

- 1. 06/22/20 CK COMPLETED DESIGN FOR CLIENT & CITY REVIEW.
- 2. 08/12/20 CK UPDATED PER REVIEW COMMENTS DATED 07/09/20.
- 3. 08/27/20 CK UPDATED PER REVIEW COMMENTS DATED 08/17/20. 4. 09/29/20 CK - MOVED LIFT STATION 20' NORTH PER EMAIL DATED 09/24/20.
- 5. 02/03/21 CK REVISED LIFT STATION FROM REGIONAL TO LOCAL & RELATIVE
- SIZING & CAPACITY WAS REDUCED.
- 6. 04/13/21 KH CHANGED TO CHOPPER PUMP, REMOVED GRINDER.





Taylor Landing Local Lift Station Improvement Plans

JUNE 2020

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Sheet	7 - Plan & Details



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Engineer's Notice To Contractors THE EXISTENCE AND LOCATION OF ANY UNDERGROUND UTILITY PIPES OR STRUCTURES SHOWN ON THESE PLANS WERE OBTAINED FROM AVAILABLE INFORMATION PROVIDED BY OTHERS. THE LOCATIONS SHOWN ARE APPROXIMATE AND SHALL BE CONFIRMED IN THE FIELD BY THE CONTRACTOR, SO THAT ANY NECESSARY ADJUSTMENT CAN BE MADE IN ALIGNMENT AND/OR GRADE OF THE PROPOSED IMPROVEMENT. THE CONTRACTOR IS REQUIRED TO CONTACT THE UTILITY COMPANIES AND TAKE DUE PRECAUTIONARY MEASURE TO PROTECT ANY UTILITY LINES SHOWN, AND ANY OTHER LINES OBTAINED BY THE CONTRACTOR'S RESEARCH, AND OTHERS NOT OF RECORD OR NOT SHOWN ON THESE PLANS. (Project Contact: **(Developer Contact:** Project Manager: JEREMY DRAPER Visionary Homes 50 East 2500 North Project Engineer: KEN HUNTER North Logan, UT 84341 PH: (435) 265-3853

VERIFY ROADWAY DESIGN ELEVATIONS PRIOR TO INSTALLATION OF LIFT STATION. CONTACT ENGINEER OF RECORD IF ISSUES ARISE.











