

DRAWING NOTES

DESIGN CODE: 2018 IBC.
 USE GROUP: U
 CONSTRUCTION TYPE VB
 DESIGN CATEGORY: RISK CATEGORY I – LOW RISK

SEISMIC CRITERIA:
 DESIGN CATEGORY D
 SOIL SITE CLASS D (ASSUMED)
 R = 2.5 (LIGHT-FRAME WALLS WITH SHEAR PANELS OF ALL OTHER MATERIALS)
 SS= 0.78g, S1= 0.27g; SDS= 0.62g
 ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
 BASE SHEAR= 18,235 LBS

WIND LOAD:
 ULTIMATE WIND DESIGN SPEED: 97 MPH 3 SEC. GUST
 (RISK CATEGORY I, 2018 IBC FIGURE 1609.3(4))
 TERRAIN EXPOSURE C

FROST DEPTH: 40 IN
 SITE ELEVATION: 4970 FT

SNOW LOAD:
 GROUND SNOW LOAD: 53 PSF
 ROOF DESIGN SNOW LOAD: 33 PSF MAIN; 59 PSF LEAN

DEAD LOADS:
 ROOF 4 PSF
 WALLS 4 PSF

ROOF LIVE LOAD: 20 PSF

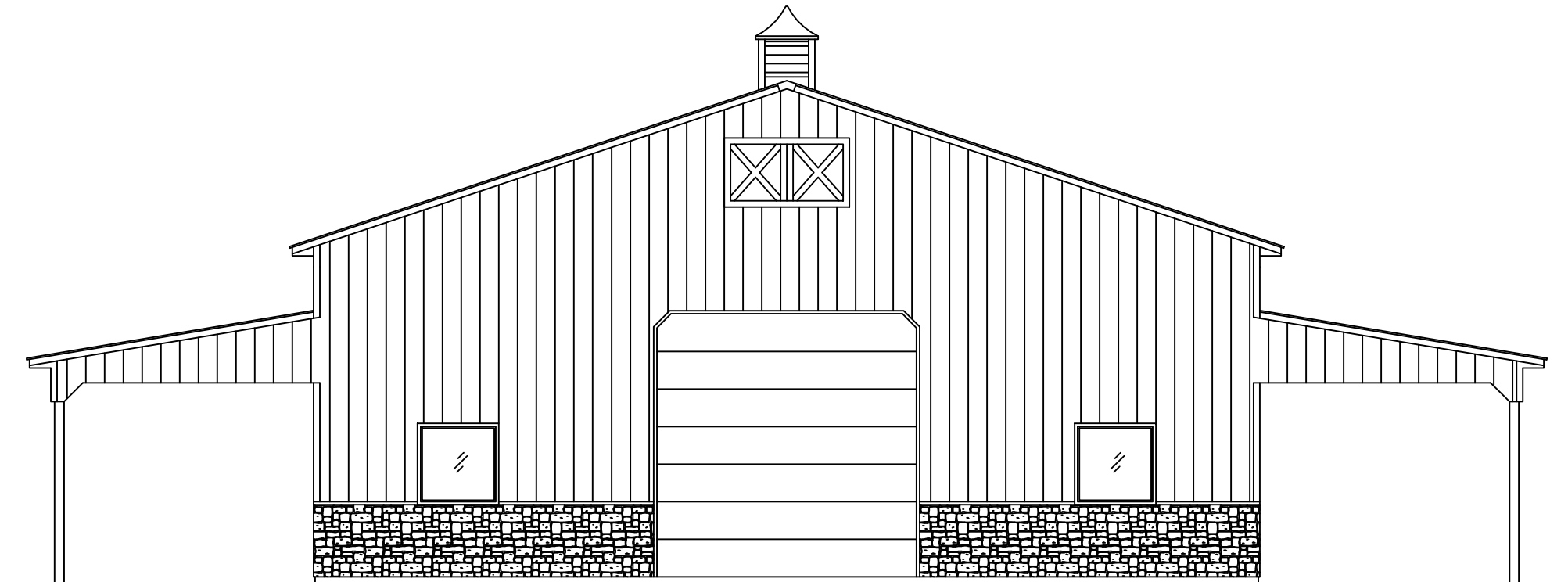
CONCRETE FOUNDATION NOTES:
 1. 28 DAY STRENGTH (F'C) W/ NORMAL 145 PCF DENSITY:
 1.1. FOOTINGS: 3000 PSI
 1.2. SLABS ON GRADE: 3000 PSI REQ'D, 3500 PSI RECOMMEND
 2. ALL SLABS: PROVIDE A MIN. THICKNESS OF 4" W/ 4" DEEP MIN. CRUSHED GRAVEL BASE.
 3. CONTRACTION/CONTROL JOINTS SHALL BE INSTALLED IN SLABS ON GRADE SO THE LENGTH TO WIDTH RATIO OF THE SLAB IS NO MORE THAN 1.5:1. CONTROL JOINT SPACING SHALL NOT EXCEED 30 TIMES THE SLAB THICKNESS IN ANY DIRECTION, UNLESS OTHERWISE NOTED.
 4. CONTROL JOINTS SHALL BE COMPLETED WITHIN 6-18 HOURS OF CONCRETE PLACEMENT.
 5. CONTROL JOINTS SHALL BE TOOLED OR SAWED TO THE GREATER DEPTH OF 1" DEEP OR 1/4 THICKNESS OF CONCRETE SLAB.

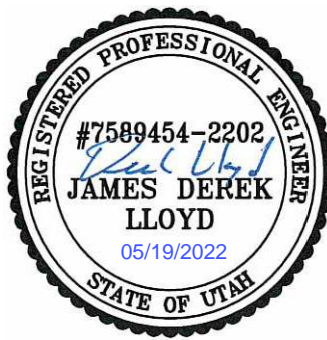

SOILS AND EXCAVATION:
 6. NO SOILS REPORT PROVIDED – STABLE SOIL CHARACTERISTICS ASSUMED. ALL DESIGN WAS BASED ON STABLE SOIL CHARACTERISTICS. GEOTECHNICAL HAZARDS FOUND ON OR AROUND THE SITE, INCLUDING EXPANSIVE CLAYS, OR SOILS FOUND AT THE SITE WHILE EXCAVATION OCCURS WHICH DIFFERS FROM THOSE ASSUMED SHOULD BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND ENGINEER.
 7. ALLOWABLE BEARING PRESSURE: 1500 PSF
 8. NATIVE MATERIAL SURROUNDING FOOTING TO BE DISTURBED MINIMALLY DURING EXCAVATION.
 9. FOOTINGS SHALL BE PLACED ENTIRELY IN UNDISTURBED NATIVE SOILS OR STRUCTURAL FILL WHICH IS BEARING ON UNDISTURBED NATIVE SOILS AND IS COMPACTED TO 95% OF THE MODIFIED PROCTOR DENSITY.

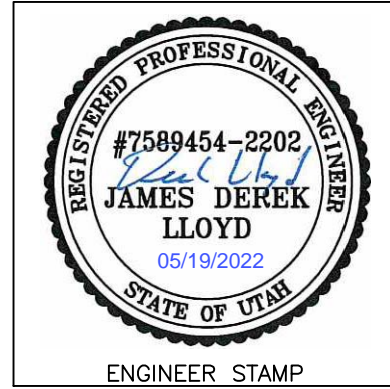
GENERAL:
 10. PLEASE REFER TO THE STRUCTURAL CALCULATIONS FOR ALL SPECIFICATIONS AND DESIGN CRITERIA NOT LISTED HERE. WHERE DETAILS ARE NOT SPECIFIED, TYPICAL DETAILS AS SPECIFIED ON PLANS AND ON THE STRUCTURAL DETAILS SHEET SHALL APPLY.
 11. FOR SPECIFICATIONS NOT SHOWN REFER TO THE IBC.

NORTH STAR BUILDINGS

WADE AND CHELSEE TOLMAN BUILDING



| BUILDING INFORMATION | CONTRACTOR | DRAFTING & ENGINEERING | DRAWING INDEX | |  ENGINEER STAMP |
|--|--|--|---------------|-----------------|---|
| SITE INFORMATION: ADDRESS: 425 S 8300 E HUNTSVILLE, UTAH BUILDING INFORMATION: DIMENSIONS: 50' x 60' TOTAL SQUARE FOOTAGE: 4680 S.F. MAIN BUILDING: 3000 S.F. LEANS: 1680 S.F. | NORTH STAR BUILDINGS CONTACT: ALAN WALKER ALAN@NORTHSTARBUILDINGS.COM PHONE: 385-254-1086 | MOUNTAIN POINT ENGINEERING CONTACT: DEREK LLOYD DEREK@MOUNTAINPOINTENGINEERING.COM PHONE: 801-450-5332  | SHEET | DESCRIPTION | |
| | | | 00 | COVER SHEET | |
| | | | 01 | FOUNDATION PLAN | |
| | | | 02 | FLOOR PLAN | |
| | | | 03 | ROOF PLAN | |
| | | | 04 | ELEVATIONS | |
| | | | 05 | GIRT PLAN | |
| | | | 06 | PANEL LAYOUT | |
| | | | 07-09 | DETAIL SHEETS | |



