BY:

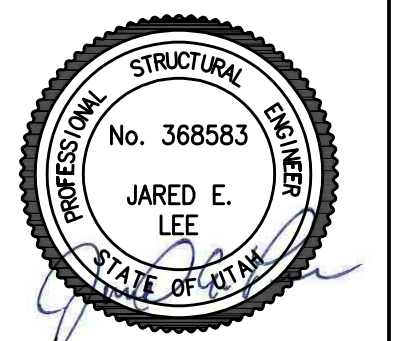
Designed by:

JMB

Drawn by:

S.E.S.

Checked by:



05/12/2023

THE GOOSE

2103 N. 5600 E.
SUNNYFIELD FARM EDEEN, UT

FOUNDATION SPECIAL INSPECTION

Mark	Description	Date	By
A	Issued For Approval	04/18/2023	AML
B	Issued For Permit	05/10/2023	AML

DATE 05/10/2023

PROJ. NO. 23002

DRAWING NO.

SS152

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DESIGNED BY: JMB
DRAWN BY: SES
CHECKED BY:



05/12/2023

THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM, EDEN, UT
FOUNDATION SCHEDULES

Mark	Description	Date	By
A	Issued For Approval	04/18/2023	AML
B	Issued For Permit	05/10/2023	AML

DATE 05/10/2023

PROJ. NO. 23002

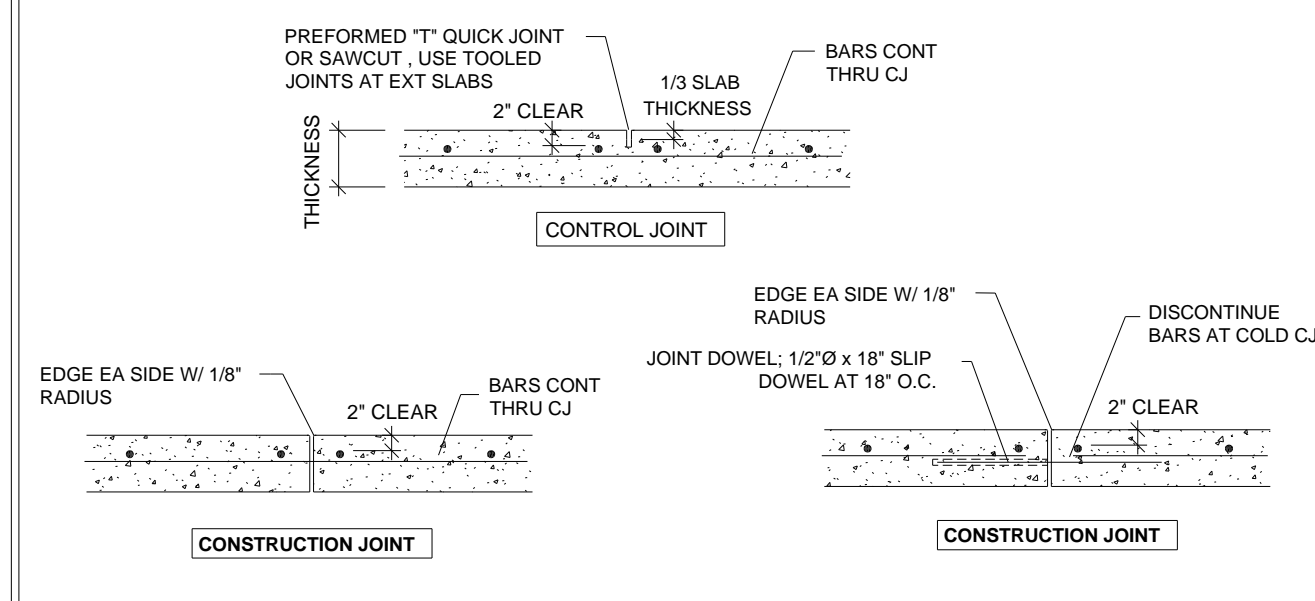
DRAWING NO.

SS153

CONCRETE SLAB-ON-GRADE SCHEDULE

MARK	SLAB THICKNESS 'T'	REINFORCING				COMMENTS
		LONGITUDINAL		TRANSVERSE		
		SIZE	SPACING	SIZE	SPACING	
CS1	4"	#4 BAR	24" O.C.	#4 BAR	24" O.C.	SAW CUT 10' MAX.
CS2	6"	#4 BAR	18" O.C.	#4 BAR	18" O.C.	SAW CUT 12' MAX.

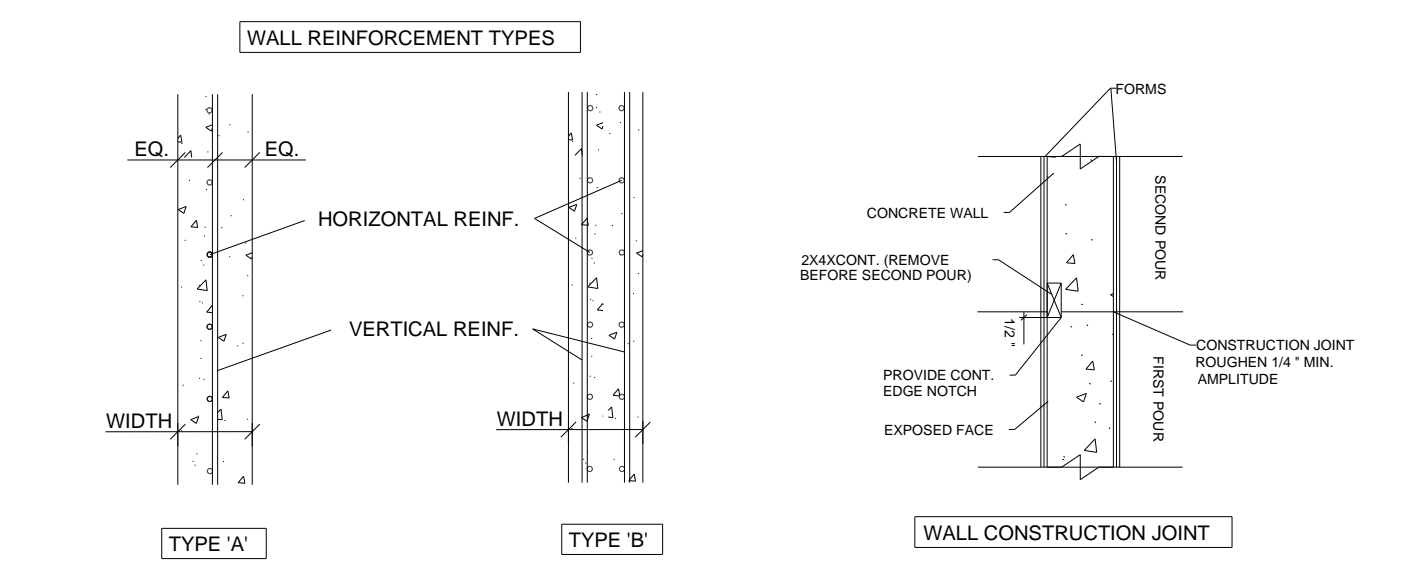
- SLAB ON GRADE SHALL BE PER SCHEDULE. SLAB SHALL BE UNDERLAIN BY 4" MIN OF FREE DRAINING MATERIAL.
- THICKENING OF SLAB IS NOT REQUIRED FOR SLABS WITH THICKNESS OF 6" OR GREATER. SEE PLAN FOR AREAS REQUIRING THICKENING.
- BLOCK OUT FOR ALL PIERS LOCATIONS AND INTERIOR COLUMN BASE PLATES. BLOCK OUT SHALL BE A MINIMUM OF 4" FROM EDGE OF PIER/BASE PLATES.
- COORDINATE JOINT LOCATIONS WITH ARCHITECT.
- FILL ALL INTERIOR CONTROL JOINTS WITH SEALER AFTER 28 DAYS OF PLACEMENT OF CONCRETE.
- REINFORCEMENT SHALL RUN CONTINUOUSLY THROUGH COLD JOINTS. (SEE DETAIL BELOW).
- SEE PLAN FOR LOCATIONS OF RECESSED SLABS. REINFORCEMENT SHALL CONTINUE THROUGH RECESSED AREAS.



CONCRETE WALL SCHEDULE

MARK	WALL WIDTH	WALL HEIGHT	REINFORCING				WALL TYPE	COMMENTS
			VERTICAL		HORIZONTAL			
			SIZE	SPACING	SIZE	SPACING		
CW4	8"	4'	#4 BAR	24" O.C.	#4 BAR	18" O.C.	A	
CW6	8"	6'	#4 BAR	24" O.C.	#4 BAR	18" O.C.	A	
CW8	8"	8'	#4 BAR	20" O.C.	#4 BAR	18" O.C.	A	
CW10	8"	10'	#5 BAR	14" O.C.	#5 BAR	18" O.C.	A	
CW12	10"	12'	#4 BAR	18" O.C.	#4 BAR	16" O.C.	B	(2) CURTAINS

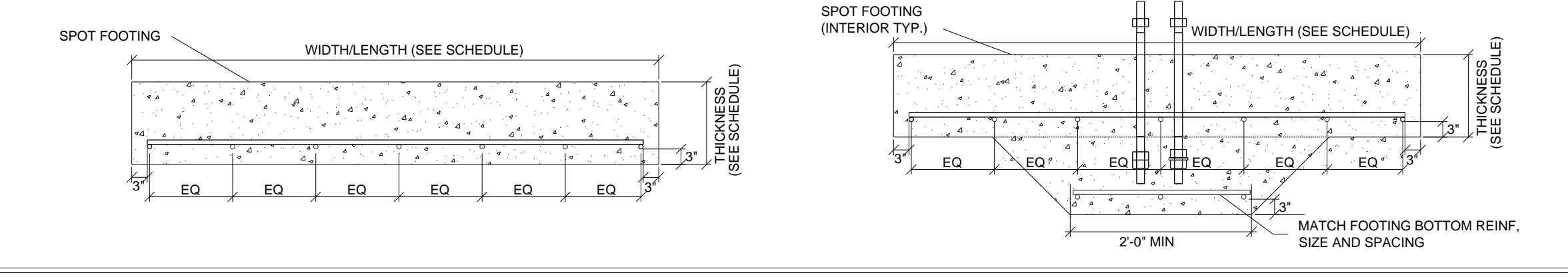
- FOUNDATION WALL NOTES:
- SEE GENERAL STRUCTURAL NOTES FOR CLEAR COVER AND OTHER REQUIREMENTS NOT INCLUDED IN THIS SCHEDULE.
 - SEE FOOTING AND FOUNDATION PLAN FOR FOUNDATION WALL CALLOUTS AND T.O.W. ELEVATIONS.
 - WALLS SHALL HAVE TWO HORIZONTAL BARS IN THE TOP AND BOTTOM.
 - PLACE STEEL IN THE CENTER OF THE WALL FOR TYPE 'A'. ALL WALLS THICKER THAN 10" SHALL HAVE TWO CURTAINS OF REINFORCEMENT PLACED NEAR EACH FACE OF WALL.



CONCRETE FOOTING SCHEDULE

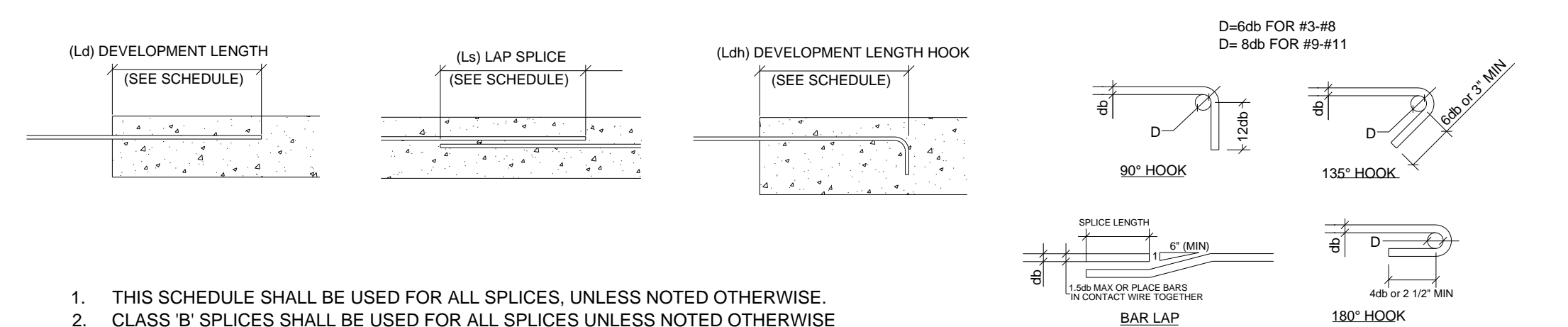
	MARK	WIDTH	LENGTH	DEPTH	REINFORCEMENT								COMMENTS	
					LONGITUDINAL				TRANSVERSE					
					AMOUNT	SIZE	LENGTH	SPACING	AMOUNT	SIZE	LENGTH	SPACING		
CONTINUOUS FOOTINGS	FC 1.5	18"	CONT.	12"	2	#4 BAR	CONT.	12"	NOT REQ.	-	-	-	-	
	FC 3.0	36"	CONT.	12"	3	#5 BAR	CONT.	15"	CONT.	#5 BAR	30"	18" O.C.		
SPOT FOOTINGS	F2.0	2'-0"	2'-0"	12"	3	#4 BAR	18"	6" O.C.	3	#4 BAR	18"	6" O.C.		
	F3.0	3'-0"	3'-0"	12"	5	#4 BAR	30"	6" O.C.	5	#4 BAR	30"	6" O.C.		
	F4.0	4'-0"	4'-0"	12"	4	#5 BAR	42"	11" O.C.	4	#5 BAR	42"	11" O.C.		
	F5.0	5'-0"	5'-0"	12"	5	#5 BAR	54"	11" O.C.	5	#5 BAR	54"	11" O.C.		
	F5.0T	5'-0"	5'-0"	12"	5	#5 BAR	54"	11" O.C.	5	#5 BAR	54"	11" O.C.	USE TOP REINFORCING	
	F6.0	6'-0"	6'-0"	12"	6	#5 BAR	66"	10" O.C.	6	#5 BAR	66"	10" O.C.		
	F6.0T	6'-0"	6'-0"	12"	7	#5 BAR	66"	10" O.C.	7	#5 BAR	66"	10" O.C.	USE TOP REINFORCING	
	F7.0	7'-0"	7'-0"	14"	7	#6 BAR	78"	12" O.C.	7	#6 BAR	78"	12" O.C.		
F7.0T	7'-0"	7'-0"	14"	7	#6 BAR	78"	12" O.C.	7	#6 BAR	78"	12" O.C.	USE TOP REINFORCING		
F8.0	8'-0"	8'-0"	16"	8	#6 BAR	90"	11" O.C.	8	#6 BAR	90"	11" O.C.			

- SPOT FOOTING NOTES:
- SEE FOUNDATION PLAN FOR TOP OF FOOTING ELEVATIONS.
 - ALL SPOT FOOTINGS SHALL BE CENTERED UNDER COLUMNS.
 - TOP REINFORCING (INDICATED BY 'T' ON THE END OF FOOTING MARK), SHALL BE THE SAME SIZE, AMOUNT AND SPACING AS THE BOTTOM REINFORCING AND BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
 - PLACE ALL BOTTOM FOOTING REINFORCEMENT IN BOTTOM OF FOOTING WITH 3" OF CLEAR CONCRETE COVER.
 - RUN CONTINUOUS FOOTING REINFORCEMENT THROUGH SPOT FOOTINGS.
 - IF FOOTINGS ARE EARTH FORMED, FOOTING SIDES SHALL BE INCREASED 6" IN WIDTH AND LENGTH.
 - SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER FOOTING REQUIREMENTS.
 - FOOTINGS MAY BE FORMED THICKER THAN THE DEPTH SPECIFIED IN LOCATIONS WHERE ANCHOR RODS EXTEND BELOW BOTTOM OF FOOTINGS IN SCHEDULE. SEE DETAIL BELOW.
 - INTERIOR FOOTINGS SHALL BE BLOCKED OUT FOR COLUMNS WITH 4" MIN DIST. FROM COLUMN B.P. TO EDGE OF BLOCK OUT DIAMOND.



CONCRETE REINFORCEMENT DEVELOPMENT LENGTH & LAP SCHEDULE (IN)

BAR SIZE	f'c=3000				f'c=4000				f'c=5000				f'c=6000				f'c ALL					
	Ld	Lt	Ls	Lst	Ld	Lt	Ls	Lst	Ld	Lt	Ls	Lst	Ld	Lt	Ls	Lst	Ldc	Lsc				
#3	18	24	24	32	9	16	20	21	26	8	14	18	19	24	7	13	17	23	6	7	12	
#4	24	31	32	41	11	21	27	28	36	11	19	24	25	32	11	17	22	23	29	11	11	15
#5	30	39	39	51	14	26	34	34	45	14	23	30	30	39	14	21	28	28	37	14	14	19
#6	36	47	47	62	17	31	40	41	52	17	28	36	37	47	17	26	33	34	43	17	17	23
#7	42	54	55	71	20	36	47	47	62	20	33	42	43	55	20	30	39	39	51	20	20	27
#8	48	62	63	81	22	41	54	54	71	22	37	48	49	63	22	34	45	45	58	22	22	30
#9	54	70	71	91	25	47	61	62	80	25	42	54	55	71	25	38	50	50	65	25	25	34
#10	61	79	80	103	28	53	68	69	89	28	47	61	62	80	28	43	56	56	73	28	28	39
#11	67	87	88	114	31	58	76	76	99	31	52	68	68	89	31	48	62	63	81	31	31	43



- THIS SCHEDULE SHALL BE USED FOR ALL SPLICES, UNLESS NOTED OTHERWISE.
- CLASS 'B' SPLICES SHALL BE USED FOR ALL SPLICES UNLESS NOTED OTHERWISE.
- SPLICES ARE NOT NECESSARY FOR TIES AND STIRRUPS.
- IF LIGHTWEIGHT CONCRETE IS USED, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3.
- FOR EPOXY COATED BARS, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5 FOR BARS WITH CLEAR COVER LESS THAN 3db OR CLEAR SPACING LESS THAN 6db, OTHERWISE MULTIPLY BY 1.2.
- IF REINFORCING HAS CLEAR COVER LESS THAN 1db, LAP LENGTH SHALL BE MULTIPLIED BY 1.5.
- FOR REINFORCING NOT ENCLOSED IN TIES OR STIRRUPS WITH SPACING LESS THAN 2db ON CENTER, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5.
- IF BARS OF DIFFERENT SIZES ARE LAPPED, THE SPLICE LENGTH SHALL BE THE LARGER OF 'Ld' OF THE LARGER BARS AND THE 'Ls' OF THE SMALLER BAR.

DEFINITIONS:

Ld: TENSION DEVELOPMENT LENGTH WITH THE FOLLOWING CLEAR SPACING AND COVER: CLEAR SPACING OF BARS BEING DEVELOPED OR LAP SPLICE NOT LESS THAN db, CLEAR COVER AT LEAST db, AND STIRRUPS OR TIES THROUGHOUT Ld NOT LESS THAN THE CODE MINIMUM OR CLEAR SPACING OF BARS BEING DEVELOPED OR LAP SPLICED AT LEAST 2db AND CLEAR COVER AT LEAST db.

Lt: TENSION DEVELOPMENT LENGTH FOR TOP BARS. TOP BARS ARE DEFINED AS ANY HORIZONTAL REINFORCEMENT WITH MORE THAN 12 IN OF CONCRETE CAST BELOW, THIS WOULD INCLUDE WALL REINFORCEMENT.

Ls: TENSION LAP SPLICE LENGTH FOR BOTTOM BARS. DEFINED AS ALL OTHER BARS THAT ARE NOT CONSIDERED TOP BARS OR HAVE LESS THAN 12 IN OF CONCRETE CAST BELOW. THIS WOULD INCLUDE SLAB REINFORCEMENT.

Lst: TENSION LAP SPLICE LENGTH FOR TOP BARS

Ldc: COMPRESSION DEVELOPMENT LENGTH

Lsc: COMPRESSION LAP SPLICE LENGTH, THIS WOULD INCLUDE COLUMNS AND PEDESTALS.

db: DIAMETER OF REINFORCING BARS (IN).

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PREPARED BY:
Designed by: JMB
Drawn by: SES
Checked by:



THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT
FOUNDATION PIERS

Mark	Description	Date	By
A	Issued For Approval	04/18/2023	AML
B	Issued For Permit	05/10/2023	AML

DATE 05/10/2023

PROJ. NO. 23002

DRAWING NO.

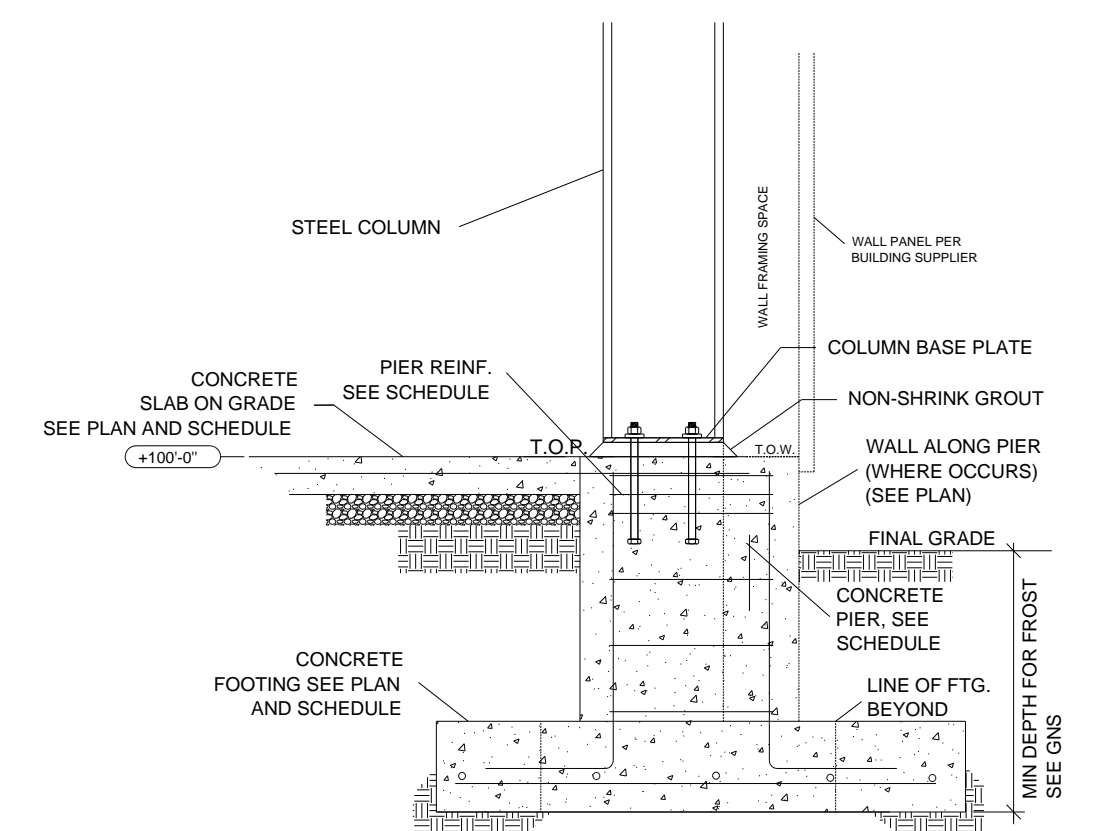
SS154

CONCRETE PIER SCHEDULE

MARK	PIER WIDTH (SHORT FACE)	PIER LENGTH (LONG FACE)	VERTICAL REINFORCING			TIES	COMMENTS/ELEVATION
			# OF BARS IN SHORT FACE	# OF BARS IN LONG FACE	TOTAL # OF VERTICAL BARS		
			AMOUNT/BAR SIZE	AMOUNT/BAR SIZE			
P1	16	24	(3) #5 BAR	(5) #5 BAR	12	#3 BAR @ 8" O.C.	T.O.P. 100'-0"
P2	24	24	(5) #5 BAR	(5) #5 BAR	16	#3 BAR @ 8" O.C.	T.O.P. 100'-0"

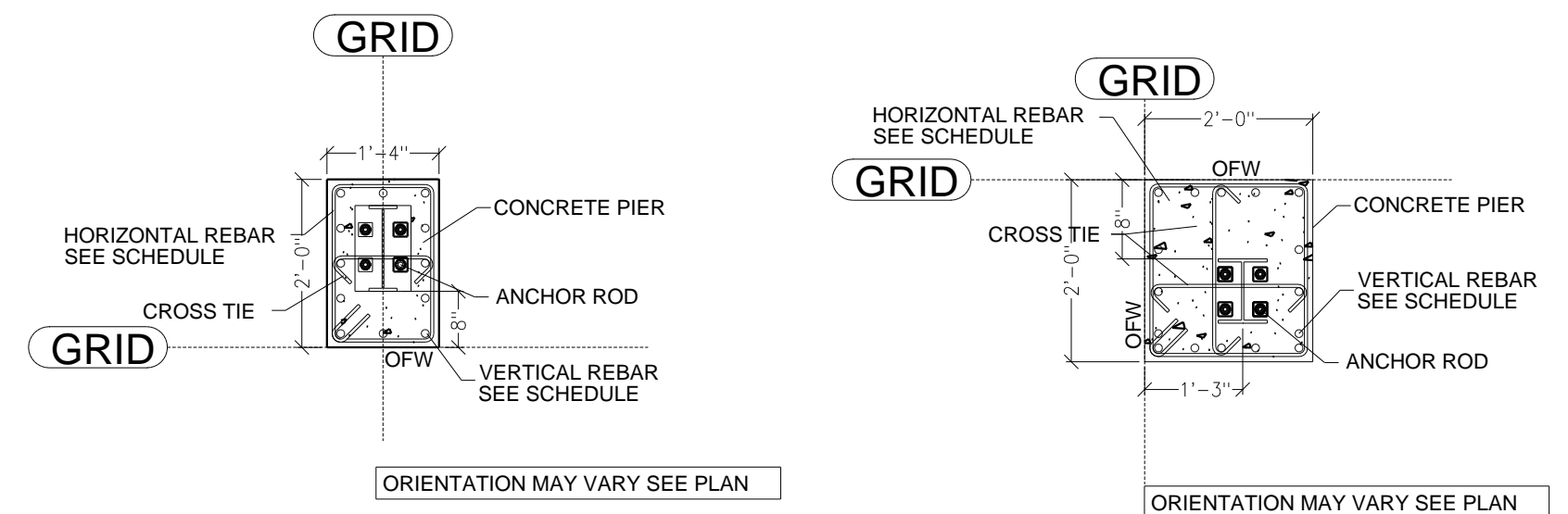
- PLACE (3) TIES IN THE TOP 5' OF EACH PIER.
- CROSSTIES TO BE #3 BAR U.N.O. (SEE SCHEDULE) WITH THE SAME VERTICAL SPACING AS TIES, SEE PIER DETAILS FOR LOCATIONS AND PLACEMENT.
- PIER ORIENTATION MAY VARY (SEE PLAN).
- VERTICAL REINFORCING TO BE SPACED UNIFORMLY IN EACH SIDE OF PIER WITH 2" OF CLEAR SPACING FROM OUTSIDE OF PIER. (SEE GENERAL NOTES FOR CLEAR SPACING)
- TOP OF PIER (T.O.P.) ELEVATIONS TO BE 100'-0".
- HORIZONTAL WALL REINFORCING SHALL RUN CONTINUOUS THROUGH PIER WHEN PIER IS POURED MONOLITHICALLY WITH CONCRETE WALL.

PIER AT EXTERIOR WALL



PIER AT WALL ELEVATION W/ PROJECTED WALL

PIER CONFIGURATIONS



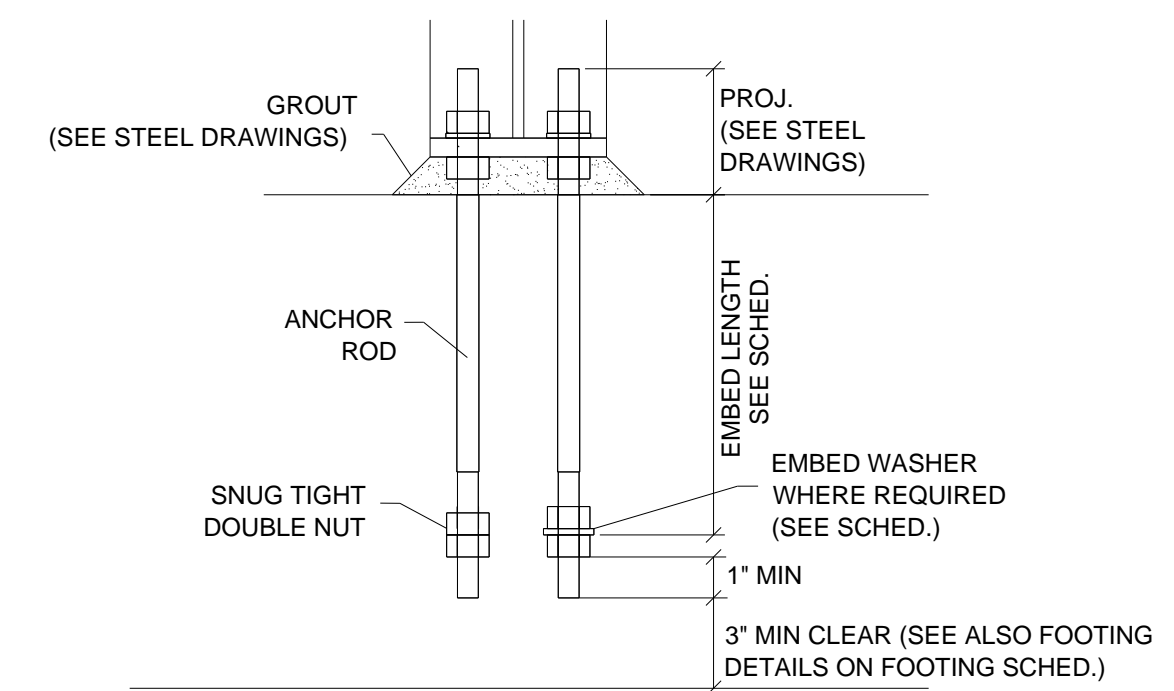
CONFIGURATION P1

CONFIGURATION P2

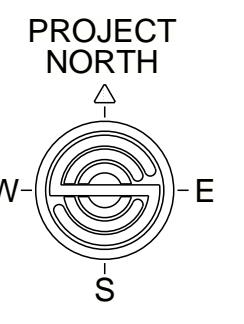
ANCHOR ROD AND BASE PLATE SCHEDULE

BASE PLATE DETAILS STEEL DRAWINGS	BOLT DIAMETER	EMBED LENGTH	ANCHOR GRADE	COMMENTS
1/SS252	3/4"	16"	F1554-36	USE EMBED WASHER
2/SS252	3/4"	16"	F1554-36	USE EMBED WASHER

- SEE SS251 FOR GUIDELINES FOR PLACEMENT OF ANCHOR RODS.
- SNUG TIGHTEN NUTS ON THREADED RODS.
- LOCATE ANCHOR ROD PATTERN FROM GRID LINE TO CENTER. USE CAST PLATE CENTER DIAMOND. (STEP 3 ON SS251)
- USE EMBED WASHERS WHERE INDICATED. WASHER TO BE PLACED AS SHOWN IN DETAIL BELOW.