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					(A)
					DC. DS ORS E ARCHITECTS
LEGEND PRECAST CONCRETE WET WELL, HS-2 WALL THICKNESS. COAT EX HIGH PRECAST CONCRETE VALVE VAUL HS-20 RATED. COAT EXTERIOR V ALUMINUM DOUBLE LEAF ACCESS DOC SAFETY GRATE – OPENING DIMENSION PUMP MANUFACTURERS REQUIREMENTS WATER-TIGHT. PROVIDE RECESSED, LC SURFACE. INSTALL DOOR SUCH THAT	NOTES O RATED WITH MONOLITHIC XTERIOR WITH WATERPROOF T WITH PLASTIC COATED S WITH WATERPROOF TREATME OR WITH STAINLESS STEEL S AND DOOR LOCATION SH S. HATCH SHALL BE HS-20 OCKABLE HASP COVERED WI ENTRY SYSTEM IS NOT IN	BASE. DESIGN PER GEOTECH. 14 TREATMENT. TEPS. 12" WALL THICKNESS. NT. HARDWARE AND ORANGE ALL BE IN ACCORDANCE WITH TRAFFIC RATED AND ITH HINGED LID FLUSH WITH CONFLICT WITH DOOR.	-" MIN	Ceed	ASSOCIALES, I 5 SOUTH 1500 WEST, RIVERDALE, UTAH 8440 EL: (801) 621-3100 WWW.FEGVE-95SOC.COM PLANNERS * CML ENCINERS * LAND SURVEY ELERS * STRUCTURAL ENCINEERS * LANDSCOPI
HS-20 TRAFFIC RATED AND WATER-TI WITH HINGED LID FLUSH WITH SURFAC CONFLICT WITH DOOR. PLUMB HATCH	GHT. PROVIDE RECESSED, CE. INSTALL DOOR SUCH TH RIM DRAIN TO VAULT FLOC	LOCKABLE HASP COVERED HAT ENTRY SYSTEM IS NOT IN DR DRAIN.			
6"Ø SCHEDULE 40 WELDED STEEL VE STAINLESS STEEL. WELDED STAINLESS CLEARANCE FROM ROOF OF VAULT. USE "LINK-SEAL" OR APPROVED EQU, MANHOLE JOINT WITH EXTRUDED BUTY OUT OR VULLCEM 16 JOINT SEALANT APPLY SPECTRASHIELD LINER OR APP ARE SEALED AND PIPES ARE INSTALLE	NT. HOT DIP GALVANIZE AF STEEL SCREEN WITH 1/4" AL FOR HIGHER HYDROSTAT (L RUBBER SEAL OR EQUIV CAULKING, TYP. ROVED EQUAL TO INTERIOR ED.	TER FABRICATION OR USE OPENINGS. MIN. 12" IC GROUNDWATER PRESSURES. ALENT. GROUT JOINT INSIDE AND R OF WET WELL AFTER ALL JOINTS		2	5
CRUSHED AGGREGATE (3/4" MINUS) (UNLESS INDICATED OTHERWISE IN GEC	COMPACTED TO 95% ASTM DTECH REPORT.	D-698 OR MODIFIED PROCTOR			
C-900 PIPE STAINLESS STEEL CABLE SUPPORT BR IS SHOWN SCHEMATICALLY IN THESE THE CABLES AND FLOATS ARE EASILY AVOID CONFLICTS WITH PUMP REMOVA	ACKET FOR POWER CABLES DRAWINGS. THE SUPPORT E ACCESSIBLE FROM THE AC L AND TO OPTIMIZE FLOAT	S AND FLOAT SWITCHES, BRACKET BRACKET NEEDS TO BE LOCATED S CCESS HATCH. FIELD ADJUST TO SWITCH PERFORMANCE.	0 [NOL	
STAINLESS STEEL PUMP REMOVAL SYS SUPPORT BRACES. STAINLESS STEEL LIFTING CHAIN OR C CLEVIS FITTING AT EACH END. 5.0HP SITHE SCDF PUMP MODEL 4SC	TEM, COMPLETE WITH MOU CABLE (MIN. STRENGTH 6,00 DF50N4 4" DISCHARGE	NTING BRACKETS AND INTERMEDIAT	E	DESCRIPT	
4" PUMP QUICK DISCONNECT DISCHAF STEEL MOUNTING BOLTS, VERIFY SIZE	RGE ELBOW AND MOUNTING WITH PUMP MANUFACTURE	BASE WITH EPOXY-SET STAINLESS R. PLACE ON 1-1/2" LEVELING G	S ROUT.	DATE	
DUAL 6" FLEXIBLE SLEEVE-TYPE PIPE					
PROVIDE SUFFICIENT LENGTH FOR WIR 6"Ø CAST IRON FLOOR DRAIN, P-TRAI REFER TO GEOTECHNICAL REPORT 6" ALUMINUM PIPE TO PRESSURE CAF STANDARD IMPELLER SIZES Pump HP Impeller Dia mm (in.)	P. C.	JND SURFACE. _ SLOPED AT 2% MIN.	Reeve & Associates, Inc Solutions Y	COUNTY, UTAH	Station
7.5 190 (7.48) 5.0 180 (7.09) 3.0 160 (6.30)	Sewer De	Pump Station esign Data		ing weber	ift
	156 SINGLE FAMILY LOTS AVERAGE FLOW IN = 32. PEAK FLOW IN (2.0 PEAK NO EMERGENCY STORAGE PROVIDED) WET WELL VAULT = 8' D LID/RIM ELEVATION = 39 INVERT IN = 20.50' DEEF ALARM FLOAT = 19.50' D DUAL PUMP FLOAT = 20 SINGLE PUMP FLOAT = 20 SINGLE PUMP FLOAT = 20 OFF FLOAT = 22.50' DEE BOTTOM = 23.50' DEEP ELEVATION HEAD = 17 F	© 300 GAL/PER LOT 5 GPM KING FACTOR) = 65 GMP PROVIDED (EMERGENCY GENERATO MAMETER .00' EL P DEEP .50' DEEP 21.50' DEEP EP T	R	Taylor Landi	
	ESTIMATED TDH = 30 FT SINGLE PUMP FLOW @ 3 EORCE MAIN VELOCITY @	0 TDH = 270 GPM	/s	PROFES	SION
200 <u>850</u> 300 <u>850</u> 400 450	VOLUME TO PUMP 1 ON VOLUME TO PUMP 1 &	= 375 GAL 2 ON = 375 GAI		5338	480
MODEL 4SCDF50N4	SINGLE PUMP CYCLE TIME	$E = \sim 11.5$ MINUTES @ AVG FLOW $E = \sim 5.8$ MINUTES @ PEAK FLOW		JEREMY A. 04/13/	DRAPER
FOR CORRECT QUANTITIES AND PIPE S (BOTH VERTICAL AND HORIZONTAL). MENSIONED DRAWING SHOWING ALL ORDINATE ALL WORK WITH RELATED	AVG FLOW TIME FROM SII PEAK FLOW TIME FROM S PUMP TYPE:	NGLE ON TO OFF = ~ 1.6 MIN. SINGLE ON TO OFF = ~ 2.0 MIN.			F UIB-
FOR FALLING LEVEL ARM OFF	(DESIGNED FOR 10 STAR BACK-UP GENERATOR: CATERPILLAR DG150-2 G	ENERATOR SET W/	- [[[Troject Inf Engineer: JEREMY A. D Drafter:	d. Raper, p.e.
ALTERNATE LEAD/LAG	8.8L, V8, 4-CYCLE ENGI	NE. NATURAL GAS FUELED.	E	Begin Date: JUNE	2020
INGS, SPECIFICATIONS, AND			1 -	Name: TAYLOR I	
QUIRED. ED OTHERWISE. DAMAGE BY		Blue Stakes Location Center		LUCAL LIF	016-09
/E BOX SHALL BE DUCTILE IRON CLASS RIOR AND EXTERIOR SURFACES.		1-800-662-4111		Sheet	7
SLABS.	TIONS MADE TO THESE PLANS OF	THE DESIGN THEREON WITHOUT THEIR CO		5	Sheets

- GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL OR COLUMN AT FACH FND. MASONRY LINTELS ML-1 THRU ML-4 SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. WHEN A LINTEL IS SPECIFIED ON THE PLANS, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SPECIFIED ON THE PLANS WHICH HAVE A SPAN GREATER THAN 10'-0". MASONRY LINTELS ML-1 THRU ML-4 SHALL NOT BE LOCATED DIRECTLY BELOW
- FLOOR OR ROOF BEAMS OR GIRDERS UNLESS NOTED OTHERWISE ON THE PLANS. JOISTS SHALL NOT BEAR ON ANY LINTEL LESS THAN 16" DEEP. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SHOWN ON THE PLANS WHICH ARE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR GIRDFRS. EXTEND ALL HORIZONTAL REINFORCING BEYOND THE EDGE OF ALL OPENINGS
- IF HORIZONTAL REINFORCING CANNOT EXTEND LAP SPLICE LENGTH BEYOND EDGE OF OPENING, PROVIDE 90° STANDARD HOOK. SPLICE TOP BARS AT MIDSPAN OF LINTEL ONLY AND BOTTOM BARS OVER SUPPORTS ONLY.
- HORIZONTAL WALL REINFORCING SHALL CONTINUE THRU MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING. DOWEL VERTICAL REINFORCING OF WALL ABOVE LINTEL INTO THE FULL DEPTH
- OF LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