MOUNTAIN POINT
ENGINEERING

FOUNDATION PLAN

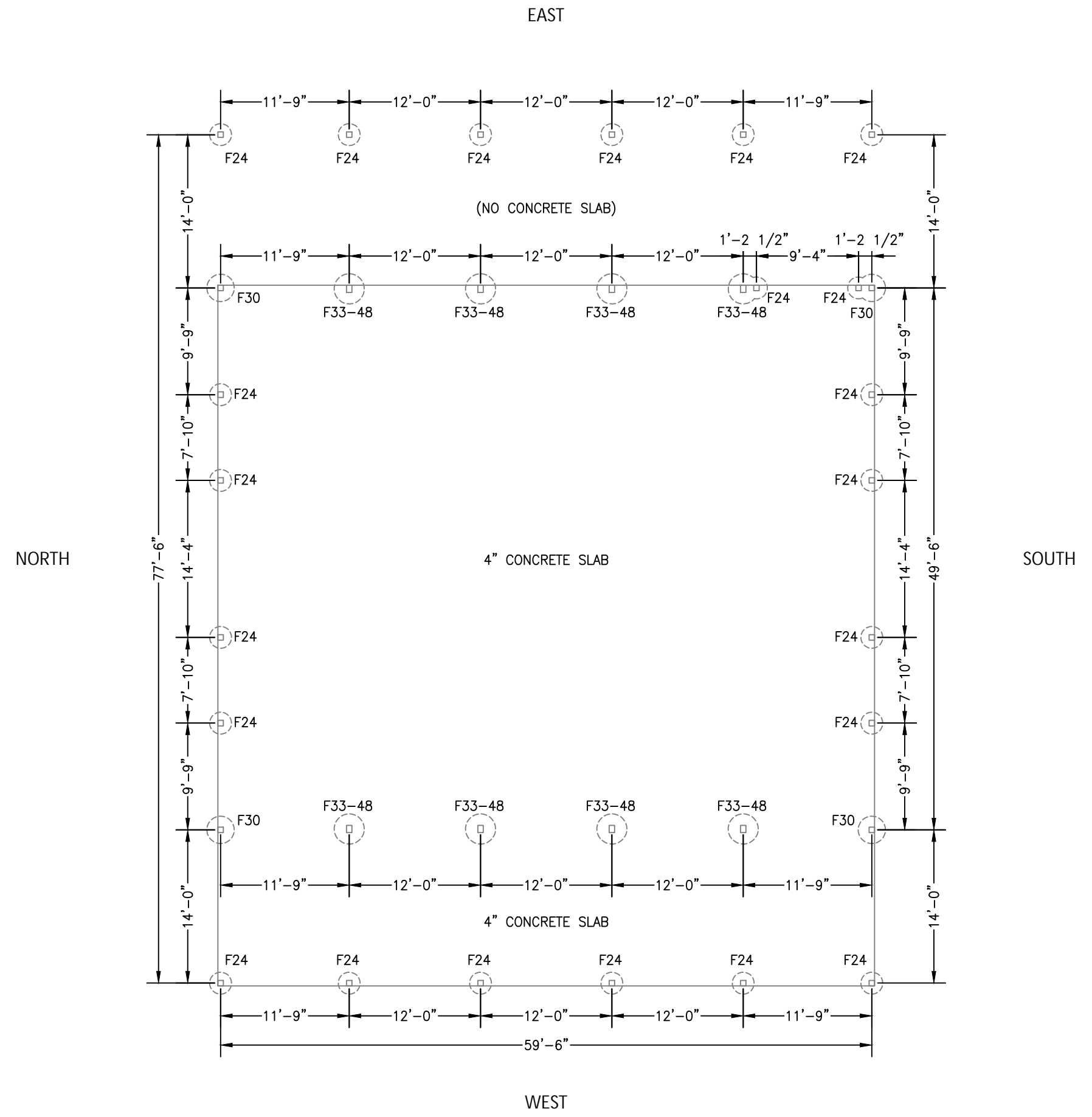
WADE & CHELSEE TOLMAN
BUILDING
HUNTSVILLE, UTAH

PROJECT
NS77

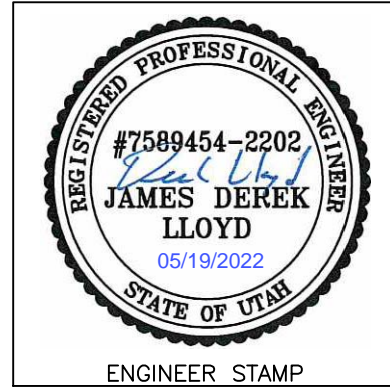
SCALE
3/32" = 1'-0"

DATE
05/19/2022

SHEET
01



| FOOTING SCHEDULE | | | | |
|------------------|----------|------------|-----------------|-------------------------|
| MARK | DIA (IN) | DEPTH (IN) | SPIKES PER SIDE | REINFORCEMENT |
| F24 | 24 | 40 | 1 | NONE |
| F30 | 30 | 40 | 1 | NONE |
| F33-48 | 33 | 48 | 2 | #4 BAR @ 6" O.C. EA WAY |



MOUNTAIN POINT
ENGINEERING

FLOOR PLAN

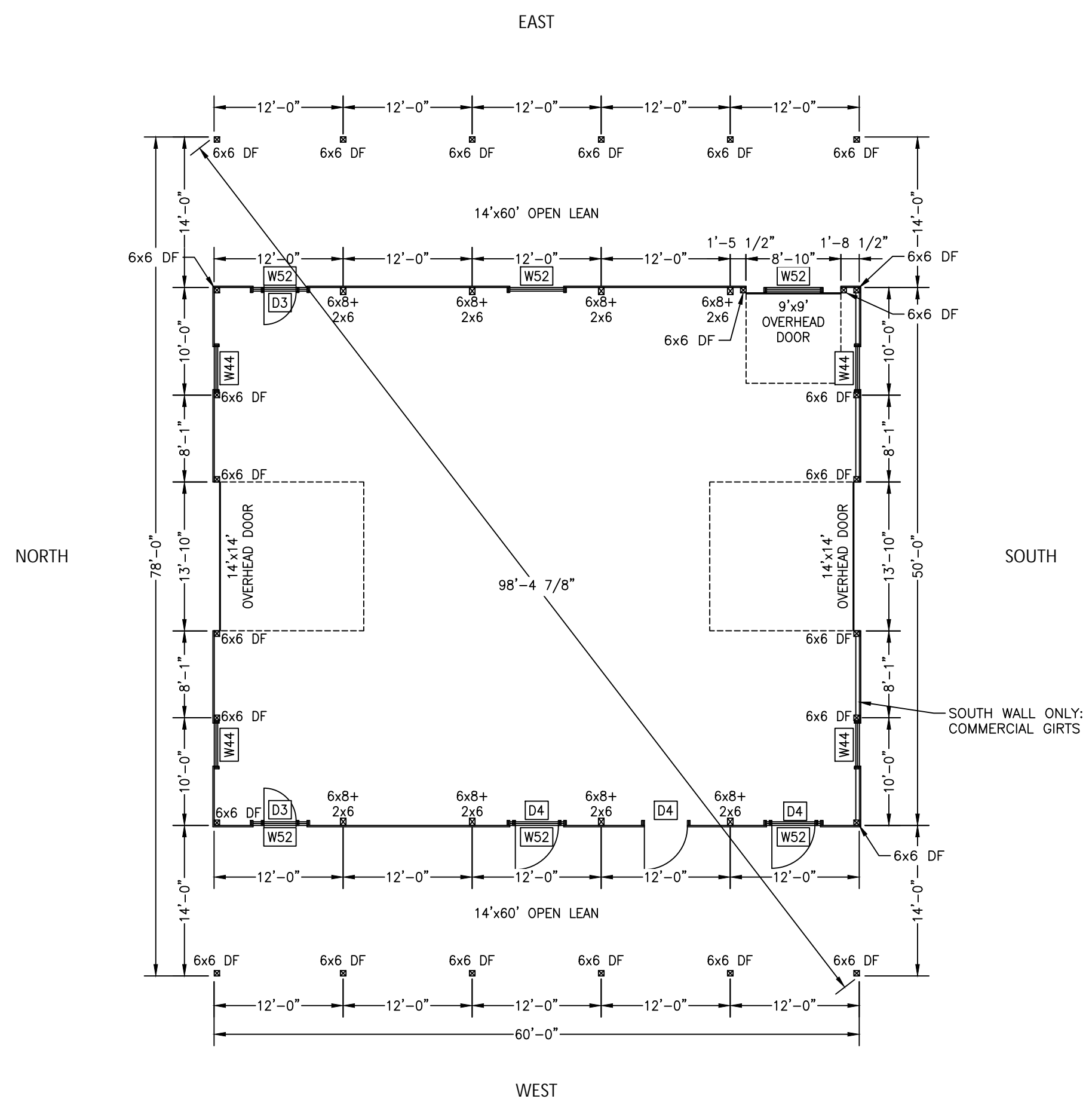
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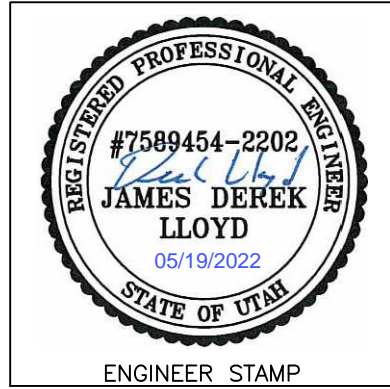
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SHEET
02