TYPICAL ANCHOR ROD PATTERN



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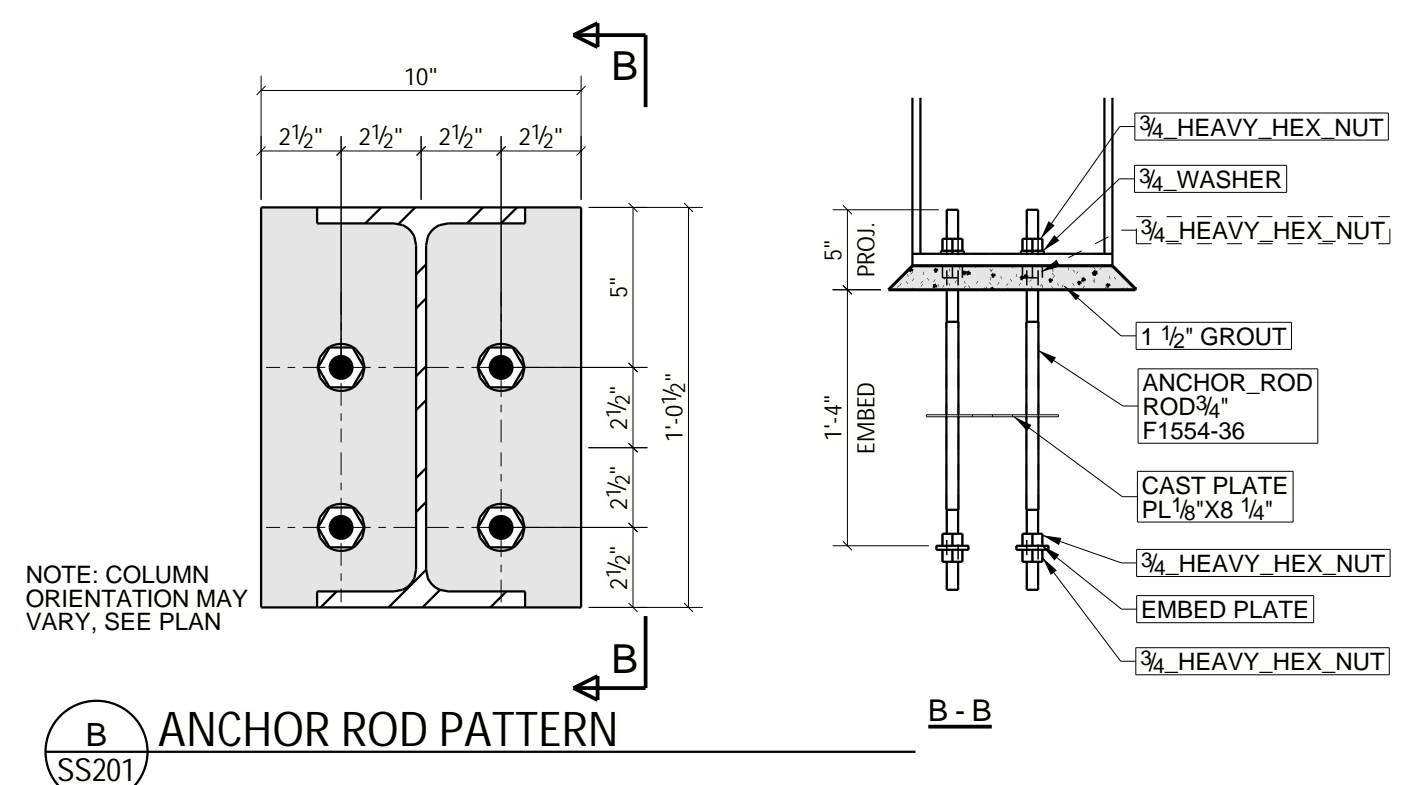
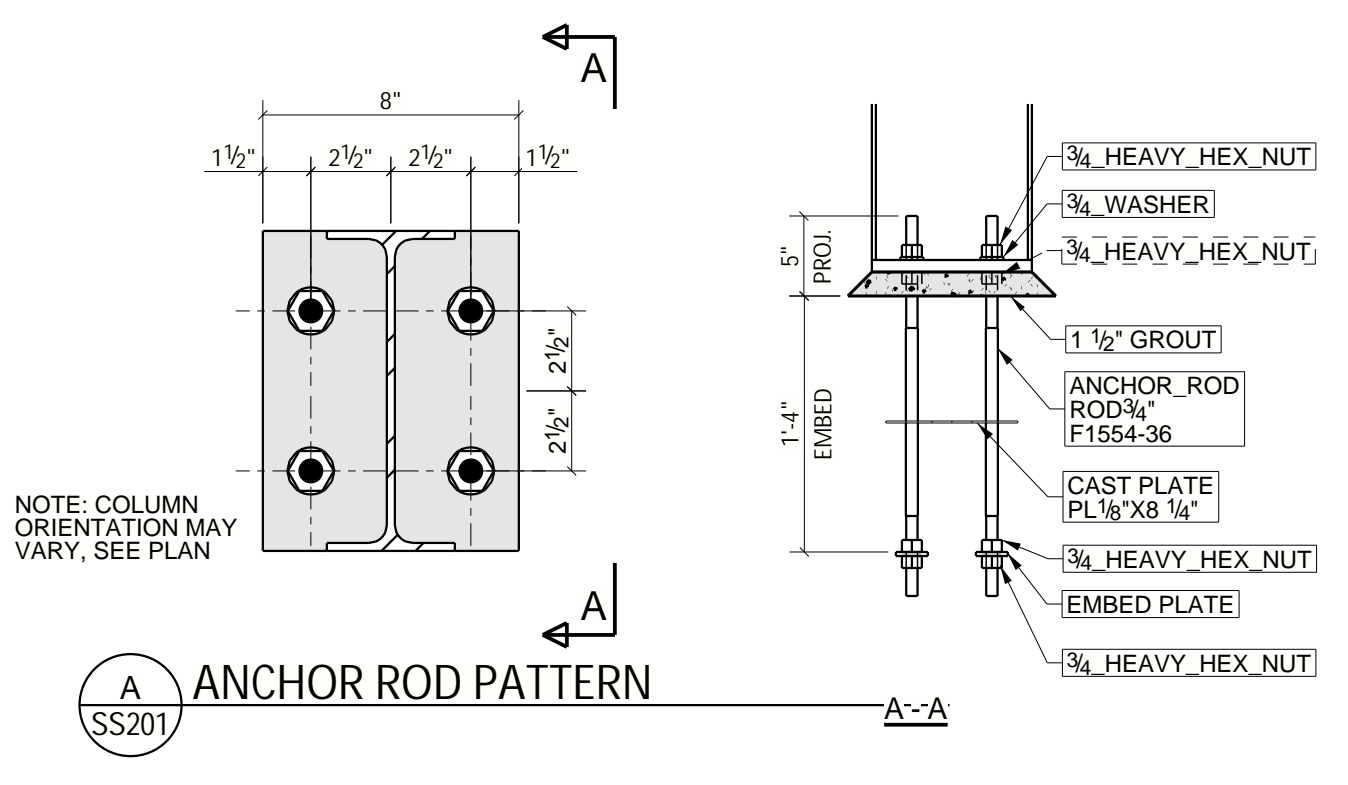
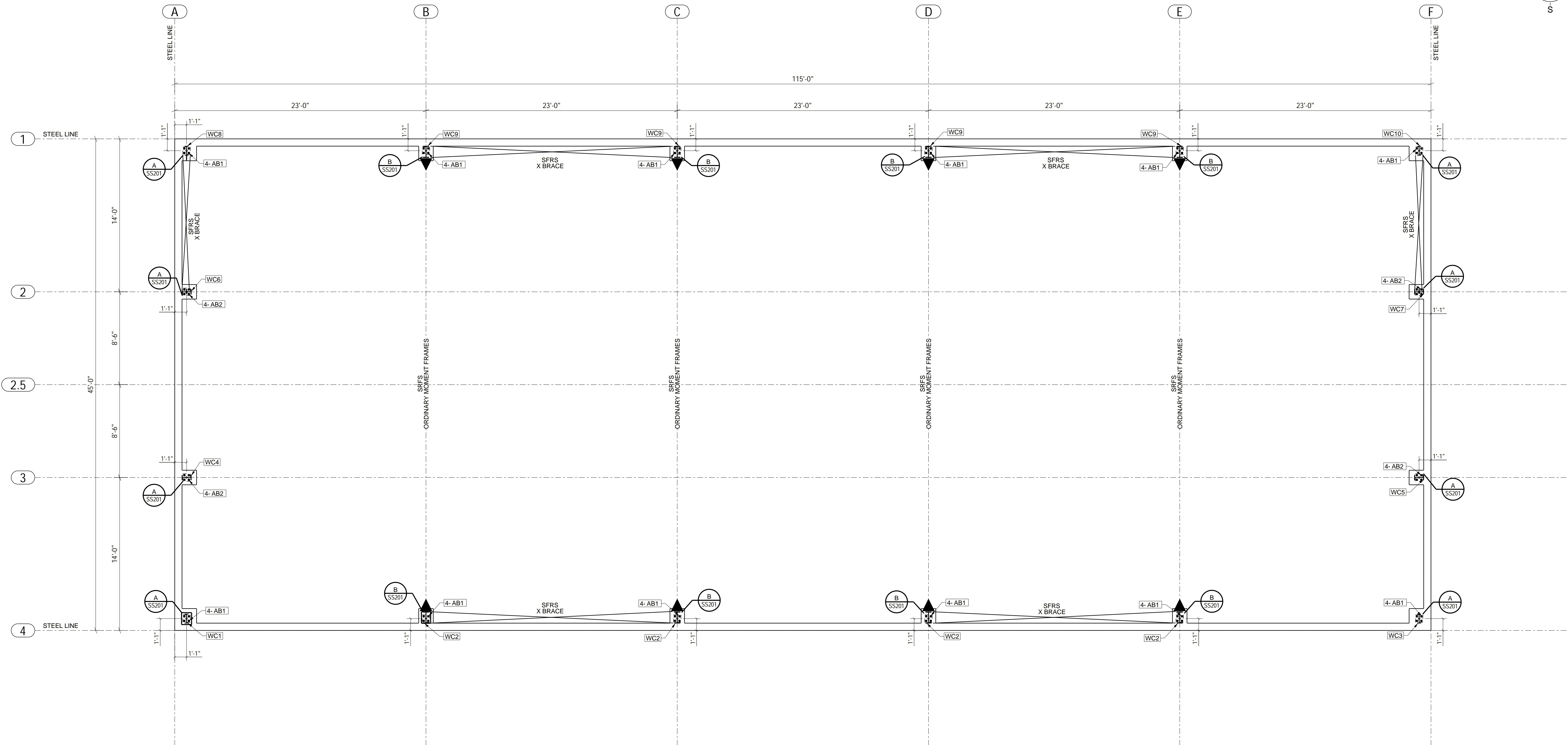


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Drawn by: AML
Checked by: J.L.



THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT
ANCHOR ROD PLAN



- ANCHOR ROD PLAN NOTES:**
1. ALL ANCHOR RODS TO BE F1554, GRADE 36
 2. FINISH FLOOR SLAB ELEVATION: 100'-0"
 3. PERIMETER BOTTOM OF BASE PLATE ELEVATION = 100'-1.5"
 4. 1.5" GROUT UNO
 5. SFRS IS:
EAST-WEST DIRECTION: ORDINARY CONCENTRICALLY BRACES FRAMES
 $R=3.25$, $P=1.0$, $\Omega=2.0$ NORTH-SOUTH DIRECTION:
ORDINARY MOMENT FRAMES $R=3.5$, $P=1.0$, $\Omega=3.0$
 6. ▲ REFERS TO MOMENT CONNECTION AT ROOF
 7. COLUMNS AT LINES 1 & 4 HAVE BEEN DESIGNED FOR A 12' FUTURE EXPANSION. THE GIRTS AT LINES 1 & 4 ARE TO REMAIN IN PLACE AFTER THE EXPANSION.

Mark	Description	Date	By
A	Issued For Approval	04/18/2023	AML
B	Issued For Permit	05/10/2023	AML

DATE 05/10/2023
PROJ. NO. 23002
DRAWING NO. **SS201**

SETTING ANCHOR RODS IN THE FOUNDATION (CONCRETE AND FOUNDATION SUB CONTRACTOR)

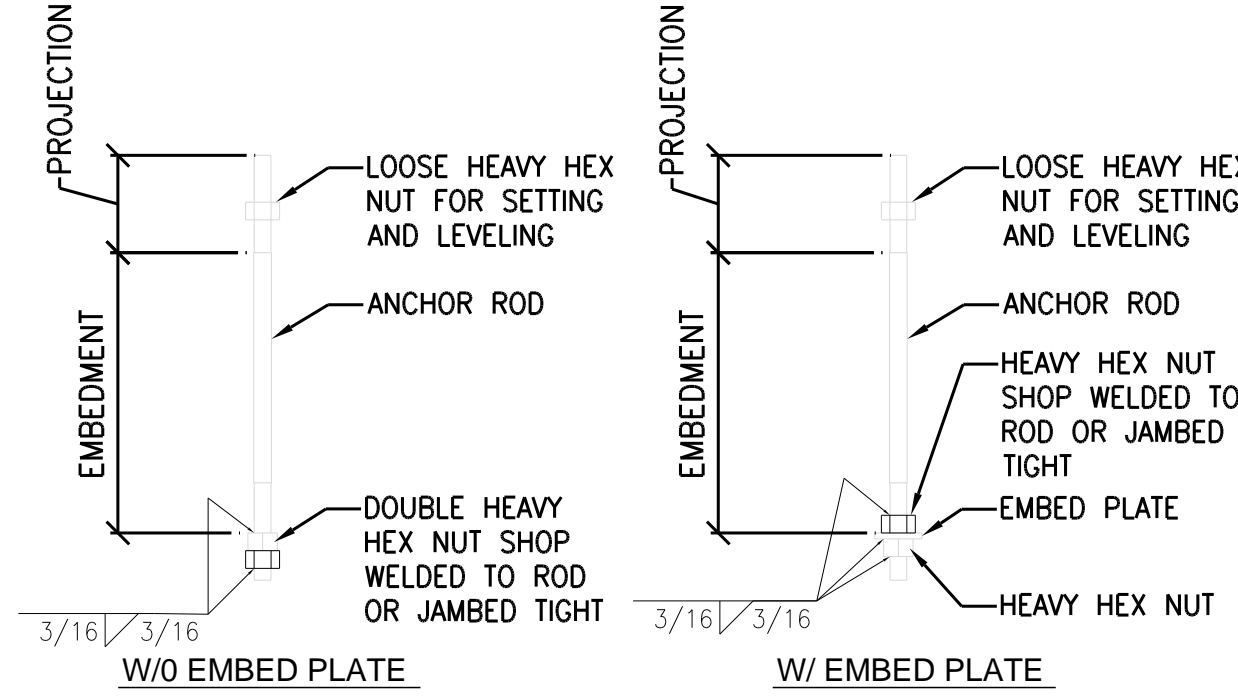
3/4" DIAMETER ANCHOR RODS IN A 4" BY 4" PATTERN SHOWN FOR REFERENCE. SEE ANCHOR ROD DETAILS FOR SIZE AND PATTERN SPECIFIC TO EACH LOCATION.

ANCHOR ROD PLACEMENT NOTES

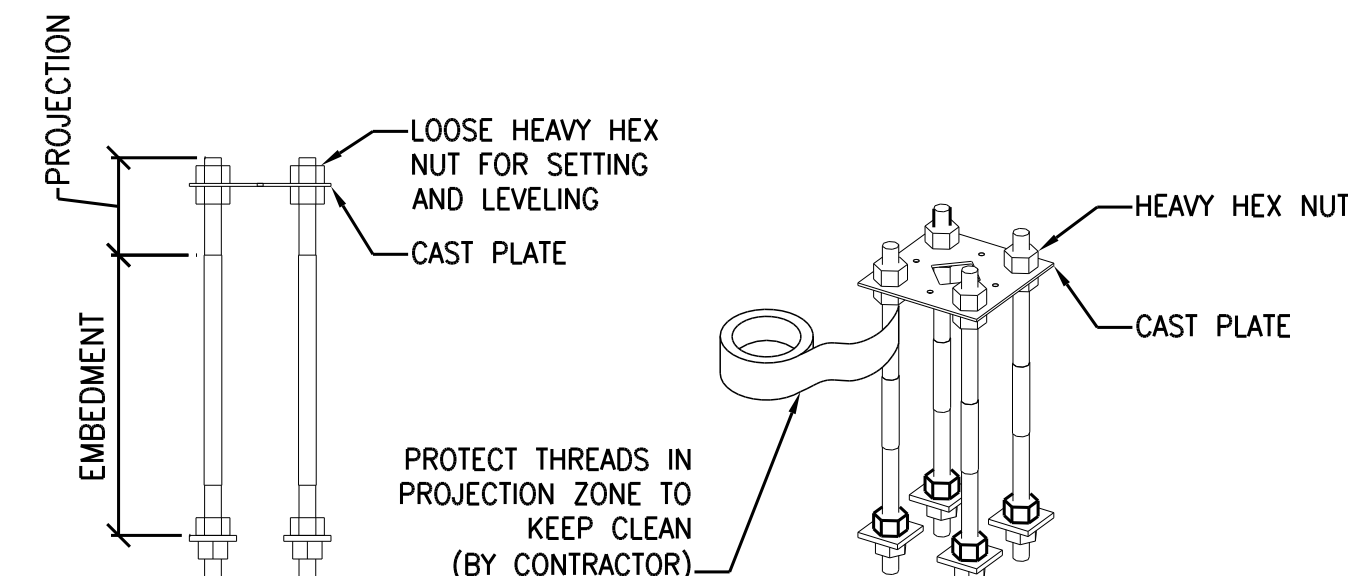
- SHOP FABRICATED 1/8" THICK STEEL TEMPLATES "CAST PLATE" MAY HAVE BEEN PROVIDED IN LIEU OF FIELD FABRICATED PLYWOOD TEMPLATES BY CUSTOMER REQUEST. (SEE CONTACT)
- INDICATES: TOP OF ANCHOR ROD FROM SPECIFIED ELEVATION VERTICAL DEVIATION $\pm 1/2$ IN.

** 1" DIFFERENCES IN ELEVATION MAY OCCUR. LEVELING BASE PLATES TO THE LOCATION WITH THE HIGHEST CONCRETE LEVEL WILL ENSURE THE MOST EFFECTIVE GROUTING AND SIMPLEST LEVELING.

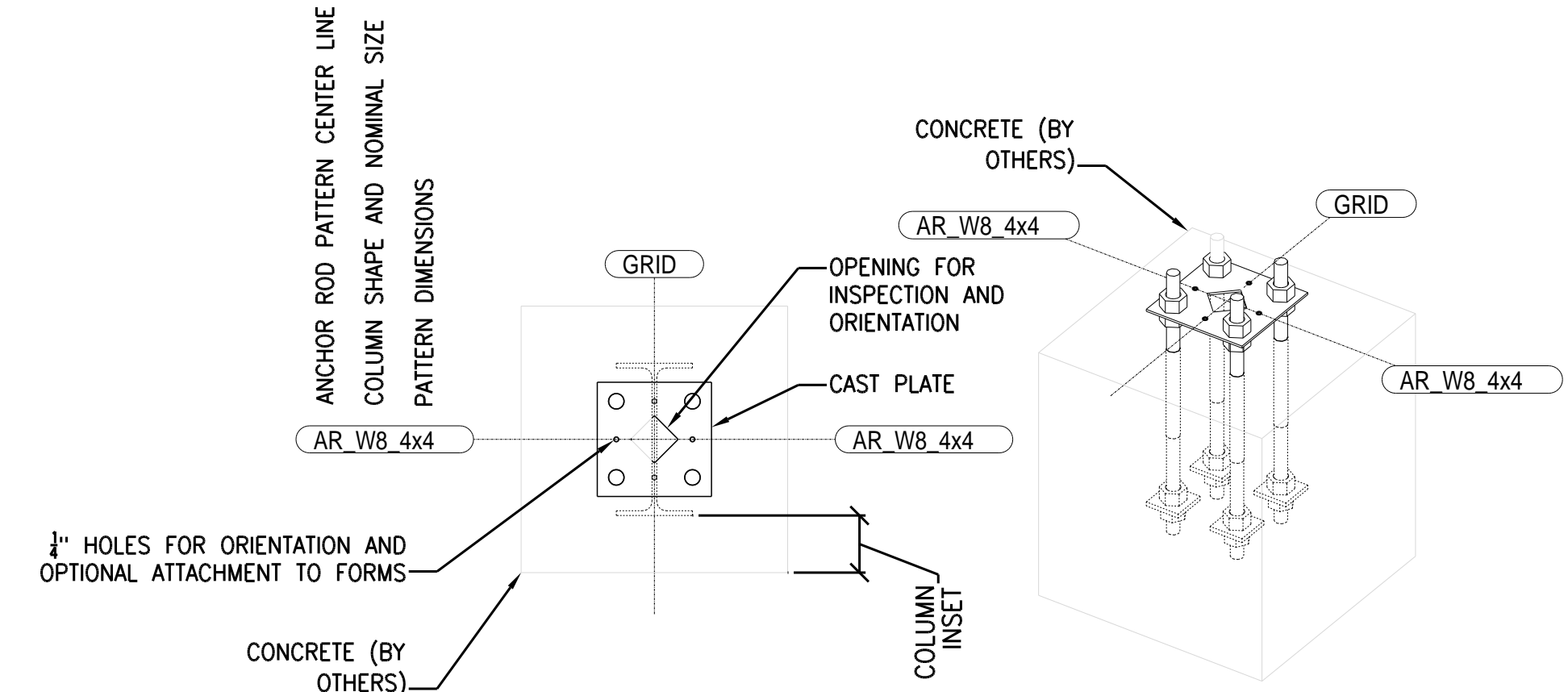
- INDICATES: CENTERLINE OF INDIVIDUAL ANCHOR BOLTS FROM SPECIFIED LOCATION HORIZONTAL DEVIATION 3/4" AND 7/8-IN. DIAMETER BOLTS: $\pm 1/4$ IN. ; 1-, 1-1/4-, AND 1-1/2-IN. DIAMETER BOLTS: $\pm 3/8$ IN. ; 1-3/4, 2-, AND 2-1/2-IN. DIAMETER BOLTS: $\pm 1/2$ IN.



ANCHOR ROD ASSEMBLY
STEP 1



CAST PLATE ASSEMBLY / GROUP TEMPLATE
STEP 2



ANCHOR ROD PLACEMENT
STEP 3

SETTING ELEVATION AND LEVELING (GENERAL CONTRACTOR)

3/4" DIAMETER ANCHOR RODS IN A 4" BY 4" PATTERN SHOWN U.N.O. FOR REFERENCE. SEE ANCHOR ROD DETAILS FOR SIZE AND PATTERN SPECIFIC TO EACH LOCATION.

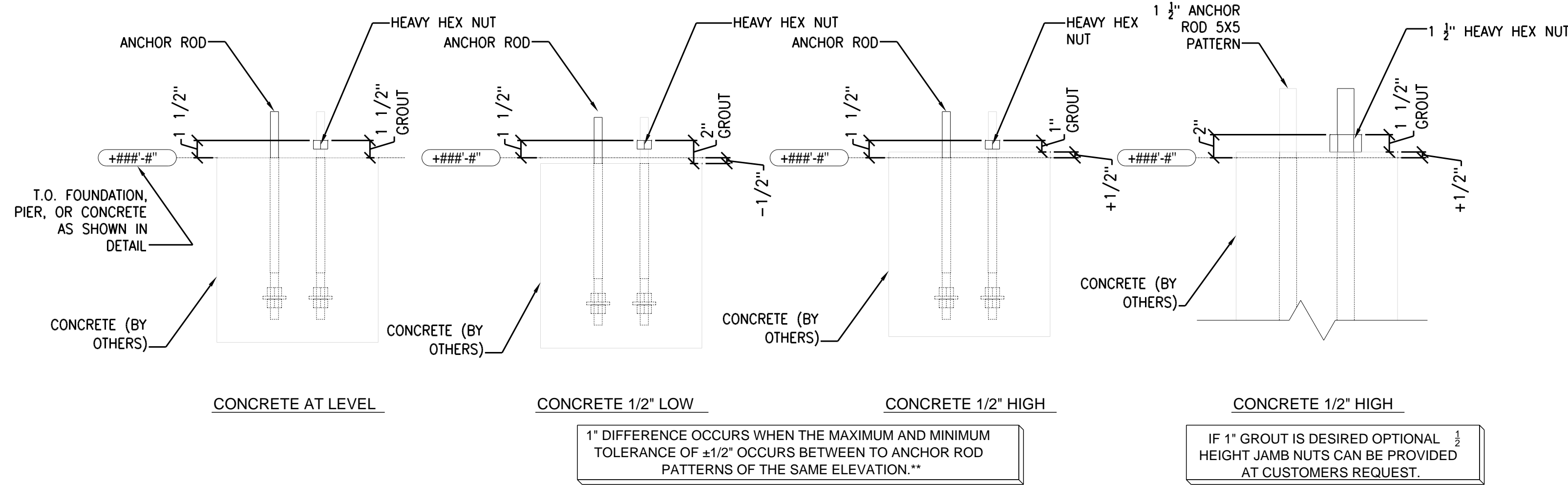
ANCHOR ROD INSTALLATION NOTES

- COMMENCEMENT OF ERECTION WORK DOES NOT CONSTITUTE ACCEPTANCE OF THE SITE, CONCRETE, OR ANCHOR BOLT PLACEMENT. ANCHOR RODS SHALL BE STRAIGHT, CLEAN, AND FREE OF THREAD DAMAGE.
- ACCURACY OF BOLT LOCATIONS IS HIGHLY CRITICAL TO AN EXPEDITED ERECTION. COSTLY DELAYS WILL OCCUR IF AN ERROR IN SQUARENESS, DIMENSIONS, OR ELEVATION PROHIBITS THE ERECTOR FROM MAINTAINING REQUIRED TOLERANCE. ANY DEVIATION FROM PLANS, DETAILS, NOTES, AND/OR TOLERANCES MUST BE COMMUNICATED TO THE METAL BUILDING SUPPLIER.
- STEEL TEMPLATES "CAST PLATE" MUST BE REMOVED BEFORE LEVELING NUTS ARE SET.
- (1) LEVELING NUT PER BOLT PATTERN MUST BE LOCATED AT BOTTOM OF BASE PLATE ELEVATION AND SPRAY PAINTED TO ESTABLISH PROPER GROUT THICKNESS AND ELEVATION PRIOR TO SETTING COLUMNS.

** 1" DIFFERENCES IN ELEVATION MAY OCCUR. LEVELING BASE PLATES TO THE LOCATION WITH THE HIGHEST CONCRETE LEVEL WILL ENSURE THE MOST EFFECTIVE GROUTING AND SIMPLEST LEVELING.

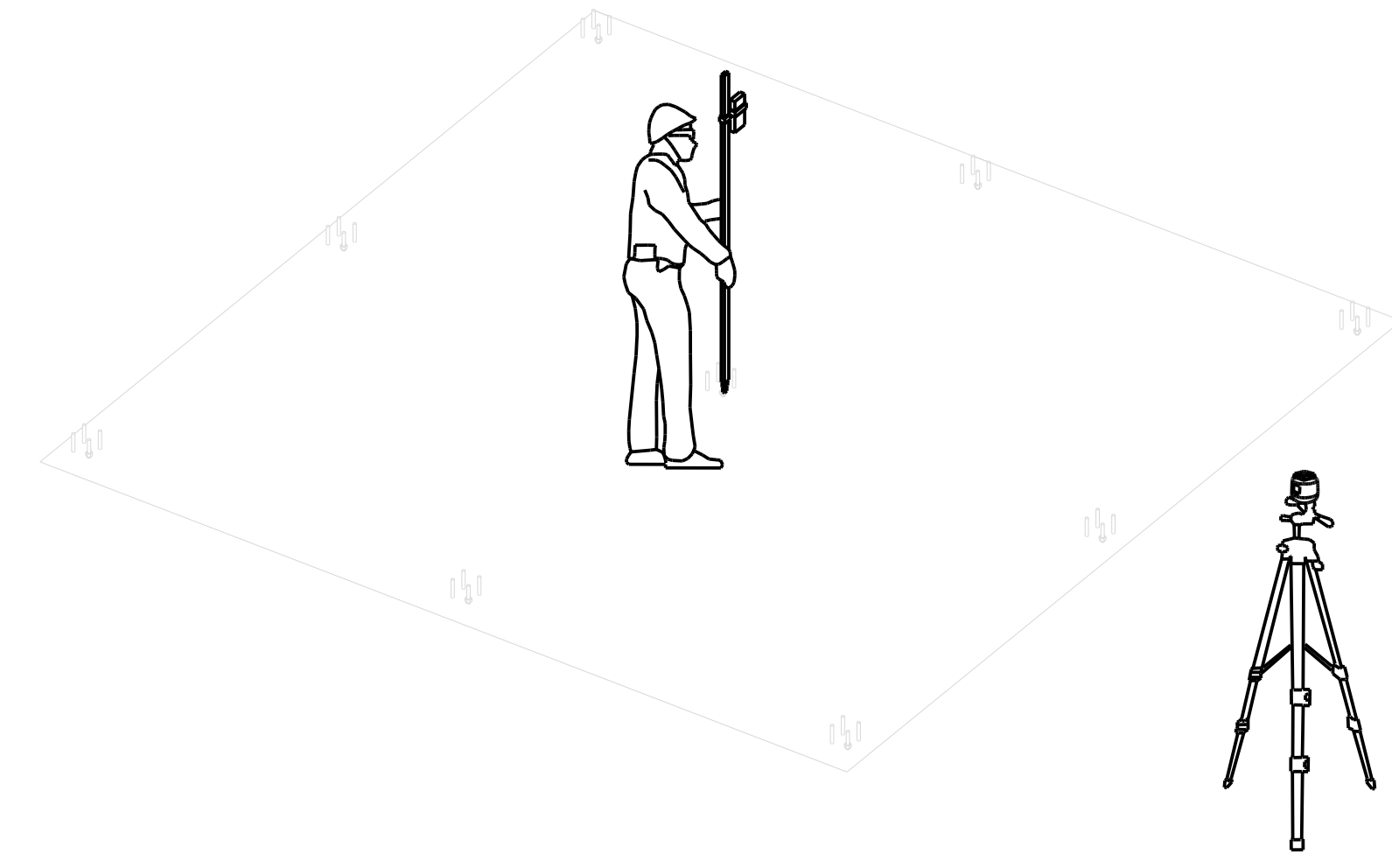
- BASE PLATES DESIGNED TO BE PLACED AT THE TOP OF 1 1/2" GROUT U.N.O. SEE NOTED GROUT THICKNESS FOR ANCHOR RODS GREATER THAN 1 1/2" DIAMETER.

- BASE PLATE LEVELING AND/OR SHIMMING TO $\pm 1/2$ IN. OF DESIGNED GROUT IS ALLOWED.



HIGHEST FOUNDATION LOCATION **
STEP 1

(CONDITIONS SHOWN ARE WITHIN TOLERANCES GIVEN BY AISC 303-16 SECTION 7.5.1 AND AISC DESIGN GUIDE 1 SECTION 2.8.) LOCATE ONE LEVELING NUT FOR THE HIGHEST BOLT PATTERN AT BOTTOM OF BASE PLATE ELEVATION AND SPRAY PAINT.)



LEVELING NUTS ON SAME ELEVATION ANCHOR ROD GROUPS
STEP 2

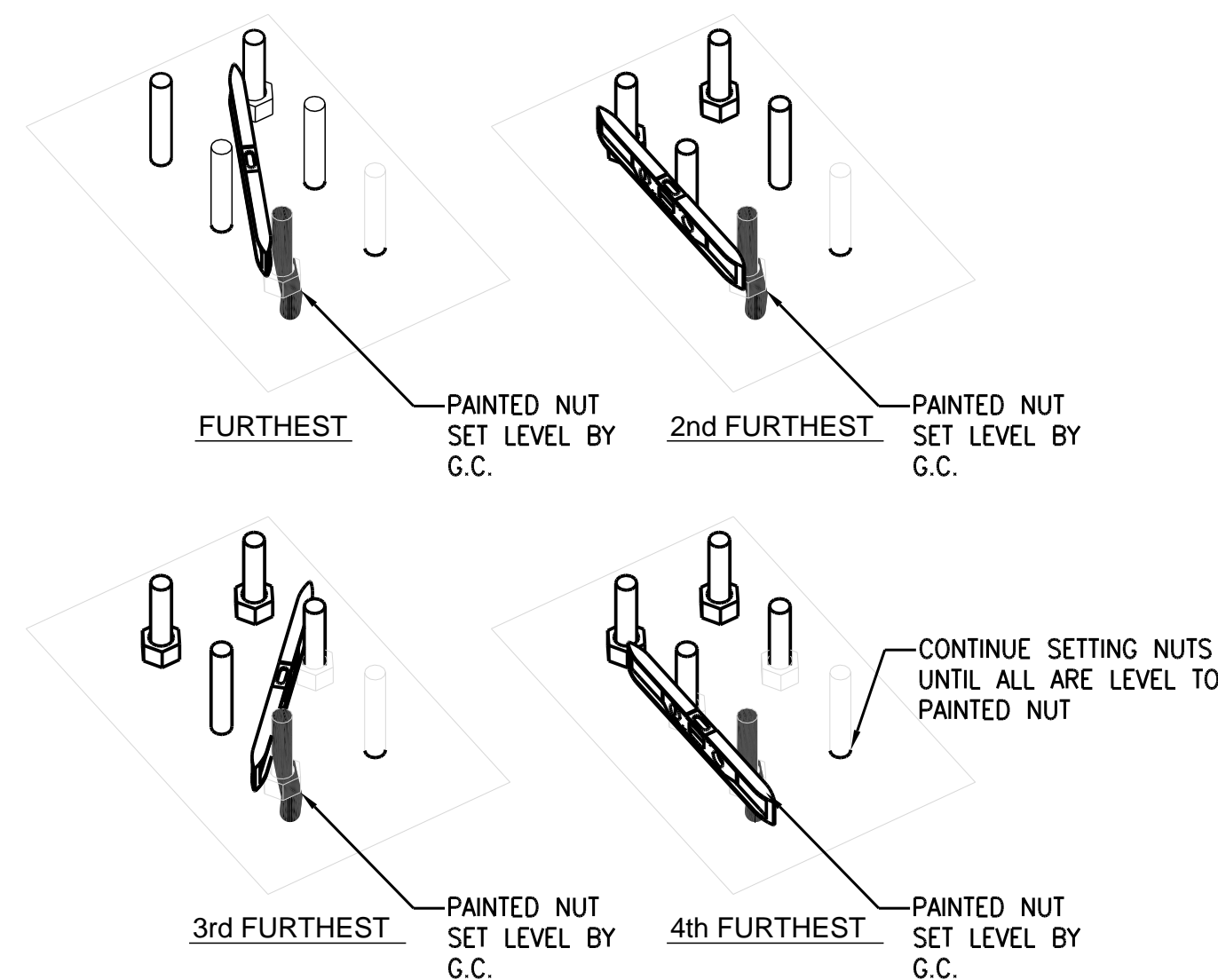
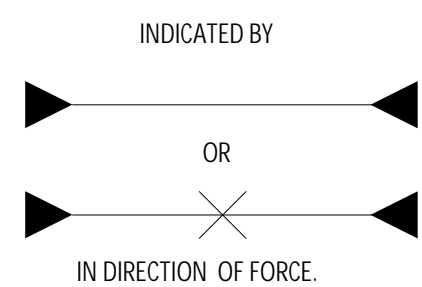
(LOCATE ONE LEVELING NUT PER BOLT PATTERN AT BOTTOM OF BASE PLATE ELEVATION ESTABLISHED AT HIGHEST LOCATION AND SPRAY PAINT.)

SETTING COLUMNS (STEEL ERECTION SUB CONTRACTOR)

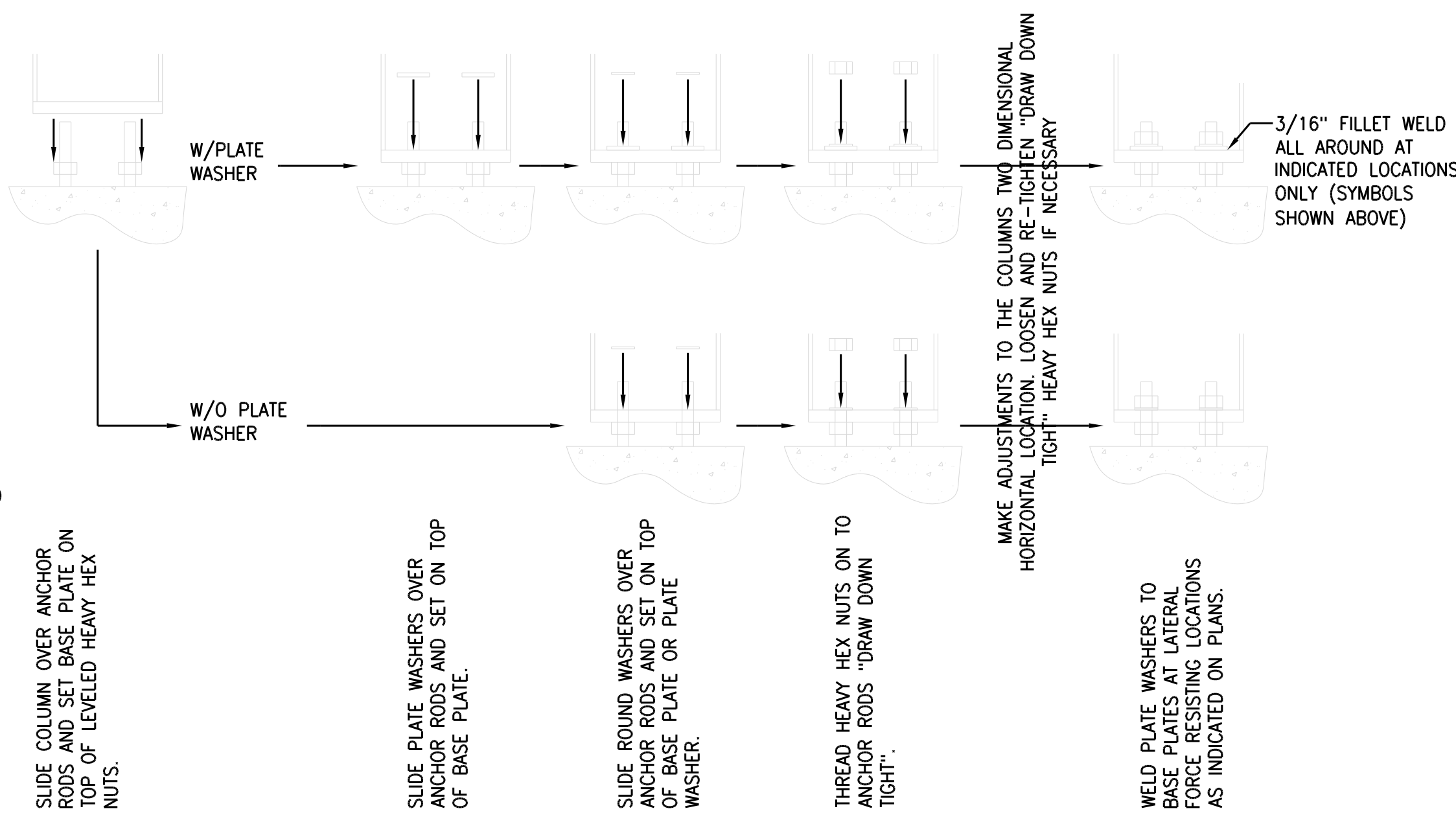
WIDE FLANGE COLUMN AND 5X5X5 PATTERN SHOWN FOR REFERENCE. SEE ROOF FRAMING PLAN, ANCHOR ROD PLAN, AND DETAILS FOR COLUMN TYPE AND SIZE SPECIFIC TO EACH LOCATION.