	FOOTING SCHEDULE								
MARK	WIDTH	LENGTH	THICK	LENGTHWISE REINF.		ISE CROSSWISE REINF.		REINF.	
				NO.	SIZE	NO.	SIZE	SPACING	NOTES
FC-20	20"	CONT.	10"	2	#4				REBAR CONTINUOUS
FC-24	24"	CONT.	10"	3	#4				REBAR CONTINUOUS
FC-30	30"	CONT.	12"	3	#5		#5	12" OC	REBAR CONTINUOUS
FC-36	36"	CONT.	12"	4	#4		#5	12" OC	REBAR CONTINUOUS
FC-48	48"	CONT.	12"	5	#5		#5	12" OC	REBAR CONTINUOUS
FC-54	54"	CONT.	12"	5	#5		#5	12" OC	REBAR CONTINUOUS
FT-18	18"	CONT.	10"	2	#4				THICKENED SLAB, REBAR CONTINUOUS
FT-24	24"	CONT.	10"	3	#4				THICKENED SLAB, REBAR CONTINUOUS
F-24	24"	24"	10"	3	#4	3	#4	EQ.	
F-30	30"	30"	10"	3	#4	3	#4	EQ.	
F-36	36"	36"	10"	4	#4	4	#4	EQ.	
F - 42	42"	42"	12"	4	#5	4	#5	EQ.	
F-48	48"	48"	12"	5	#5	5	#5	EQ.	
F-54	54"	54"	12"	5	#5	5	#5	EQ.	
F-60	60"	60"	12"	6	#5	6	#5	EQ.	
F-66	66"	66"	12"	6	#5	6	#5	EQ.	
F-72	72"	72"	12"	7	#5	7	#5	EQ.	
TYPICAL FOOTING SECTION 3" CLEAR									
	TYPICAL FOOTING REINF								

MASON	RY NOTES:
1. MAT	TERIALS, UNLESS NOTED OTHE
1.1.	CONCRETE MASONRY UNITS
1.2.	HOLLOW CLAY UNITS: HOLLO
1.3.	3,000 PSI) F'M = 1800 PSI SOLID CLAY UNITS: GRADE S
1 /	1800 PSI MORTAR: TYPE "S" (1800 PSI)
1.4. 1.5.	GROUT SHALL ATTAIN A MINI
16	28 DAYS. REINFORCING STEEL
1.7	DEFORMED BAR ANCHORS (D
1.8.	HEADED STUD ANCHORS (HS
1.9	HEAVY HEX NUTS AND HARDE
2. REI	NFORCEMENT SHALL HAVE TH
2.1.	COVERAGE FROM THE EXPOS
2.2.	OTHER REINFORCEMENT SHA
	EXPOSED TO SOIL, MIN. COVE
3. CON	
3.1.	ALL HEAD JOINTS SHALL BE F
	IN FROM THE FACE OF THE U
	LONGITUDINAL FACE SHELLS
3.2.	MASONRY WALLS, BEAMS AN
33	RUNNING BOND U.N.O.
5.5.	SPECIFICALLY NOTED.
3.4.	ALL CELLS CONTAINING REIN
	MECHANICAL VIBRATION DUR
	MOISTURE HAS BEEN ABSOR
3.5	PUDDLING OR RODDING OF G
	TERMINATE FLUSH WITH THE
	CELLS WITH VERTICAL REINF
3.6.	GROUT POURS SHALL BE LIM
0.7	PROCEDURES ARE FOLLOWE
3.8.	VERTICAL CELLS TO BE FILLE
	ALIGNMENT SUFFICIENT TO M
	SHALL BE SECURED AGAINST
	POSITIONERS OR OTHER SUI
	200 BAR DIAMETERS OR 10'-0' VERTICAL REINFORCING SHA
	UNLESS NOTED OTHERWISE.
3.9.	REINFORCING BARS SHALL N
	SUBSTITUTE REINFORCING B
3.10.	CONTROL JOINTS: SPACING S
3.11.	GROUT ALL BEAM AND JOIST
3.12.	EMBED CHANNELS AND PLAT
2 1 2	SURFACE WITH THE FACE OF
3.13.	CELL. ANCHOR BOLTS AND HEADEL
	SURROUNDING THE SHANK A
4. DET	AILING REQUIREMENTS
4.1.	LAP ALL MASONRY REINFORC
	$\frac{\text{REQUIRED LAP LENGTHS FC}}{\#3 = 16"}$
	#4 = 22" #7 = 60"
	#5 = 26" #8 = 72" REQUIRED AP ENGTHS EC
	CELL WITH 2.5" COVER:
	#3 = 16" #6 = 54" #9 #4 = 22" #7 = 63"
	#5 = 32" #8 = 72"
4.1.	LAP ALL MASONRY REINFORC
4.2.	ALL VERTICAL REINFORCING
	FOOTING (STRUCTURE BELO)
	WALL REINFORCING ABOVE.
4.3.	CORNER BARS: HORIZONTAL
	THE REQUIRED LAP SPLICE L
4.4.	WALL OPENINGS 24" WIDE AN
	OPENINGS. PROVIDE HORIZO
	DETAILS. VERTICAL BARS SH
	SHALL EXTEND A MINIMUM O
	THE OPENING. WHERE A 48 E
	THE BAR(S) WITH A 90° STAN
4.5.	HORIZONTAL WALL REINFOR
	CONCRETE WALLS, MASONRY KEY BETWEEN THE WALL AND
	WALL REINFORCING SHALL B
46	REINFORCING. HORIZONTAL REINFORCING S
т. О.	EDGE OF OPENINGS AND END
Δ7	SHOWN IN DETAILS.
¬ ./.	HOOK AT EACH SIDE OF CON
٨٥	LEVELS, LINTELS, BEAMS AND
4.0.	BAR DIAMETER FXTFNSION (4

ML-4

ML-2

ML-1

ML

FOUNDATION WALL SCHEDULE							
MARK	MAX HEIGHT	WALL THICKNESS	VE F	RTICAL REINF.	HORIZONTAL REINF.		
			SIZE	SPACING	SIZE	SPACING	
FW-1	8'-0"	8"	#4	16" O.C.	#4	12" O.C.	
FW-2	9'-0"	8"	#4	15" O.C.	#4	12" O.C.	
FW-3	10'-0"	8"	#5	18" O.C.	#4	12" O.C.	
FW-4	12'-0"	10"	#5	12" O.C.	#5	15" O.C.	