- D3 3068 DOOR
- D4 4070 DUTCH DOOR
- W44 4040 WINDOW
- W52 5020 WINDOW

- NOTES:
1. 6x6 DF: 6x6 DF-L #2 TREATED POST.
 2. 6x8 DF: 6x8 DF-L #2 TREATED POST.
 3. 6x8+(1)2x6: 6x8 DF-L #2 TREATED POST WITH TREATED 2x6 DF-L #2 ATTACHED PER DETAIL ON SHEET 08.
 4. EMBED POST INTO CONCRETE PIER. SEE DETAIL ON SHEET 8.
 5. ROOF: 29 GA STEEL PANEL
 6. WALLS: STANDARD GIRTS, 29 GA STEEL PANEL



ROOF PLAN

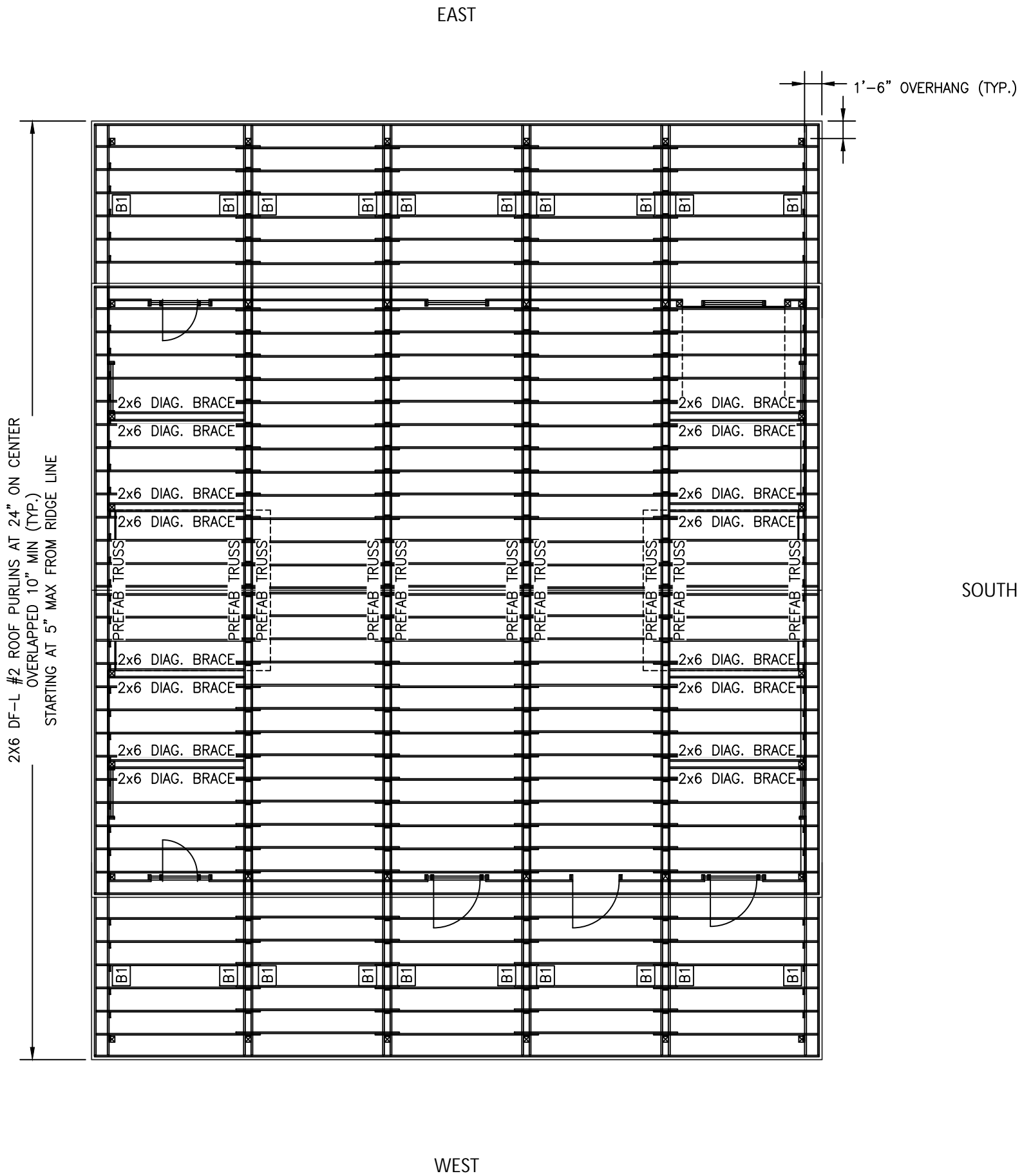
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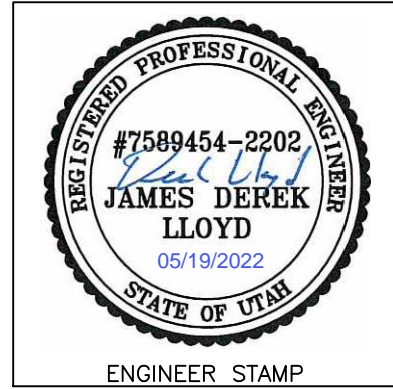
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03