COLUMN ERECTION NOTES

- VERIFY THAT THE DRAWINGS IN POSSESSION ARE FOR CONSTRUCTION AND HAVE THE MOST CURRENT REVISIONS
- VERIFY ELEVATION NUT IS SET AND PAINTED BY THE GENERAL CONTRACTOR AT ALL ANCHOR ROD PATTERNS BEFORE STAGING STEEL.
- NON-SHRINKING GROUT MUST BE INSTALLED PRIOR TO ANY ADDITIONAL LOADING TO THE STEEL FRAMING.
- PLATE WASHERS AT OVERSIZED BASE PLATE HOLES MUST BE WELDED AT ALL ANCHOR ROD GROUPINGS WITH LATERAL FORCE RESISTING SYSTEMS (LFRS) INDICATED. NON LATERAL FORCE RESISTING SYSTEMS DO NOT REQUIRE PLATE WASHERS TO BE WELDED AT ANCHOR ROD GROUPINGS. LATERAL FORCE RESISTING SYSTEM INDICATED BY



SETTING LEVELING NUTS IN ANCHOR ROD PATTERNS
STEP 1



STANDING COLUMNS
STEP 2

GROUTING COLUMNS (GENERAL CONTRACTOR)

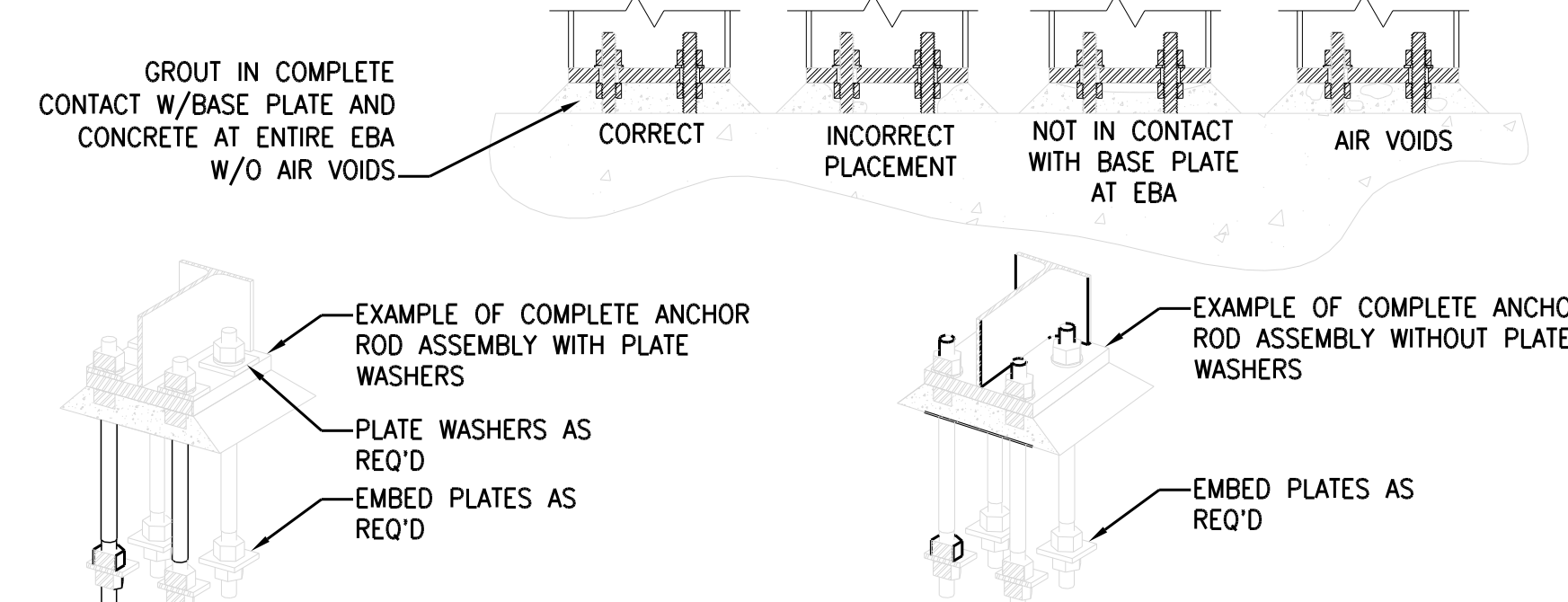
3/4" DIAMETER ANCHOR RODS IN A 4" BY 4" PATTERN SHOWN U.N.O. FOR REFERENCE.

ADJUST HEAVY HEX LEVELING NUTS AND LOOSENING/RE-TIGHTENING "DRAW DOWN TIGHT" HEAVY HEX ANCHOR NUTS TO PLUMB THE COLUMN WHILE INSTALLING BRACING ELEMENTS WILL HELP TO ENSURE PROPER SQUARENESS, DIMENSIONS, AND LOCATION.

AISC SECTION 16.3-40 PARAGRAPH 7.7. GROUTING SHALL BE THE RESPONSIBILITY OF THE OWNER'S DESIGNATED REPRESENTATIVE FOR CONSTRUCTION (G.C.). LEVELING PLATES AND LOOSE BASE AND BEARING PLATES SHALL BE PROMPTLY GROUTED AFTER THEY ARE SET AND CHECKED FOR LINE AND GRADE. COLUMNS WITH ATTACHED BEARING PLATES, BEAMS WITH ATTACHED BEARING PLATES AND OTHER SIMILAR MEMBERS WITH ATTACHED BEARING DEVICES THAT ARE TEMPORALLY SUPPORTED ON LEVELING NUTS AND WASHERS, SHIMS OR OTHER SIMILAR LEVELING DEVICES, SHALL BE PROMPTLY GROUTED AFTER THE STRUCTURAL STEEL FRAME OR PORTION THEREOF HAS BEEN PLUMBED.

AISC SECTION 14-7 "LEVELING BOLTS OR NUTS SHOULD NOT BE USED TO SUPPORT THE COLUMN DURING ERECTION. IF GROUTING IS DELAYED UNTIL AFTER STEEL ERECTION, THE BASE PLATE MUST BE SHIMMED TO PROPERLY DISTRIBUTE LOADS TO THE FOUNDATION WITHOUT OVERSTRESSING EITHER THE BASE PLATE, [LEVELING NUTS], OR THE CONCRETE."

EFFECTIVE BEARING AREA (EBA) IS DEFINED BY THE AREA DIRECTLY UNDER THE COLUMN BASE PLATE. THE EBA MUST MATCH THE AREA OF THE BASE PLATE IN ORDER TO PROPERLY TRANSMIT LOAD FROM THE COLUMN TO THE FOUNDATION/PIER. THE EBA CAN BE AFFECTED BY GROUT LEAKAGE, UNLEVELED PLATE, SHRINKAGE OF GROUT, INADEQUATE MIXING OF GROUT, INCORRECT PLACEMENT METHOD, POOR GROUT FLOW OR ENTRAPPED AIR VOIDS. EBA CAN LEAD TO POINT LOADING ON THE GROUT WHICH IN TURN CAN RESULT IN CRACKING AND CRUMBLING OF THE GROUT UNDER THE BASE PLATE. THIS COULD HAVE DISASTROUS CONSEQUENCES.



PLUMBING AND GROUTING COLUMNS
STEP 3

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Drawn by: AML
Checked by: J.L.



05/12/2023

THE GOOSE

2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT

GENERAL ANCHOR ROD DETAILS

By	Date	Description
AML	04/18/2023	Issued For Approval
AML	05/10/2023	Issued For Permit

DATE 05/10/2023

PROJ. NO. 23002

DRAWING NO.

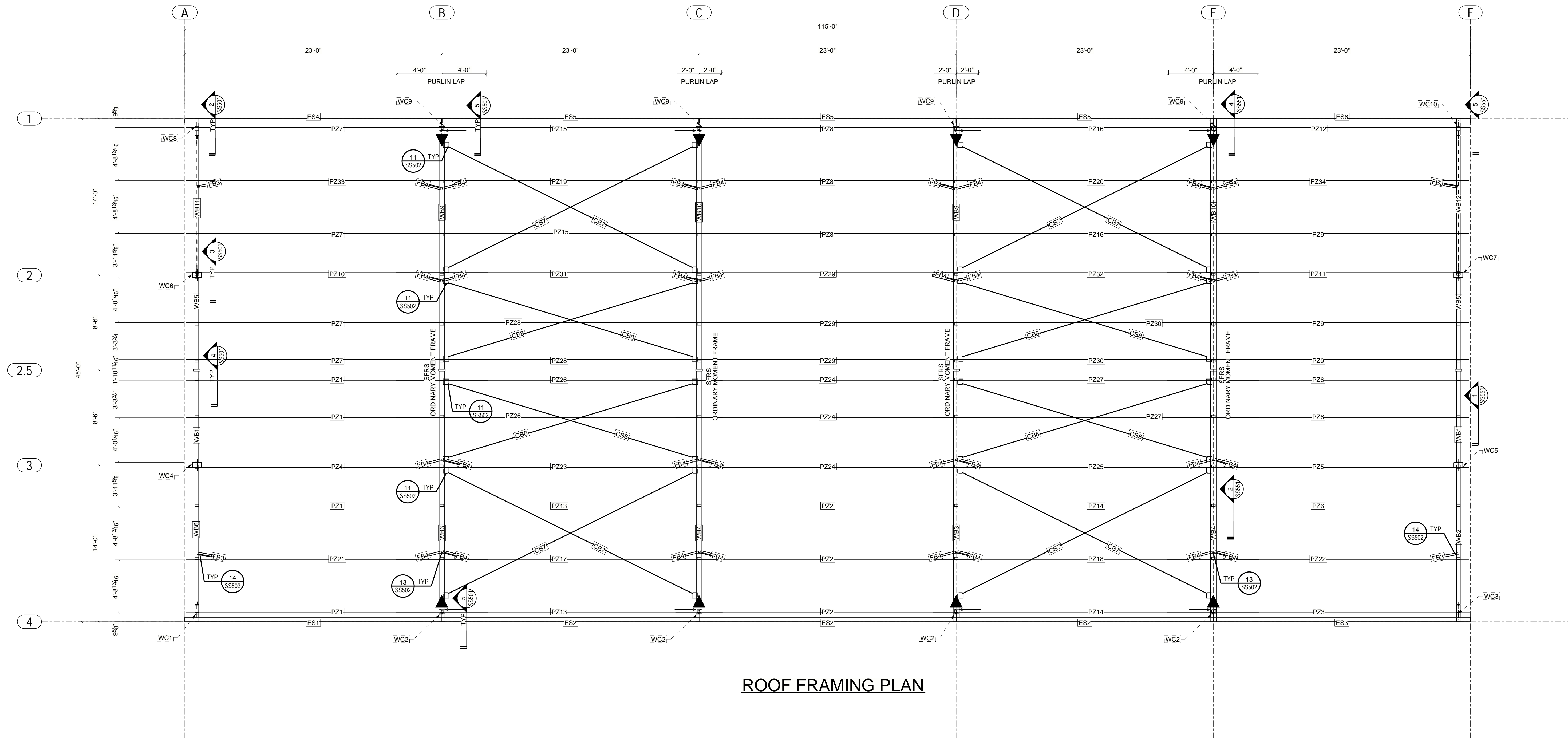
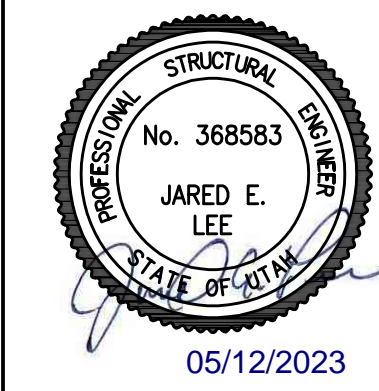
SS251

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Drawn by:
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Checked by:
JL



ROOF FRAMING PLAN

- ROOF NOTES:
1. ROOF SLOPE = 4:12
 2. PURLINS ALIGNED WITH TOP FLANGE DIRECTED TOWARD HIGH EAVE
 3. ▲ INDICATES MOMENT CONNECTION

THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT
ROOF FRAMING PLAN

Mark	Description	Date	By
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DATE 05/10/2023

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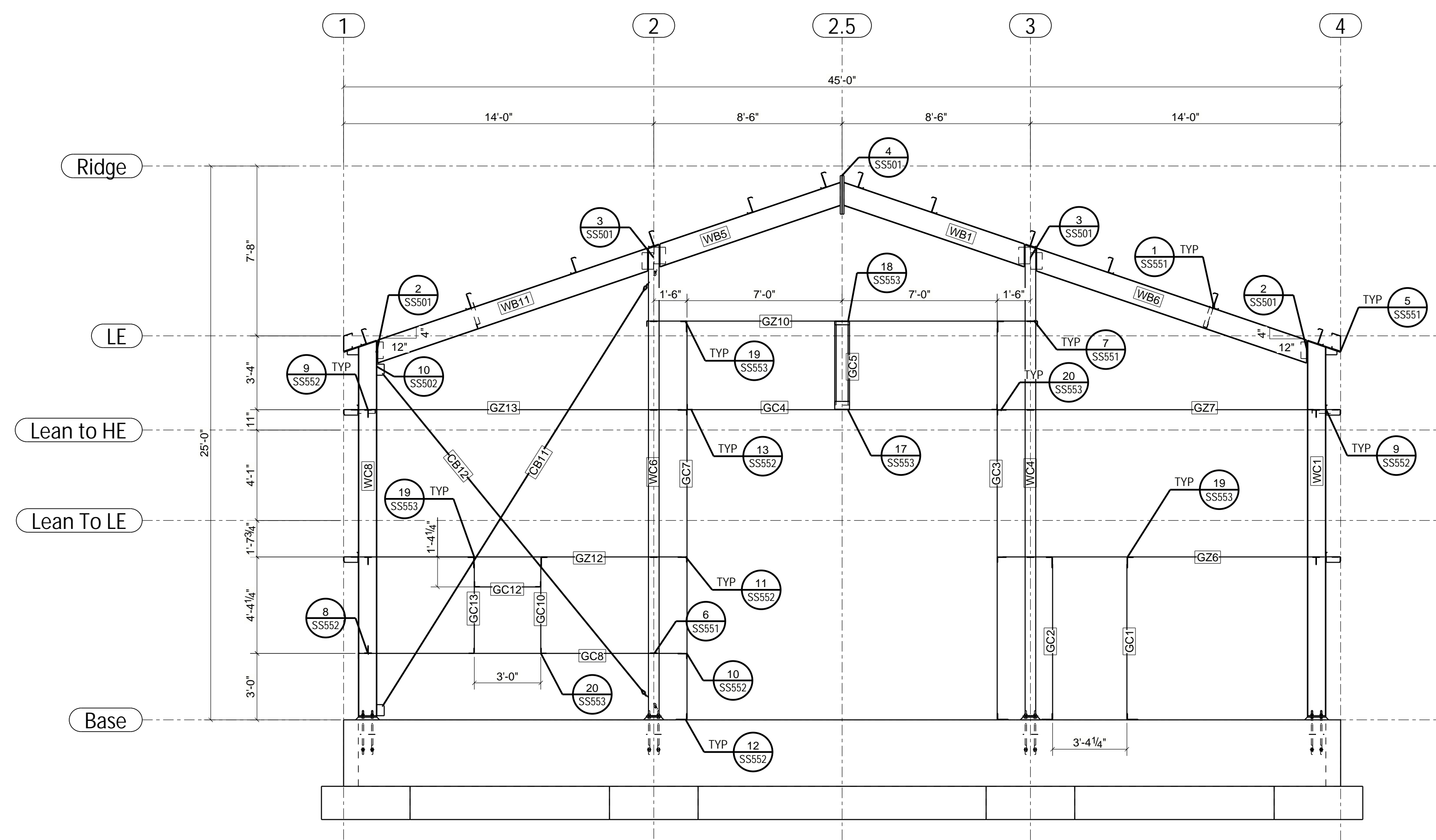
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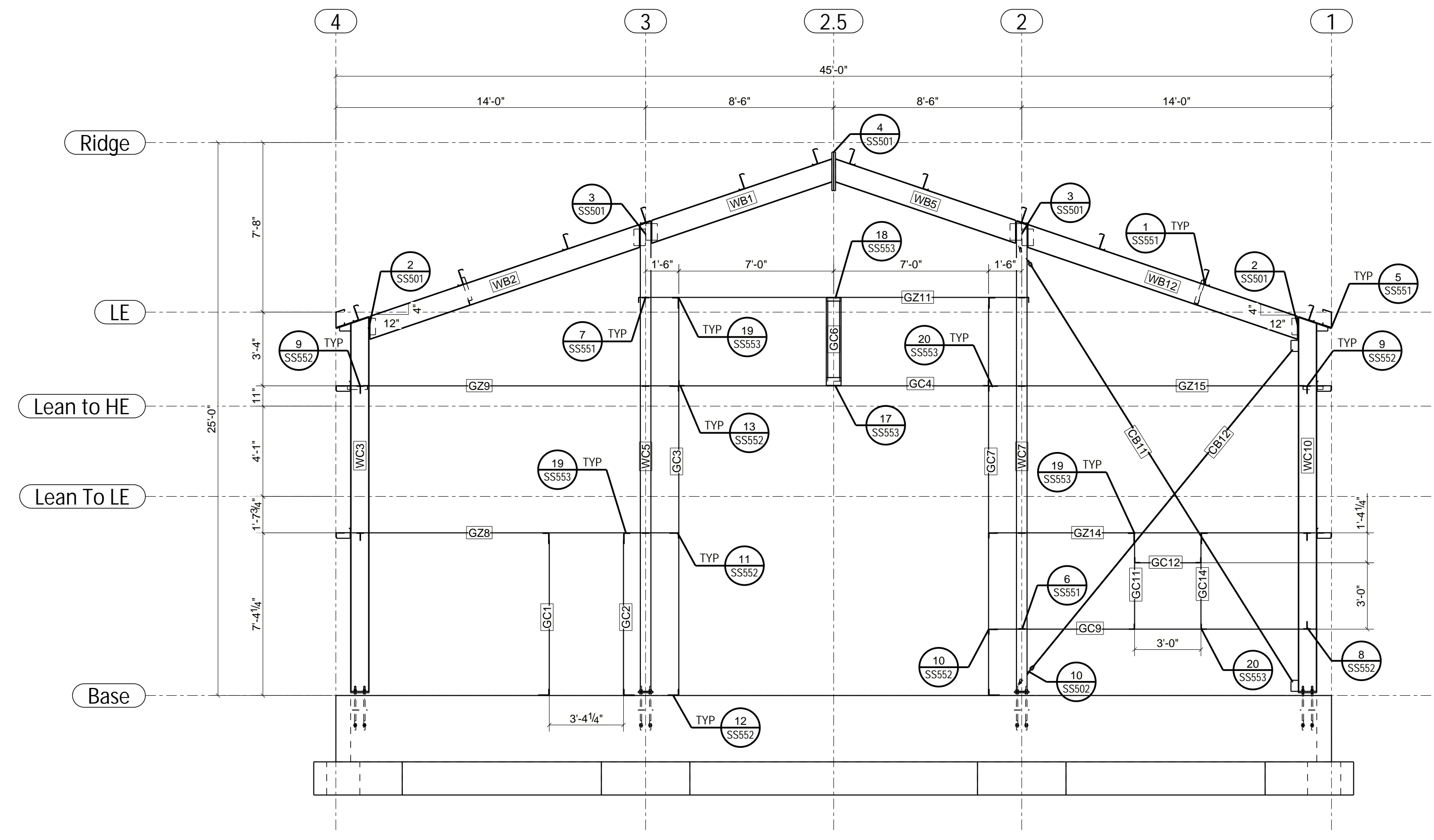


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ELEVATION ON GRID A



ELEVATION ON GRID F

THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT
ELEVATION @ GRID A & F

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PROJ. NO. 23002

DRAWING NO.

SS401

Table Structures

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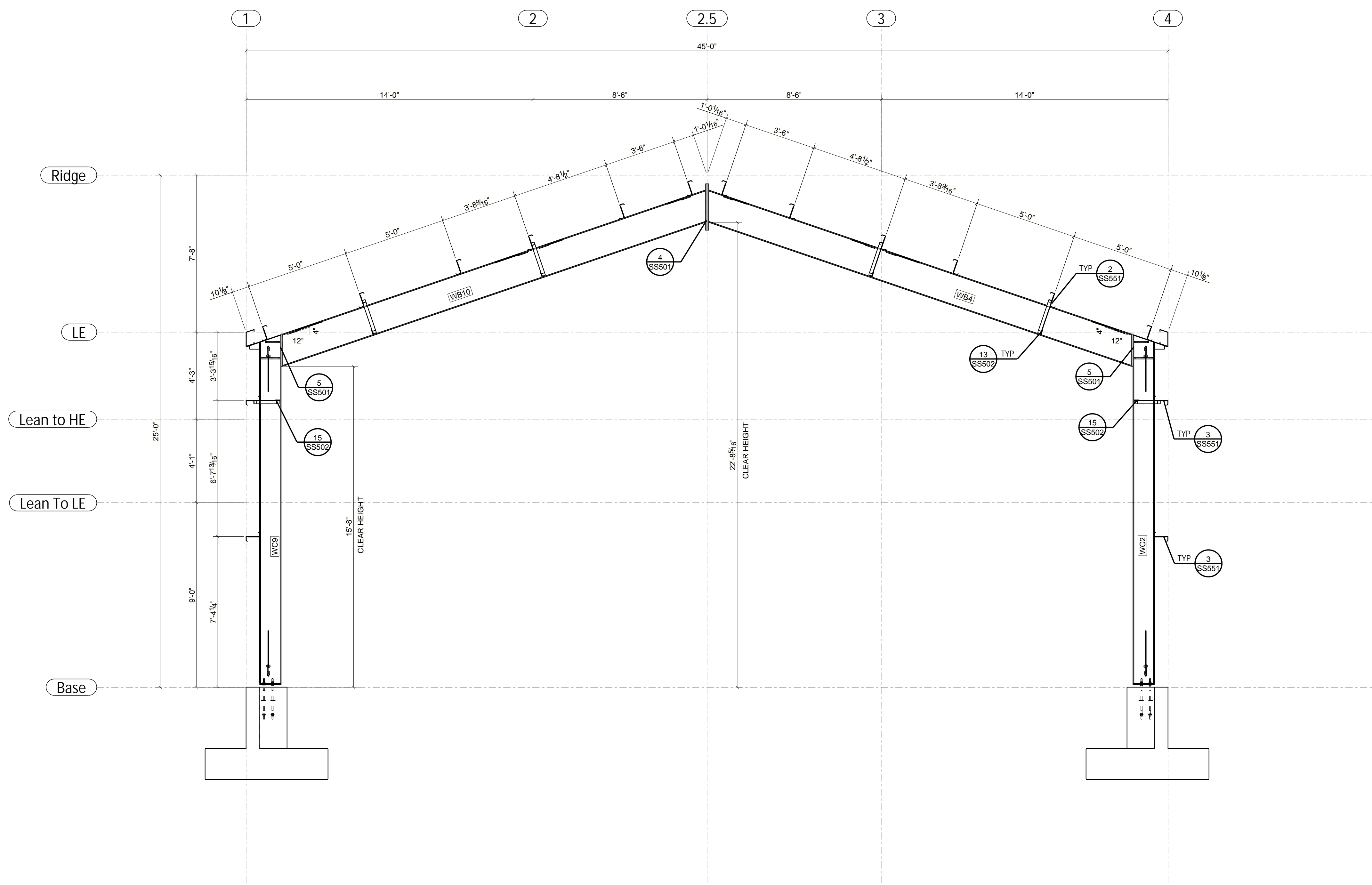


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CROSS SECTION @ GRID 3

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CROSS SECTION @ GRID 3

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A	Issued For Approval	04/18/2023	AML
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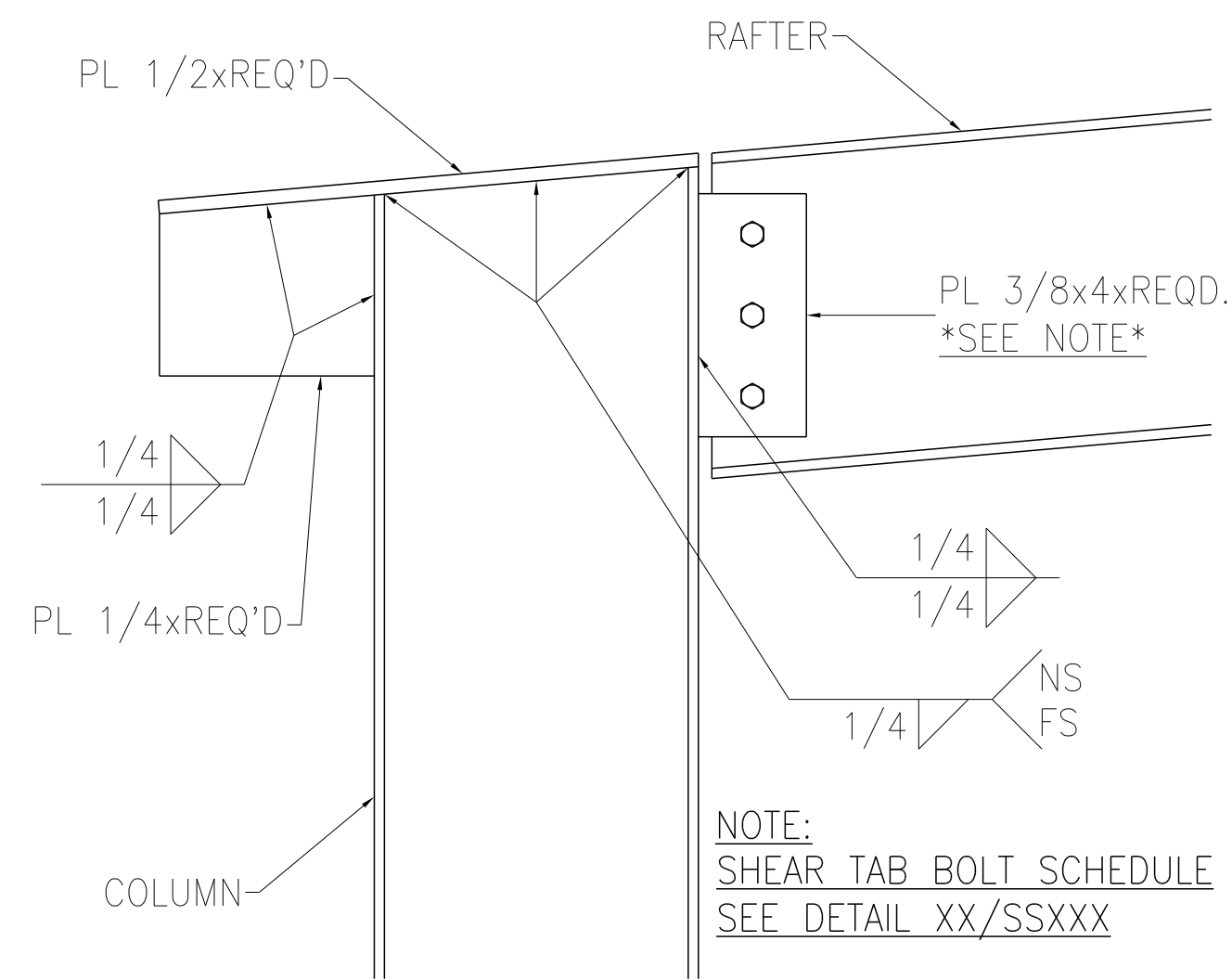
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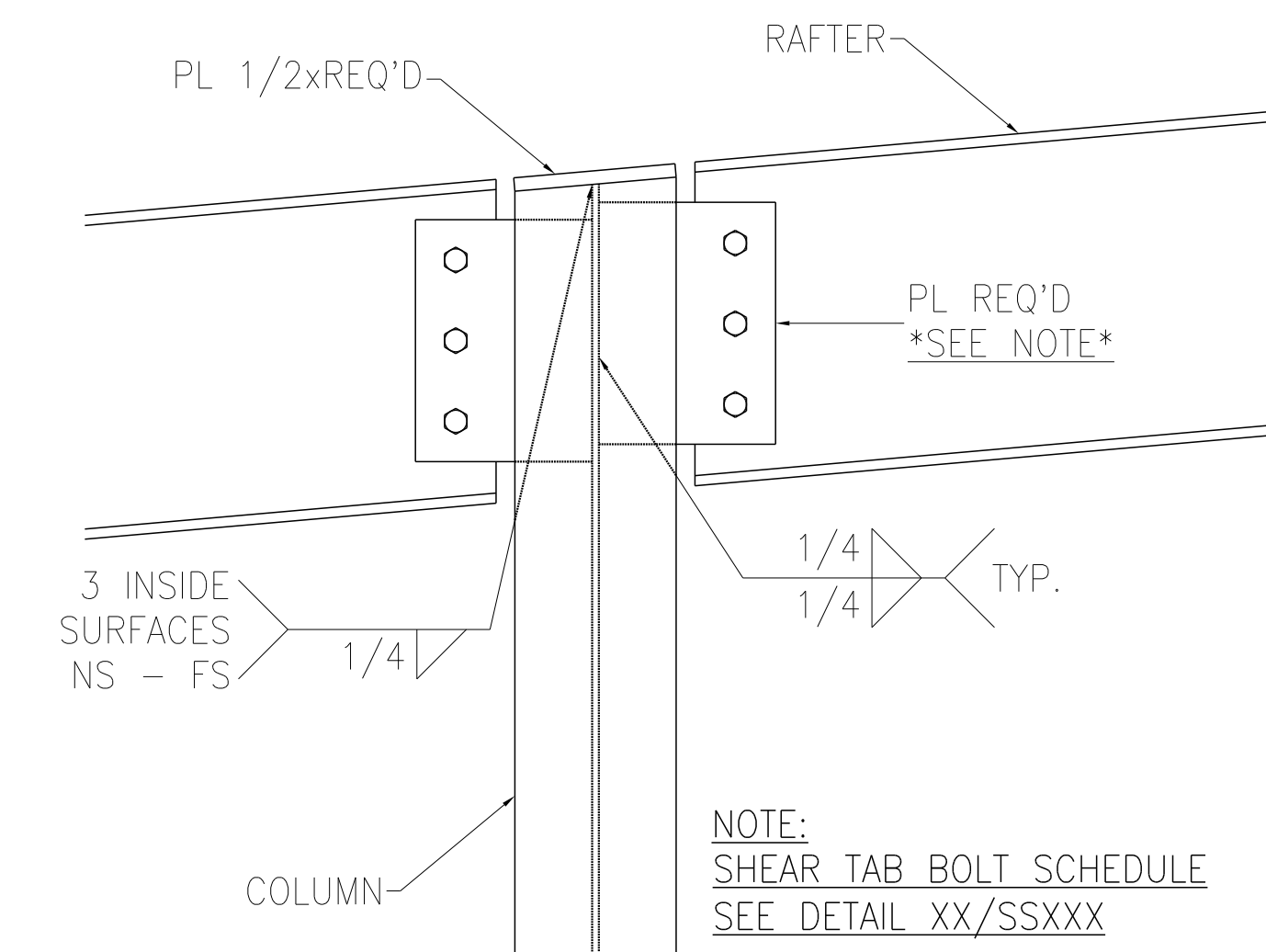
SS403

SHEAR TAB BOLT SCHEDULE					
WIDE FLANGE BEAM	# OF BOLTS	BOLT ϕ	# OF ROWS	SHEAR TAB THICKNESS	SHEAR TAB WELD
W8 & W10	2	3/4" ϕ	1	3/8"	1/4
W12 & W14	3	3/4" ϕ	1	3/8"	1/4
W16	4	3/4" ϕ	1	3/8"	1/4
W18	5	3/4" ϕ	1	3/8"	1/4
W21	6	3/4" ϕ	1	3/8"	5/16"
W24	7	3/4" ϕ	1	1/2"	5/16"
W24**	7	1" ϕ	2	3/4"	3/8"
W30	8	1" ϕ	1	3/4"	3/8"

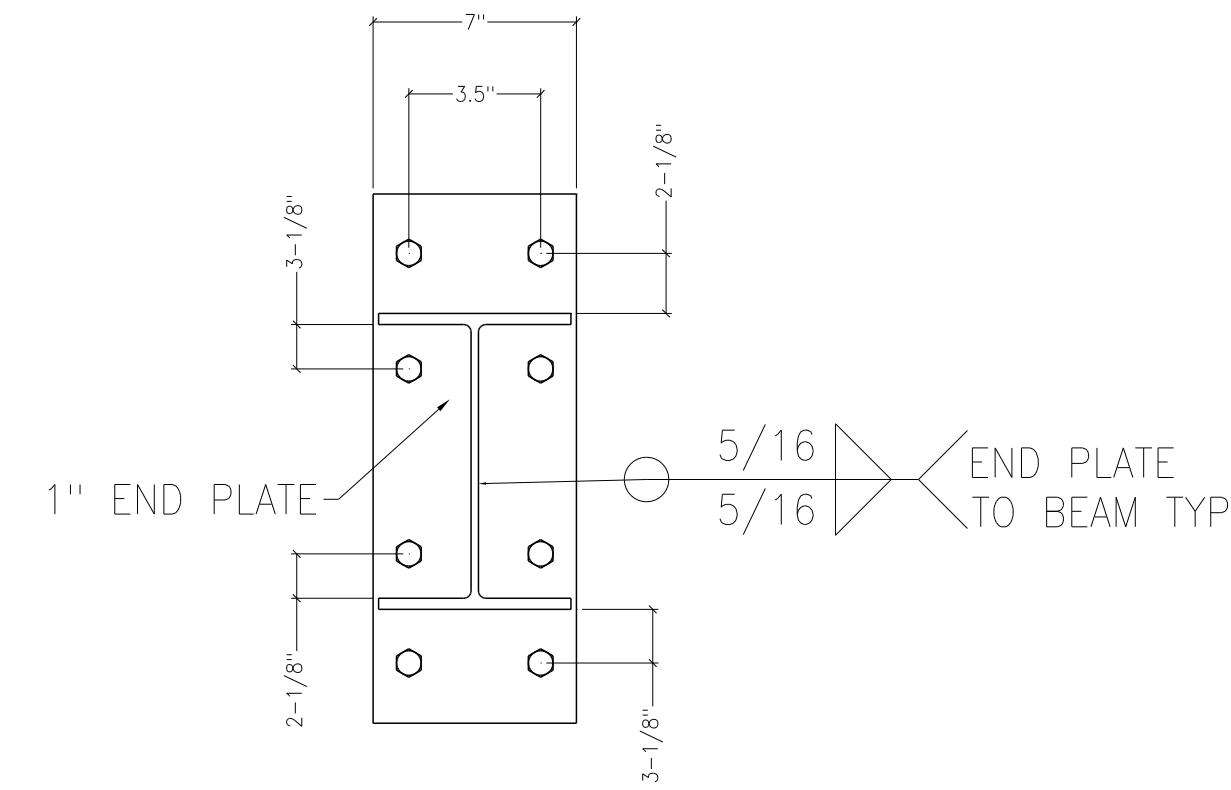
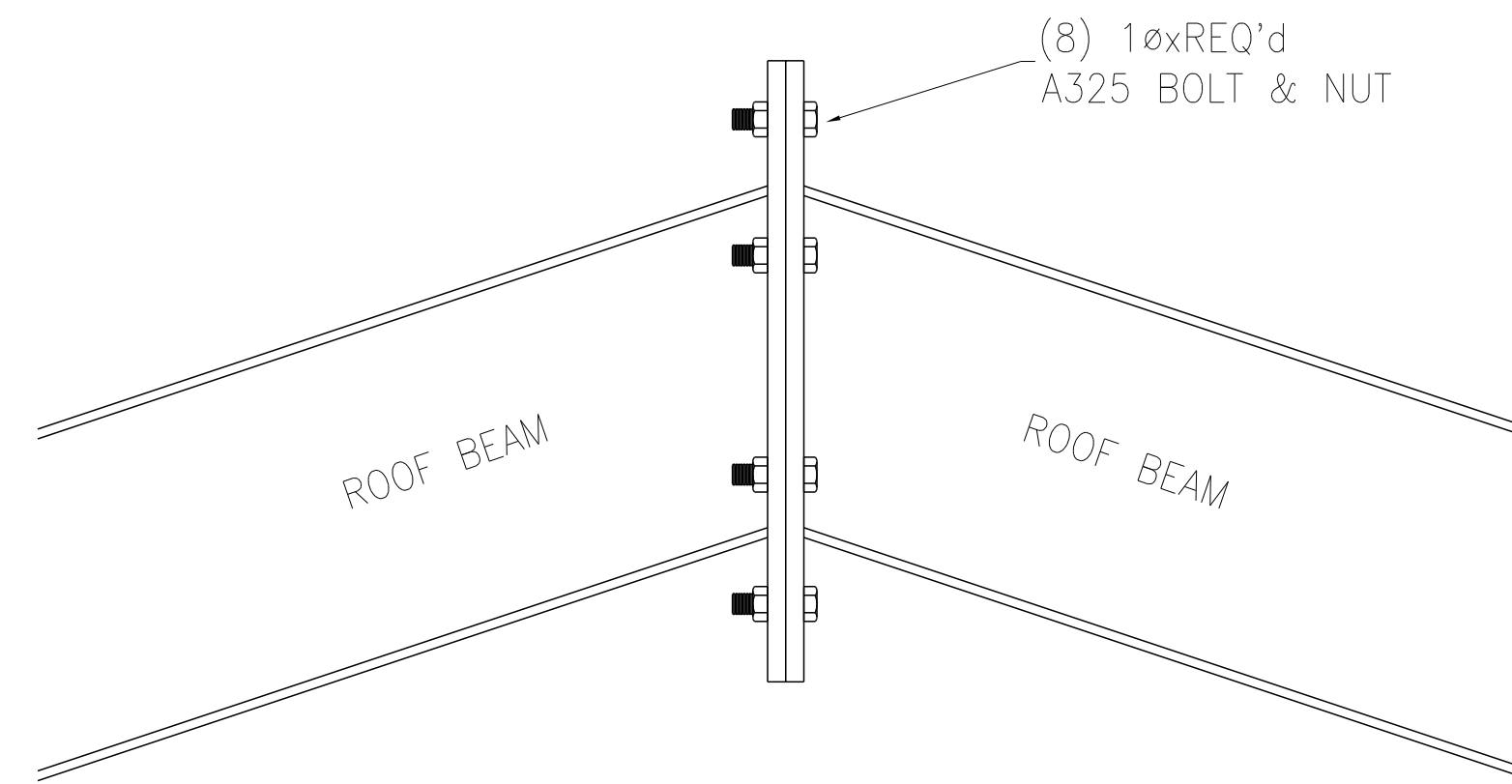
1 SS501 SHEAR TAB BOLT SCHEDULE