GENERAL NOTES:

- FRWISE
- (CMU): MEDIUM WEIGHT (115PCF) (MINIMUM UNIT OW BRICK, GRADE I (MINIMUM UNIT STRENGTH OF
- SW (MINIMUM UNIT STRENGTH OF 3000 PSI) F'M =
- VINIMUM COMPRESSIVE STRENGTH) IMUM COMPRESSIVE STRENGTH OF 2000 PSI AT
- ----- ASTM 615 GRADE 60 (FY = 60 KSI) ----- ASTM A496
- A)----- ASTM A108
- ----- ASTM A307 ENED WASHERS------ ASTM A563
- IE FOLLOWING COVER: LL HAVE NOT LESS THAN 5/8" MORTAR
- SED FACE ALL HAVE A MIN. COVERAGE OF ONE BAR
- RS, BUT NOT LESS THAN 3/4". WHEN MASONRY IS ERAGE SHALL BE 1 1/2".
- TH FULL MORTAR BEDS ON THE FACE SHELLS. FILLED SOLIDLY WITH MORTAR FOR A DISTANCE INITS NOT LESS THAN THE THICKNESS OF THE . CELLS WHICH ARE TO BE GROUTED SHALL
- ID COLUMNS SHALL BE CONSTRUCTED WITH
- IS UNACCEPTABLE EXCEPT WHERE
- VFORCEMENT, EMBEDS, ANCHOR BOLTS, ETC. I GROUT. GROUT SHALL BE PLACED BY RING PLACING AND REVIBRATED AFTER EXCESS RED BUT BEFORE WORKABILITY IS LOST. GROUT IS NOT ALLOWED.
- OUTED SOLID, EACH GROUT POUR SHALL TOP OF THE UPPERMOST UNIT EXCEPT AT FORCING WHERE THE GROUT SHALL BE 1 1/2" VIDE CONSTRUCTION KEY.
- AITED TO 4'-0" UNLESS HIGH LIFT GROUTING E SHALL BE SOLID GROUTED.
- ED WITH GROUT SHALL HAVE VERTICAL /IAINTAIN A CLEAR, UNOBSTRUCTED, VERTICAL THAN 2" BY 3". ALL STEEL REINFORCEMENT DISPLACEMENT PRIOR TO GROUTING BY WIRE ITABLE DEVICES AT INTERVALS NOT EXCEEDING " MAXIMUM, OR AT BAR SPLICE LOCATIONS.
- ALL BE LOCATED AT THE CENTER OF THE WALL NOT BE WELDED UNLESS SPECIFICALLY SHOWN SES, USE ONLY AWS STANDARDS. DO NOT
- BARS FOR DBAS OR HSAS. SHALL NOT EXCEED 26'-0". SEE ARCHITECTURAL
- POCKETS SOLID AFTER INSTALLATION OF
- TES SHALL BE PLACED SO AS TO CREATE A FLUSH THE WALL. D STUD ANCHORS SHALL BE SET IN A GROUTED HEADED STUD ANCHORS SHALL HAVE 1/2" GROUT AT ITS PENETRATION. GROUT SHALL BE FLUSH THE MASONRY.
- CING PER BAR SIZE AS FOLLOWS: OR SINGLE BARS CENTERED IN EACH CELL:
- OR FLUSH WALL PILASTER/COLUMN, 2 BARS PER 9 = 82"
- CING PER THE "MASONRY REINFORCING BAR LAP **JED WITHIN THE CONTRACT DRAWINGS.** SHALL BE DOWELED TO THE FOUNDATION WALL, W) AND TO THE STRUCTURE BELOW WITH THE G (AND IN THE SAME CORE) AS THE VERTICAL
- L REINFORCEMENT SHALL BE CONTINUOUS AT RSECTING WALLS. PROVIDE CORNER BARS WITH
- FNGTH ND WIDER: FOR UNSCHEDULED OPENINGS,
- LL SIDES PER DETAILS. ALSO, FOR ALL ONTAL BAR AT BOTTOM OF OPENING PER HALL EXTEND FROM FLOOR LEVEL BELOW TO THE OVE. HORIZONTAL BARS FOR ALL OPENINGS F 48 BAR DIAMETERS BEYOND THE CORNERS OF BAR DIAMETER EXTENSION IS NOT POSSIBLE,
- ND THE OPENING AS POSSIBLE AND TERMINATE IDARD ACLHOOK CING SHALL BE CONTINUOUS THROUGH JOINING Y WALLS, COLUMNS, AND PILASTERS. PROVIDE A D THE COLUMN OR PILASTER. HORIZONTAL
- BE PLACED INSIDE THE COLUMN VERTICAL SHALL TERMINATE WITH A STANDARD HOOK AT
- DS OF WALLS WITHOUT CORNER BARS AS
- CING SHALL TERMINATE WITH A STANDARD 180° ITROL JOINTS EXCEPT AT FLOOR AND ROOF D AT TOP OF PARAPETSAS SHOWN IN DETAILS. SHALL TERMINATE WITH 135° HOOKS PLUS A 6 4" MIN).