BEAMS:
1. B1: 1-3/4"x11-7/8" LVL



MOUNTAIN POINT
ENGINEERING

ELEVATIONS

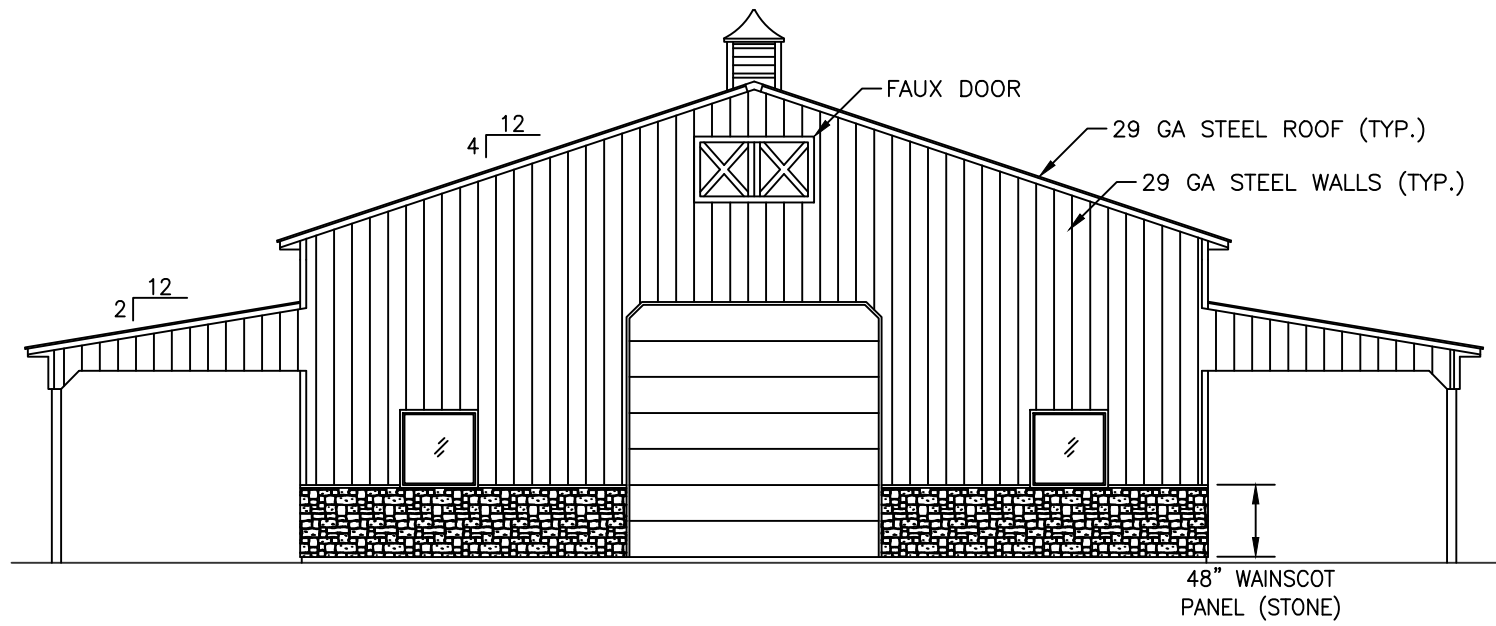
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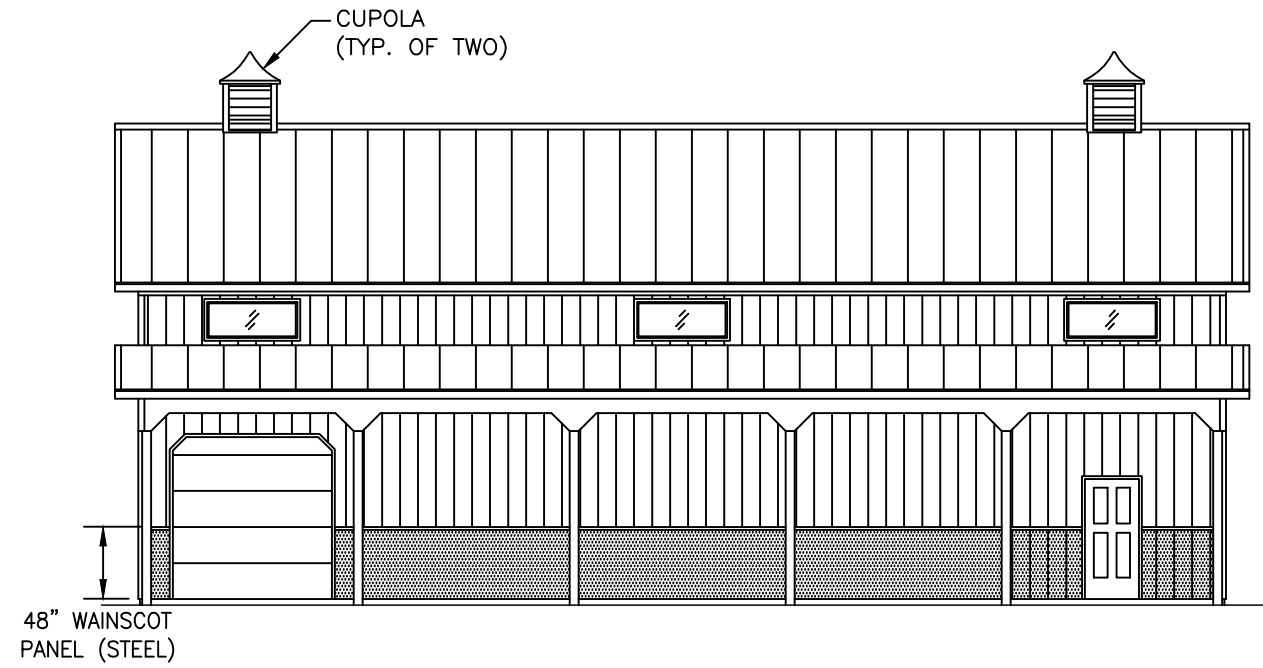
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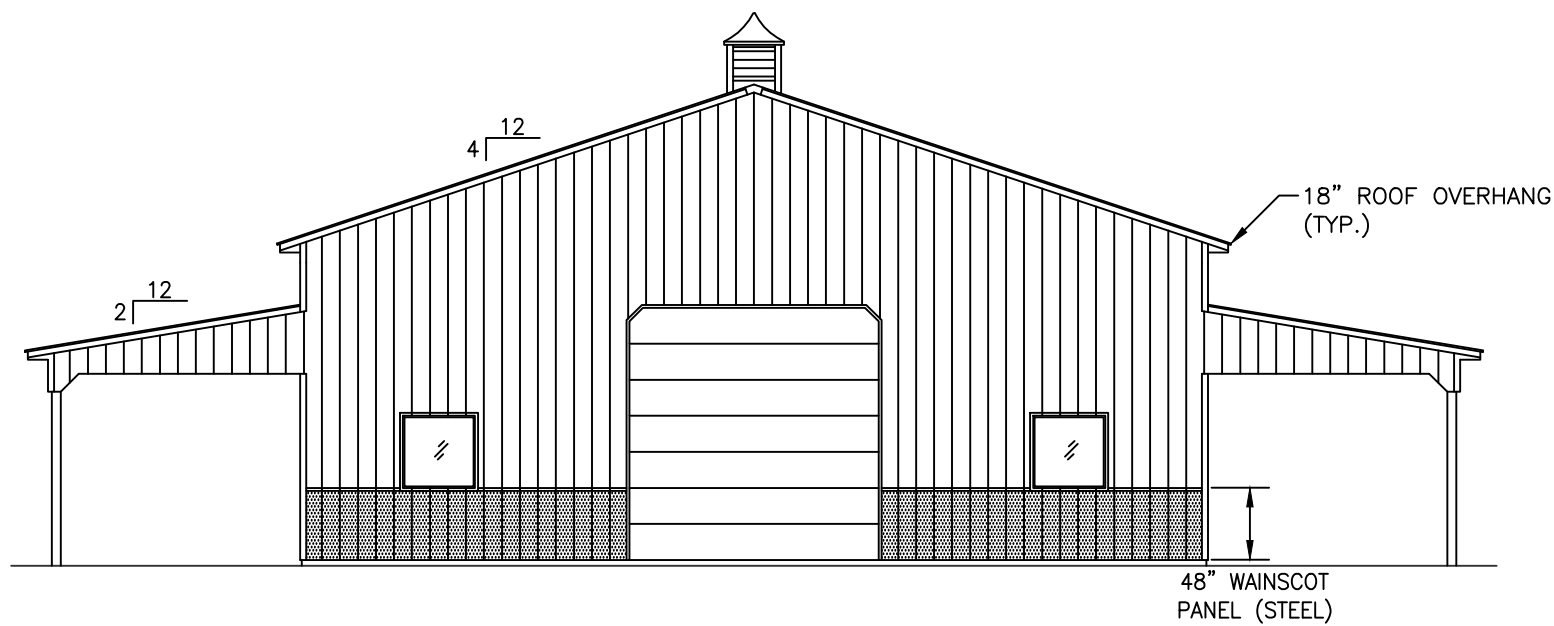
SHEET
04