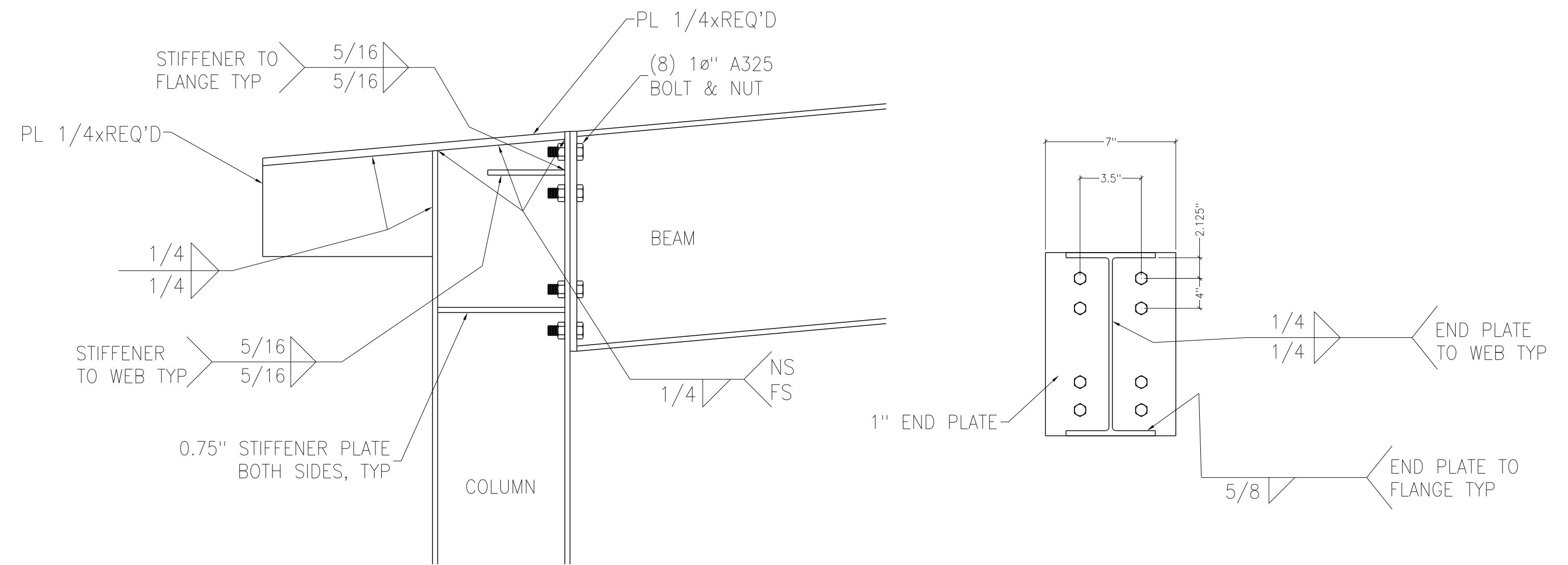
2 SS501 RAFTER TO COLUMN



3 SS501 RAFTER TO COLUMN



4 SS501 RIDGE BEAM SPLICE END PLATE MOMENT CONNECTION



5 SS501 END WALL COLUMN AND BEAM END PLATE MOMENT CONNECTION

6 SS501

7 SS501

8 SS501

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THE GOOSE

2103 N. 5600 E.
SUNNYFIELD FARM EDEEN,UT

PRIMARY FRAMING DETAILS

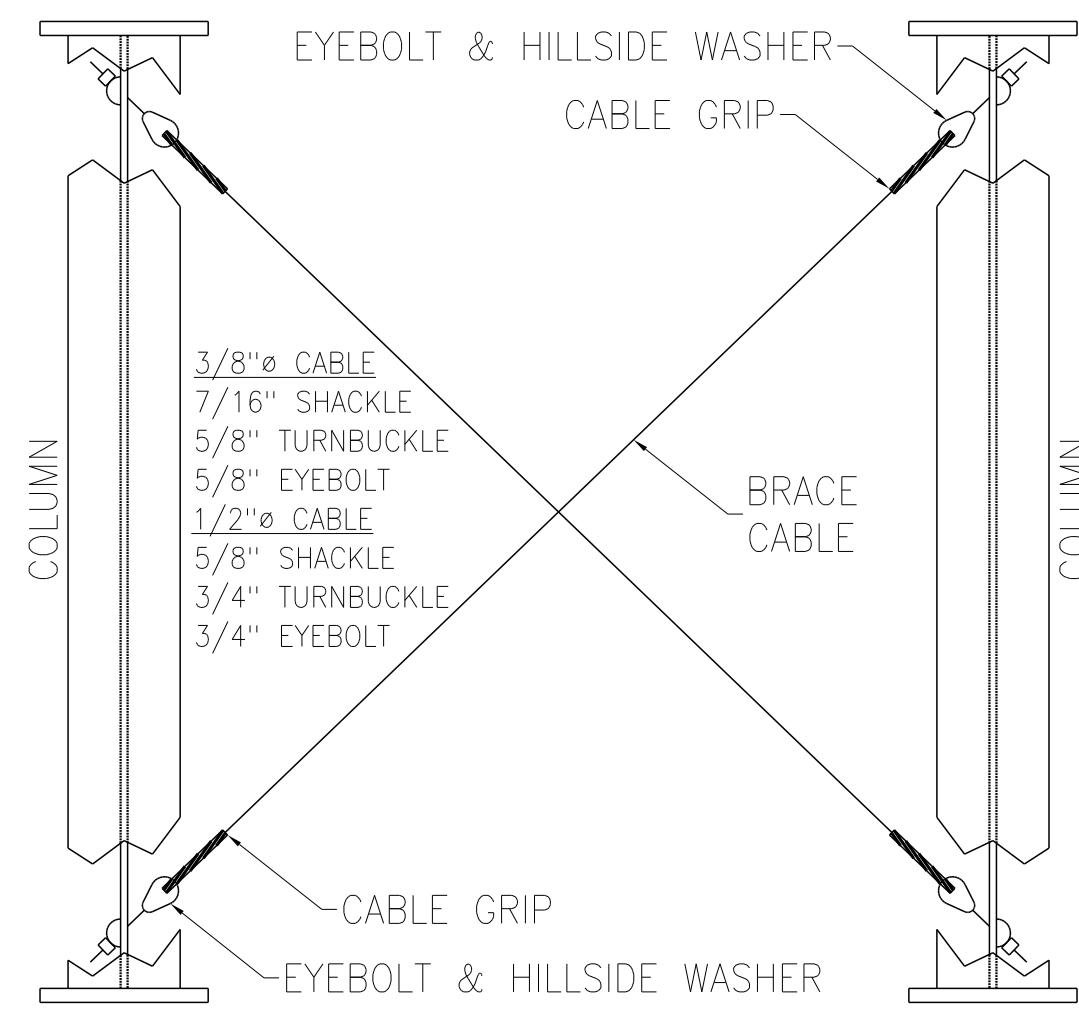
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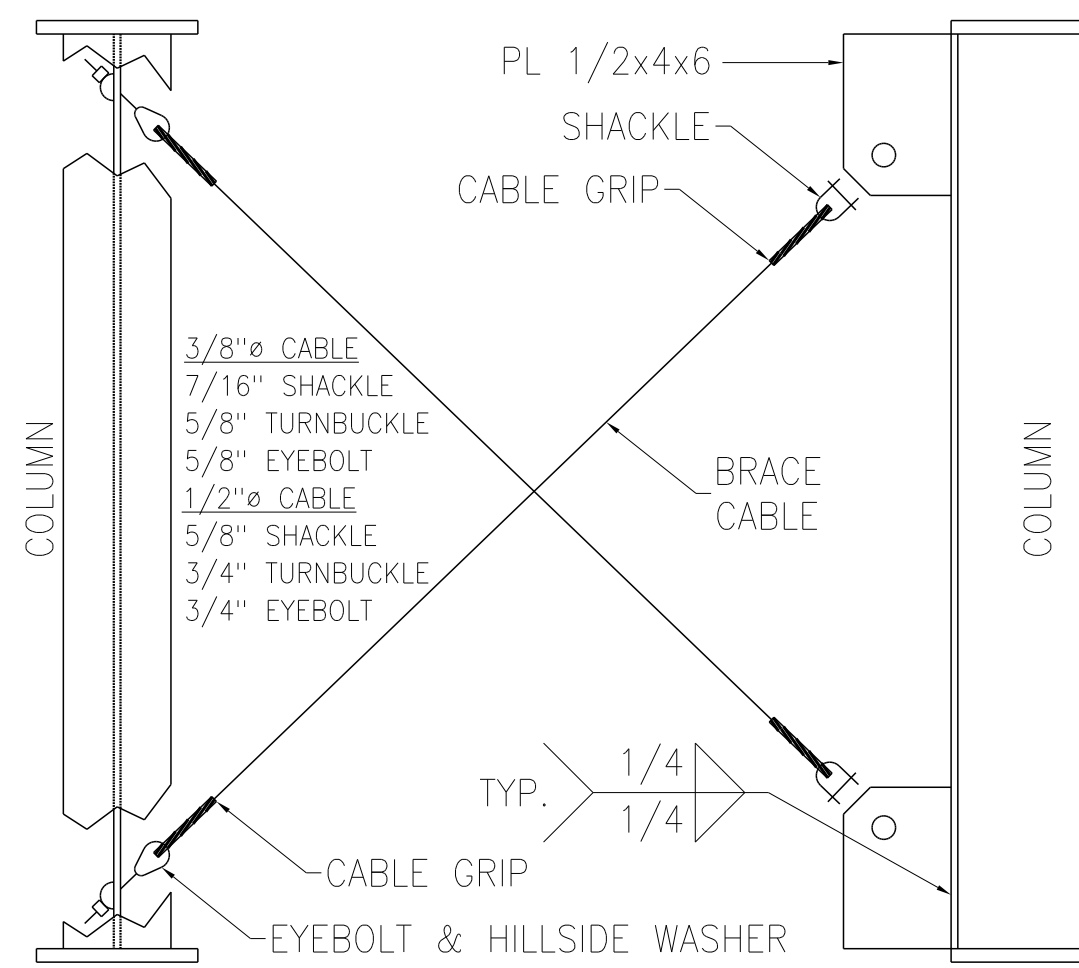
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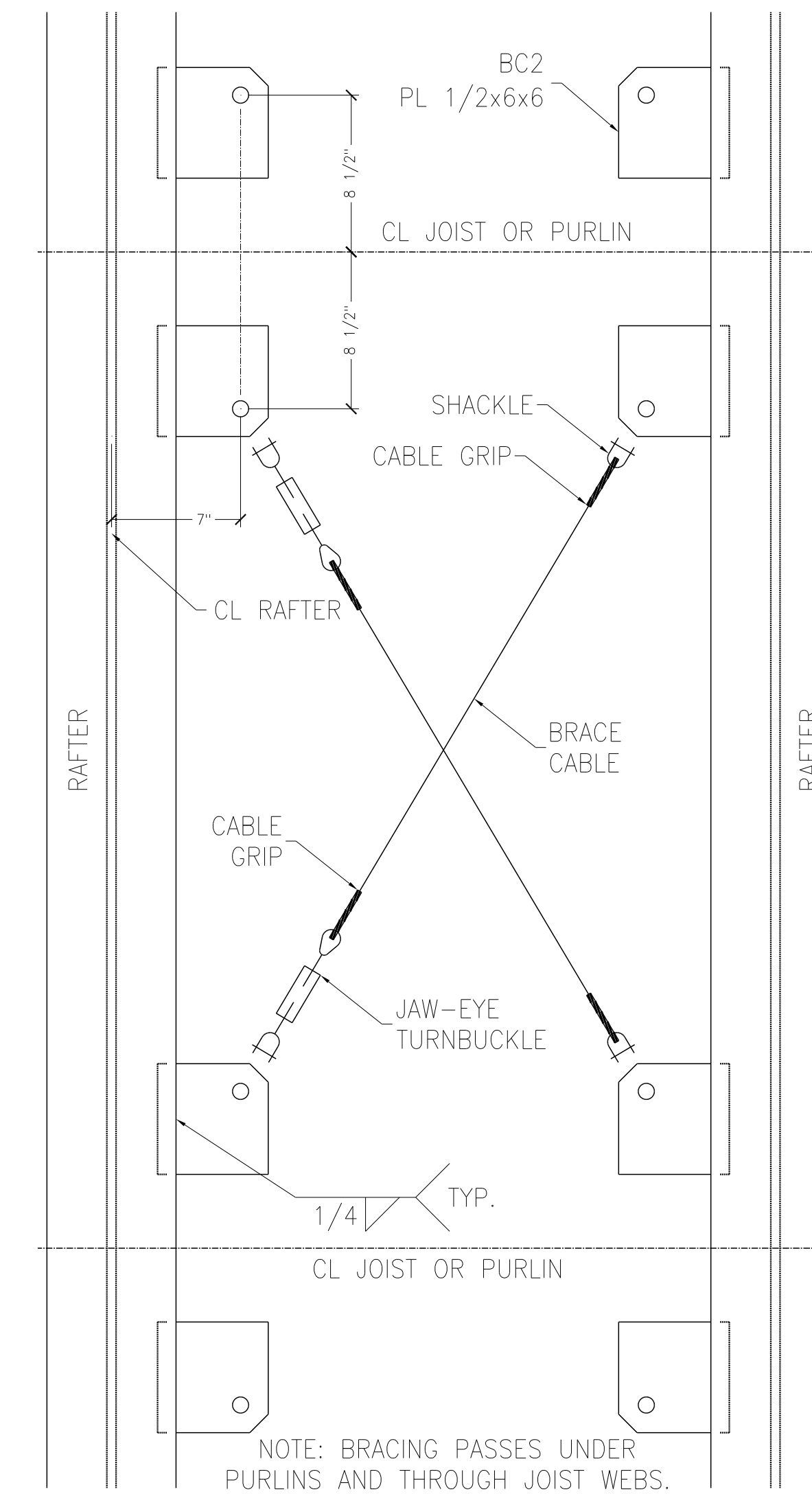
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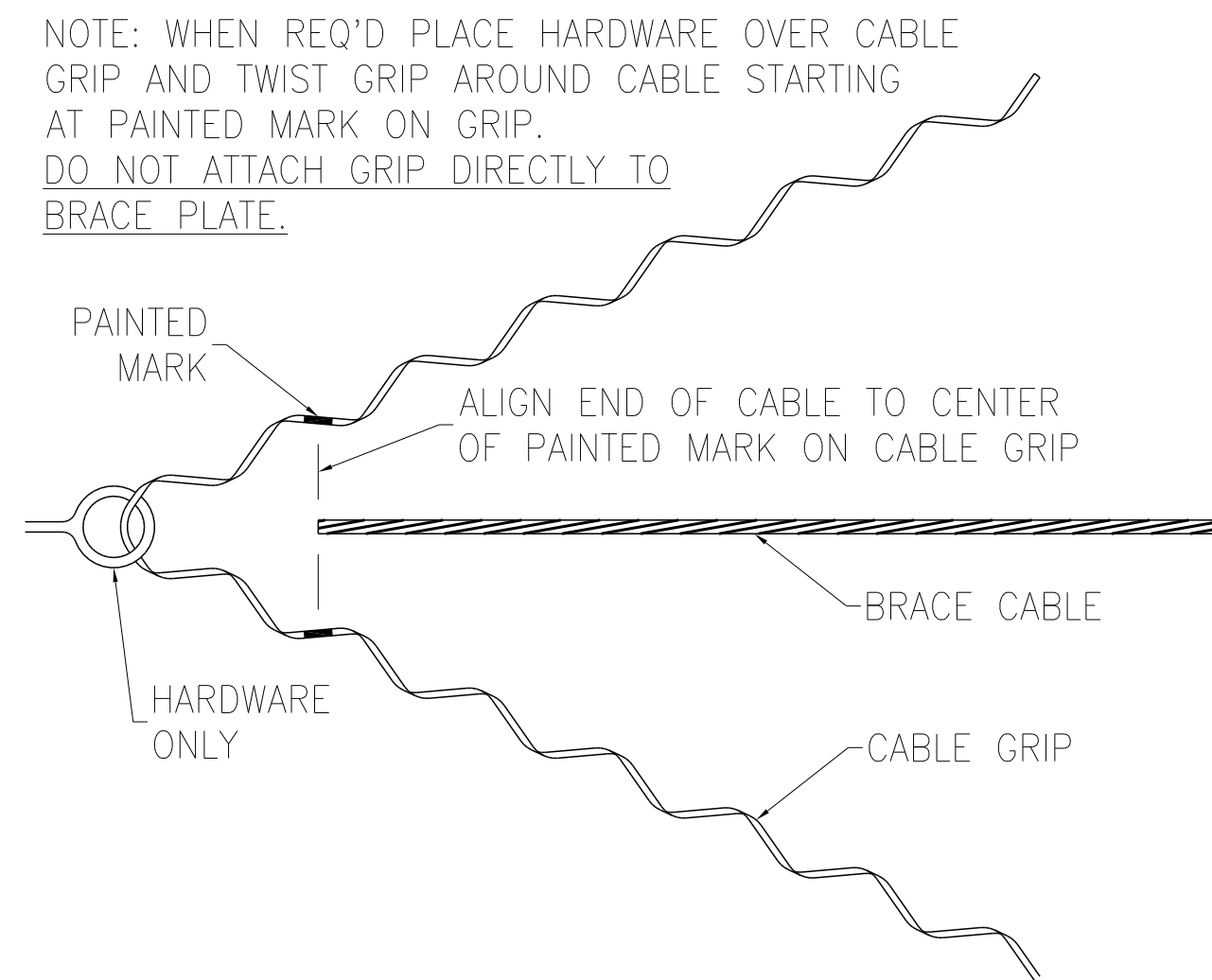
9 CABLE BRACING TO COLUMN
SS502



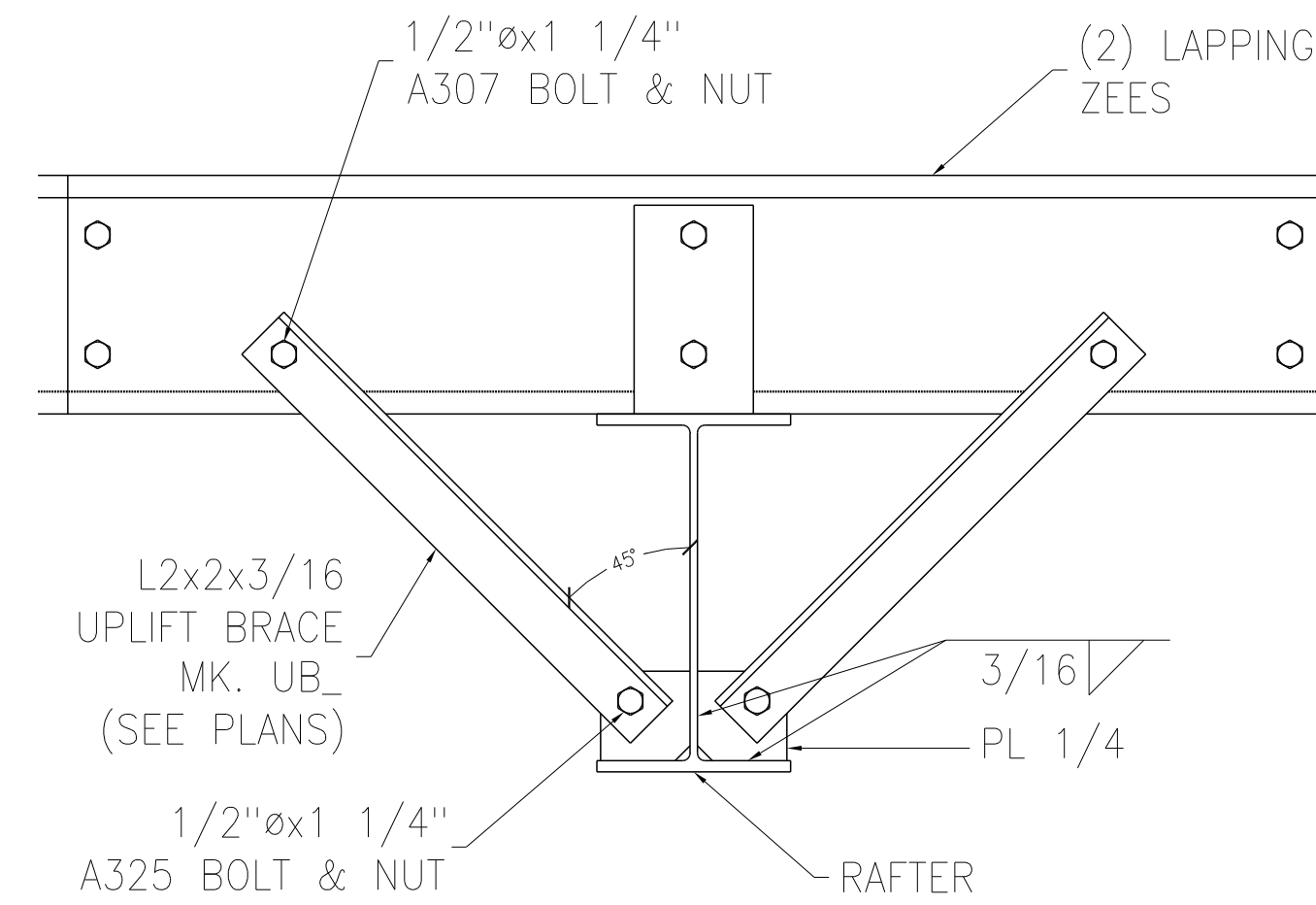
10 CABLE BRACING TO COLUMN
SS502



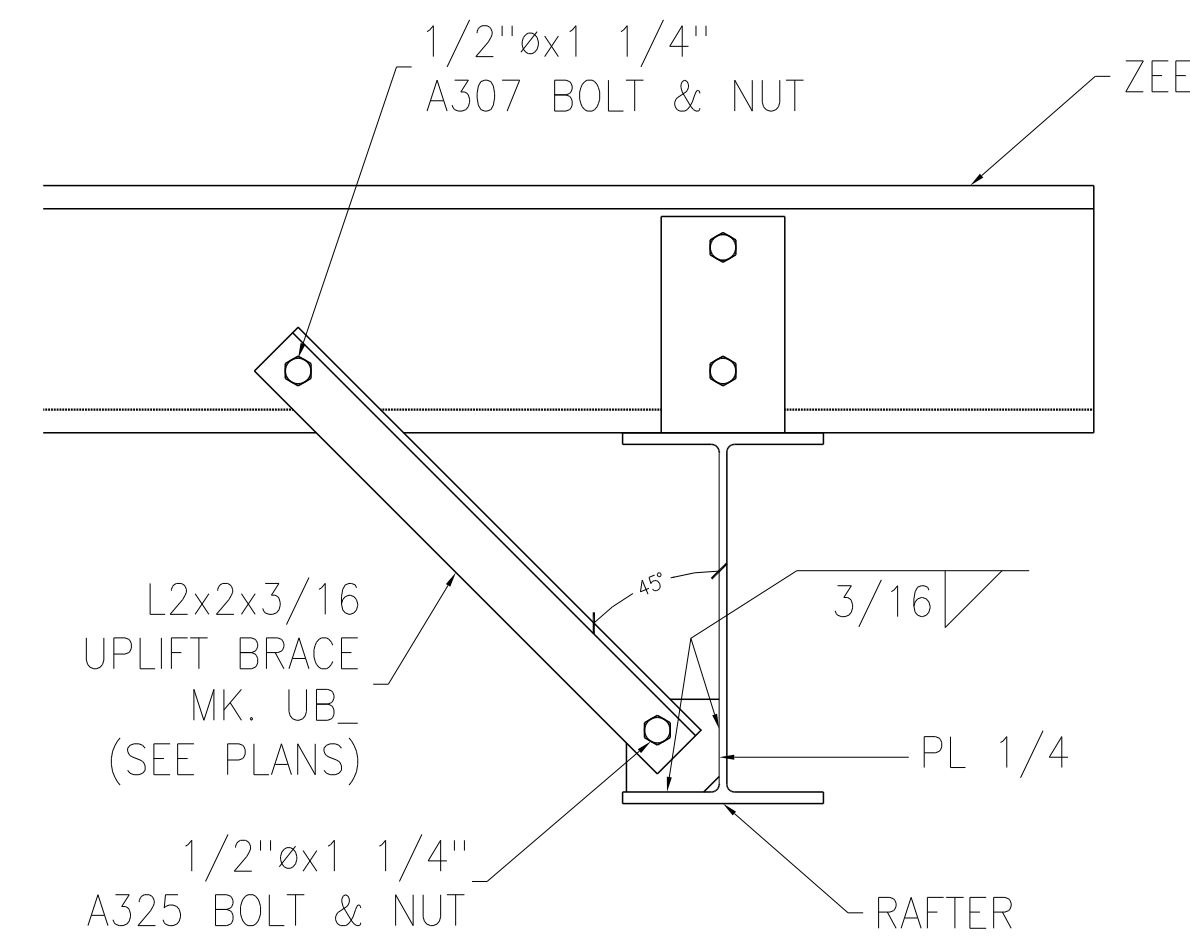
11 CABLE BRACING TO RAFTER
SS502



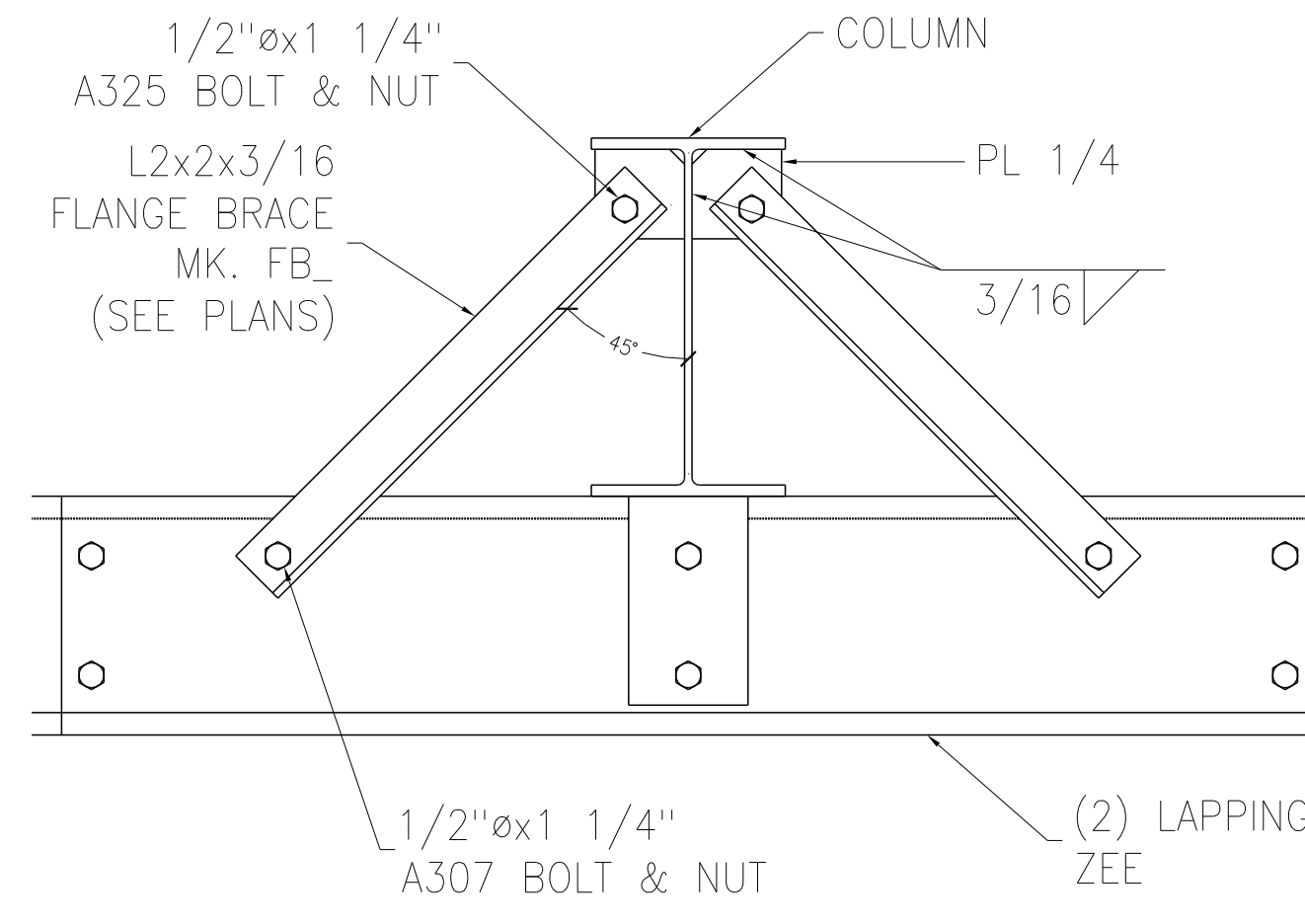
12 CABLE GRIP TO BRACE CABLE
SS502



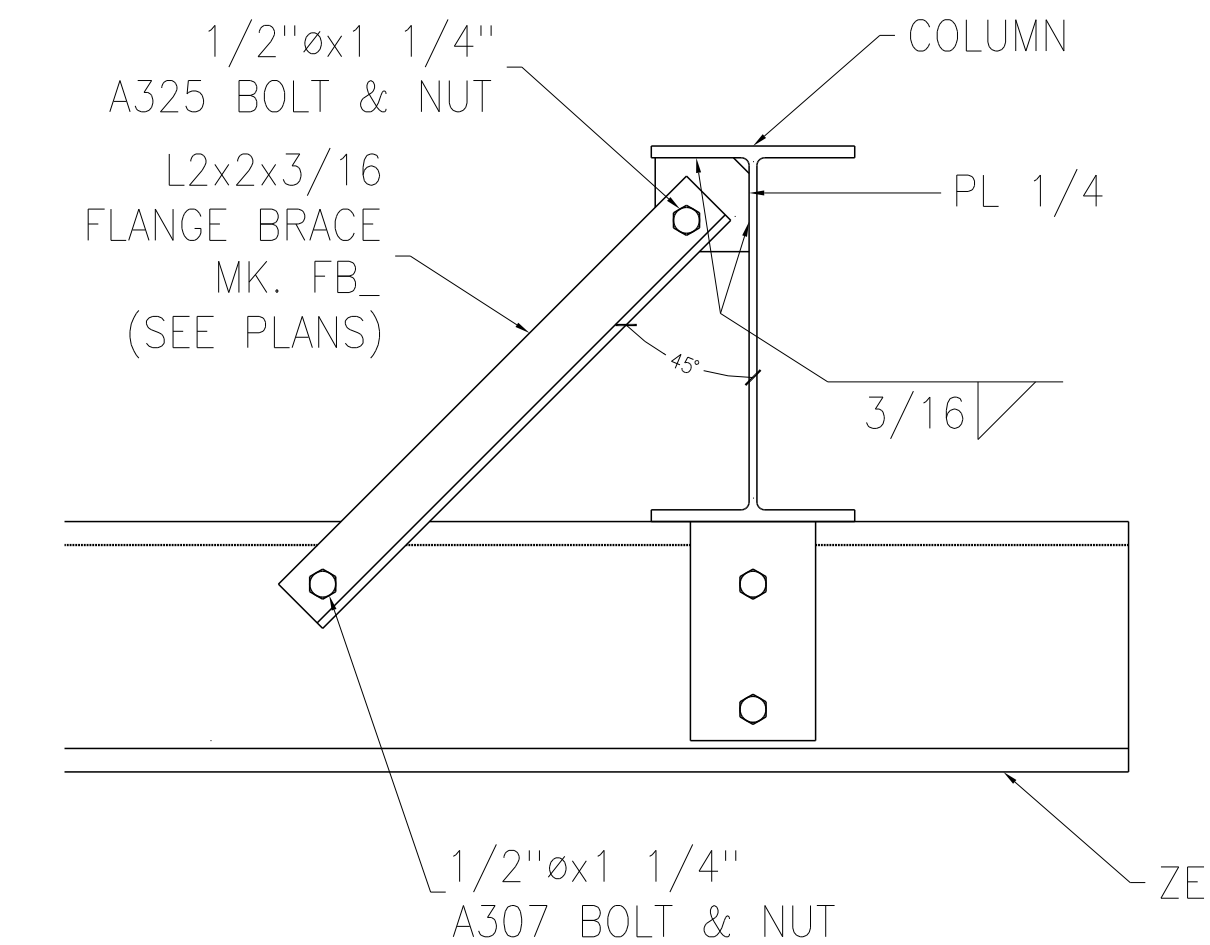
13 UPLIFT BRACE TO RAFTER & ZEE
SS502



14 UPLIFT BRACE TO RAFTER & ZEE
SS502



15 FLANGE BRACE TO COLUMN & GIRT
SS502



16 FLANGE BRACE TO COLUMN & GIRT
SS502

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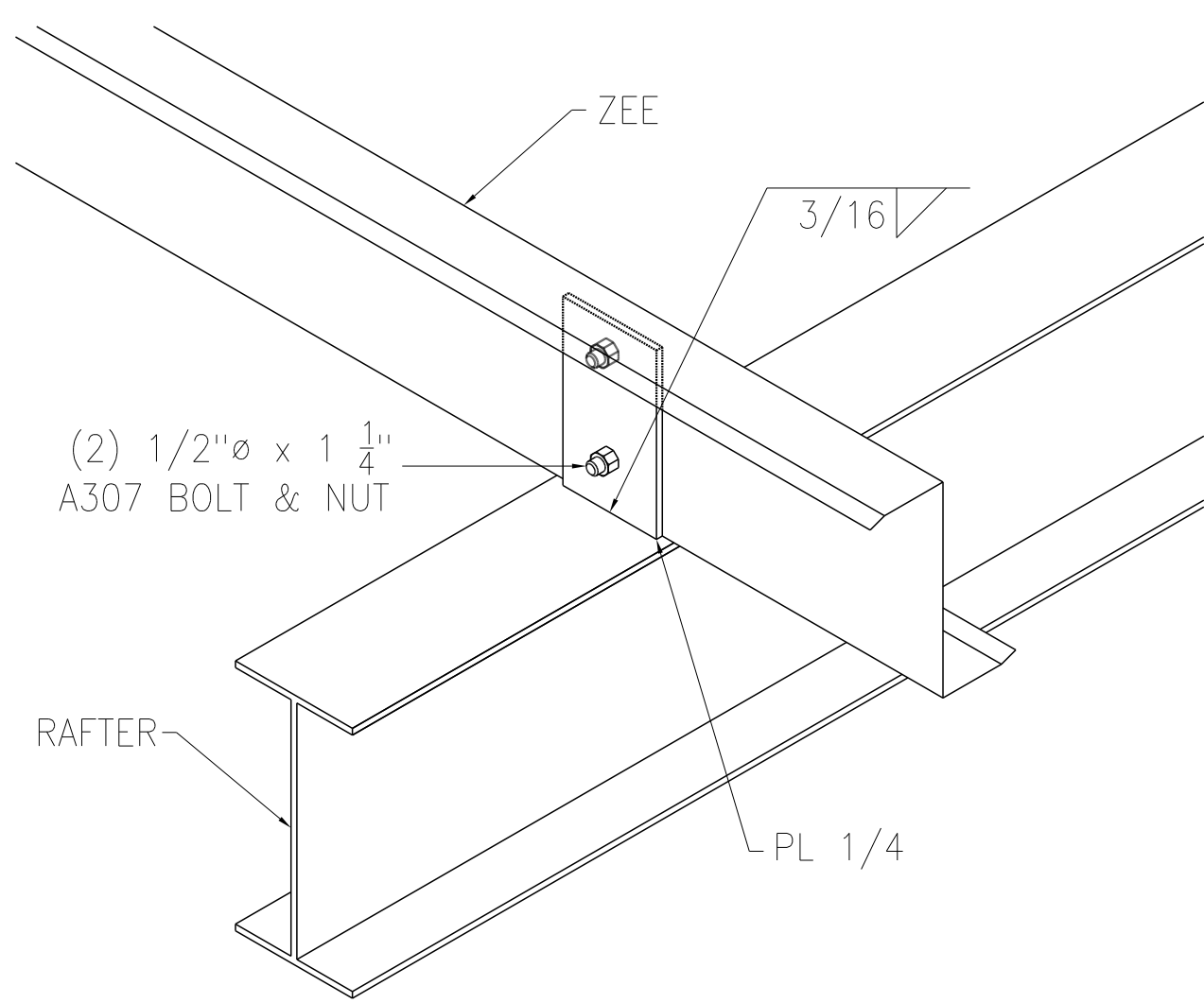
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THE GOOSE
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PRIMARY FRAMING DETAILS