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- 1. VISITS TO THE JOB SITE BY REPRESENTATIVES OF THE ENGINEER DO NOT SUBSTITUTE APPROVAL OF THE WORK PERFORMED BY THE CONTRACTOR OR HIS SUBCONTRACTORS AND ARE MERELY FOR THE PURPOSE OF OBSERVING THE WORK PERFORMED.
- CONTRACTOR SHALL NOTIFY ENGINEER/ARCHITECT OF ANY DISCREPANCIES, OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS BEFORE PROCEEDING WITH ANY WORK INVOLVED. IN ALL CASES, UNLESS OTHERWISE DIRECTED, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN AND BE PERFORMED.
- CONTRACTOR SHALL VERIFY ALL CONDITIONS, DIMENSIONS AND ELEVATIONS, ETC., AT THE SITE AND SHALL COORDINATE WORK PERFORMED BY ALL TRADES. SEE ARCHITECT'S PLANS FOR DIMENSIONS. DO NOT SCALE DRAWINGS SHOP DRAWINGS SHALL BE REVIEWED BY THE ENGINEER/ARCHITECT PRIOR TO FABRICATION OR ERECTION FOR ANY PREFABRICATED OR
- MANUFACTURER-DESIGNED COMPONENTS AND SHALL BE STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THIS STRUCTURE RESIDES. SIZES, LOCATIONS, LOADS, AND ANCHORAGES OF EQUIPMENT SHALL BE VERIFIED
- IN THE FIELD WITH EQUIPMENT MANUFACTURERS (SUPPLIERS) PRIOR TO FABRICATION OR INSTALLATION OF SUPPORTING STRUCTURES. 6. TEMPORARY BRACING SHALL BE PROVIDED WHEREVER NECESSARY TO TAKE
- CARE OF ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED, INCLUDING WIND. SUCH BRACING SHALL BE LEFT IN PLACE AS LONG AS MAY BE REQUIRED FOR SAFETY, OR UNTIL ALL THE STRUCTURAL ELEMENTS ARE INSTALLED. DURING AND AFTER CONSTRUCTION THE CONTRACTOR AND/OR OWNER SHALL
- KEEP LOADS ON THE STRUCTURE WITHIN THE LIMITS OF THE DESIGN LOAD. CONTRACTOR AND ALL SUBCONTRACTORS SHALL PERFORM THEIR TRADES AND DUTIES IN A MANNER CONFORMING TO THE PROCEDURES AND REQUIREMENTS AS STATED IN THE 2015 INTERNATIONAL BUILDING CODE, (OR LATEST ACCEPTED CODE ADOPTED BY THE LOCAL BUILDING OFFICIALS).
- ANY SPECIAL INSPECTIONS REQUIRED BY THE BUILDING OFFICIAL OR THE INTERNATIONAL BUILDING CODE ARE THE RESPONSIBILITY OF THE OWNER. 10. CONTRACTOR SHALL BE RESPONSIBLE FOR SAFETY AND PROTECTION WITHIN
- AND ADJACENT TO THE JOB SITE.
- FOOTINGS, FOUNDATIONS AND SLAB ON GRADE NOTES:
- 1. ALL FOOTING SIZES ARE BASED ON AN ALLOWABLE SOIL BEARING PRESSURE AS SHOWN IN THE DESIGN CRITERIA. ANY SOIL CONDITION ENCOUNTERED DURING EXCAVATION THAT IS CONTRARY TO THOSE USED FOR DESIGN OF FOOTINGS AS OUTLINED IN WORKING DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING.
- SOIL PREPARATION UNDER FOOTINGS AND SLABS ON GRADE SHALL BE IN ACCORDANCE WITH THE SOILS REPORT. FOR PROJECTS WITHOUT A SOILS REPORT CONTRACTOR/OWNER IS TO VERIFY ADEQUATE SOIL CONDITIONS ARE PROVIDED.
- ALL FOOTINGS SHALL BEAR ON UNDISTURBED NATIVE SOIL OR ENGINEERED GRANULAR FILL COMPACTED TO 95% OF MAX. DENSITY, BASED ON ASTM D 1557 METHOD OF COMPACTION. FILL SHALL BE PLACED IN LAYERS NOT TO EXCEED SIX INCHES IN DEPTH AFTER COMPACTION AND SHALL EXTEND DOWN TO IN-SITU SOILS. FILL SHALL BE COMPACTED UNDER ALL CONCRETE WORK ON THE SITE. NO FOOTINGS SHALL BE PLACED IN WATER, SNOW, FROZEN GROUND, OR UNSTABLE SOILS.
