HUNTSVILLE TOWN CORPORATION WISHING WELL CONNECTION TO SYSTEM

HUNTSVILLE, WEBER COUNTY, UTAH

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PROJECT NO. 55-18-114 JUNE 2019

AGENCY REVIEW SET

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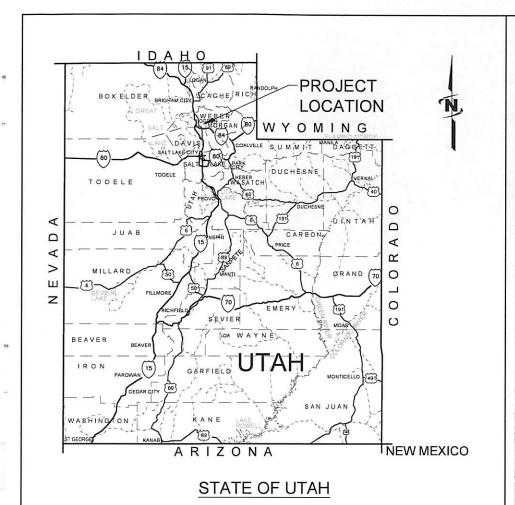


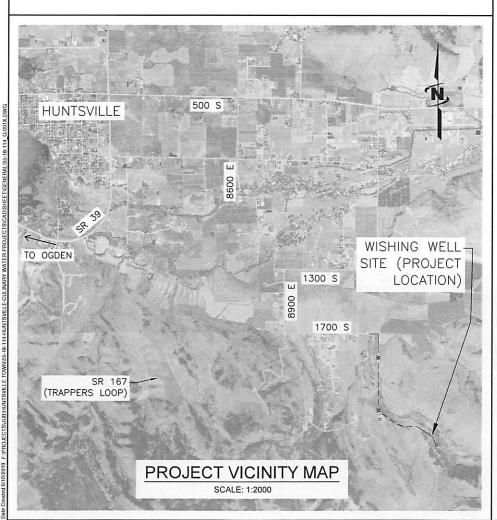
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J-U-B ENGINEERS, INC.

AGENCY

WISHING WELL CONNECTION TO SYSTEM HUNTSVILLE TOWN CORPORATION VICINITY MAP AND PROJECT LOCATION MAP

ONE INCH —

AT FULL SIZE, IF NOT ON INCH SCALE ACCORDING

HEET NUMBER:

G-002

GENERAL PROJECT NOTES

GENERAL:

- A. THE GENERAL NOTES AND SPECIFICATIONS SUPPLEMENT THE PROJECT WRITTEN TECHNICAL SPECIFICATIONS AND THE PROJECT DRAWINGS.
- THE CONTRACTOR IS RESPONSIBLE FOR ALL CONSTRUCTION BRACING, TEMPORARY SHORING, AND OTHER SITE SAFETY CONTROLS REQUIRED DURING CONSTRUCTION IN ACCORDANCE WITH ALL APPLICABLE LOCAL, STATE AND FEDERAL REGULATIONS, TO ENSURE THE STABILITY AND SAFETY OF ALL CONSTRUCTION UNTIL IT IS COMPLETED.
- C. DETAILS ON THESE PLANS ARE INTENDED TO DEPICT THE GENERAL CONSTRUCTION DETAILS AND METHODS FOR THIS PROJECT. DETAILS AND CONDITIONS NOT SPECIFICALLY SHOWN THAT ARE SIMILAR IN NATURE TO THOSE THAT ARE SPECIFIED SHALL BE ASSUMED ONE AND THE SAME. IF QUESTIONS REGARDING THE APPLICATION OF DETAILS ARE ENCOUNTERED, NOTIFY THE ENGINEER FOR CLARIFICATION OR INSTRUCTION.
- D. PRIOR TO IMPLEMENTING ANY CHANGES TO THESE PLANS, THE ENGINEER SHALL BE NOTIFIED IN WRITING FOR THEIR WRITTEN APPROVAL. CHANGES IMPLEMENTED WITHOUT THE ENGINEERS WRITTEN APPROVAL SHALL RELIEVE THE ENGINEER OF ANY CLAIM OR LIABILITY RESULTING FROM THAT PORTION OF THE PROJECT CHANGED OR AFFECTED BY THE CHANGE.