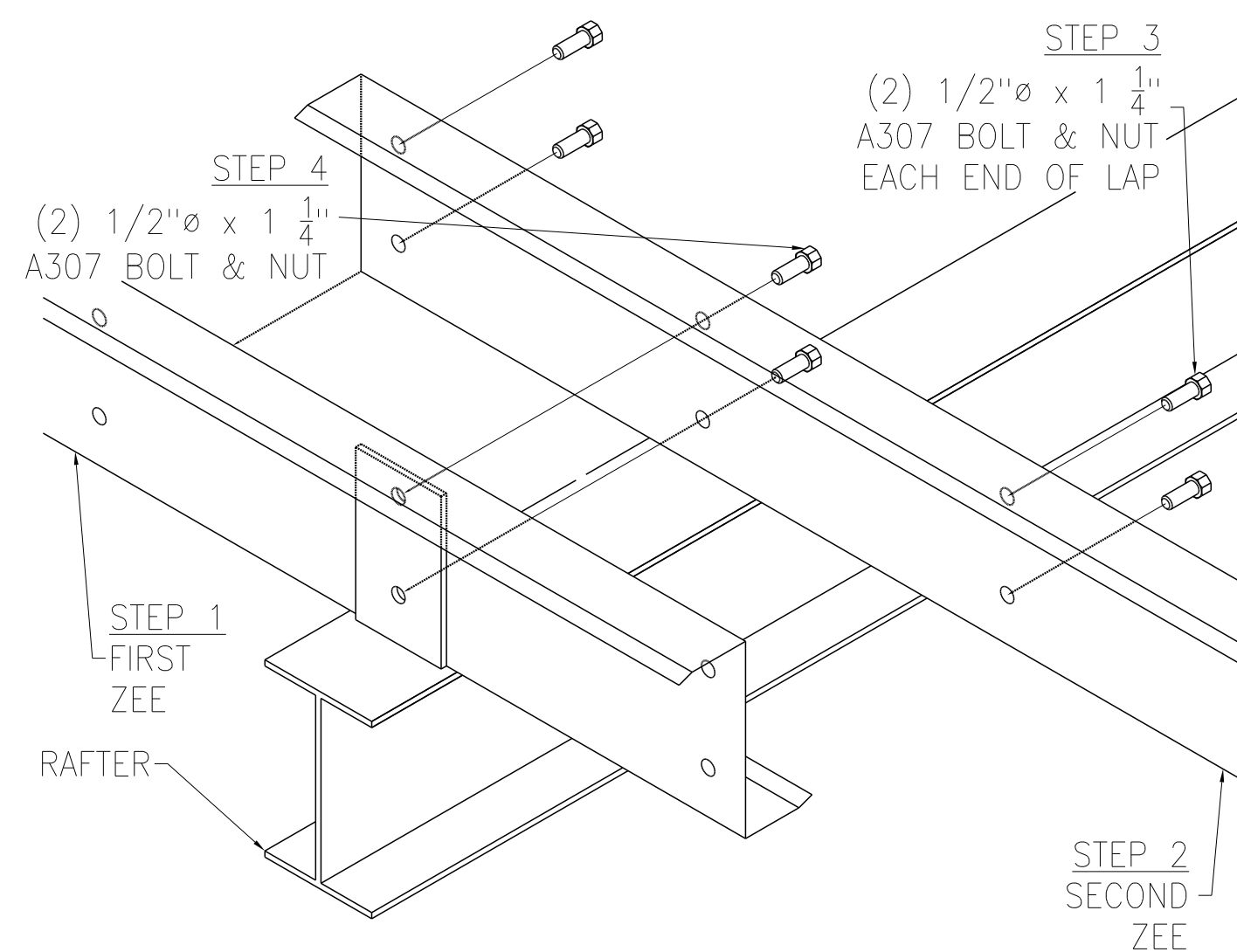
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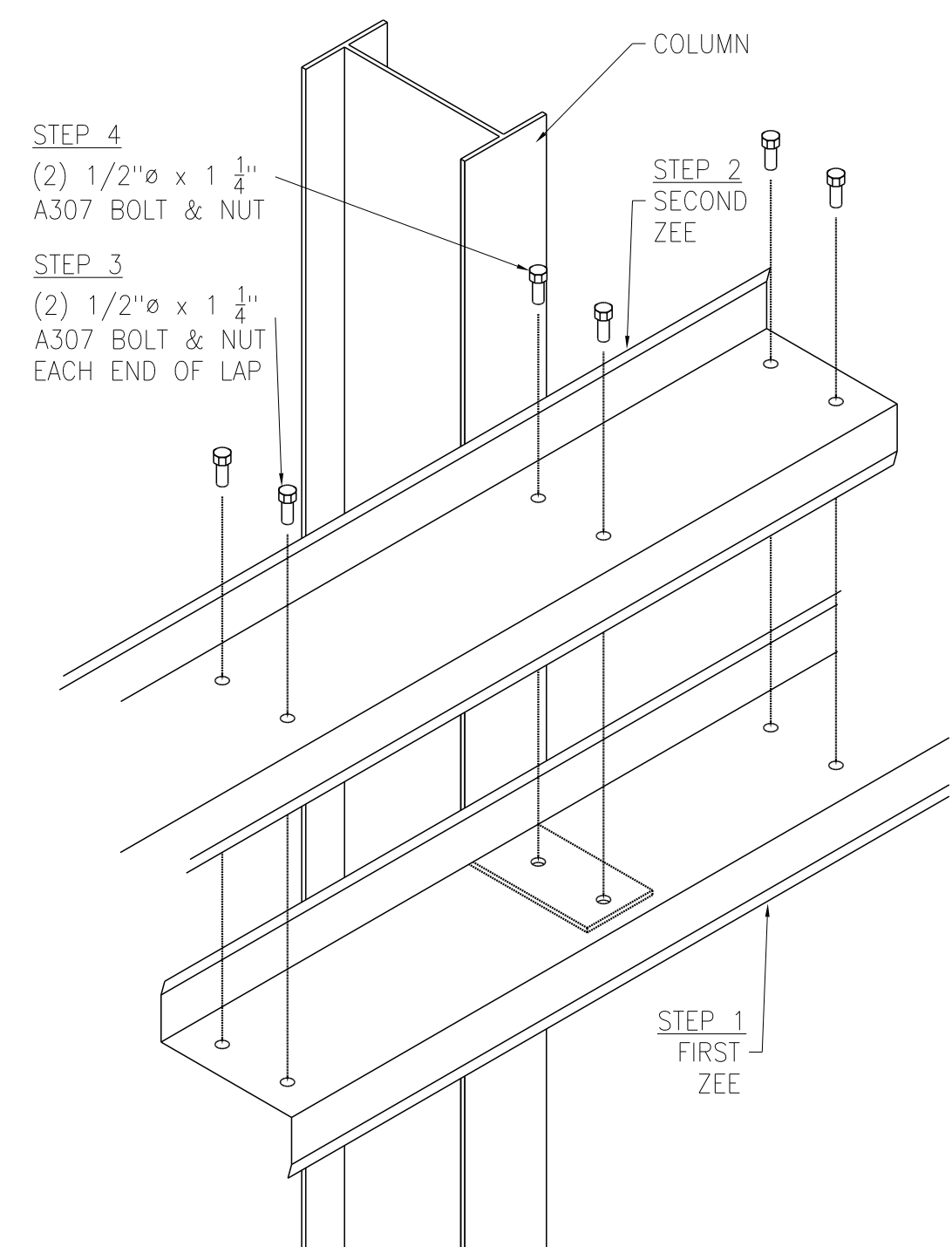
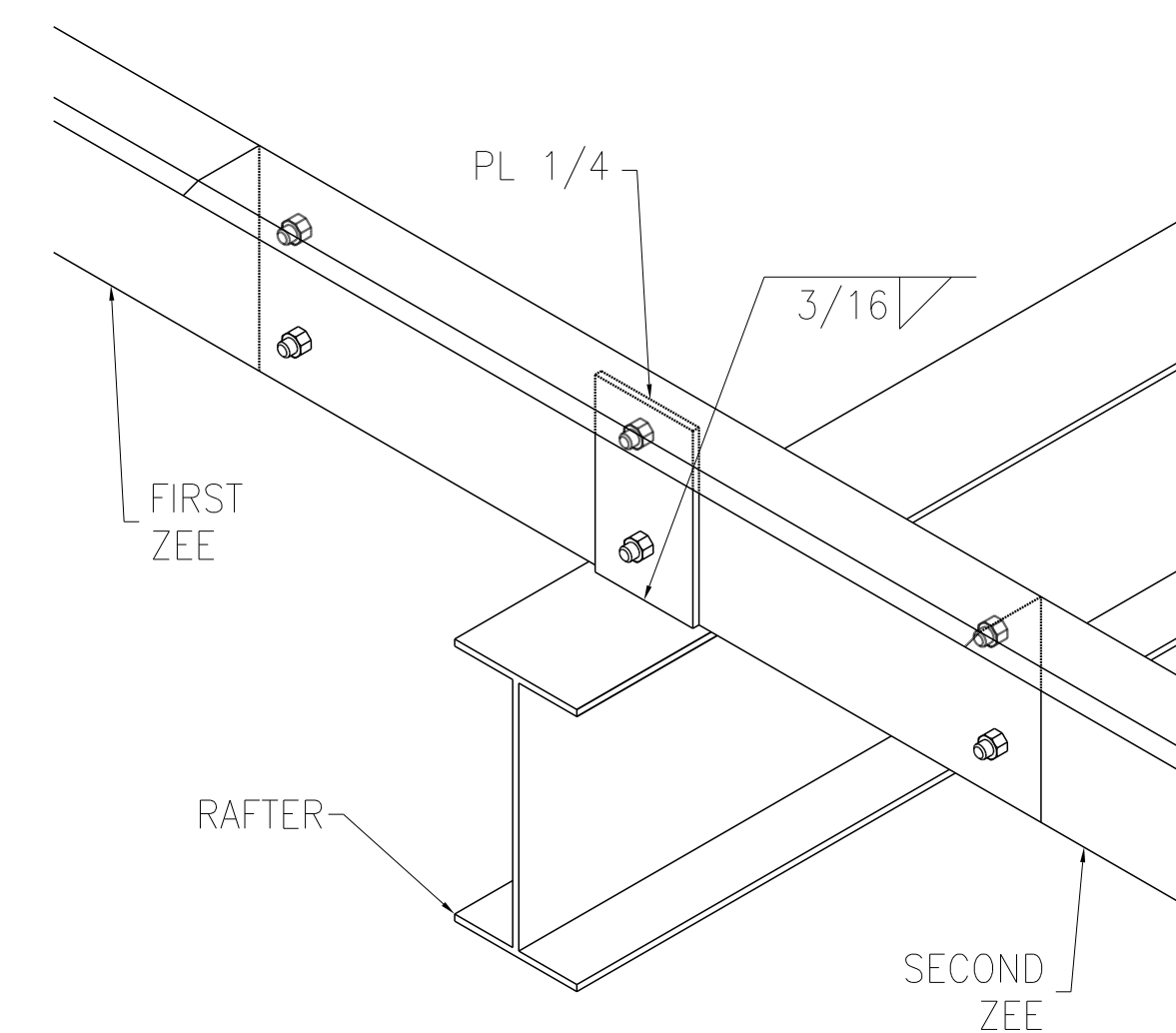
1 ZEE TO RAFTER

SS551



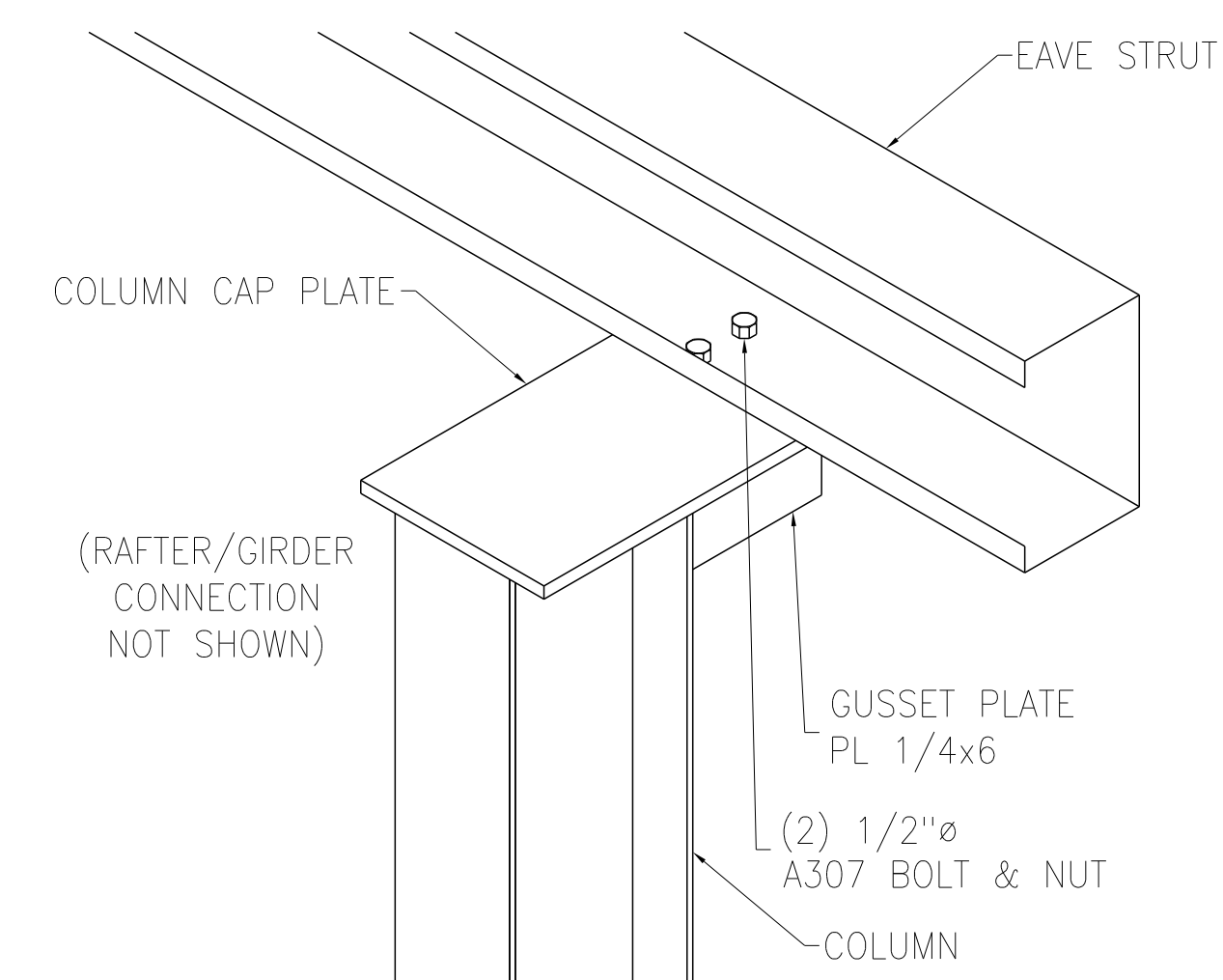
2 LAPPED ZEES TO RAFTER

SS551



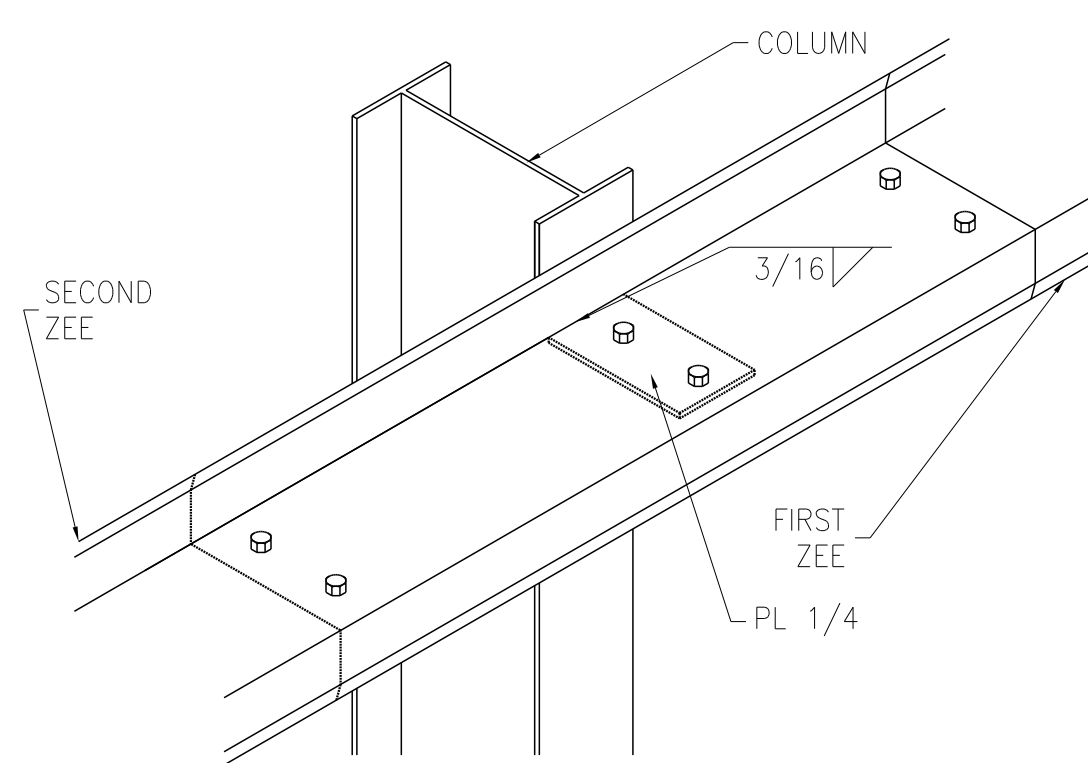
4 EAVE STRUT TO INSET COLUMN

SS551



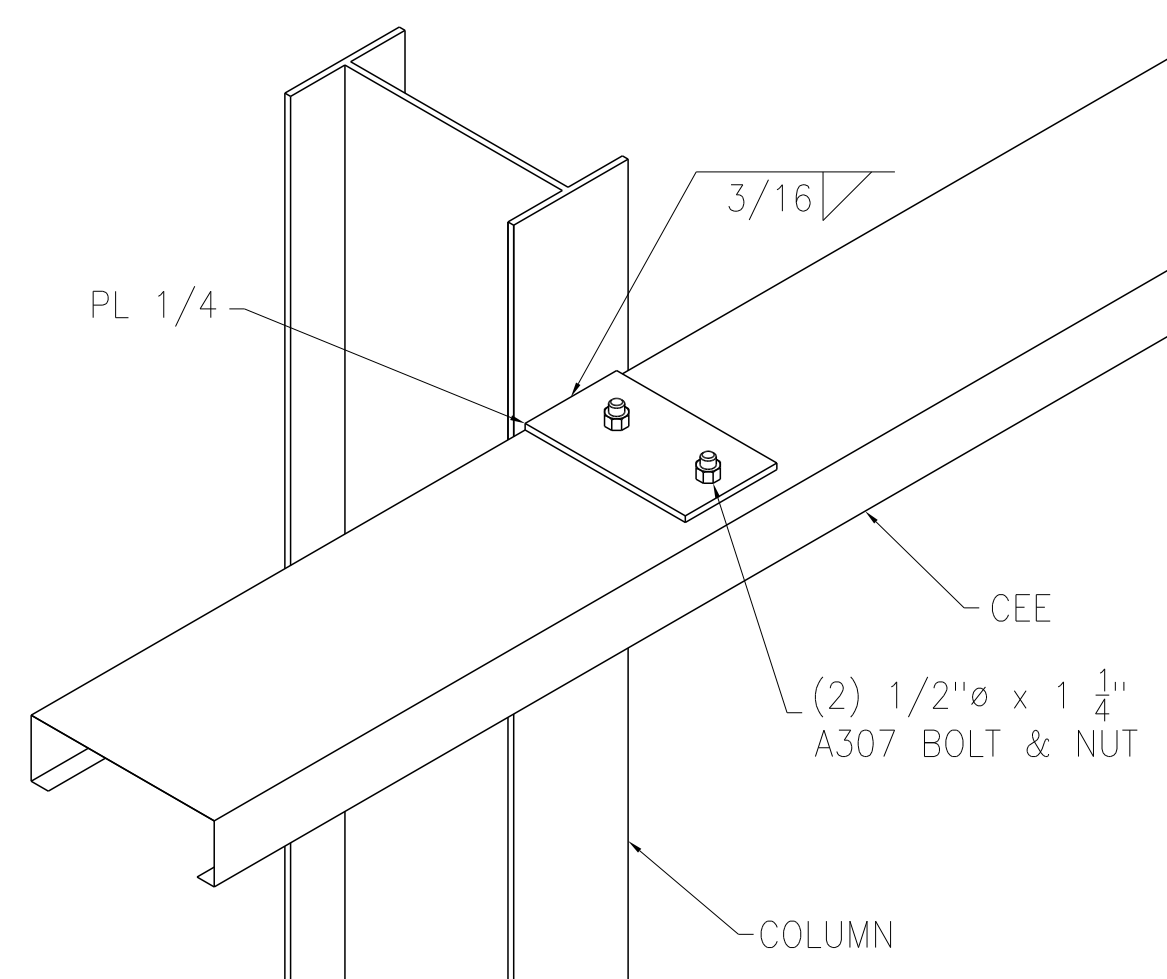
5 EAVE STRUT TO INSET COLUMN

SS551



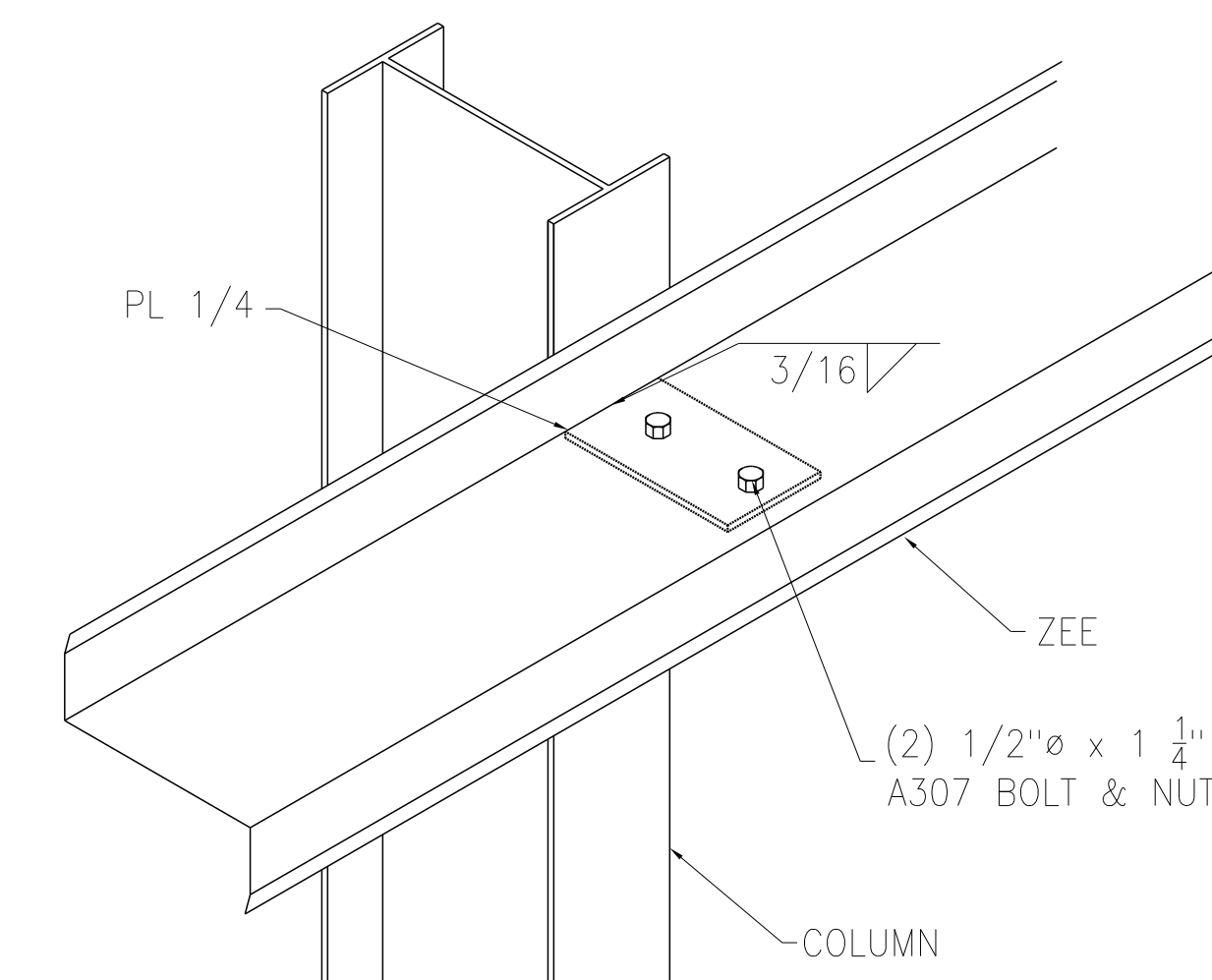
3 ZEE LAP @ COLUMN

SS551



6 CEE AT COLUMN

SS551



7 ZEE AT COLUMN

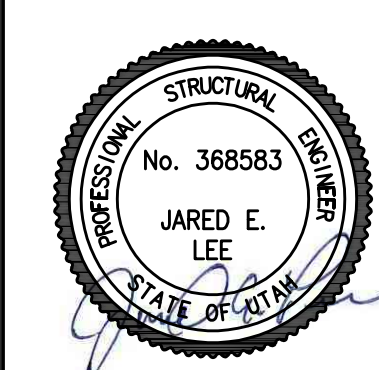
SS551

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Checked by:
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05/12/2023

THE GOOSE

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SECONDARY FRAMING DETAILS

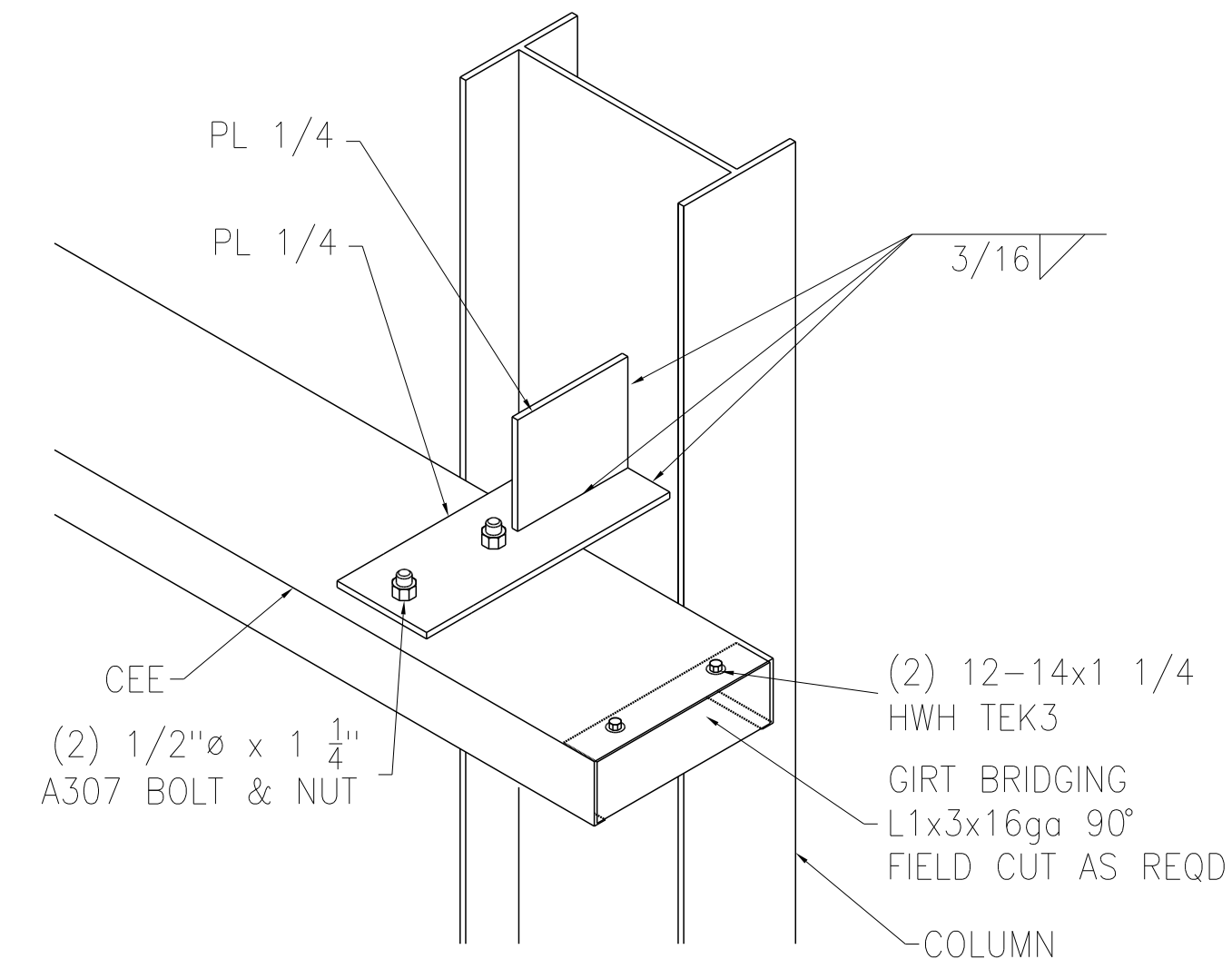
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A	Issued For Approval	04/18/2023	AML
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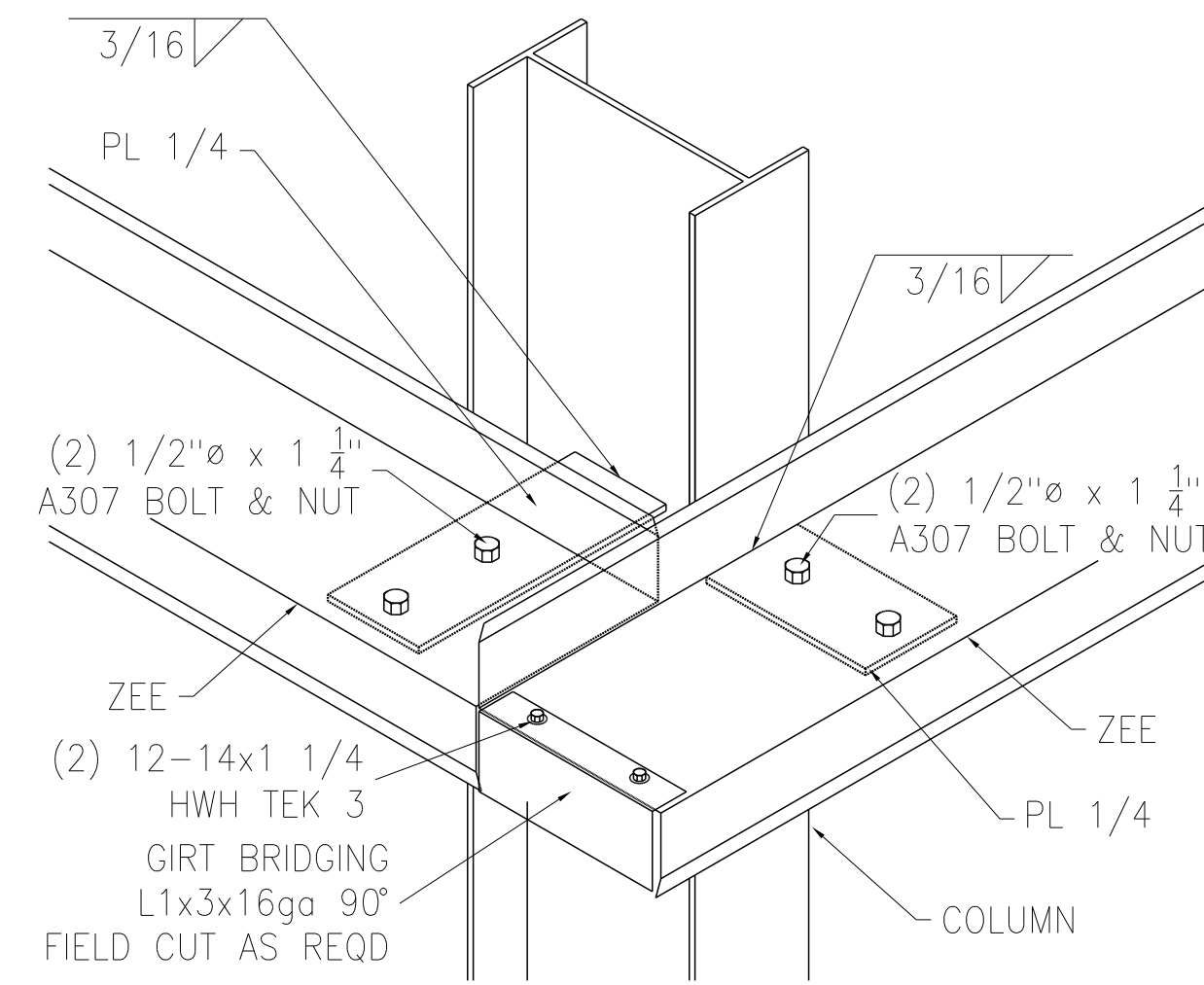
PROJ. NO. 23002