- ALL EXCAVATIONS ADJACENT TO AND BELOW FOOTING ELEVATION FOR OTHER TRADES SHALL BE ACCOMPLISHED PRIOR TO POURING ANY FOOTINGS. CONTRACTOR SHALL BE RESPONSIBLE FOR LATERALLY SUPPORTING ALL
- RETAINING TYPE FOUNDATION WALLS WHILE COMPACTING BEHIND WALLS AND UNTIL ALL SUPPORTING MEMBERS HAVE BEEN PLACED (SUCH AS FLOOR). ALL REINFORCEMENTS SHALL BE SECURELY TIED IN PLACE PRIOR TO POURING CONCRETE.
- PROVIDE DOWELS IN FOOTING AND FOUNDATIONS TO MATCH ALL VERTICAL BARS IN WALLS AND COLUMNS ABOVE, UNLESS NOTED OTHERWISE. PROVIDE CONTROL JOINTS IN SLABS AT A MAX. OF 15 FT. O.C. EACH WAY AND AS
- SHOWN ON PLANS. AT EXTERIOR SLABS AND GARAGE FLOORS POUR SLABS BETWEEN CONTROL JOINTS SO THAT ADJACENT POURS ARE STAGGERED AT LEAST TWO DAYS APART. 10. ALL EXTERIOR FOOTINGS MUST BEAR AT OR BELOW FROST DEPTH, MEASURED
- FROM LOWEST ADJACENT FINAL GRADE. 11. UNLESS NOTED OTHERWISE, ALL FOOTINGS AT COLUMNS TO BE CENTERED
- BELOW COLUMNS. 12. UNLESS NOTED OTHERWISE, ALL FOOTINGS SHALL HAVE VERTICAL FACES FORMED WITH STANDARD FORMING MATERIALS (WOOD, METAL, ETC.). WITH PRIOR APPROVAL OF ARCHITECT AND ENGINEER, CONCRETE FOR FOOTINGS CAN BE PLACED IN EXCAVATED "SOIL" FORMS PROVIDED THAT THE DIMENSIONS ARE INCREASED 3" ON EACH SIDE.
- 13. SLABS ON GRADE SHALL BE 4 INCHES THICK CONCRETE UNDERLAIN BY FREE DRAINING MATERIAL.
- CONCRETE NOTES:
- ALL COLUMNS, RETAINING WALLS AND ALL EXTERIOR FLATWORK, CURBS, GUTTERS, ETC., SHALL BE NORMAL WEIGHT CONCRETE WITH A COMPRESSIVE STRENGTH EQUAL TO AT LEAST 4,000 LBS. PER SQUARE INCH WITHIN 28 DAYS AFTER POURING.
- ALL SUSPENDED SLABS AND BEAMS SHALL BE NORMAL WEIGHT CONCRETE WITH A COMPRESSIVE STRENGTH EQUAL TO AT LEAST 5,000 LBS. PER SQUARE INCH WITHIN 28 DAYS AFTER POURING
- ALL FOOTINGS, FOUNDATIONS, INTERIOR SLABS ON GRADE, AND SUSPENDED SLABS ON DECK SHALL BE NORMAL WEIGHT CONCRETE WITH A COMPRESSIVE STRENGTH EQUAL TO A LEAST 3,000 LBS. PER SQUARE INCH WITHIN 28 DAYS AFTER POURING.
- UNLESS OTHERWISE NOTED, ALL FOUNDATION WALL VERTICAL COLD JOINTS SHALL BE KEYED WITH A KEY 1-1/2" DEEP, A LENGTH 2" LESS THAN THE MEMBER, AND A WIDTH 1/2 OF THE MEMBER. REINFORCING SHALL BE CONTINUOUS THRU JOINT
- ALL OPENINGS IN CONCRETE WALLS SHALL BE REINFORCED WITH (2) #5 BARS 5. EXTENDING 2'-0" MIN. BEYOND THE EDGE OF THE OPENING AT EACH FACE OF OPENING.
- 6. ALL CONCRETE WORK SHALL BE PLACED, CURED, STRIPPED, AND PROTECTED AS DIRECTED BY THE SPECIFICATIONS AND ACI STANDARDS AND PRACTICES. BEFORE CONCRETE IS POURED CHECK WITH ALL TRADES TO INSURE PROPER
- PLACEMENT OF ALL OPENINGS, SLEEVES, CURBS, CONDUITS, BOLTS, INSERTS, ETC. RELATIVE TO WORK. CONTRACTOR IS RESPONSIBLE FOR ALL SHORING AND FORMWORK. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENT, CLIPS
- OR GROUNDS, REQUIRED TO BE ENCASED IN CONCRETE AND FLOOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS. 10. FOR STEPS IN FOUNDATION GREATER THAN 2 FEET, WRAP CORNER W/(2) #4 BARS
- EXTENDING 18" EACH DIRECTION. 11. STRUCTURAL CONCRETE HAS BEEN DESIGNED AT 2,500 LBS, PER SQUARE INCH AND SPECIFIED AT A HIGHER STRENGTH CONCRETE AS STATED ABOVE. NO SPECIAL INSPECTIONS ARE REQUIRED PER IBC SECTION 1705.3.