2. CONTRACTOR RESPONSIBILITY FOR COORDINATION:

- A. IT IS THE CONTRACTORS PRIME RESPONSIBILITY TO COORDINATE THE WORK SHOWN ON ALL OF THE PROJECT DRAWINGS, GENERAL, SPECIAL, AND TECHNICAL SPECIFICATIONS.
- B. THE CONTRACTOR IS RESPONSIBLE TO VERIFY ALL EXISTING CONSTRUCTION MATERIAL TYPES, DIMENSIONS, ELEVATIONS, AND CONDITIONS.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO CAREFULLY STUDY AND COORDINATE THE CONSTRUCTION REQUIREMENTS SHOWN ON THESE DRAWINGS. WHEN CONFLICTS OR DISCREPANCIES ARE FOUND IN THESE DRAWINGS, THE CONTRACTOR SHALL REPORT THEM IMMEDIATELY TO THE PROJECT ENGINEER FOR DIRECTION AND/OR CLARIFICATION.
- D. ANY CONSTRUCTION WORK DONE BY THE CONTRACTOR BEFORE OBTAINING SUCH CLARIFICATION FROM THE PROJECT ENGINEER SHALL BE AT THE CONTRACTOR'S OWN RISK AND COST. FURTHERMORE; ANY WORK REQUIRED TO CORRECT, REPLACE AND/OR RESTORE THE WORK AS DIRECTED BY THE ENGINEER SHALL BE AT THE CONTRACTOR'S OWN RISK AND COST.
- E. THE PROJECT WILL REQUIRE COORDINATION BETWEEN SEVERAL GOVERNMENT AND PRIVATE AGENCIES. FOR ANY COORDINATION EFFORTS, THE CONTRACTOR IS TO REFER TO THE LIST OF PROJECT AGENCIES FOR THE APPROPRIATE PERSONS TO CONTACT.

3. PROJECT NOTES:

- A. THE CONTRACTOR SHALL LIMIT ACTIVITIES TO IMMEDIATE PROJECT AREA TO FULLEST EXTENT
- B. ANY DAMAGE TO PUBLIC OR PRIVATE PROPERTY RESULTING FROM CONSTRUCTION ACTIVITIES SHALL BE RESTORED TO EQUAL OR BETTER CONDITION AT THE CONTRACTOR'S EXPENSE.
- C. THE ENGINEER WILL PROVIDE VERTICAL AND HORIZONTAL CONTROLS ON THE PROJECT SITE. ANY ADDITIONAL CONSTRUCTION STAKING REQUIRED TO COMPLETE THE PROJECT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- D. THE CONTRACTOR SHALL LOCATE AND PROTECT ALL EXISTING UTILITIES AND BE RESPONSIBLE FOR DAMAGES TO EXISTING UTILITIES AND EXISTING IMPROVEMENTS AS A RESULT OF THE CONTRACTOR'S CONSTRUCTION ACTIVITIES.

4. EARTHWORK:

- A. STRIP AND REMOVE EXISTING VEGETATION, DEBRIS, AND OTHER DELETERIOUS MATERIALS FROM THE EXCAVATION LIMITS
- IT IS ANTICIPATED THAT GROUNDWATER WILL BE FOUND DURING THE EXCAVATION. IN THE EVENT THAT GROUNDWATER IS PRESENT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DEWATERING DURING THE CONSTRUCTION PERIOD.
- C. THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND CONSTRUCTING STABLE EXCAVATIONS AS REQUIRED TO MAINTAIN STABILITY OF BOTH EXCAVATION SIDES AND BOTTOM. ALL EXCAVATIONS SHOULD BE SLOPED OR SHORED IN THE INTEREST OF SAFETY FOLLOWING LOCAL. STATE, AND FEDERAL REGULATIONS, INCLUDING CURRENT OSHA EXCAVATION AND TRENCH SAFETY STANDARDS
- D. THE CONTRACTOR SHALL EXCAVATE THE SITE TO THE LIMITS AND ELEVATIONS SHOWN ON THE PLANS.

5. EXISTING UTILITIES:

- A. THE LOCATION OF EXISTING UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY.
- DEPTHS AND ELEVATIONS OF UTILITIES ARE UNKNOWN UNLESS OTHERWISE SHOWN.
- C. UNDERGROUND UTILITY LOCATION AND VERIFICATION IS TO BE AN ONGOING PROCESS.
- CONTRACTOR IS RESPONSIBLE TO:
- VERIFY EXACT LOCATIONS OF ALL UTILITIES PRIOR TO BEGINNING WORK IN THAT AREA
- FIELD VERIFY UTILITY LOCATION, DEPTHS, AND ELEVATIONS WHERE CONFLICTING UTILITIES MAY BE PRESENT A MINIMUM OF 500 FEET AHEAD OF TRENCHING OPERATIONS
- iii. BRING ANY DISCREPANCIES AND/OR CONFLICTS TO THE ATTENTION OF THE ENGINEER IMMEDIATELY.
- iv. NOTIFY APPROPRIATE UTILITY COMPANIES WHEN CONSTRUCTION MIGHT INTERFERE WITH NORMAL OPERATION OF ANY LITHTIES.
- MAINTAIN SERVICE OF EXISTING UTILITIES.
- vi. RESTORE ANY UTILITIES DAMAGED DUE TO CONSTRUCTION AT NO ADDITIONAL COST TO THE