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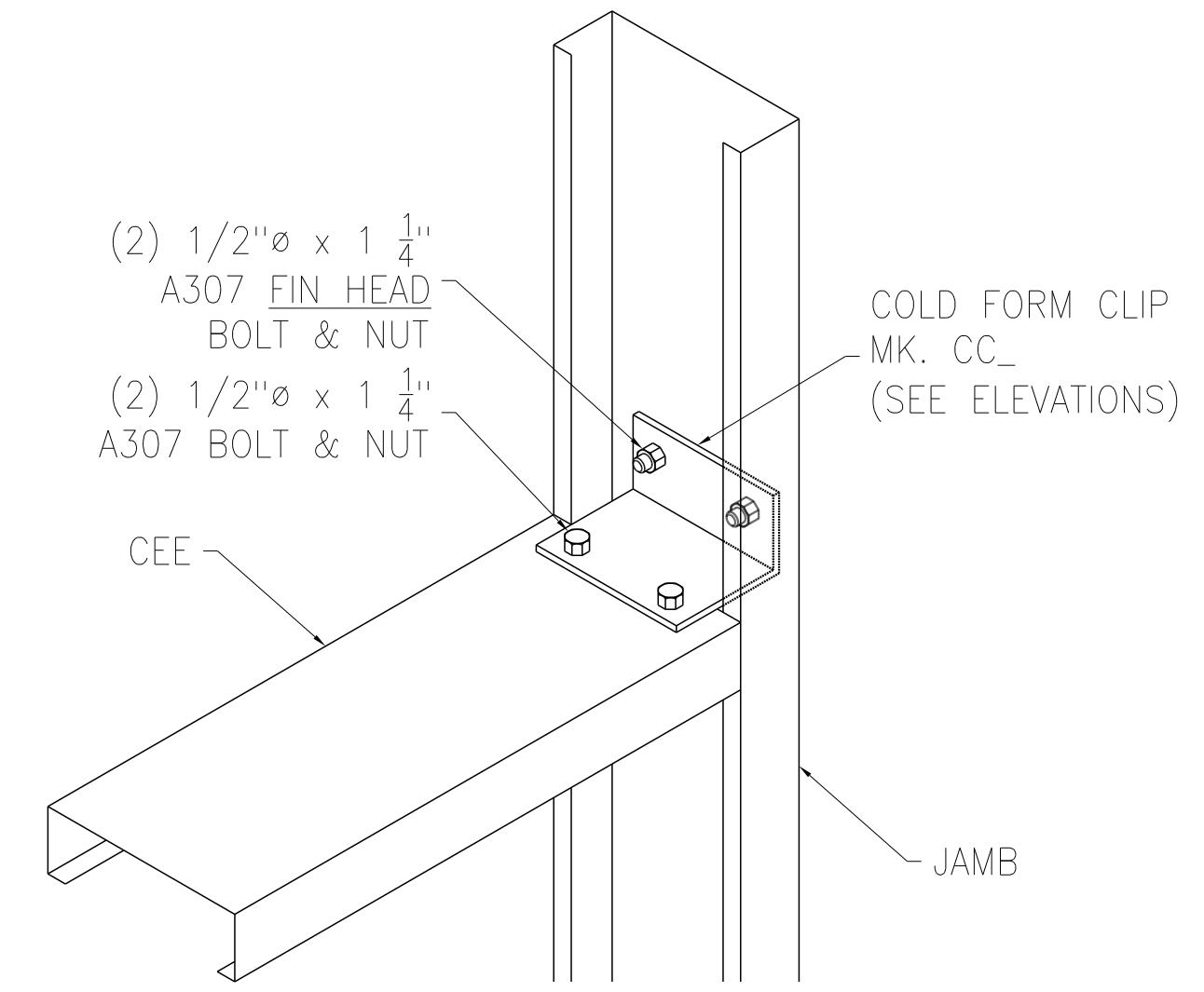
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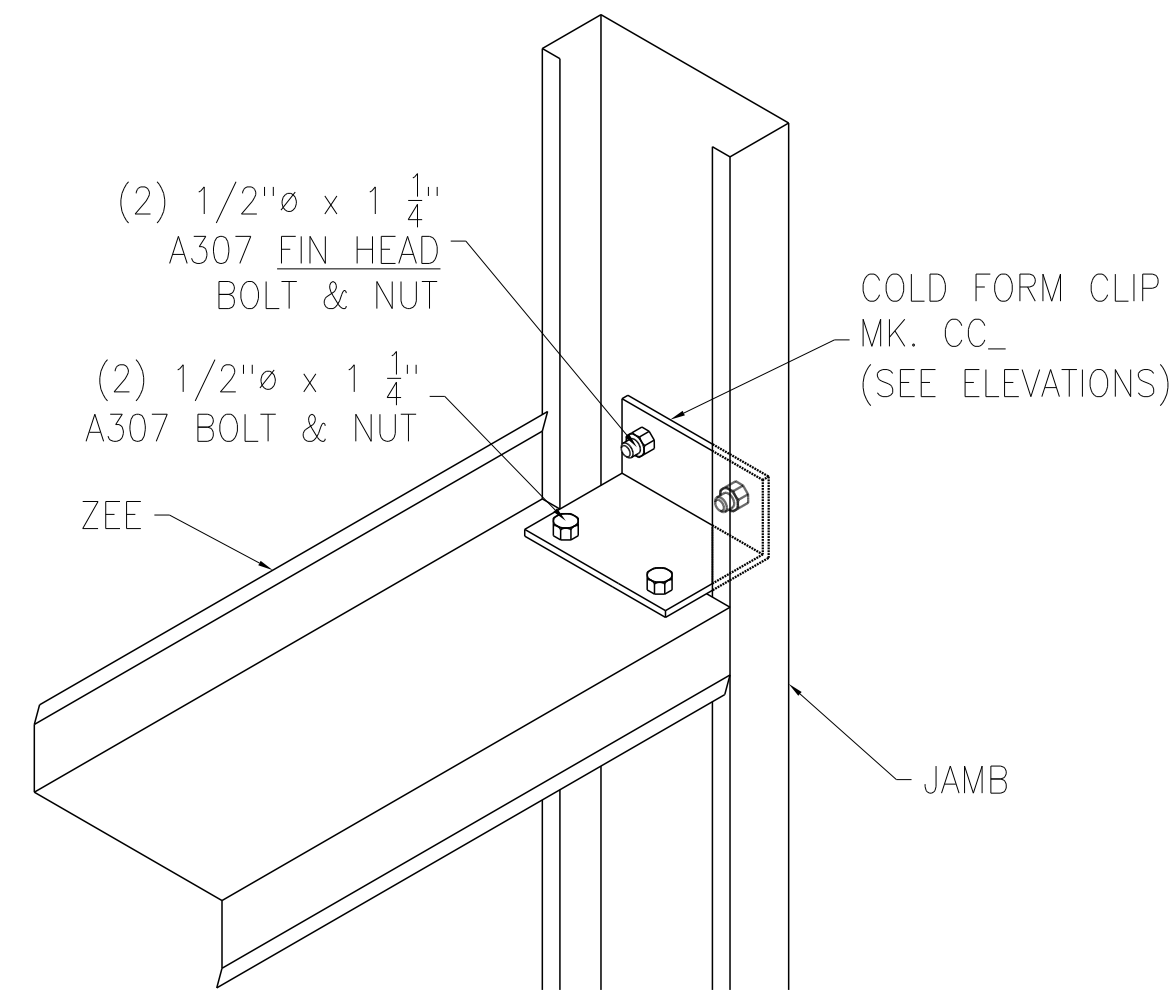
8 SS552 CEE TO COLUMN



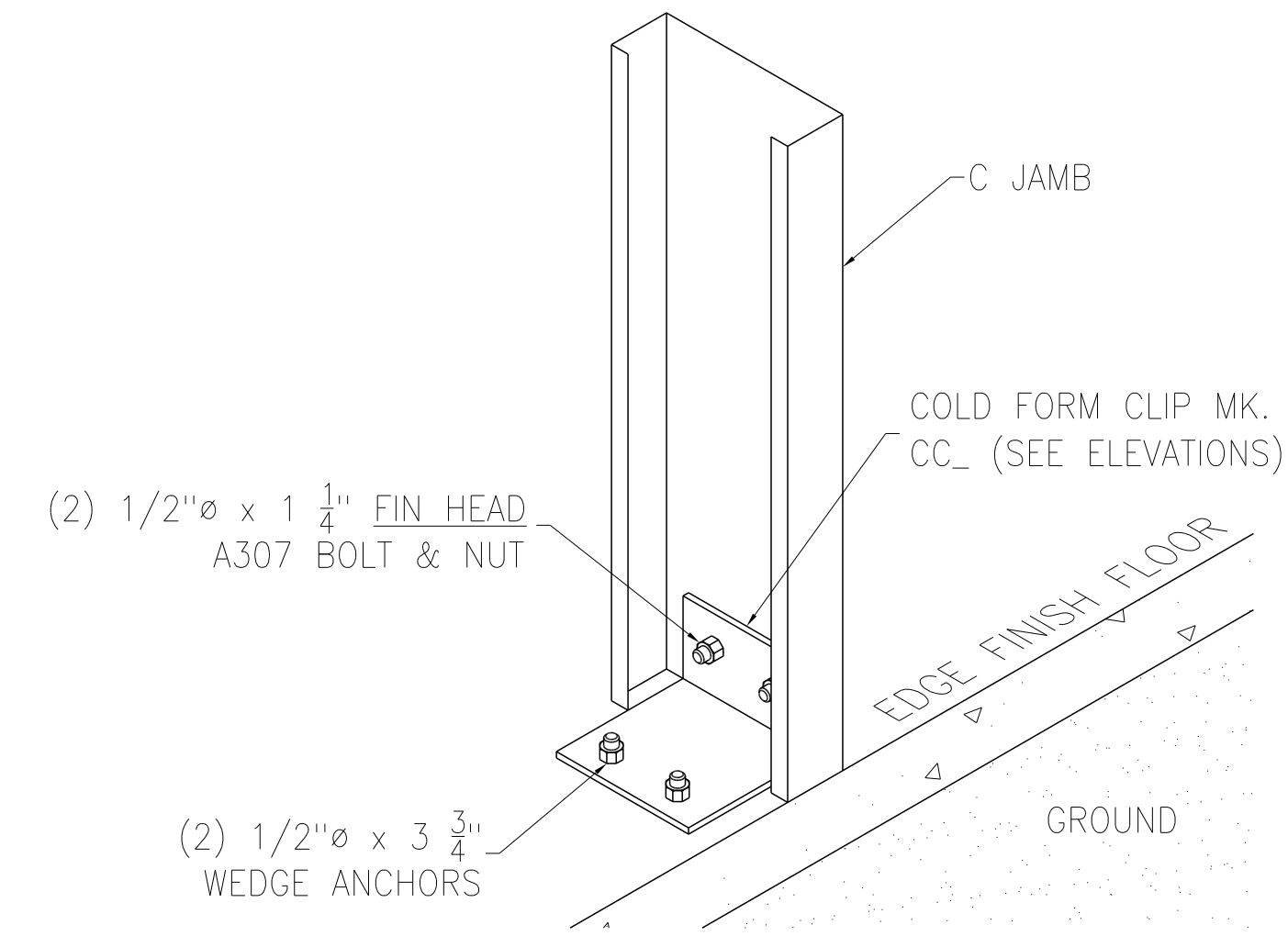
9 SS552 ZEE TO COLUMN



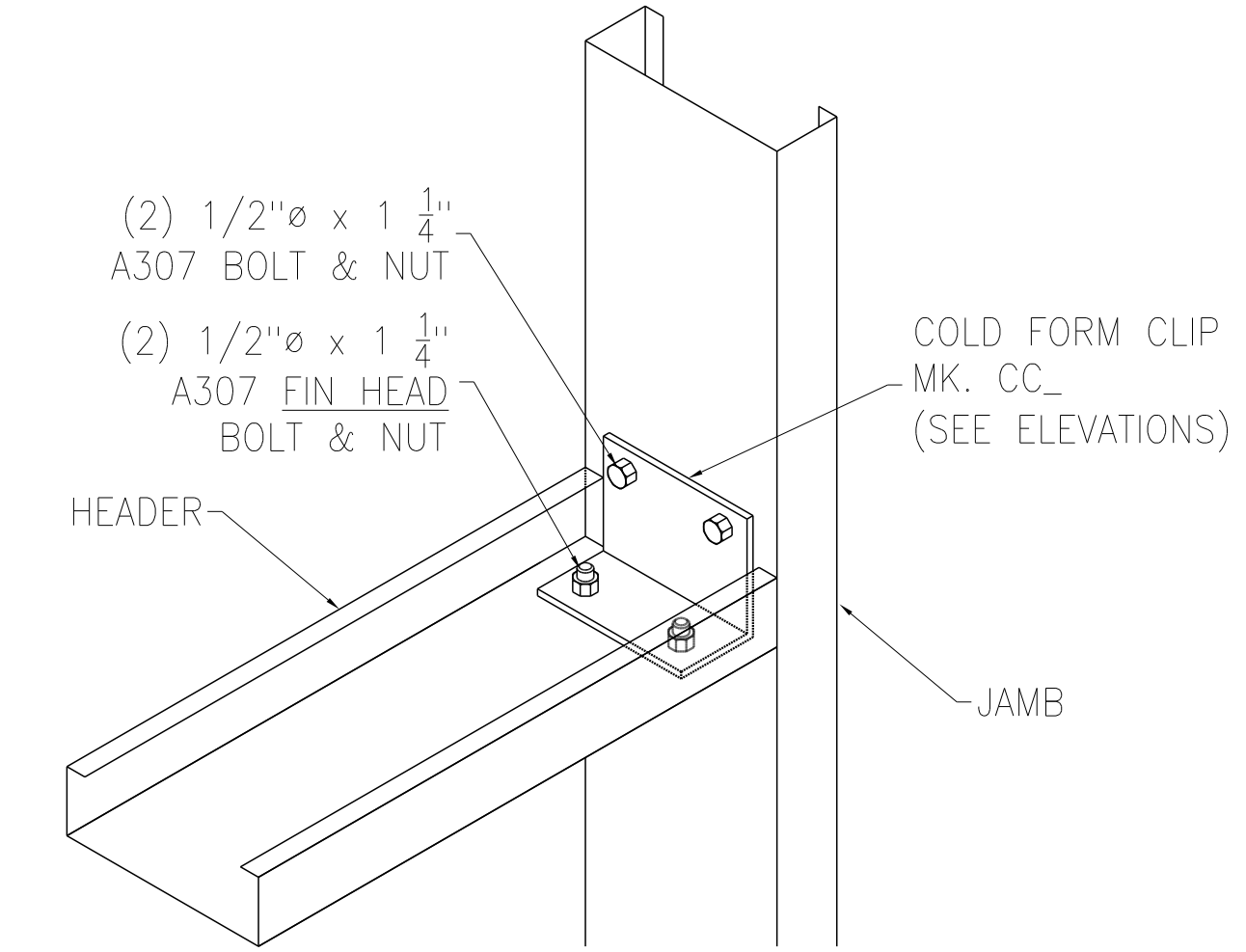
10 SS552 CEE TO JAMB



11 SS552 ZEE TO JAMB



12 SS552 JAMB TO FLOOR



13 SS552 HEADER TO JAMB

14 SS552

15 SS552

16 SS552

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SUNNYFIELD FARM EDEN, UT
SECONDARY FRAMING DETAILS

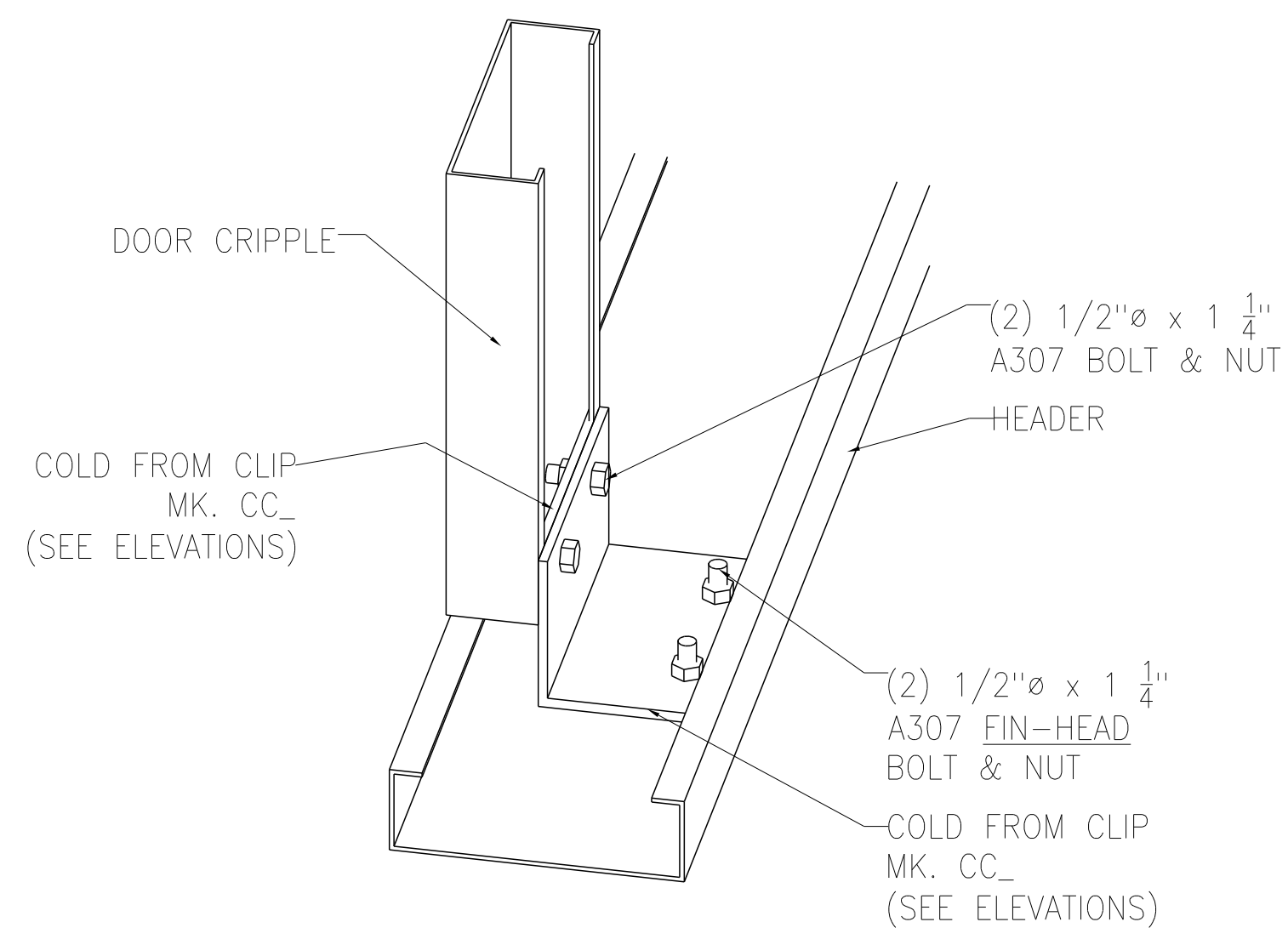
Mark	Description	Date	By
A	Issued For Approval	04/18/2023	AML
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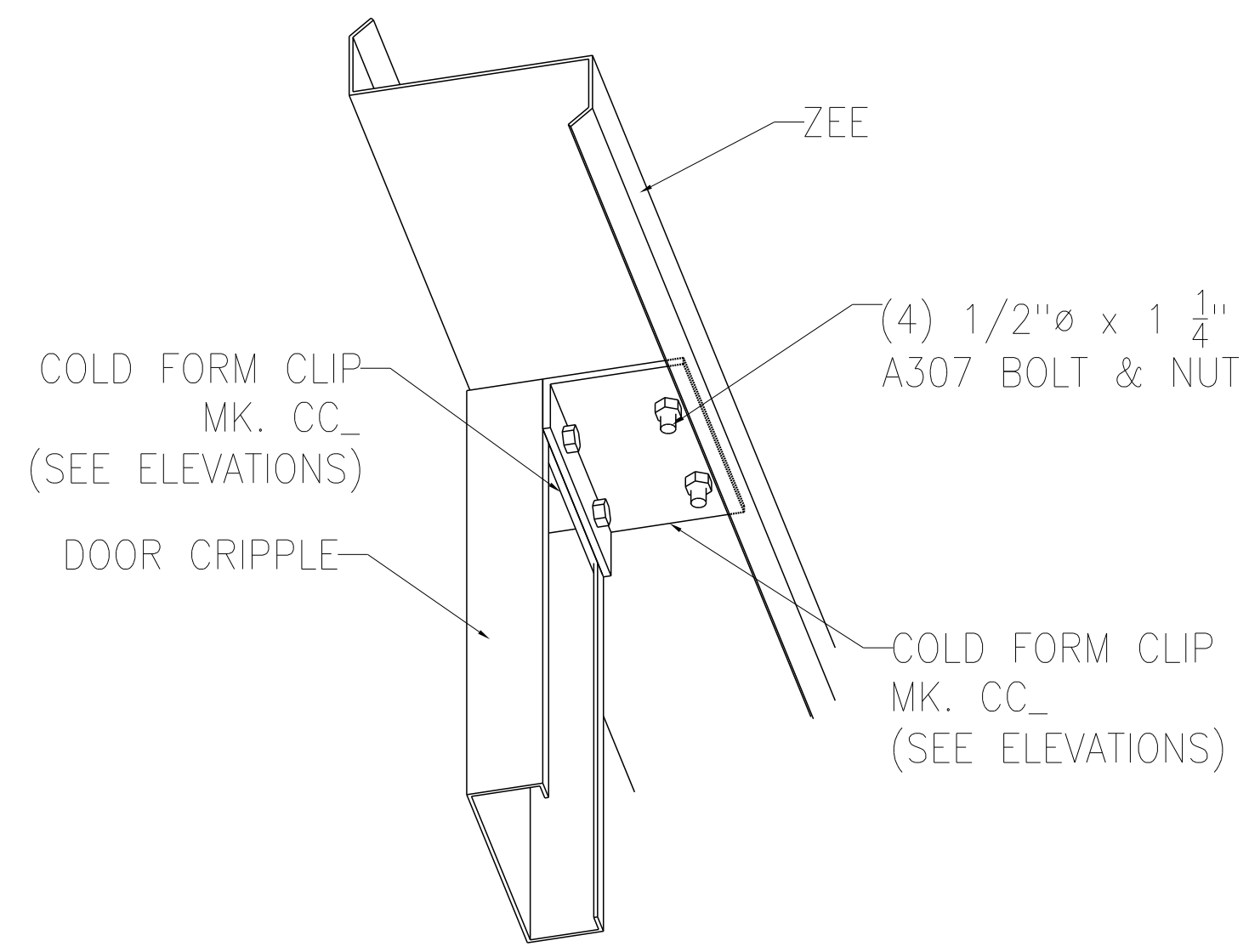
PROJ. NO. 23002

DRAWING NO.

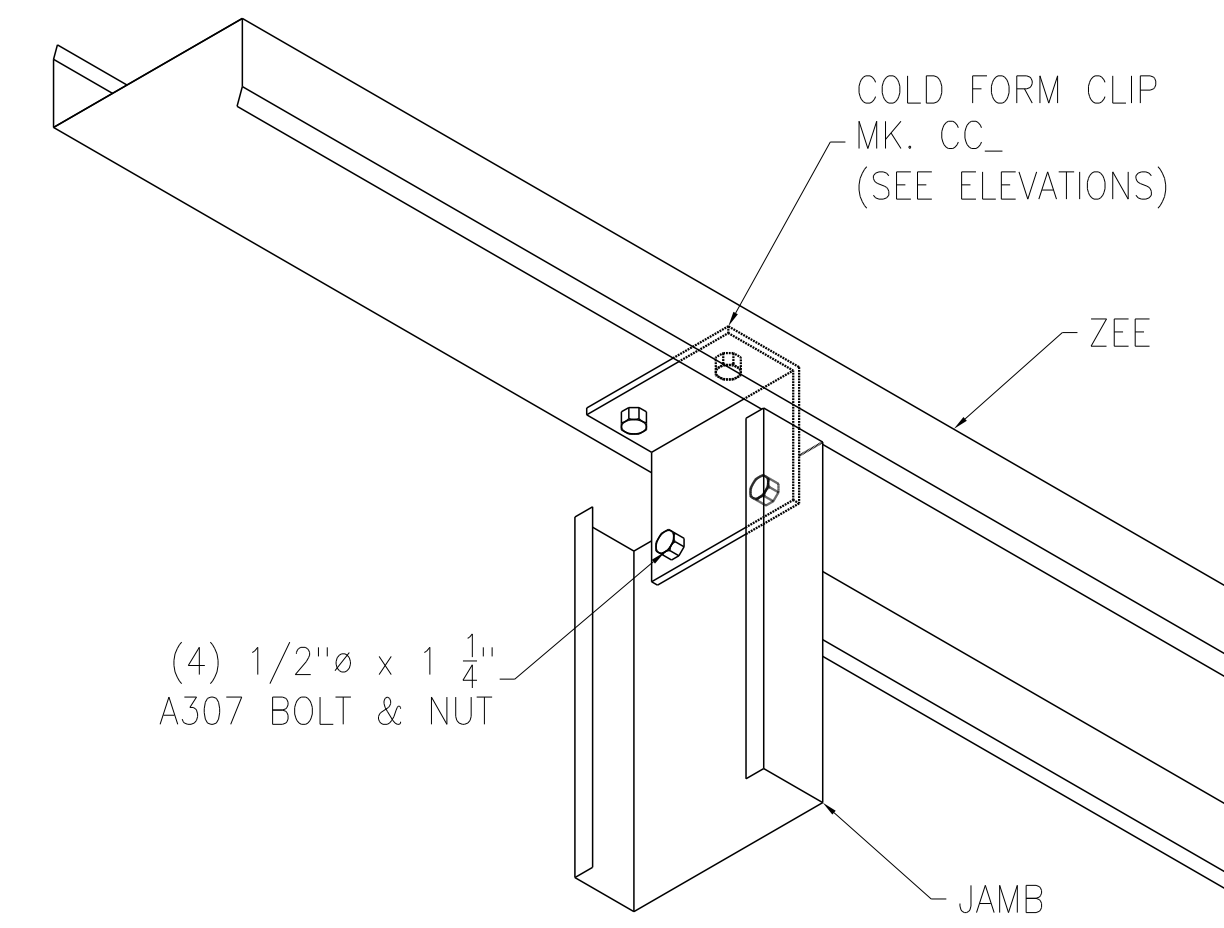
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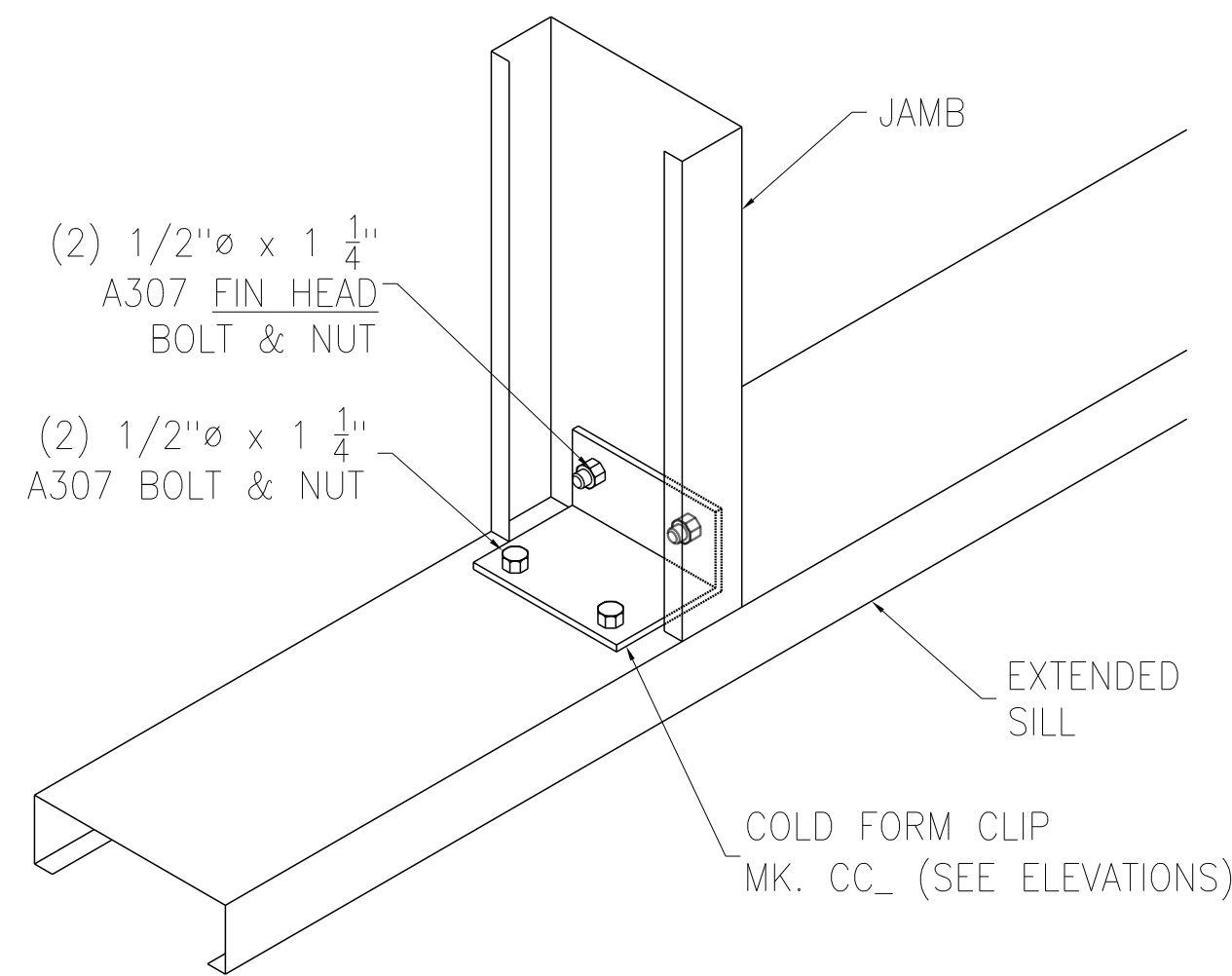
17
SS553 DOOR CRIPPLE @ HEADER



18
SS553 DOOR CRIPPLE @ ZEE



19
SS553 JAMB TO ZEE



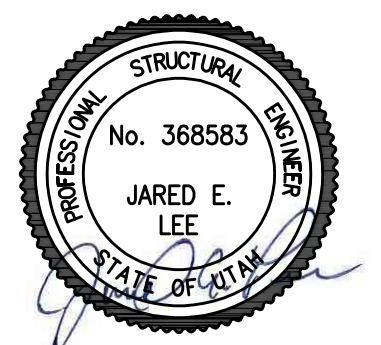
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SS553 JAMB TO EXTENDED SILL

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Checked by: JL



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THE GOOSE

2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT

SECONDARY FRAMING DETAILS

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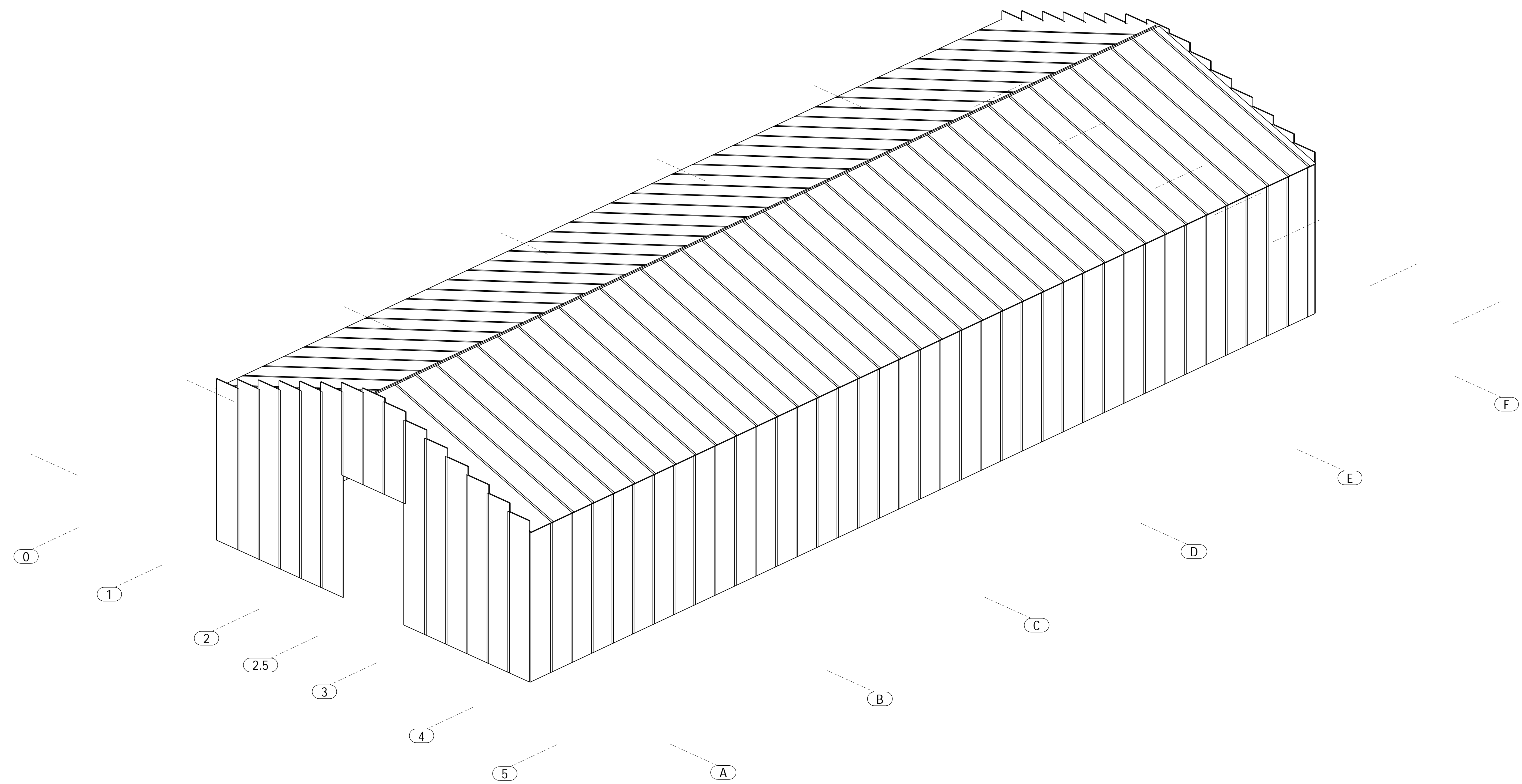
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THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT
3D CLADDING VIEW