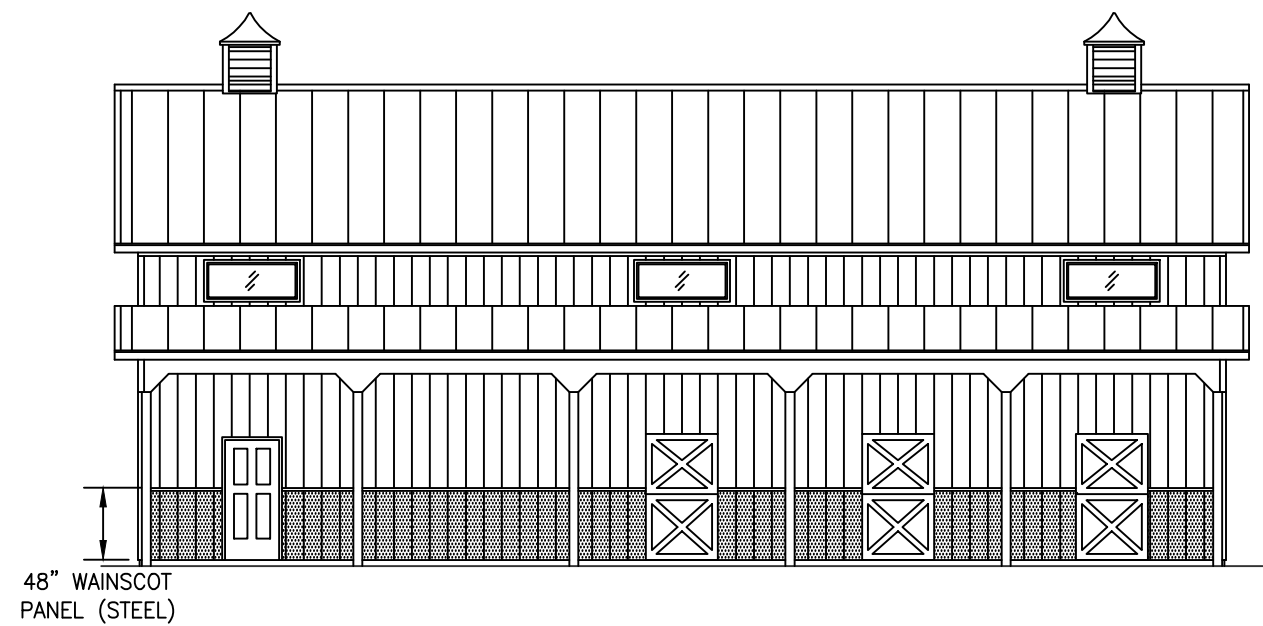
GABLE END ELEVATION - SOUTH



SIDE ELEVATION - EAST



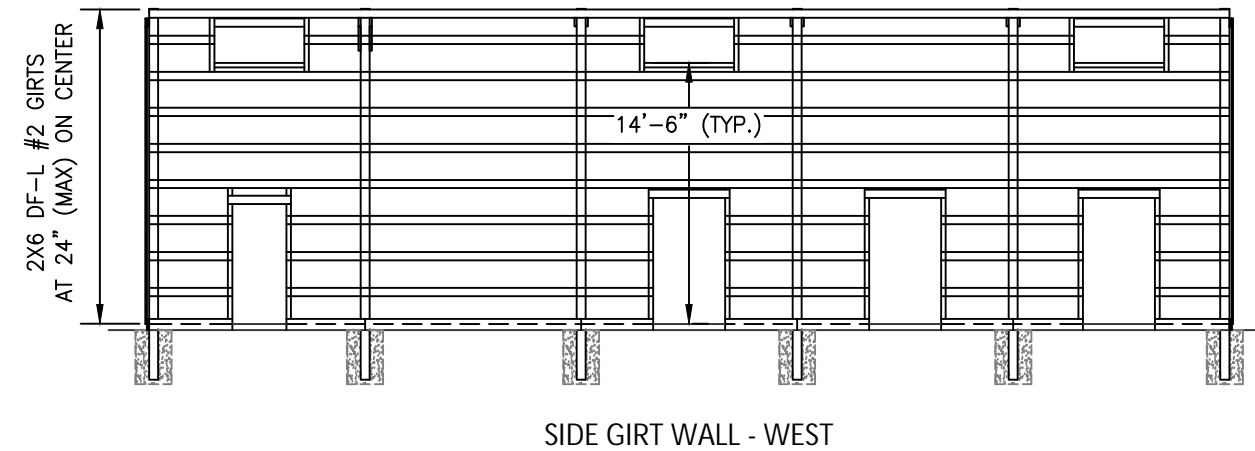
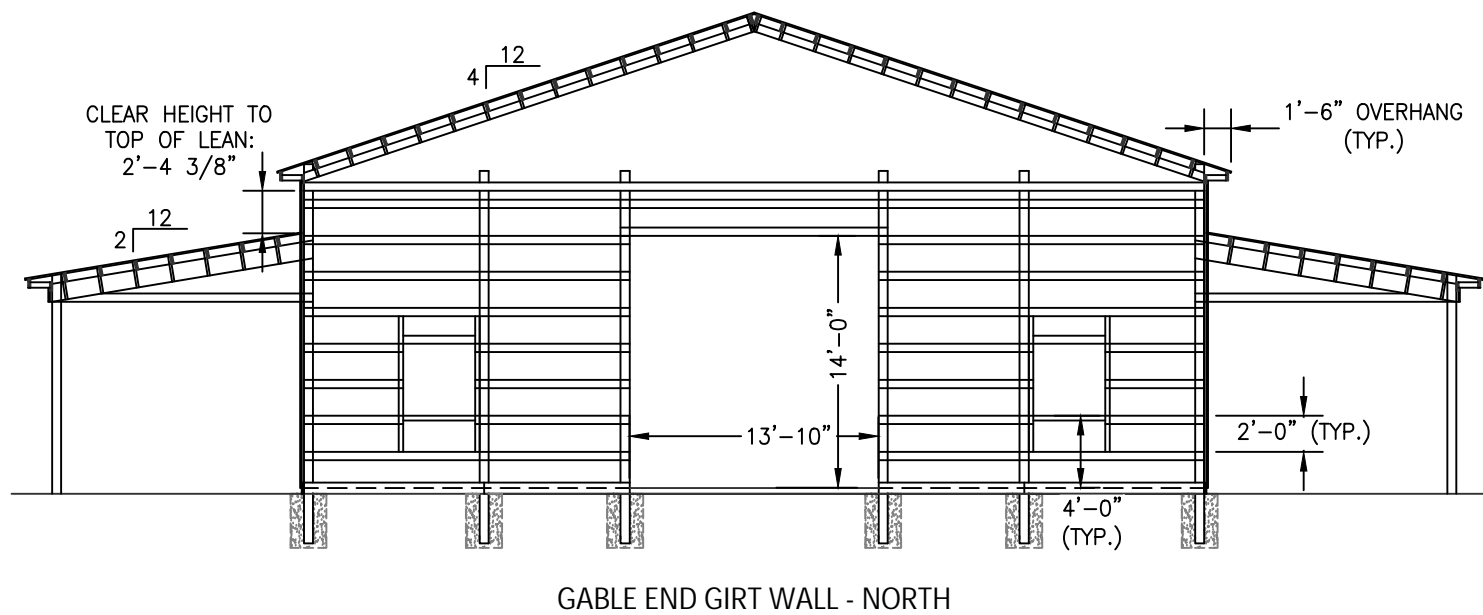
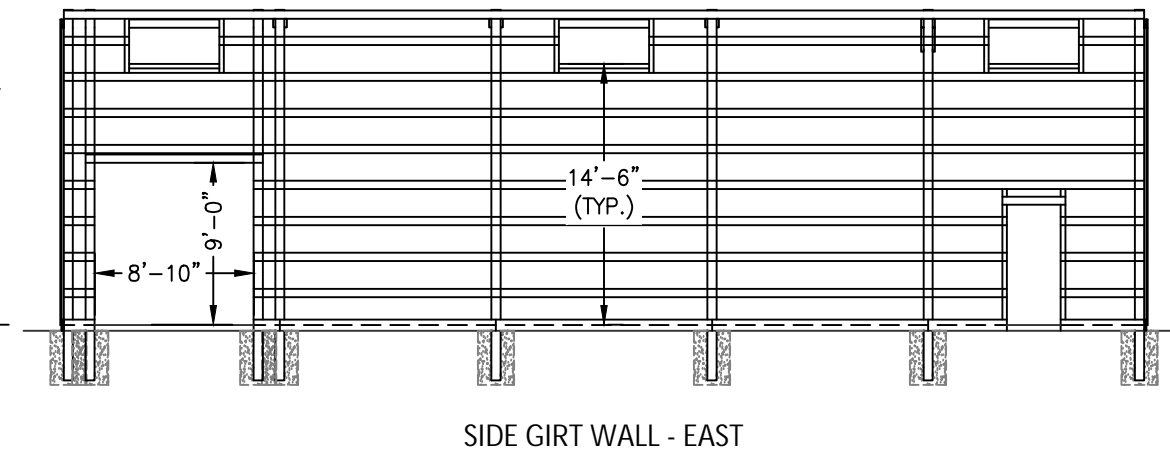
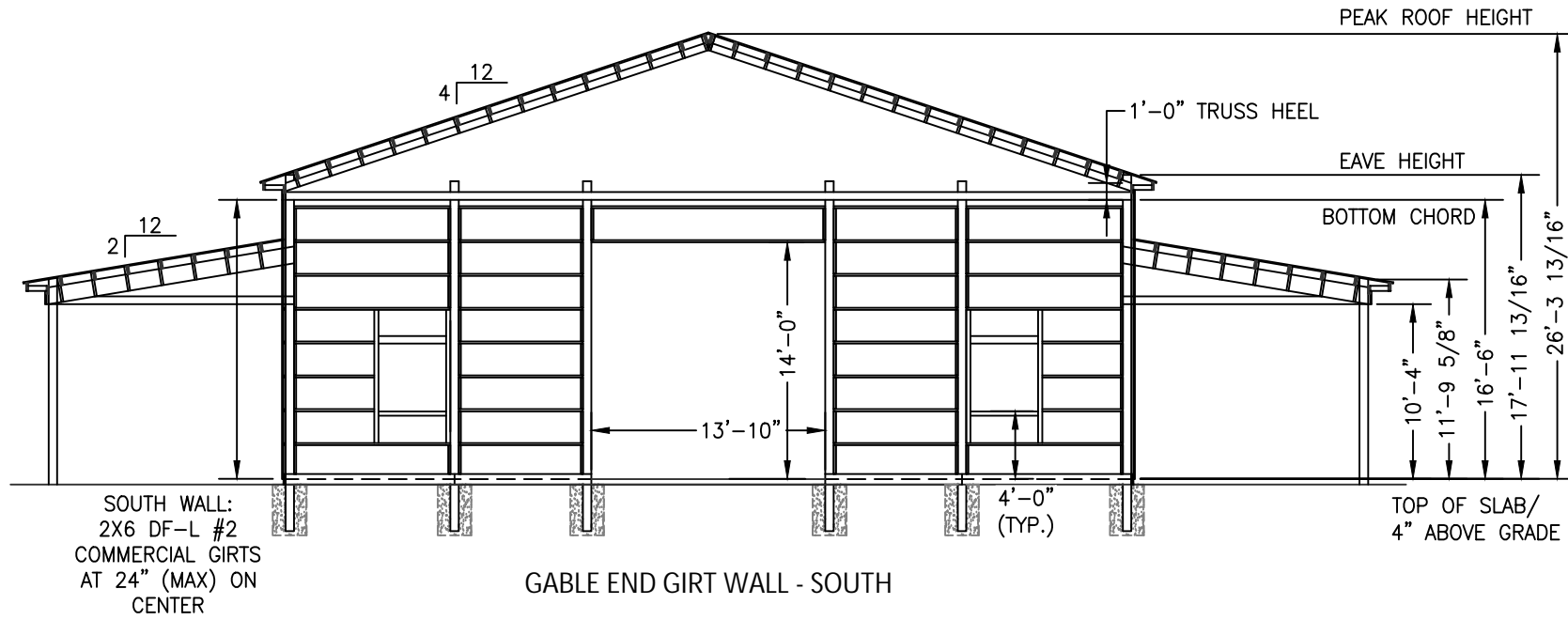
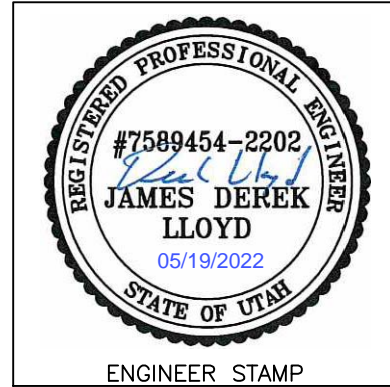
GABLE END ELEVATION - NORTH



SIDE ELEVATION - WEST

NOTES:

1. TRUSS HEEL SHOWN AT 6-3/16" ON THESE PLANS. COMPARE PLAN DIMENSIONS WITH TRUSS MANUFACTURER DRAWINGS AND ADJUST PLAN DIMENSIONS (EAVE HEIGHT, PEAK HEIGHT, PANEL LENGTHS, ETC) AS NECESSARY.