6. INSPECTION AND TESTING:

- A. THE OWNER SHALL BE RESPONSIBLE FOR MATERIALS TESTING INCLUDING BUT NOT LIMITED TO CONCRETE, ASPHALT, AND COMPACTION. ALL TESTS SHALL MEET MINIMUM ENGINEER REQUIREMENTS. SEE THE CONTRACT DOCUMENTS AND DRAWINGS FOR FREQUENCY OF TESTING. RESULTS ARE TO BE DELIVERED TO SPECIAL INSPECTOR, OWNER AND ENGINEER.
- THE CONTRACTOR IS RESPONSIBLE TO COORDINATE WITH ENGINEER AND SPECIAL INSPECTOR FOR INSPECTIONS OF WORK AT APPROPRIATE INTERVALS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PAY FOR ADDITIONAL INSPECTIONS THAT ARE THE RESULT OF HIS WORKMANSHIP.

7. PERMITTING AND COORDINATION:

- CONTRACTOR SHALL OBTAIN ALL NECESSARY PERMITS AND BUSINESS LICENSES PRIOR TO CONSTRUCTION.
- CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL LOCAL, STATE, AND FEDERAL PERMITS REQUIRED FOR STORM WATER POLLUTION PREVENTION AS A RESULT OF CONSTRUCTION ACTIVITIES. WHEN CALLED FOR IN THE CONTRACT DOCUMENTS, CONTRACTOR SHALL PREPARE A STORM WATER POLLUTION PREVENTION PLAN FOR APPROVAL BY THE ENGINEER AND FOR SUBMITTAL TO LOCAL AUTHORITIES FOR REVIEW AND APPROVAL. IF THE CONSTRUCTION WILL DISTURB MORE THAN ONE ACRE, CONTRACTOR SHALL FILE A "NOTICE OF INTENT" FOR PERMIT COVERAGE UNDER THE STATE'S UPDES STORM WATER GENERAL PERMIT FOR CONSTRUCTION ACTIVITIES (UTR300000) AND PAY ALL ASSOCIATED FEES. THE NOI MAY BE FILED ELECTRONICALLY AT THE FOLLOWING WEBSITE: TTP://DEQ.UTAH.GOV/LEGACY/PERMITS/WATER-QUALITY/UTAH-POLLUTANT-DISCHARGE-
- ELIMINATION-STORM-WATER-GENERAL-CONSTRUCTION.HTM AND FOLLOWING THE DIRECTIONS GIVEN UNDER THE HEADING "ONLINE APPLICATION PROCESS AND SEARCH FOR EXISTING PERMITS". THE CGP DOES NOT RELIEVE CONTRACTOR FROM COMPLIANCE WITH OTHER REGULATIONS OR CONTRACT REQUIREMENTS REGARDING STORM WATER POLLUTION PREVENTION INCLUDING BUT NOT LIMITED TO: PROTECTION OF SURFACE WATERS, PREVENTION OF SOIL RUNOFF INTO DRAINS, DUST CONTROL, PREVENTION OF TRACKING SOILS TO ADJACENT STREETS, FUEL CONTAINMENT, SPILL CONTROL FTC.
- B. ANY WORK DONE WITHIN A PUBLIC RIGHT-OF-WAY SHALL BE COORDINATED WITH THE APPROPRIATE TRANSPORTATION AGENCY AND SHALL MEET THE REQUIREMENTS OF THAT AGENCY AND. IN PARTICULAR, REQUIREMENTS OF ANY RIGHT-OF-WAY SPECIAL USE PERMIT, OR OTHER PERMIT. ALL WORK SHALL MEET CURRENT OSHA REQUIREMENTS.
- C. WHERE WORK IS PERFORMED ON EASEMENTS, THE CONTRACTOR SHALL TAKE EVERY PRECAUTION TO ELIMINATE ANY ADVERSE EFFECTS ON THE ADJACENT PROPERTY AND/OR TO RESTORE IT TO ITS ORIGINAL CONDITION.