Mark	Description	Date	By
A	Issued For Approval	04/18/2023	AML
B	Issued For Permit	05/10/2023	AML

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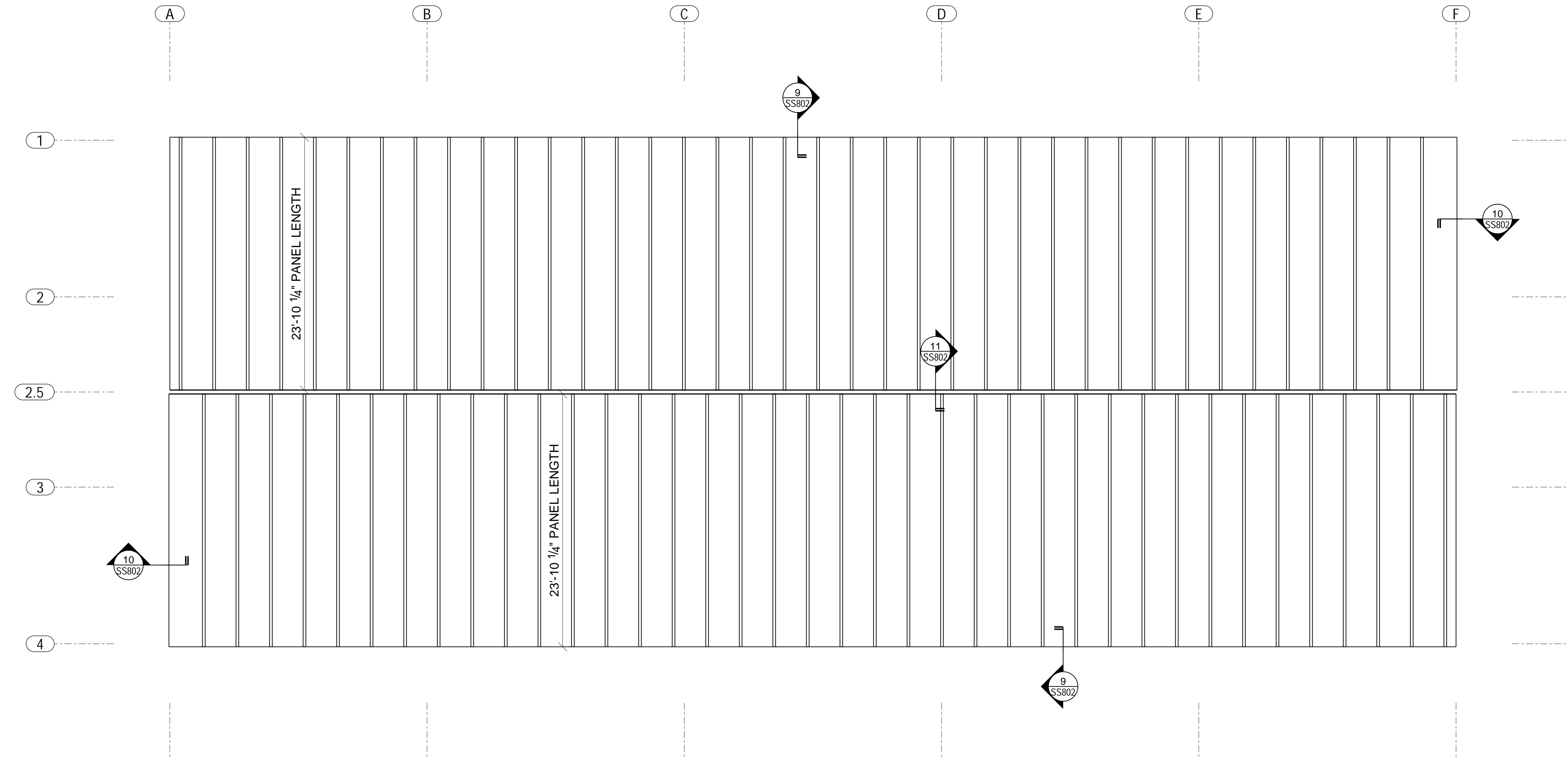
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ROOF PANEL LAYOUT PLAN
26 GA. PBR-36 WALL PANEL
(78) @ 23'-10 1/4"

THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT
ROOF PANEL LAYOUT PLAN

Mark	Description	Date	By
A	Issued For Approval	04/18/2023	AML
B	Issued For Permit	05/10/2023	AML

DATE 05/10/2023

PROJ. NO. 23002

DRAWING NO.

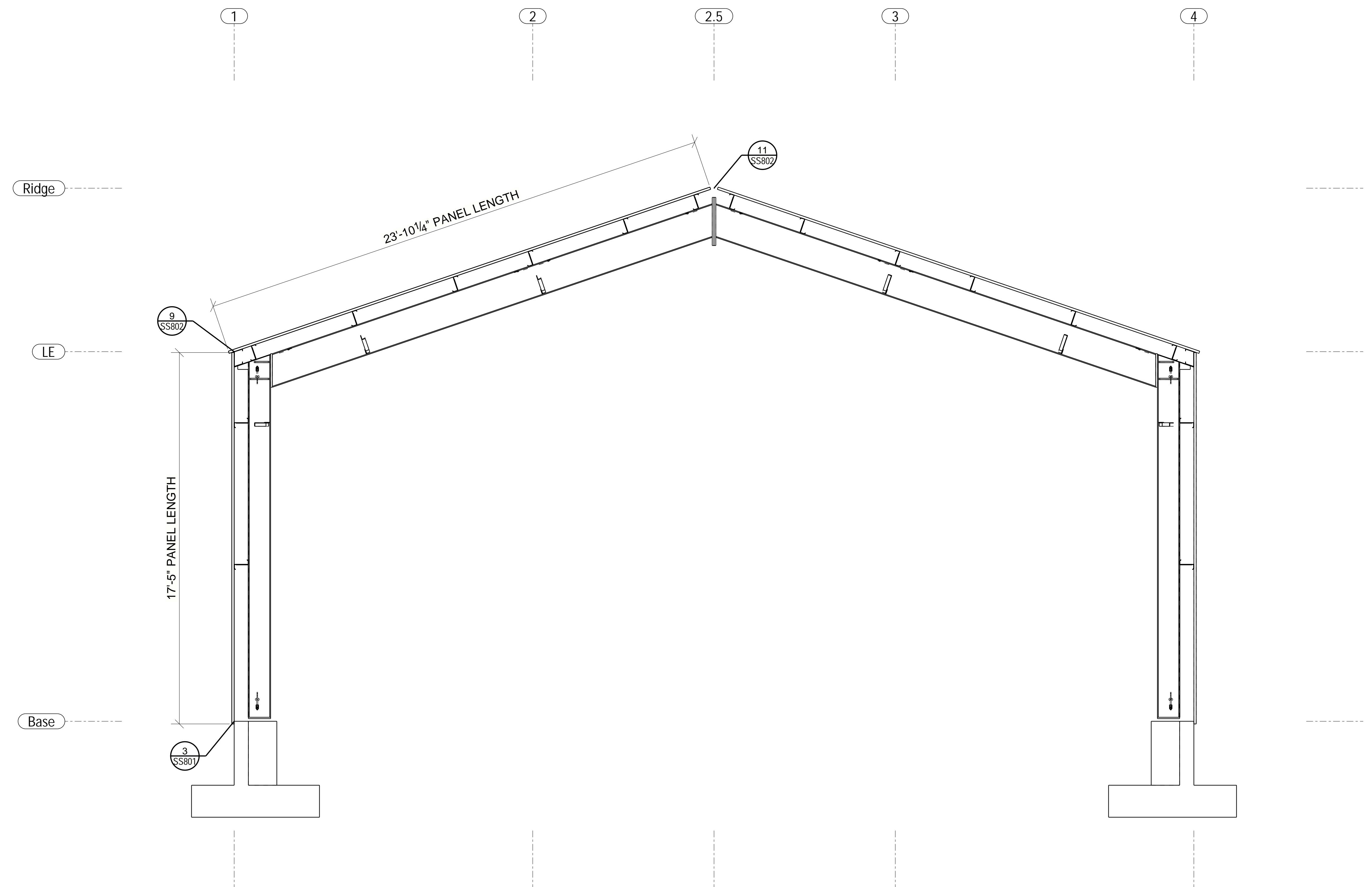
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CLADDING CROSS SECTION @ LINE B

THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT
CLADDING CROSS SECTION @ LINE B

Mark	Description	Date	By
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DRAWING NO.

SS603

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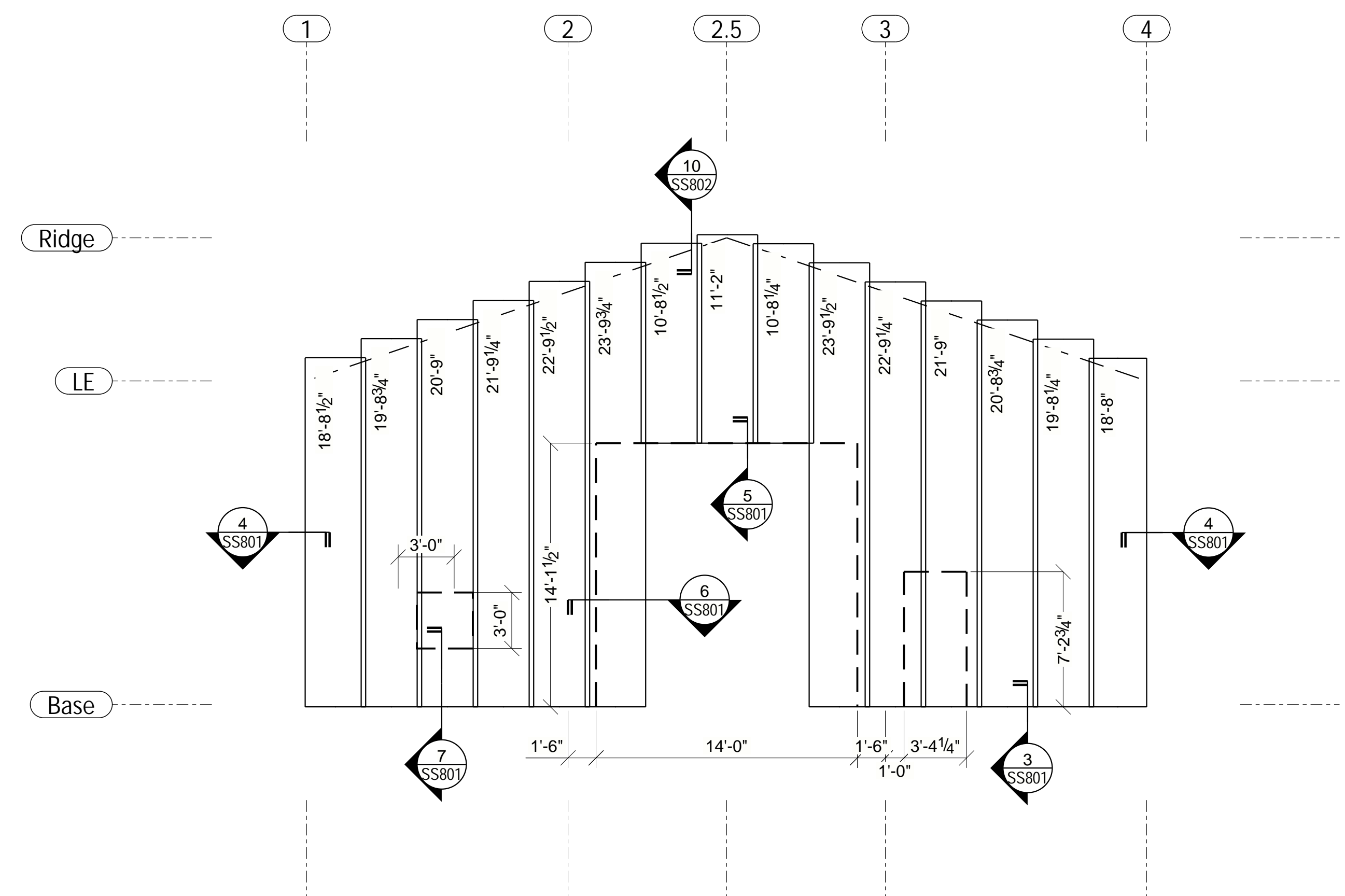


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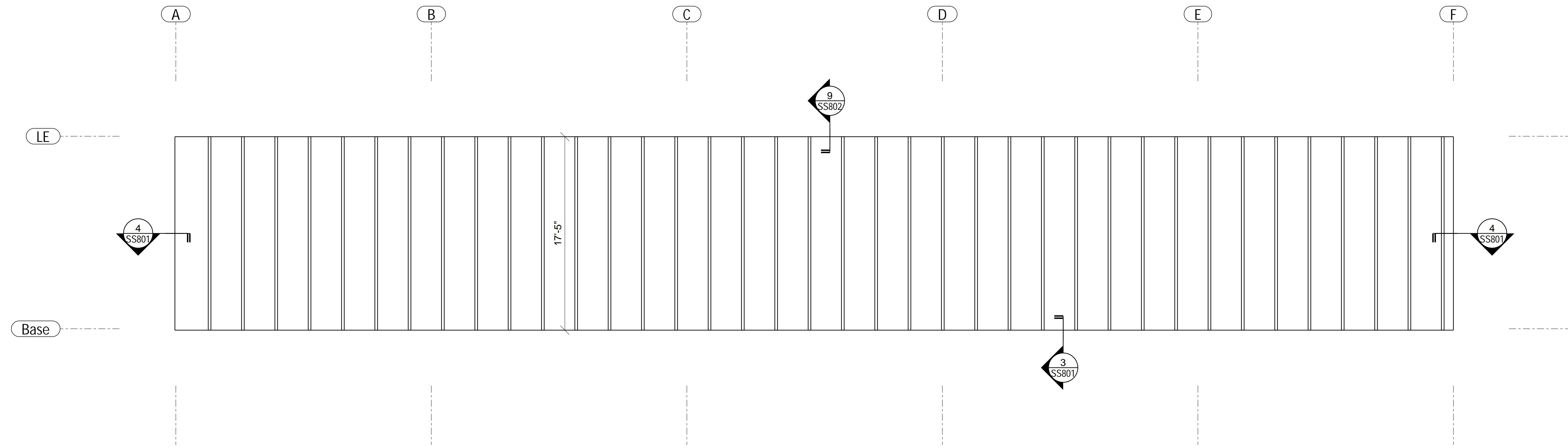


05/12/2023



CLADDING ELEVATION @ LINE A

- 26 GA. PBR-36 WALL PANEL
- | | | |
|------------------|------------------|------------------|
| (1) @ 23'-9 3/4" | (1) @ 21'-8 7/8" | (1) @ 18'-8 1/2" |
| (1) @ 23'-9 1/2" | (1) @ 20'-9" | (1) @ 18'-8 1/8" |
| (1) @ 22'-9 1/2" | (1) @ 20'-8 5/8" | (1) @ 11'-2" |
| (1) @ 22'-9 1/4" | (1) @ 19'-8 3/4" | (1) @ 10'-8 1/2" |
| (1) @ 21'-9 1/4" | (1) @ 19'-8 3/8" | (1) @ 10'-8 1/4" |



CLADDING ELEVATION @ LINE 4

- 26 GA. PBR-36 WALL PANEL
(39) @ 17'-5"

THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT
CLADDING ELEVATIONS @ LINES A & 4

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DRAWING NO.

SS701

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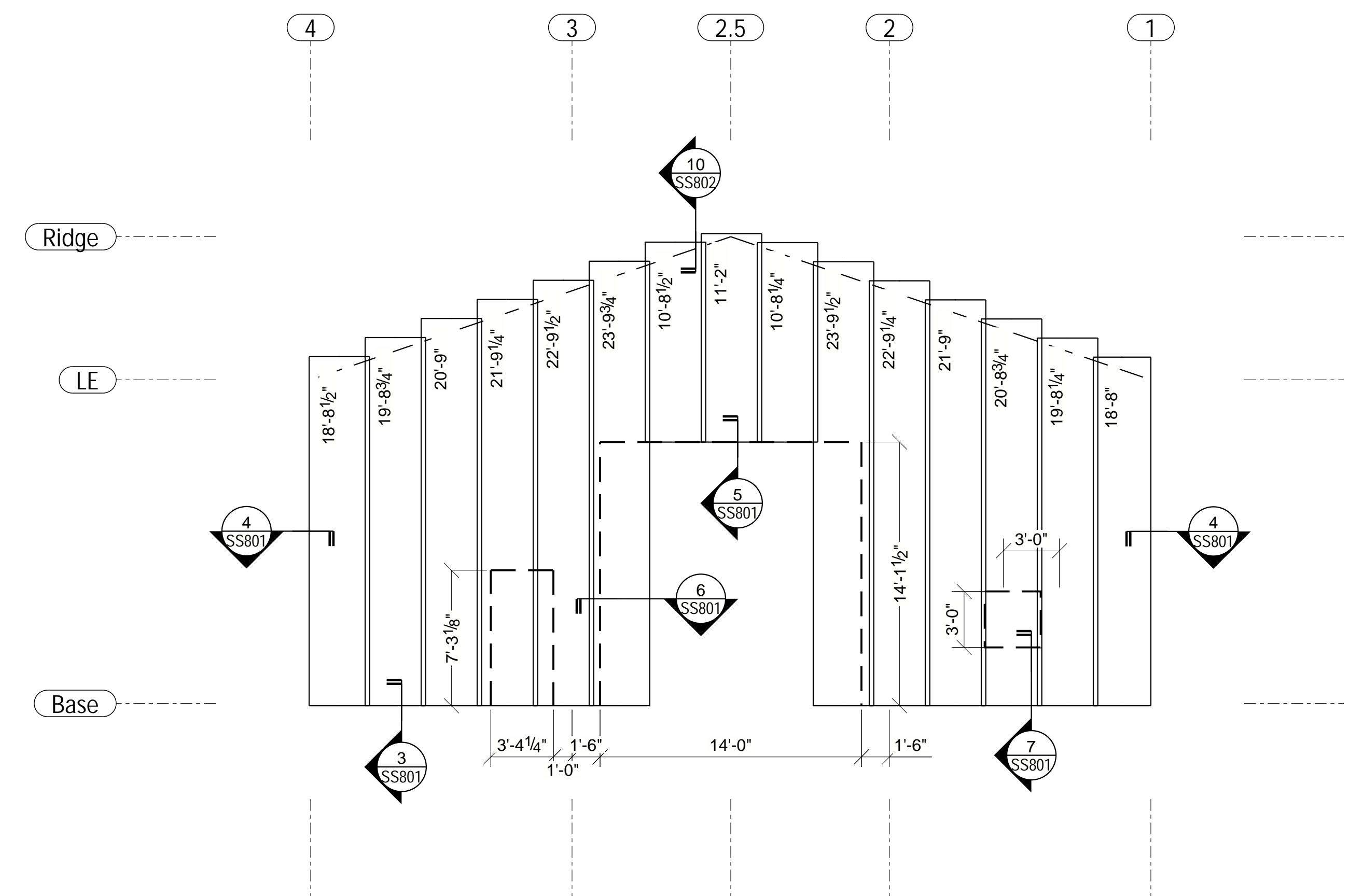


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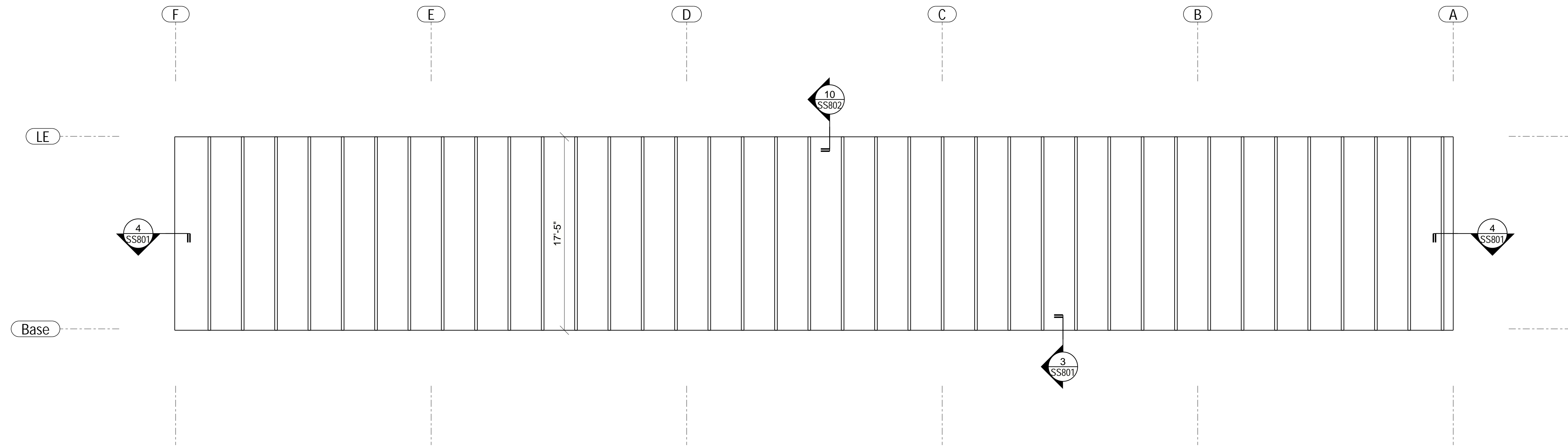


05/12/2023



CLADDING ELEVATION @ LINE F

- 26 GA. PBR-36 WALL PANEL
- | | | |
|------------------|------------------|------------------|
| (1) @ 23'-9 3/4" | (1) @ 21'-8 7/8" | (1) @ 18'-8 1/2" |
| (1) @ 23'-9 1/2" | (1) @ 20'-9" | (1) @ 18'-9 3/8" |
| (1) @ 22'-9 1/2" | (1) @ 20'-8 5/8" | (1) @ 11'-9 3/8" |
| (1) @ 22'-9 1/4" | (1) @ 19'-8 3/4" | (1) @ 10'-8 1/2" |
| (1) @ 21'-9 1/4" | (1) @ 19'-8 3/8" | (1) @ 10'-8 1/4" |



CLADDING ELEVATION @ LINE 1

26 GA. PBR-36 WALL PANEL
(39) @ 17'-5"

THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEN, UT
CLADDING ELEVATIONS @ LINES F & 1

Mark	Description	Date	By
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DRAWING NO.

SS702

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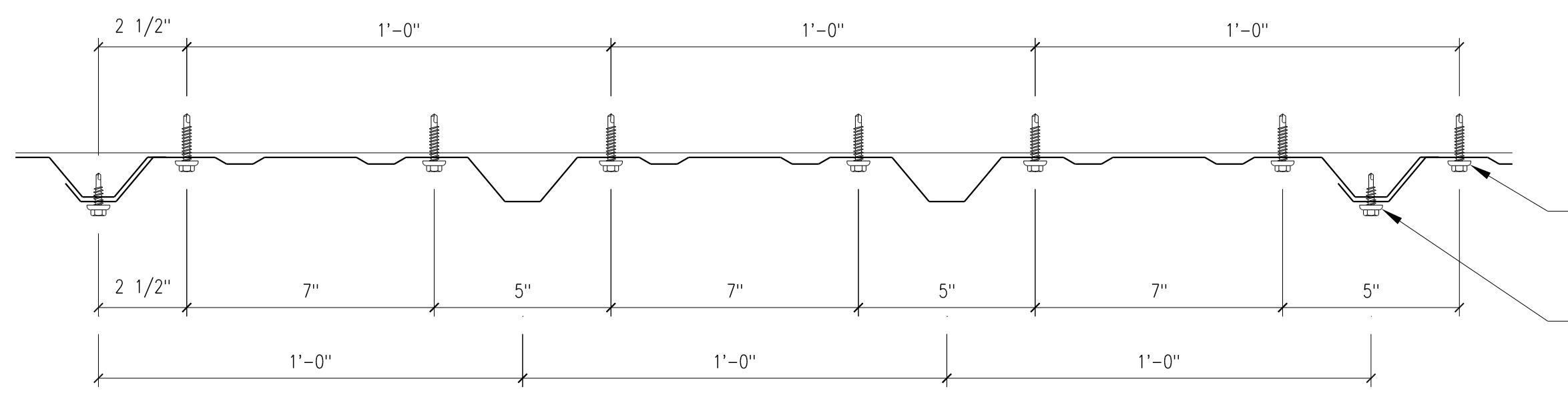


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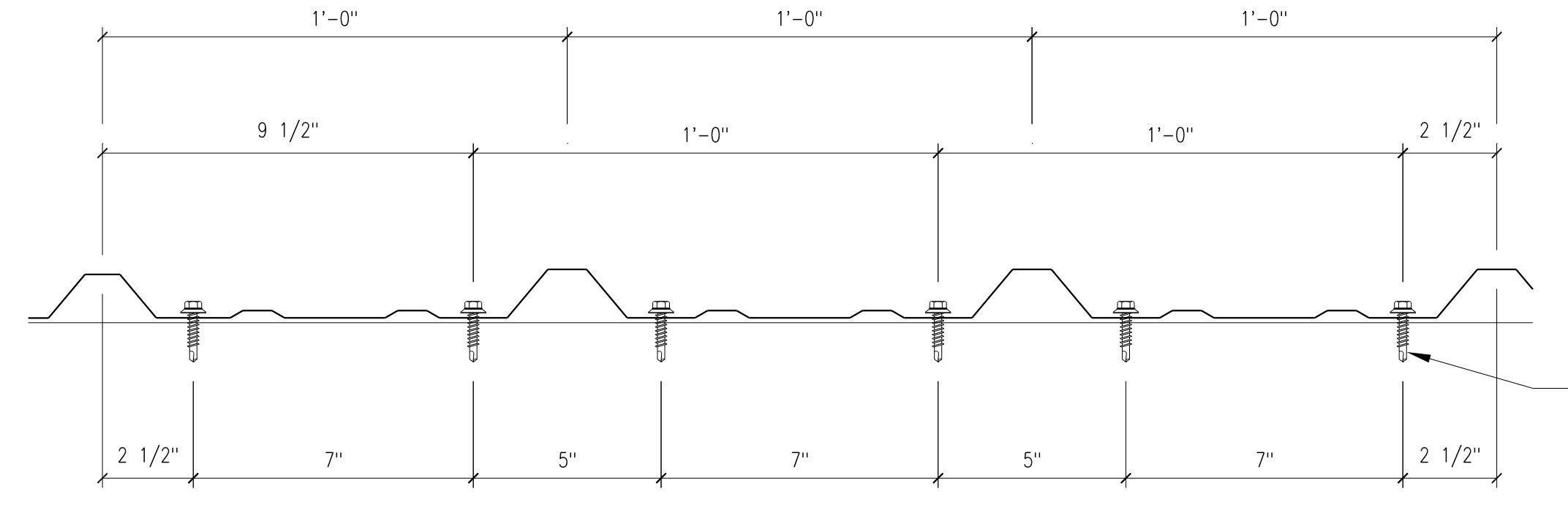
WALL FASTENER SPACING @ INTERMEDIATE SUPPORTS



12 - 14 x 1 1/4" LONG LIFE SELF DRILLER W/ WASHER [FASTENER #3]
1/4 - 14 x 7/8" LONG LIFE LAPTEK W/ WASHER [FASTENER #4A] @ 20" O.C.

WALL FASTENER SPACING @ PANEL ENDS

ROOF FASTENER SPACING @ INTERMEDIATE SUPPORTS

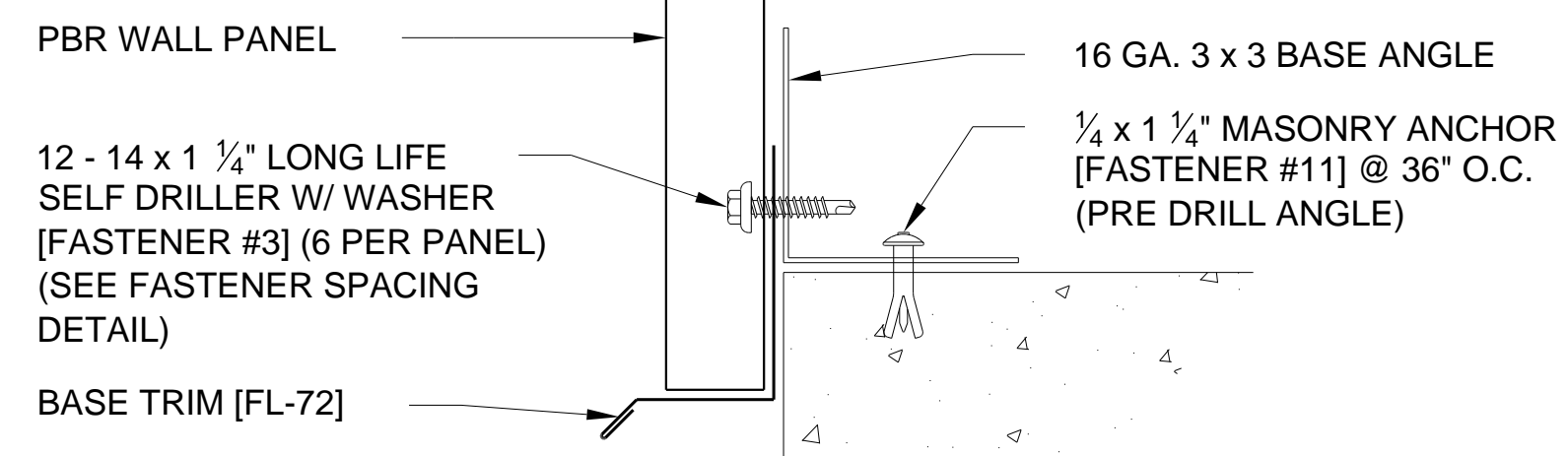


12 - 14 x 1 1/2" LONG LIFE SELF DRILLER W/ WASHER [FASTENER #3A]