GIRT PLAN

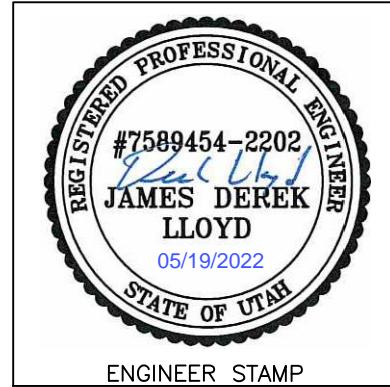
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3/32" = 1'-0"

DATE
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SHEET
05

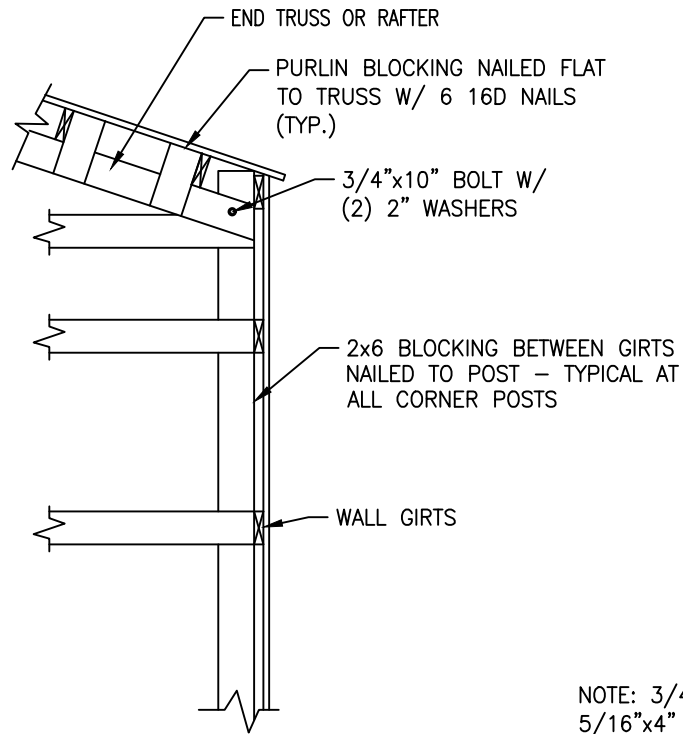


MOUNTAIN POINT
ENGINEERING

DETAIL SHEET

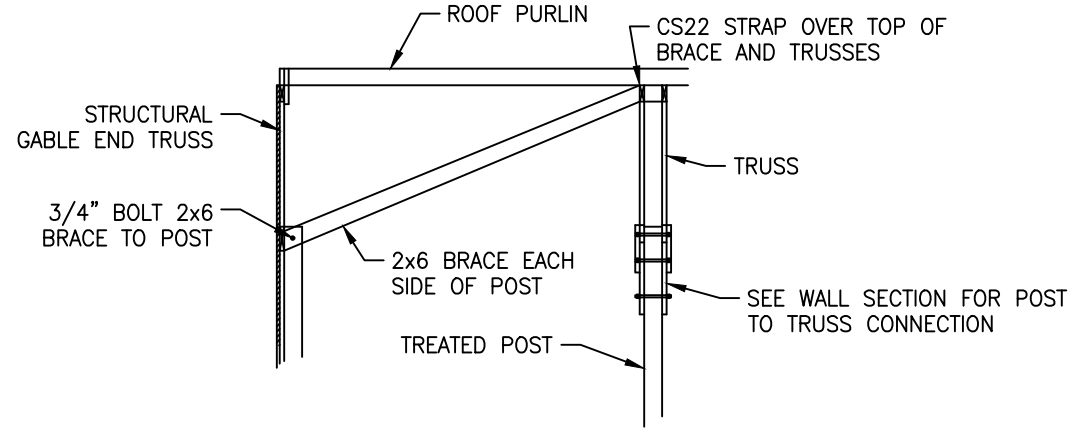
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SCALE
AS NOTED
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SHEET
07

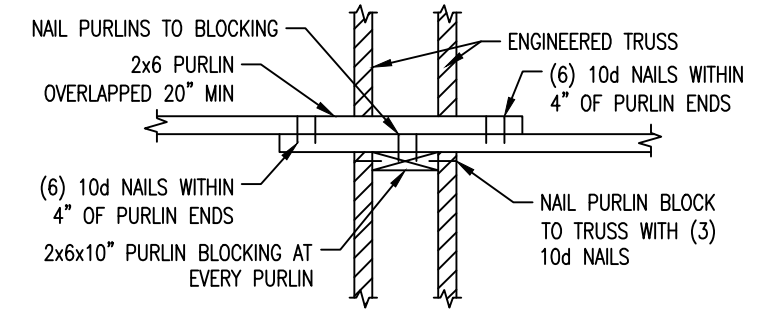


CORNER BLOCKING
NOT TO SCALE

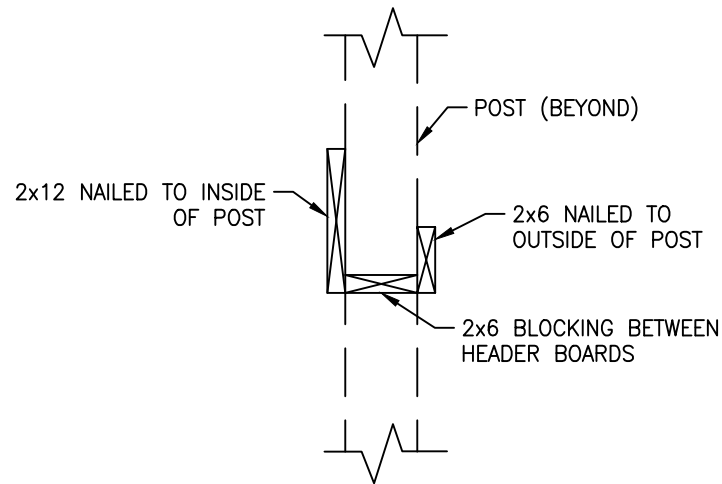
NOTE: 3/4" BOLT ALTERNATIVE: (2)
5/16"x4" POWERLAG FASTENERS
(TYP. FOR CORNER BLOCKING AND
POST TO TRUSS BRACE DETAILS)