ROOF TRUSS NOTES:

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- ROOF IS TO BE CONSTRUCTED OF A PRE-MANUFACTURED TRUSS SYSTEM DESIGNED BY TRUSS MANUFACTURER.
- DESIGN TRUSSES TO LIMIT DEFLECTION TO SPAN (IN.) DIVIDED BY 240. CHECK DIMENSIONS WITH ARCH. DRAWINGS. TRUSS MANUFACTURER IS RESPONSIBLE TO PROVIDE WEB AND CHORD MEMBERS TO SATISFY LOAD REQUIREMENTS.
- 4. SEE ARCHITECTURAL DRAWINGS FOR VAULTS, TRAY CEILINGS, CEILING HEIGHTS, ETC.
- GIRDER TO GIRDER CONNECTIONS PER TRUSS MANUFACTURER. TRUSS LAYOUT SHALL FOLLOW THE STRUCTURAL PLANS, OR TRUSS SHOP DRAWINGS NEED TO BE SUBMITTED TO REEVE AND ASSOCIATES FOR REVIEW.

LUMBER NOTES:

- 1. MEMBER GRADES SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED: GLU-LAM BEAMS . 24F-V4 DF/DF DOUGLAS-FIR/LARCH #2 JOISTS . HEADERS. DOUGLAS-FIR/LARCH #2 COLUMNS DOUGLAS-FIR/LARCH #2 STUDS NONBEARING WALLS . . DOUGLAS-FIR/LARCH #2 AS PER MANUFACTURER PRE-FAB JOISTS . SILL PLATES IN CONTACT WITH CONCRETE DOUGLAS-FIR/LARCH #2 TREATED FOR MOISTURE PROTECTION WHERE NOT NOTED OTHERWISE, CONNECT ALL WOOD TO CONCRETE, WOOD TO STEEL AND WOOD TO WOOD (EXCEPT STUD TO PLATE) WITH SIMPSON STRONG-TIE OR EQUAL STRUCTURAL CONNECTORS. ANY OTHER SUBSTITUTION MUST BE APPROVED BY THE ENGINEER.
- 3. WHERE MULTIPLE SILL PLATES ARE USED, ANCHOR BOLTS SHALL EXTEND THROUGH ALL SILL PLATES. BLOCK ALL HORIZONTAL EDGES OF PLYWOOD WALL SHEATHING WITH 2" NOMINAL
- BLOCKING. BLOCK EDGES OF PLYWOOD ON FLOORS AND ROOF AS DIRECTED ON DRAWINGS
- SOLID 2" NOMINAL BLOCKING SHALL BE PROVIDED AT ENDS OR POINTS OF SUPPORT OF ALL WOOD JOISTS. CROSS BRIDGING OF NOT LESS THAN 1"x3" MATERIAL SHALL BE PLACED IN ROWS BETWEEN SUPPORT POINTS NOT TO
- EXCEED 8'-0" APART, FOR SPANS OF 18'-0" AND GREATER. ALL LEDGER BOLTS SHALL HAVE PLATE WASHERS WITH A MIN. DIA. EQUAL TO 3 TIMES THE BOLT DIA. UNLESS SHOWN OTHERWISE IN DETAILS.
- MIN. NAILING SHALL BE AS PER SECTION 2304.10 OF THE INTERNATIONAL BUILDING CODE FASTENERS SUCH AS STAPLES, CAN ONLY BE SUBSTITUTED FOR NAILS AT A RATE EQUAL TO LOAD VALUES PROVIDED BY I.C.B.O. APPROVAL. SEE ATTACHED
- SCHEDULE. 9. JOISTS SHALL HAVE BRIDGING, BLOCKING AND NOTCHED BEARING PL AS
- RECOMMENDED BY THE MANUFACTURER WITH A MIN. OF ONE ROW OF BRACING AT MID SPAN MANUFACTURER SHALL SUPPLY AND CONTRACTOR SHALL INSTALL. 10. ALL PRE-MANUFACTURED WOOD PRODUCTS SHALL BE PROVIDED BY TRUSS
- JOIST, BOISE CASCADE CORP, OR LOUISIANA PACIFIC CORP. ANY OTHER SUBSTITUTION MUST BE APPROVED BY THE ENGINEER. 11. FASTENERS FOR PRESSURE PRESERVATIVE WOOD SHALL BE HOT-DIPPED,
- GALVANIZED STEEL OR STAINLESS STEEL.
- BEAM SIZES ARE BASED ON A MIN. STRENGTH REQUIREMENTS. SIZES MAY BE INCREASED FOR ARCHITECTURAL OR CONSTRUCTION PURPOSES. TYPICAL DOOR/WINDOW HEADERS TO BE (2) 2X8 UNLESS NOTED OTHERWISE
- 14. 2-PLY AND 3-PLY PRE-ENGINEERED WOOD BEAMS SHALL BE NAILED TOGETHER AS PER MANUFACTURER'S SPECIFICATIONS. 4-PLY AND GREATER PRE-ENGINEERED WOOD BEAMS SHALL BE ATTACHED W/ (2) ROWS 1/2"Ø THRU-BOLTS @ 12" o.c., SPACED 2" FROM TOP AND BOTTOM OF BEAM. SEE MANUFACTURES SPECIFICATIONS FOR ALL OTHER CONNECTION CONDITIONS. 15. SOLID BLOCKING OR SQUASH BLOCKS REQUIRED IN JOIST SPACE AT ALL COLUMN
- LOCATIONS. CARRY ALL COLUMN LOADS DOWN TO FTG. OR FDN. 16. ROOF SHEATHING SHALL BE 7/16" APA RATED SHEATHING W/SPAN RATING OF
- 24/16. LAY SHEATHING WITH FACE GRAIN AT RIGHT ANGLES TO FRAMING WITH END JOINTS STAGGERED.
- 17. FLOOR SHEATHING SHALL BE 3/4" T&G WAFER BOARD GLUED & NAILED. GLUE SHALL CONFORM TO AFG-01 ACCORDING TO APA SPECIFICATIONS. 18. WALL SHEATHING SHALL BE 7/16" APA RATED SHEATHING. SEE SHEAR WALL
- SCHEDULE FOR MORE INFORMATION. 19. UNLESS NOTED OTHERWISE, 8d NAILS SHALL BE USED TO FASTEN ALL ROOF AND WALL SHEATHING, AND 10d NAILS SHALL BE USED TO FASTEN ALL FLOOR
- SHEATHING TO SUPPORTING FRAMING AS FOLLOWS. A. BOUNDARY NAILING "BN": 4" O.C. AT ALL ROOF AND FLOOR SHEATHING INTO BEARING AND/OR SHEAR WALLS, TOP AND BOTTOM OF WALLS.
- PANEL EDGE NAILING "EN": 6" O.C. AT ALL OTHER PLYWOOD PANEL EDGES. PANEL FIELD NAILING "FN": 12" O.C. AT INTERIOR SUPPORTS IN FIELD OF PANEI 20. BLOCK JOISTS, RAFTERS AND/OR TRUSSES SOLID AT ALL BEARING POINTS.
- 21. PROVIDE (2) 2x STUD COLUMN AT ALL BEAMS, HEADERS, AND GIRDER TRUSS BEARING LOCATIONS TYPICAL UNLESS NOTED OTHERWISE. 22. ALL BOLTS THRU WOOD SHALL BE ASTM A307 AND SHALL HAVE HARDENED
- WASHERS UNDER ASTM A563 HEAVY HEX NUTS AND BOLT HEADS.
- 23. UNLESS NOTED OTHERWISE, ALL WALL BOTTOM PLATES TO BE ANCHORED TO FOUNDATIONS OR FOOTINGS WITH 5/8" DIAMETER ANCHOR BOLTS AT 32" O.C. WITH 8" MIN. EMBEDMENT. WALL BOTTOM PLATES AT SHEAR WALLS SHALL INCLUDE 3"x3"x1/4" STEEL PLATE WASHERS. PROVIDE A ROUND CUT WASHER
- BETWEEN THE NUT OF THE ANCHOR BOLT AND THE PLATE WASHER. 24. UNLESS OTHERWISE NOTED, ALL BEARING WALL STUDS SHALL BE 2X6 SPACED AT 16" O.C. BLOCK ALL NON-SHEATHED BEARING WALLS AT 4'-0" O.C.
- 25. EXTERIOR WALLS SHALL HAVE DOUBLE 2x TOP PLATES SPLICED WITH A MIN. OF 48" OF OVERLAP AND SHALL BE CONNECTED WITH A MIN. OF (12) 16d NAILS.

REINFORCING STEEL NOTES:

INTO FOOTING.

EPOXY

IS WELDED, USE ASTM A706 REINFORCING.

POWERS. OR APPROVED EQUAL.