8. MISCELLANEOUS:

- CONTRACTOR IS RESPONSIBLE FOR DUST ABATEMENT AND ANY LIABILITY ISSUES RELATED TO DUST AT ANY LOCATION WHICH MAY BE CAUSED BY THIS PROJECT.
- THE CONTRACTOR IS RESPONSIBLE FOR TRAFFIC CONTROL AND PROTECTION OF PEDESTRIANS IN AND AROUND THIS WORK. REFERENCE THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD LATEST EDITION FOR WORK ZONE TRAFFIC CONTROL).
- THE CONTRACTOR SHALL PRESERVE EXISTING CITY, COUNTY, STATE, AND FEDERAL LAND MONUMENTS WHENEVER POSSIBLE. IF A MONUMENT MUST BE MOVED, THE ENGINEER SHALL BE CONTACTED 2 WEEKS PRIOR TO REMOVAL TO ARRANGE FOR RELOCATION.
- D. SHOULD CONSTRUCTION BE HALTED BECAUSE OF INCLEMENT WEATHER CONDITIONS, THE CONTRACTOR WILL COMPLETELY CLEAN UP ALL AREAS AND MAINTAIN THE SURFACE IN GOOD CONDITION DURING THE SHUT-DOWN PERIOD.

9. PROJECT CONTACT LIST:

HUNTSVILLE TOWN	
J-U-B ENGINEERS	S, INC.
J-U-B ENGINEERS	SINC

SCOTT RICHARDSON (801) 698-1867 BRIAN DEETER BRANDON NIELSEN

(801) 547-0393 (801) 547-0393

	REVIATIONS
ABBREV.	TERM
ALUM	ALUMINUM
ASSY	ASSEMBLY
۷	ANGLE
©	AT (MEASUREMENTS)
BC	BEGINNING OF CURVE
BLDG	BUILDING
B.M.	BENCH MARK
BP	ALIGNMENT BEGINNING
BREAK	GRADE BREAK
BSC	BITUMINOUS SURFACE COURSE
BSW	BACK OF SIDEWALK
BVC	BEGIN VERTICAL CURVE
BVP	PROFILE START
B.W.	BOTH WAYS
С	CHANNEL (STRUCTURAL)
CJ	CONTROL JOINT
<u>Ç</u>	CENTER LINE
CLR	CLEARANCE
СМР	CORRUGATED METAL PIPE
со	CLEANOUT
CONC	CONCRETE
CONT	CONTINUOUS
CPLG	COUPLING
CTR	CENTER
CU FT	CUBIC FEET
CU YD	CUBIC YARD
DEG OR .	DEGREE
DIA OR Ø	DIAMETER
DI	DUCTILE IRON
DIST	DISTRIBUTION
DWG	DRAWING
EA	EACH
EC	END OF CURVE
ELB	ELBOW
ELEV OR EL.	ELEVATION
EOA	EDGE OF ASPHALT
EP	ALIGNMENT END
EVP	PROFILE END
E.W.	EACH WAY
EXIST	EXISTING
EVC	END VERTICAL CURVE
FF	FINISH FLOOR
FG	FINISH GRADE
FH	FIRE HYDRANT
FL	FLOW LINE
FLG	FLANGE
FT OR '	FEET
FTG	FOOTING
GALV	GALVANIZED
GB	GRADE BREAK
HORIZ	HORIZONTAL
	HIGH POINT