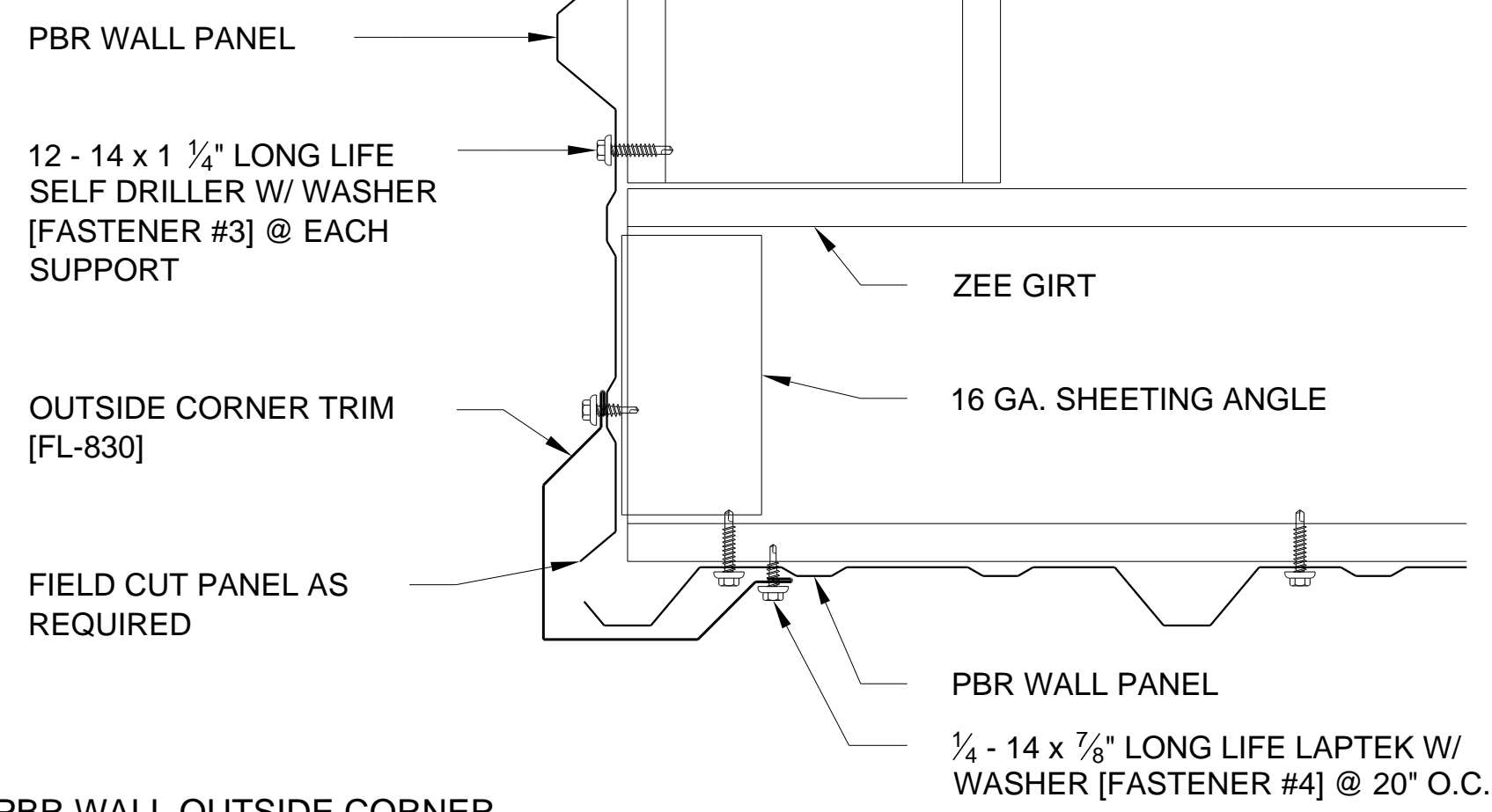
ROOF FASTENER SPACING @ PANEL ENDS

1 PBR WALL PANEL FASTENER SPACING

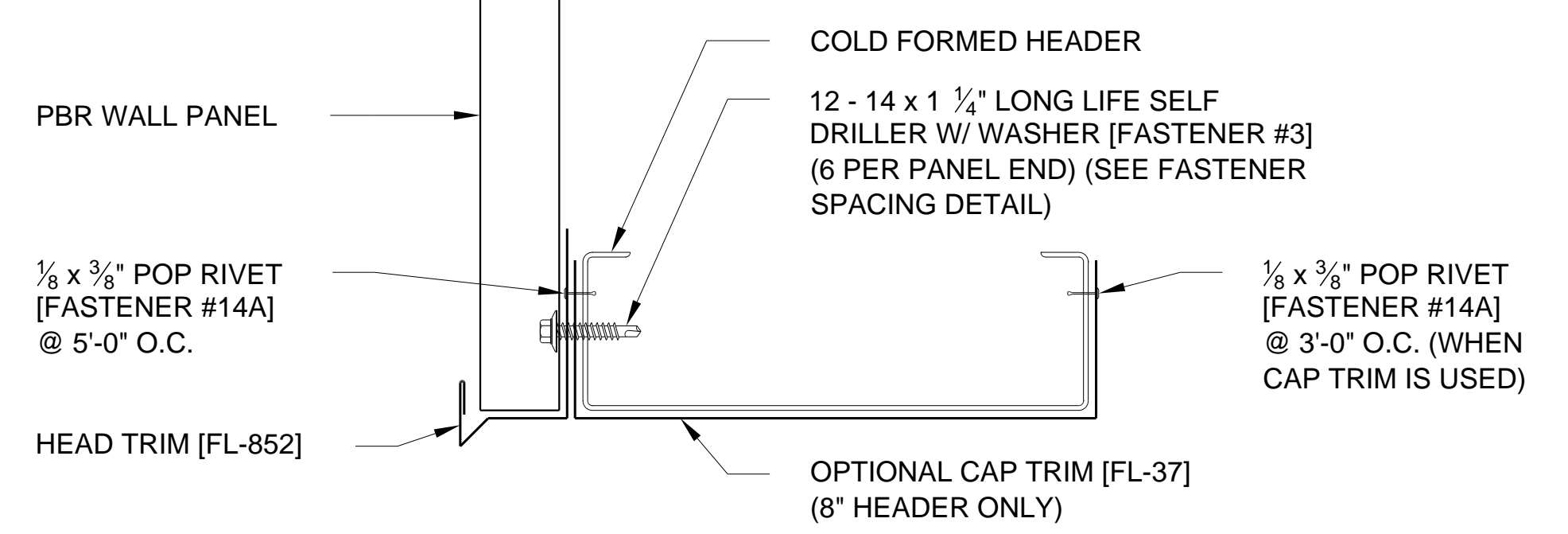
2 PBR ROOF PANEL FASTENER SPACING



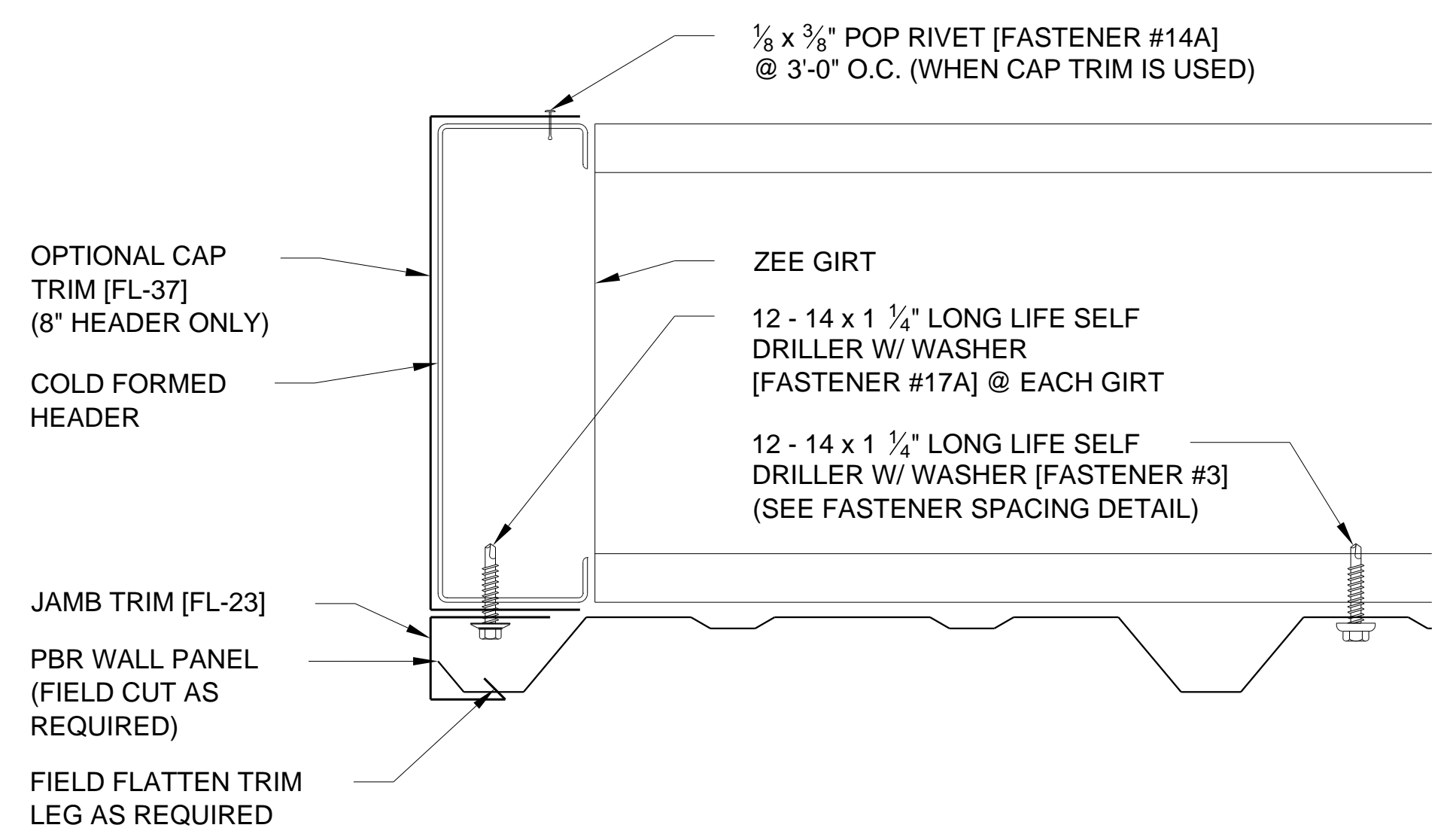
3 PBR WALL BASE



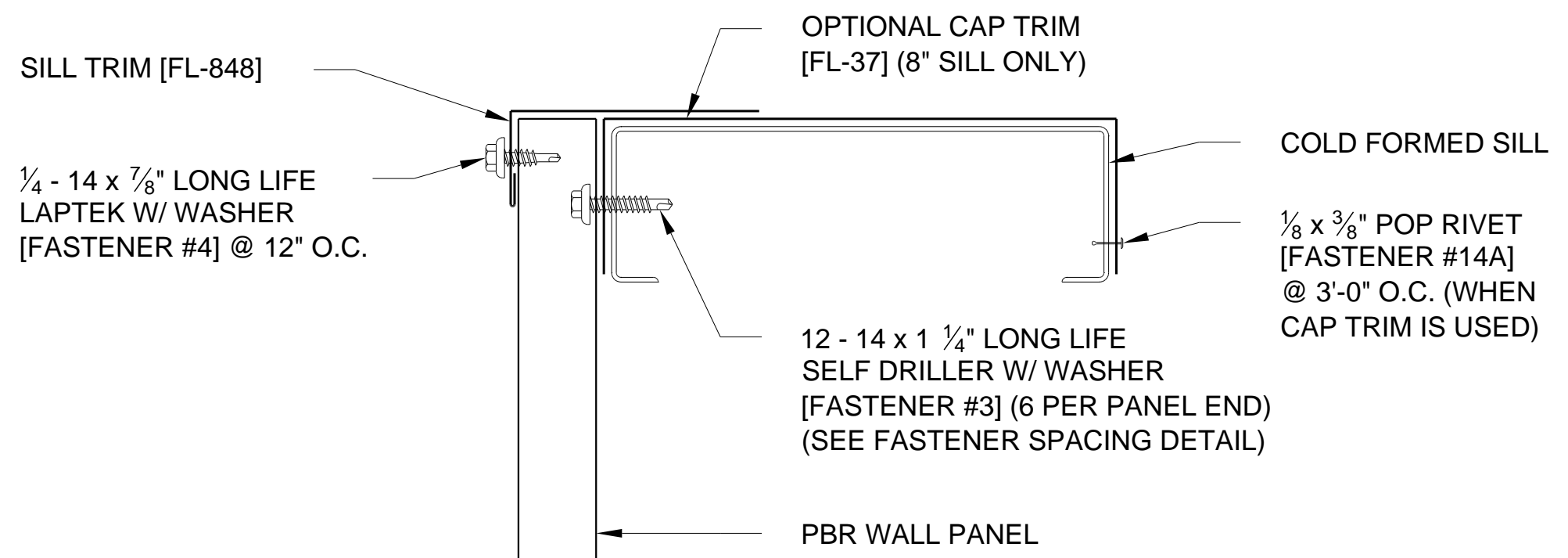
4 PBR WALL OUTSIDE CORNER



5 PBR WALL FRAMED OPENING HEADER



6 PBR WALL FRAMED OPENING JAMB



7 PBR WALL FRAMED OPENING SILL

THE GOOSE
2103 N. 5600 E.
SUNNYFIELD FARM EDEUTUM
CLADDING DETAIL SHEET

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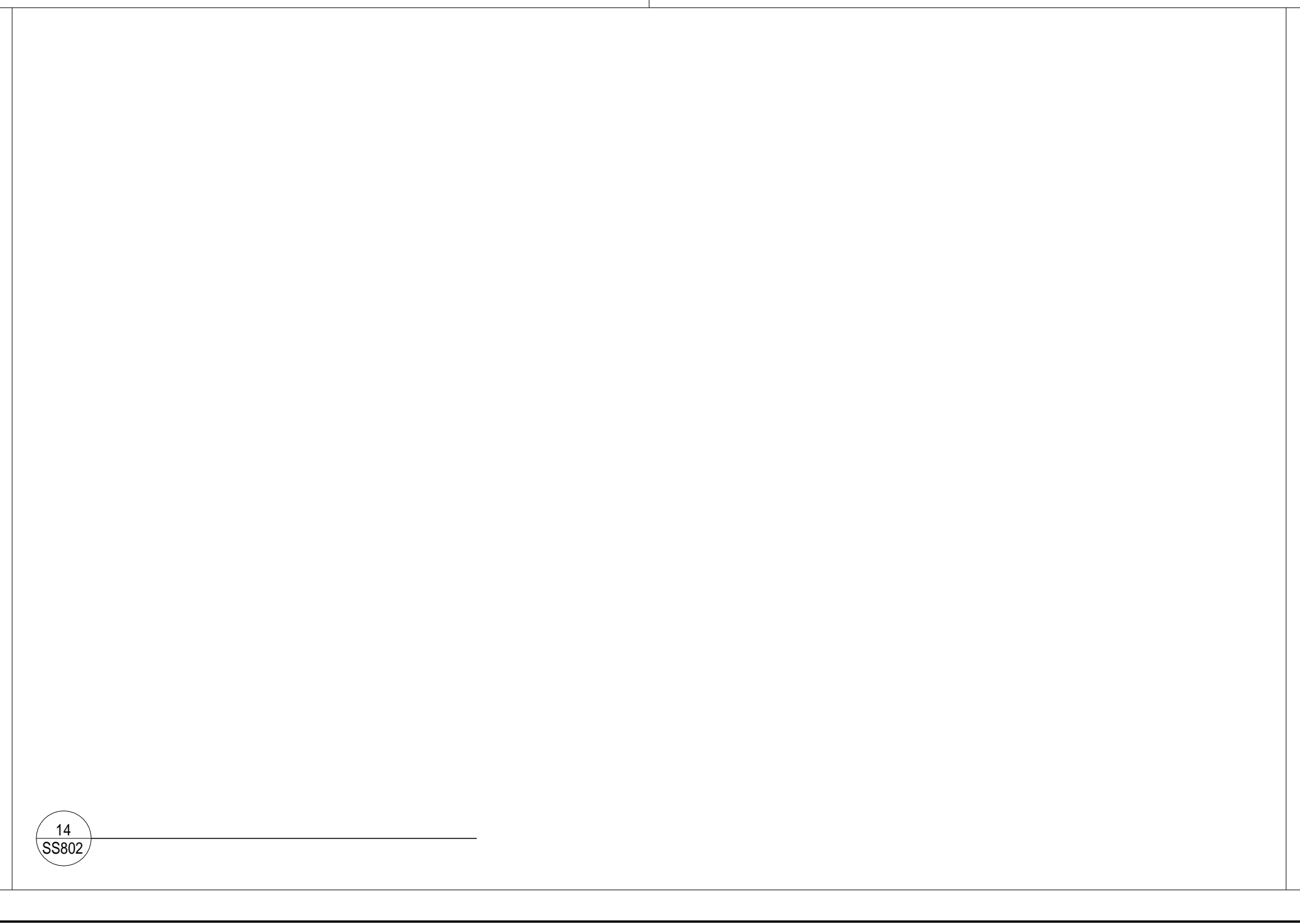
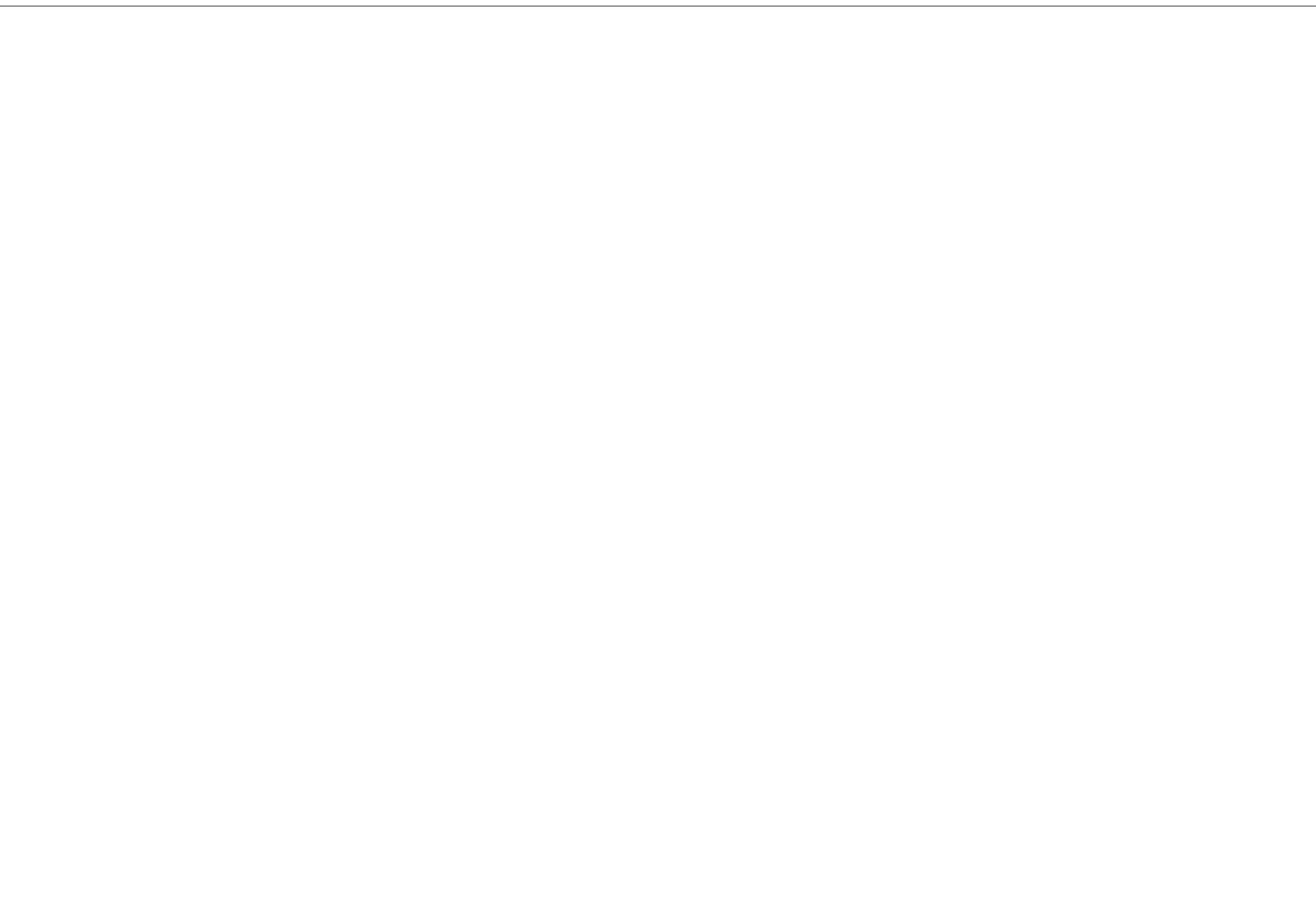
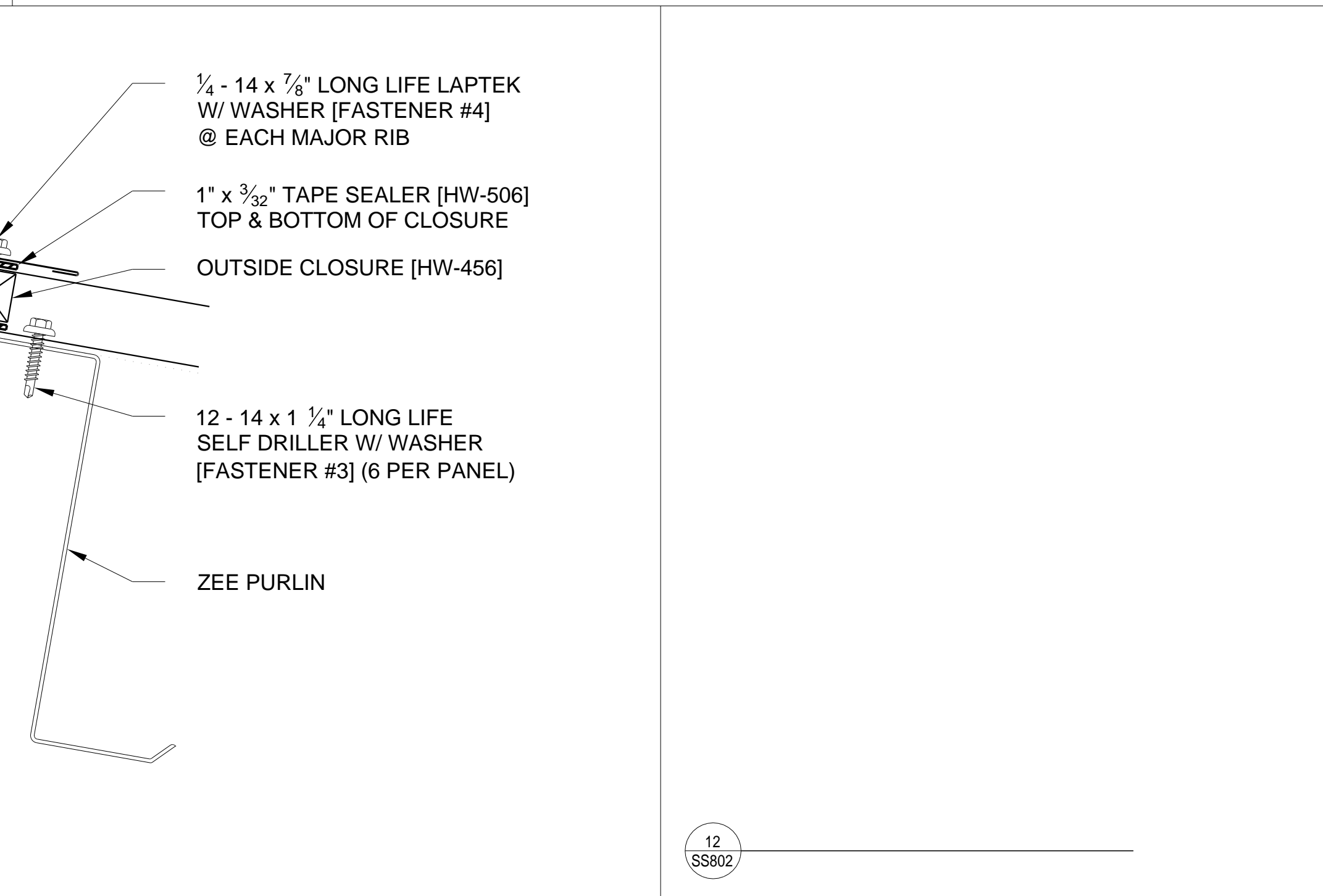
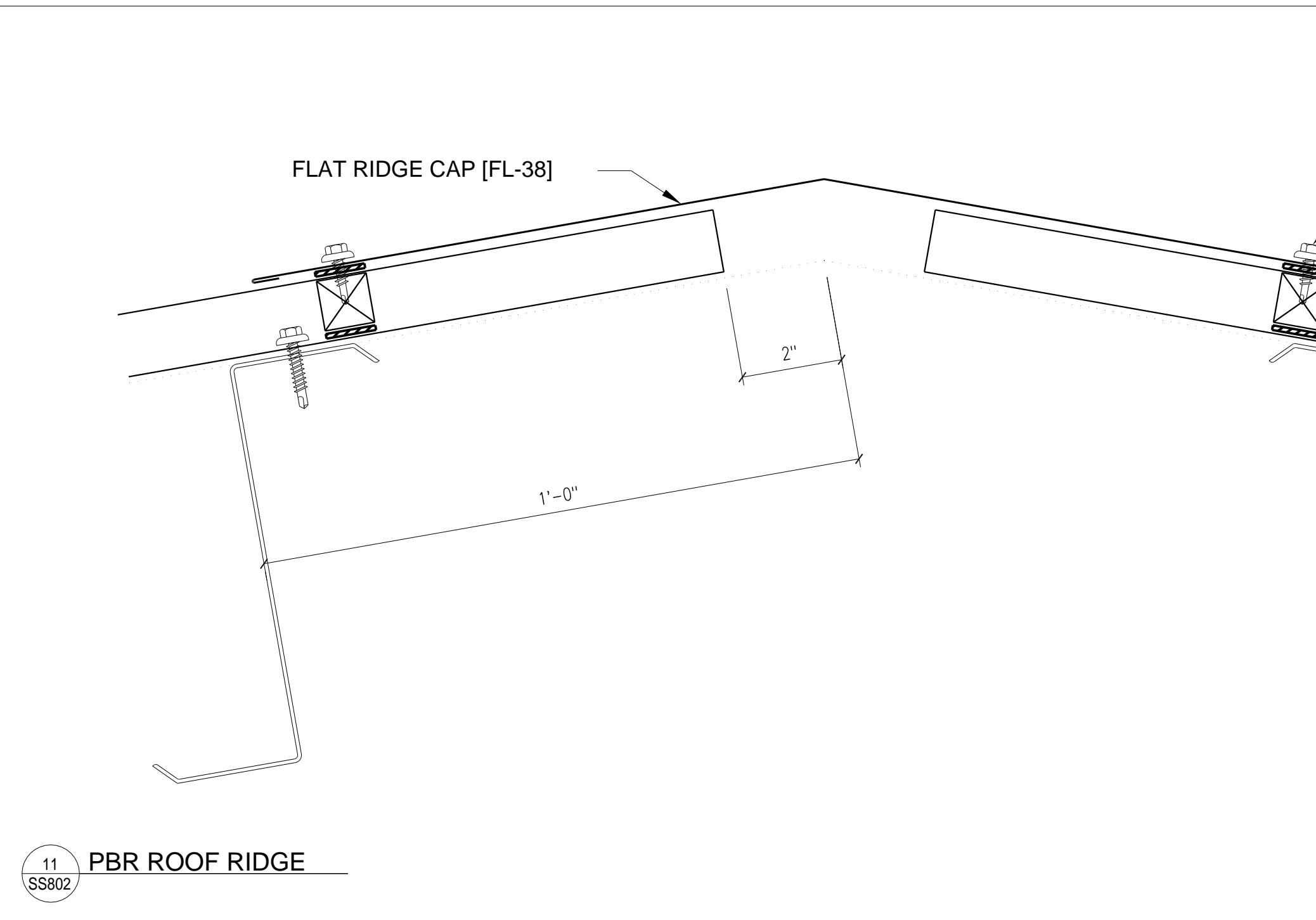
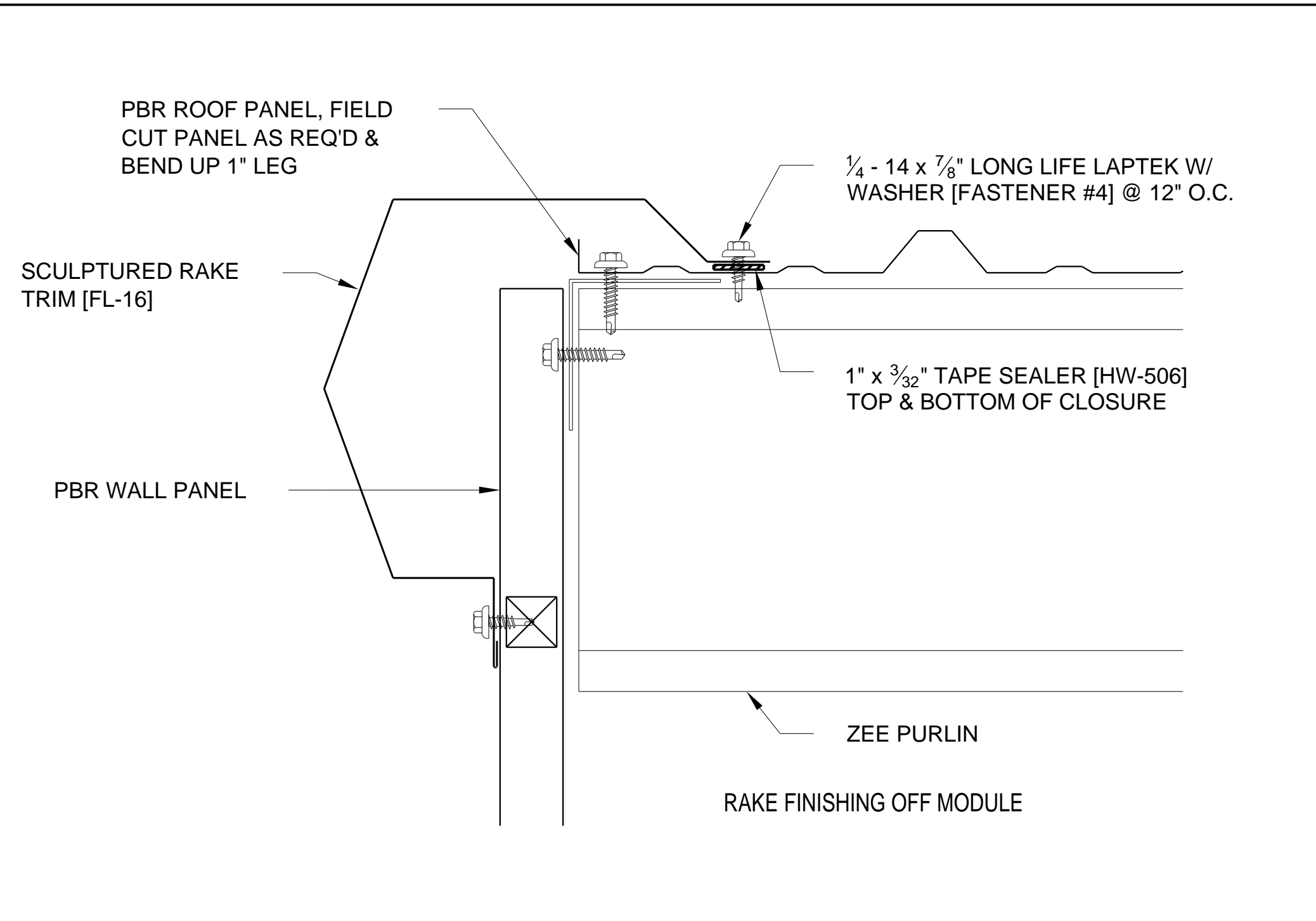
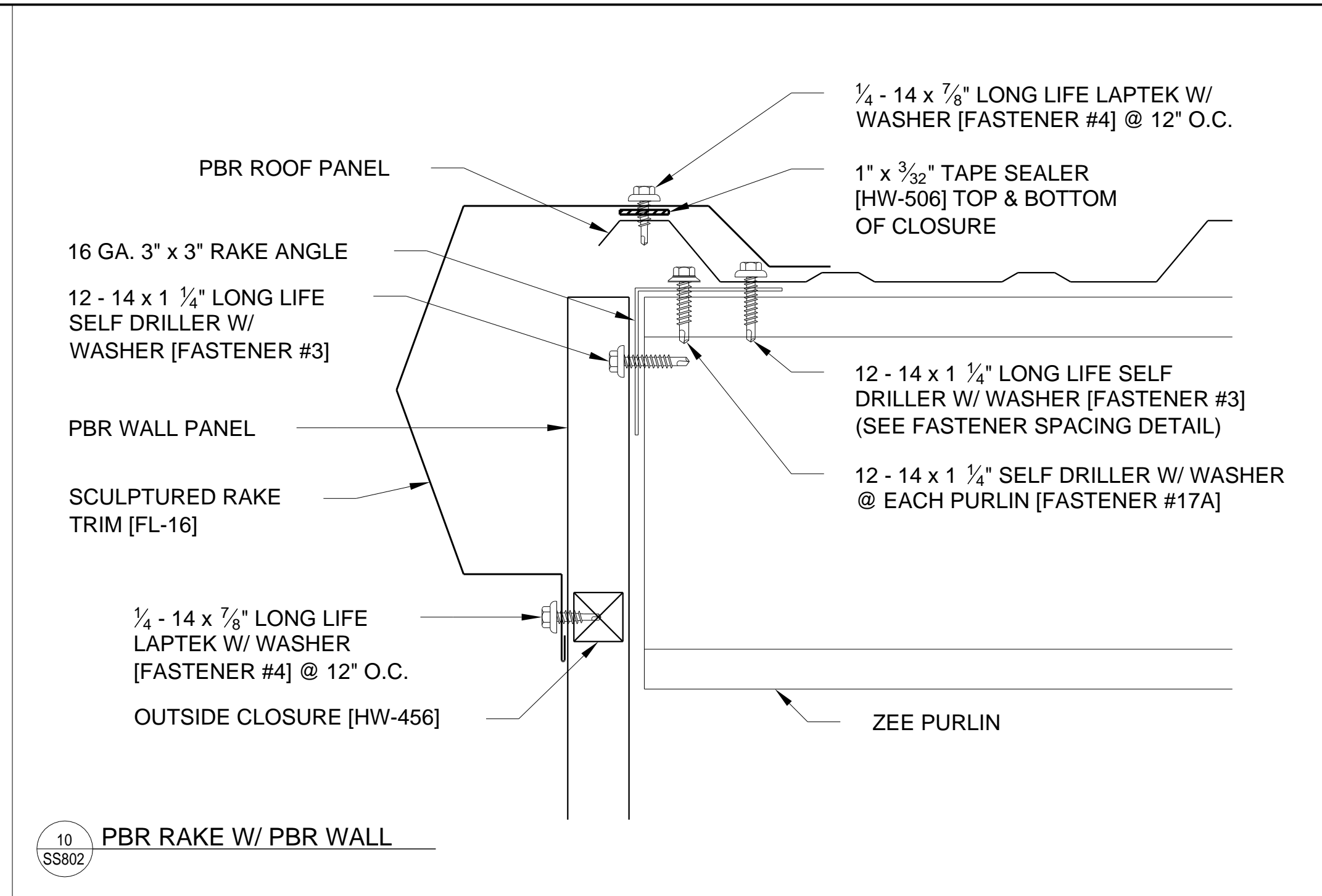
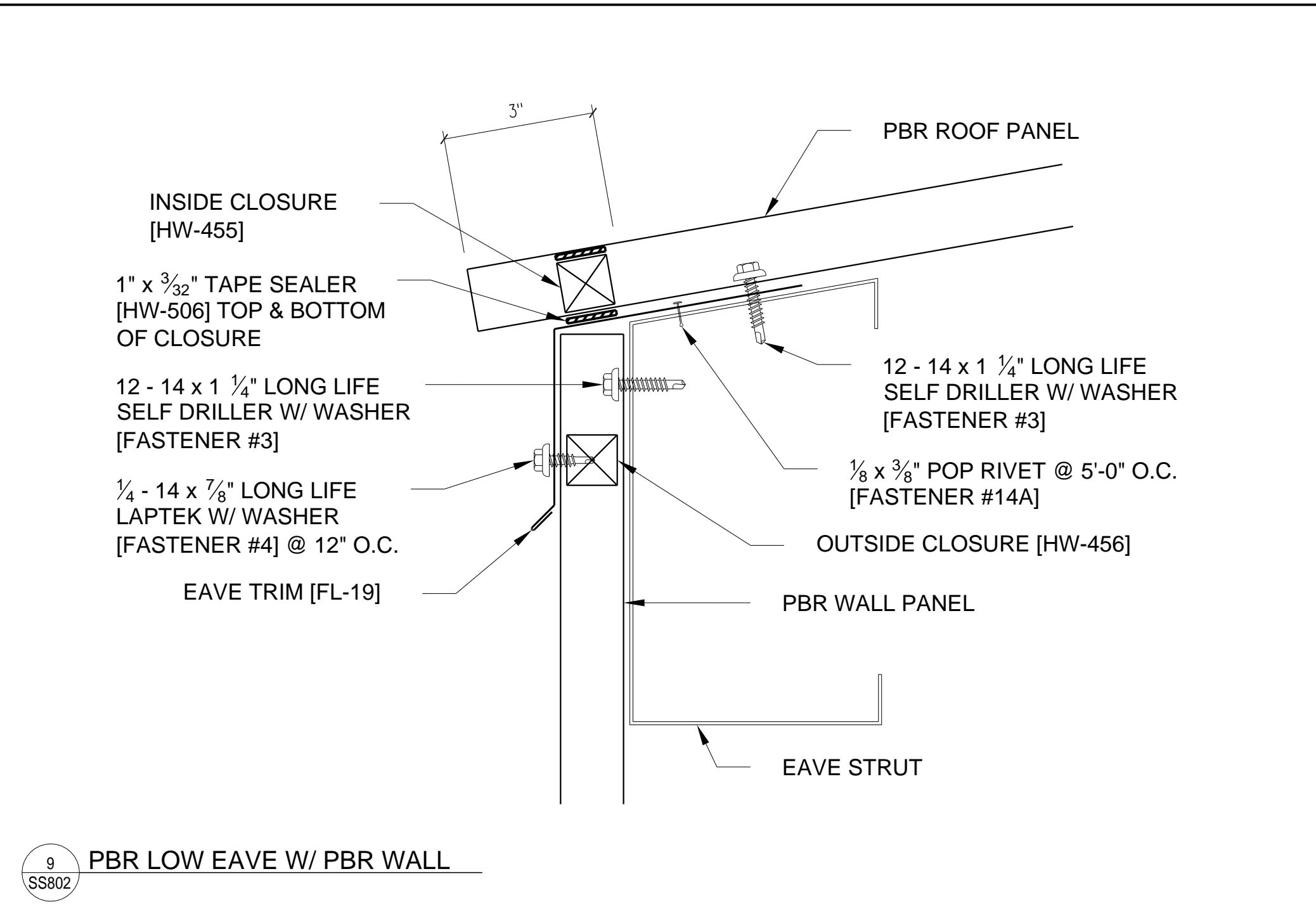
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THE GOOSE

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SUNNYFIELD FARM EDEN, UT

CLADDING DETAIL SHEET

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Taha Structures