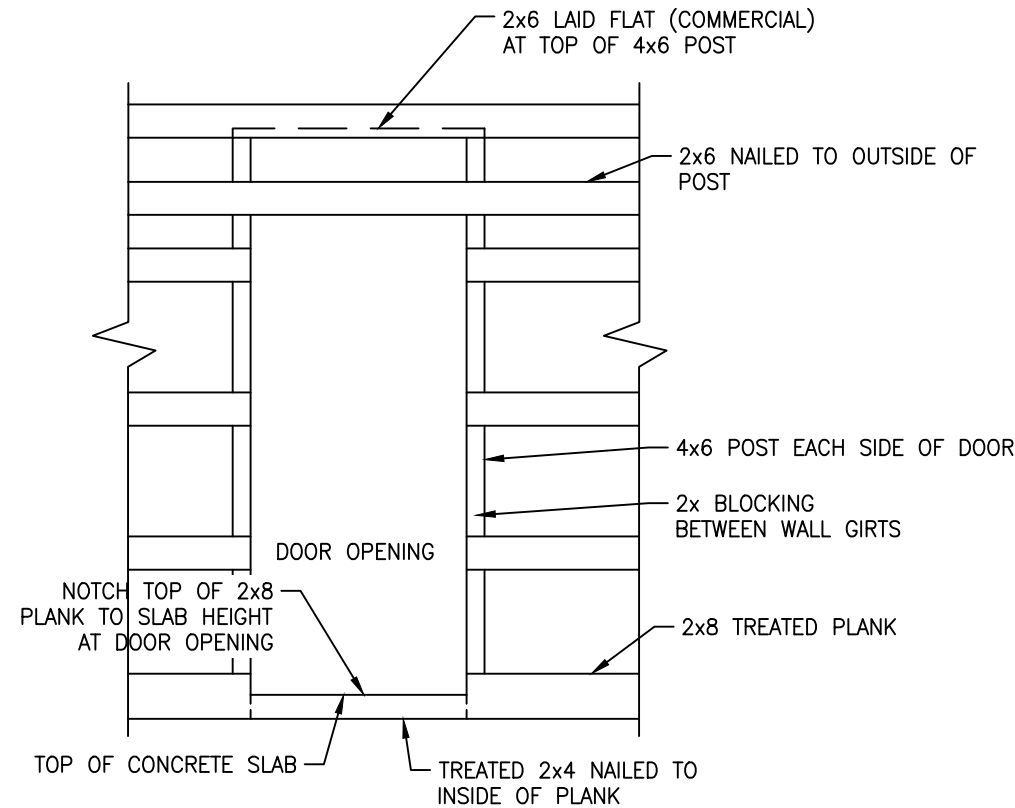
POST TO TRUSS BRACE
NOT TO SCALE



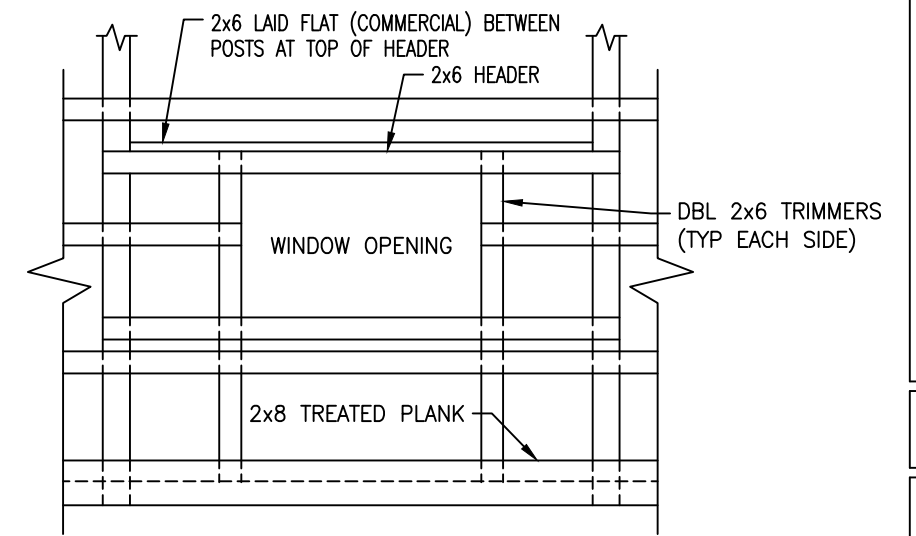
PURLIN CONNECTION
NOT TO SCALE



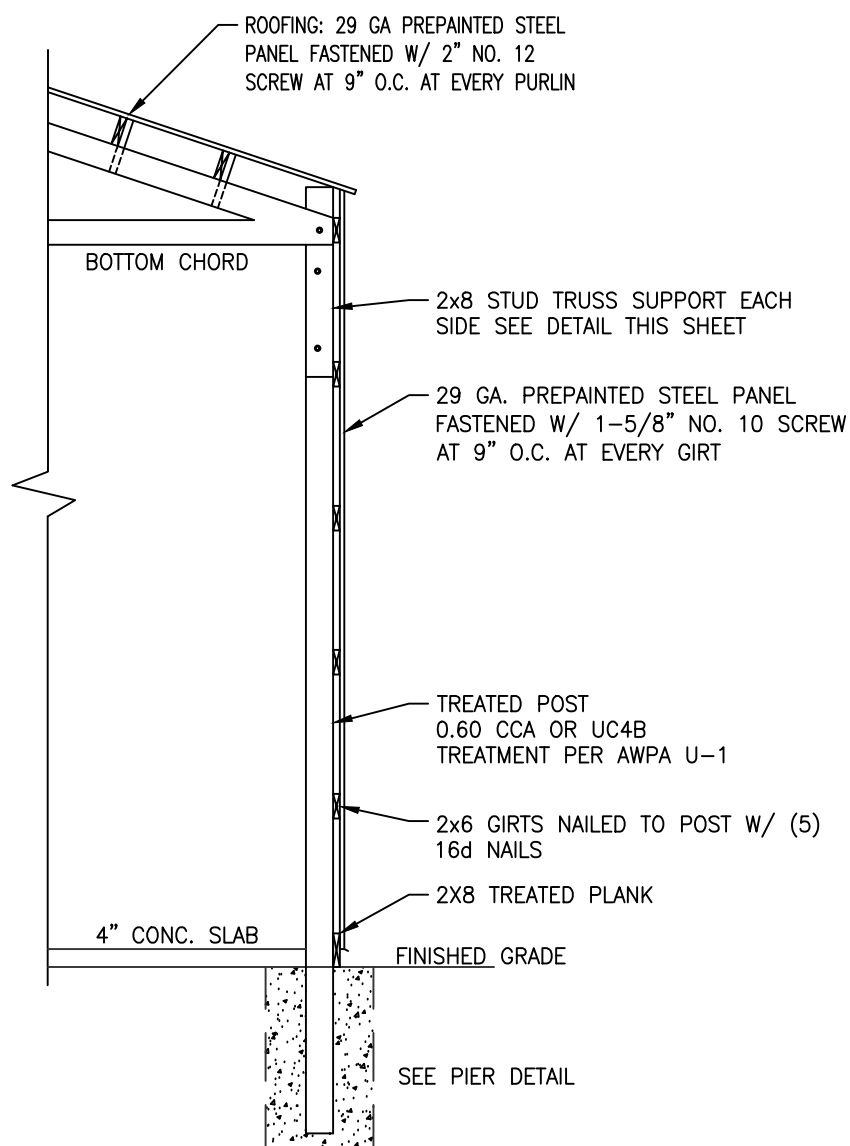
OVERHEAD DOOR HEADER
NOT TO SCALE



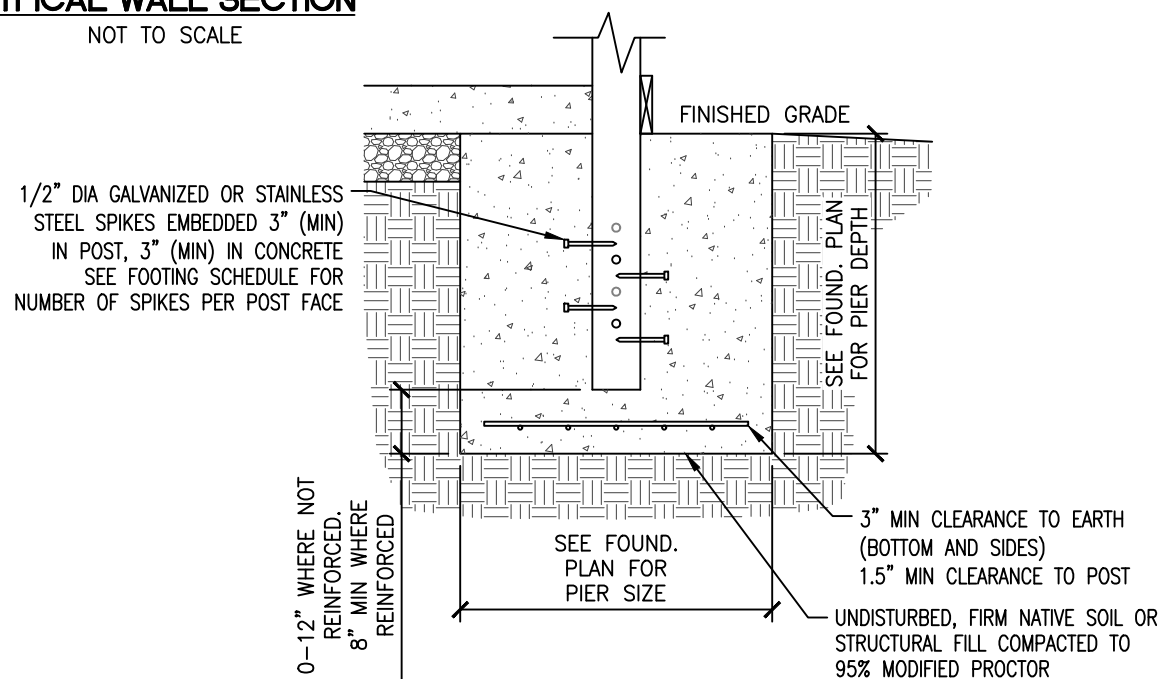
MAN DOOR OPENING
NOT TO SCALE



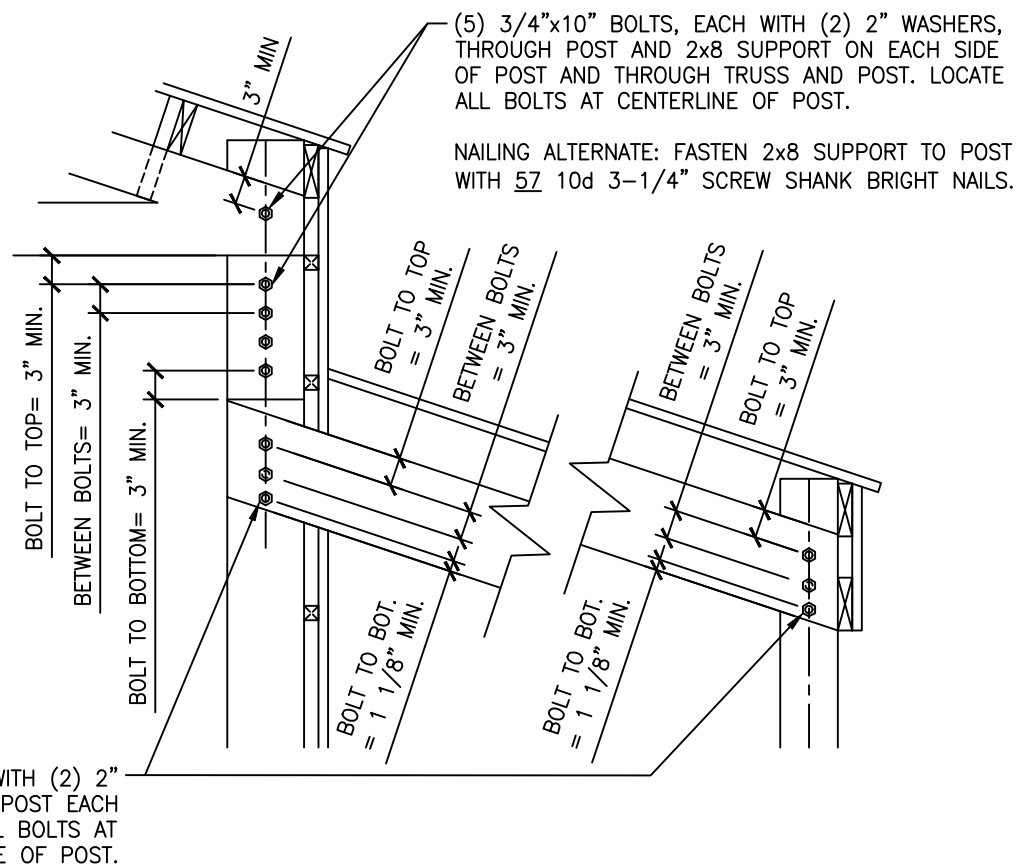
WINDOW OPENING
NOT TO SCALE



TYPICAL WALL SECTION
NOT TO SCALE

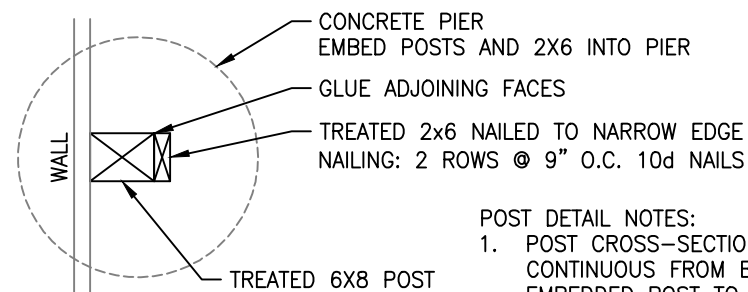


POST DETAIL
NOT TO SCALE

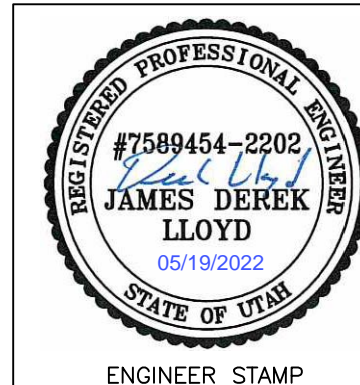


TRUSS SUPPORT AND LVL TO POST CONNECTION DETAIL

NOT TO SCALE



- POST DETAIL NOTES:
1. POST CROSS-SECTIONS SHOWN TO BE CONTINUOUS FROM BOTTOM OF EMBEDDED POST TO BOTTOM OF LOWEST BEAM/TRUSS BOTTOM CHORD.
 2. SPLICES NOT PERMITTED IN 6X8 POSTS.
 3. 2x6s AND 2x8s MAY BE SPLICED 6 FEET ABOVE TOP OF SLAB. SPLICE NAILING: 2 ROWS AT 4.5\"/>