RECOMMENDATIONS.

STANDING WATER.

- 1. ALL REINFORCING BARS SHALL CONFORM TO ASTM STANDARD A-615 GRADE 60. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM STANDARD A-185, SHALL BE SUPPLIED IN FLAT SHEETS AND SHALL HAVE A MIN. SIDE LAP OF 8 INCHES. ADEQUATELY TIE AND SUPPORT ALL REINFORCING STEEL AS SPECIFIED BY ACI 315 TO MAINTAIN EXACT REQUIRED POSITION. ALL FIELD BENT DOWELS SHALL BE
- GRADE 40 WITH SPACING INDICATED REDUCED BY 1/3. 2. REINFORCEMENT SHALL HAVE THE FOLLOWING CONCRETE COVERAGE: A. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH. . 3" B. EXPOSED TO EARTH OR WEATHER:
- C. NOT EXPOSED TO WEATHER OR EARTH: SLABS, WALLS, JOISTS, #11 & SMALLER3/4" BEAMS, COLUMNS: MAIN REINFORCING OR TIES . . 1 1/2"
- D. SLAB ON GRADE: PLACE REINFORCING AT CENTER OF SLAB UNLESS INDICATED OTHERWISE.
- EXCEPT WHERE NOTED, CONTINUOUS REINFORCEMENT SHALL BE SPLICED AT POINTS OF MIN. STRESS BY LAPPING 44 BAR DIAMETERS IN CONCRETE AND 50 BAR DIAMETERS IN MASONRY.

- 4. ALL VERTICAL REINFORCING SHALL BE DOWELED TO FOOTINGS OR STRUCTURE BELOW WITH DOWELS TO MATCH. SPLICE LENGTHS SHALL COMPLY WITH NOTE 3. DOWELS INTO FOOTINGS SHALL TERMINATE WITH A STANDARD HOOK, AND SHALL EXTEND TO WITHIN 4" OF THE BOTTOM OF THE FOOTING, BUT NOT MORE THAN 20"
- DO NOT WELD REINFORCING EXCEPT AS NOTED ON PLANS. WHERE REINFORCING
- 1. EPOXY IN CONCRETE SHALL BE "HIT RE 500 SD" BY HILTI CORPORATION, "EPCON INJECTION SYSTEM" BY RAMSET/REDHEAD, "POWER-FAST, STANDARD SET" BY
- ALL DRILLED HOLES SHALL BE SIZED PER THE MANUFACTURERS'
- AFTER DRILLING THE PROPER SIZE HOLE, CLEAN THE WALLS AND BOTTOM OF THE HOLE OF ALL DUST AND DEBRIS USING A NYLON BRUSH IN CONJUNCTION WITH OIL FREE COMPRESSED AIR. THE HOLE SHALL BE FREE OF DUST, DEBRIS AND
- 4. FOLLOW ALL MANUFACTURERS' RECOMMENDATIONS FOR EPOXY INSTALLATION.

DESIGN CRITERIA:					
GOVERNING CODE RISK CATEGORY	_ 2015 IBC _ II				
EARTHQUAKE IMPORTANCE FACTOR RESPONSE MODIFICATION COEFFICIENT SPECTRAL RESPONSE COEFFICIENTS	$\begin{array}{rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$				
SEISMIC DESIGN CATEGORY SOIL SITE CLASS BASIC SEISMIC-FORCE-RESISTING SYSTEM DESIGN BASE SHEAR ANALYSIS PROCEDURE	D D (ASSUMED) SPECIAL REINF. MASONRY V=C _S W QUIVALENT LATERAL FORCE PROCEDURE				
<u>WIND</u> BASIC WIND SPEED <u>(3 SECOND GUST)</u>	_ 115 MPH EXPOSURE C				
SOIL FROST DEPTH SOIL BEARING PRESSURE SOIL REPORT BY: REPORT # :	_ 30" MIN. _ 1500 PSF (ASSUMED)				
DATE: <u>ROOF</u> DEAD LOAD SNOW GROUND SNOW ROOF	_ 15 PSF _ 43 PSF _ 36 PSF				

LEGEND OF SYMBOLS AND ABBREVIATIONS							
AB.	=	ANCHOR BOLT					
ABV.	=	ABOVE					
ARCH.	=	ARCHITECT					
BN.	=	BOUNDARY NAILII	NG				
BLW.	=	BELOW					
CL.	=	CENTERLINE					
CMU.	=	CONCRETE MASC	DNRY UNIT				
COL.	=						
CONT.	_	CONTINUOUS					
DBA.	=	DEFORMED BAR	ANCHOR				
EN.	=	EDGE NAILING					
EQ.	=	EQUAL					
ELEV.	=	ELEVATION					
EW.	=	EACH WAY					
FDN.	=	FOUNDATION					
FN.	=	FIELD NAILING					
FIG.	=						
	=						
IBC.	_	INTERNATIONAL F	BUILDING CODE				
HSA.	=	HEADED STUD AN	ICHOR				
LLH.	=	LONG LEG HORIZ	ONTAL				
LLV.	=	LONG LEG VERTION	CAL				
MAX.	=	MAXIMUM					
MECH.	=	MECHANICAL					
MIN.	=						
OAE.	=		JUAL				
O.C.	=						
PSW	-	PERFORATED SHI					
PL.	=	PLATE					
PLM.	=	PARALLAM					
REINF.	=	REINFORCEMENT					
REQD.	=	REQUIRED					
SCHED.	=	SCHEDULE					
STRUCT.	=	STRUCTURAL					
SVV.	=						
SINI.	=	SIMILAR					
TYP	_						
UNO.	=	I TEICAL LINI ESS NOTED OTHERWISE					
VERT.	=	VERTICAL					
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•/-			HOLDOWN ANCHOR TYPE				
		\mathbf{X}					
	\bigotimes	×	OVERBUILD AREA				
		→	DEPRESS FOUNDATION WALL AND				
			POUR SLAB OVER				
		→	WOOD BEAM				

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