MOUNTAIN POINT
ENGINEERING

DETAIL SHEET

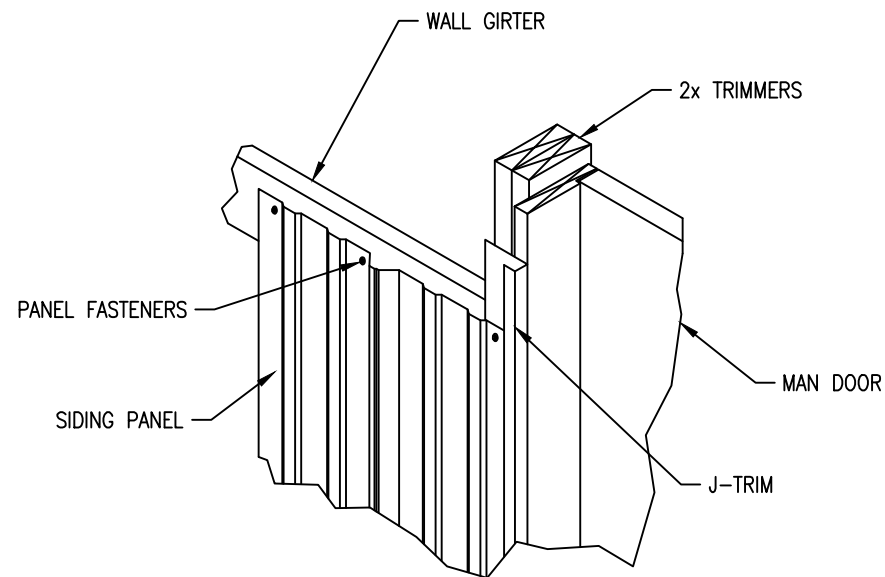
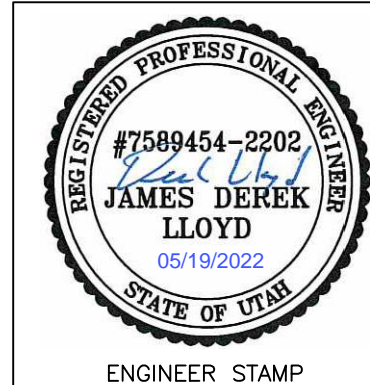
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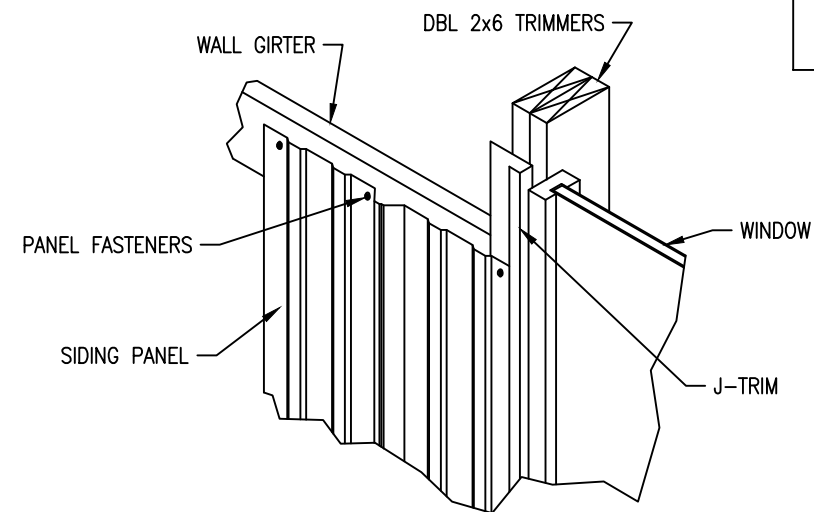
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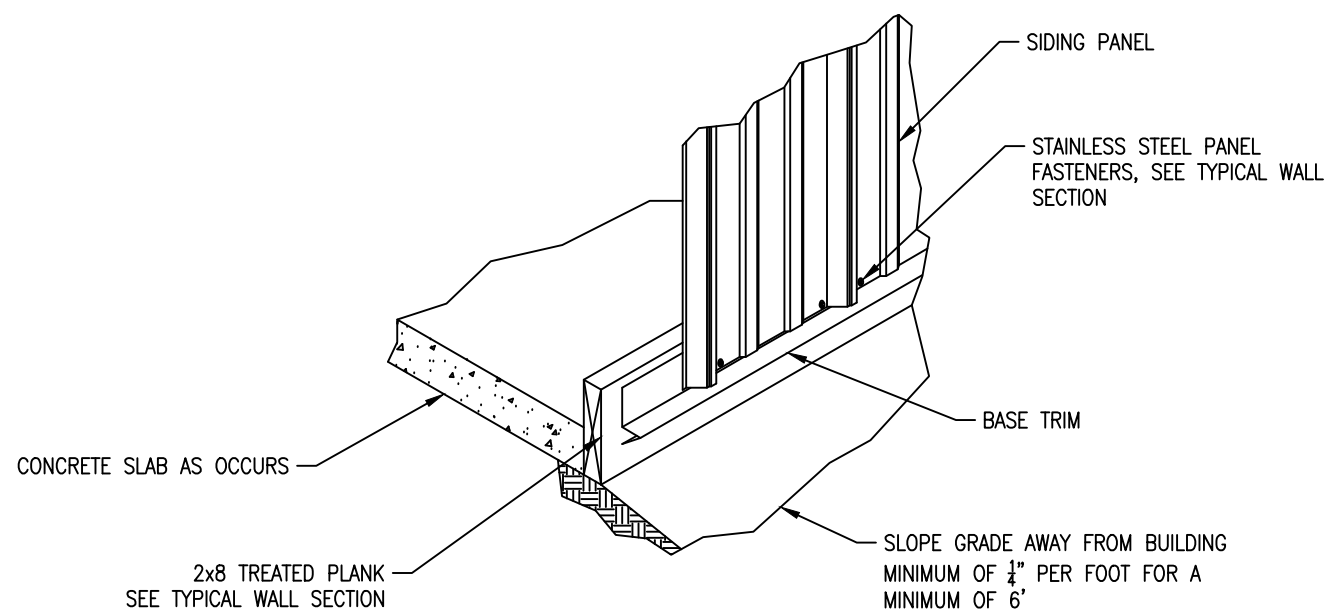
SHEET
08



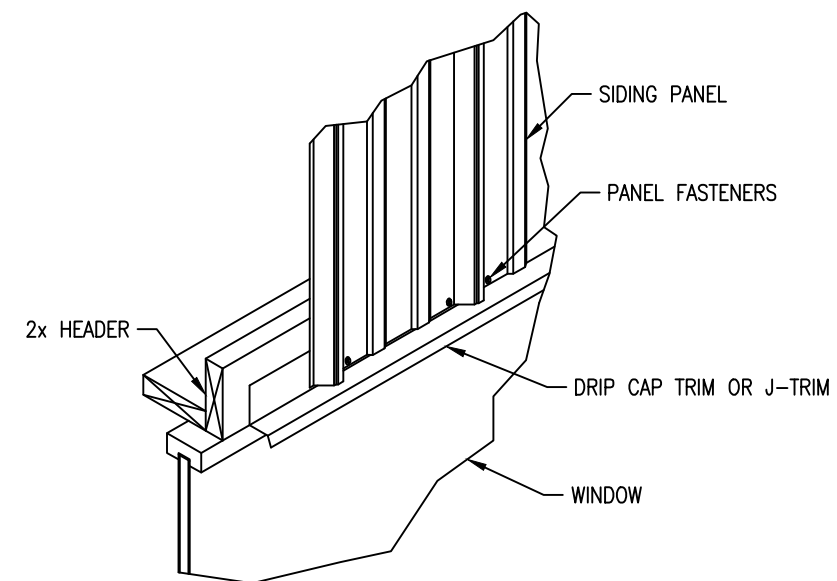
DOOR JAMB FLASHING
NOT TO SCALE



WINDOW JAMB FLASHING
NOT TO SCALE



BASE GUARD FLASHING
NOT TO SCALE



WINDOW / DOOR HEADER FLASHING
NOT TO SCALE