ABE	BREVIATIONS	ABI	BREVIATIONS
BREV.	TERM	ABBREV.	TERM
	ALUMINUM	ID	INSIDE DIAMETER
	ASSEMBLY	IE .	INVERT ELEVATION
	ANGLE	IN. OR "	INCH
	AT (MEASUREMENTS)	INV.	INVERT
	BEGINNING OF CURVE	K	CURVE COEFFICIENT
	BUILDING	<u> </u>	LEFT
	BENCH MARK	LB	LINE BEGINNING
	ALIGNMENT BEGINNING	LB OR #	POUND
	GRADE BREAK	LC	LEVEL CROWN
	BITUMINOUS SURFACE	LE	LINE END
	COURSE	LF	LINEAL FEET
	BACK OF SIDEWALK	LN	LINEAL
	BEGIN VERTICAL CURVE	LP	LOW POINT
	PROFILE START	MAN	MANUAL
	BOTH WAYS	MAX	MAXIMUM
	CHANNEL (STRUCTURAL)		
	CONTROL JOINT	MIN OR #	MINIMUM
	CENTER LINE	NO. OR #	NUMBER
	CLEARANCE	PC	POINT OF CURVATURE
	CORRUGATED METAL PIPE	PCC	POINT OF COMPOUND CURVATURE
	CLEANOUT	PE	PLAIN END
	CONCRETE		TANGENT-TANGENT
	CONTINUOUS	PI	INTERSECT
	COUPLING	PL OR PL	PLATE OR PROPERTY L
	CENTER	POLY	POLYETHYLENE
	CUBIC FEET	PRC	POINT OF REVERSE
	CUBIC YARD		CURVATURE
R ·	DEGREE	PT	POINT OF TANGENCY
		PVC	POLYVINYL-CHLORIDE
? ø	DIAMETER	PVI	POINT OF VERTICAL INTERSECTION
	DUCTILE IRON	R	RADIUS OR RIGHT
	DISTRIBUTION	RC	REVERSE CROWN
	DRAWING	REQ'D	REQUIRED
	EACH	REV	REVISION
	END OF CURVE	R/W	RIGHT-OF-WAY
	ELBOW	s s	SLOPE
K EL.	ELEVATION	SPEC	SPECIFICATION
	EDGE OF ASPHALT		STATION
	ALIGNMENT END	STA	STANDARD
	PROFILE END	STD	+
	EACH WAY	STL	STEEL STAINI FOR STEEL
	EXISTING	ST STL	STAINLESS STEEL
	END VERTICAL CURVE	TBC	TOP BACK OF CURB
	FINISH FLOOR	TFC	TOP FACE OF CONCRET
	FINISH GRADE	THD	THREADED
	FIRE HYDRANT	TOB	TOP OF BEAM
	FLOW LINE	TOC	TOP OF CONCRETE
	FLANGE	TOF	TOP OF FOOTING
,	FEET	TOP	TOP OF PIPE
	FOOTING	TOW	TOP OF WALL
	GALVANIZED	TYP	TYPICAL
	GRADE BREAK	W/	WITH
	HORIZONTAL	W/O	WITHOUT
	HIGH POINT	W/REQ'D	WHERE REQUIRED



CALL 2 BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES

Call before you dig.

J-U-B ENGINEERS, 466 North 900 V Kaysville, Utah **AGENCY** STEN TO SY CONNEC

J-U-B ENGINEERS, INC

West 84037

FILE: 55-18-114 G-001X CHECKED BY: B AT FULL SIZE, IF NOT C

WISHING WELL C HUNTSVILLE

INCH. SCALE ACCORD T UPDATED: 6/25/2019 SHEET NUMBER:

G-003

SYMBOL LEGEND					
DESCRIPTION	EXIST.	PROP.	DESCRIPTION	EXIST.	PROP.
SANITARY SEWER			IRRIGATION		
CLEANOUT	9	©	IRRIGATION VALVE	⊠ ⊠	×
SS MANHOLE	(\$)	6	IRRIGATION VALVE BOX	Φ	O O
SS VALVE	Š	Ş	SPRINKLER	Δ	A
SS METER	å	S	IRRIGATION GATE		۵
SEWER STUB	\$	©	NATURAL GAS		
STORM DRAIN			GAS METER	£	G B
CATCH BASIN	₿	B	GAS VALVE	ο̈́	Ğ
DRY WELL	(w)	6₩	GAS MANHOLE	©	©
SD MANHOLE	0	0	UTILITIES		
FLARE END	∀	0	MANHOLE (GENERIC)	0	•
GREASE TRAP	© 0	© O	PRESSURE CLEAN OUT AT GRADE	(PCG)	∞
COMMUNICATION			THRUST BLOCK	_	•
TELE. MANHOLE	T	Θ	VAULT	V	V
TELE. PEDESTAL	Œ	Ð	VALVE (GENERIC)	\bowtie	H
TELE. POLE	-0-	•	UTILITY POLE	-	
TV PEDESTAL	ī₩	껪	SITE		
GUY WIRE	Ť	T	BOLLARD		•
DOMESTIC WATER			BOULDER	0	
FIRE HYDRANT	Q	A	DRINKING FOUNTAIN	(OF)	OF)
SPIGOT	9	€	FLAGPOLE	Ð	Ø
WATER MANHOLE	₩)	Ø	GATE		
WATER METER	*	*	MAIL BOX	M	M
WATER VALVE	×	×	PARKING METER	- ₽₩	₽#
YARD HYDRANT	ç	•	POST	۰	•
ELECTRIC			SIGN	-	-
ELEC. MANHOLE	©	©	SPOT ELEVATION	×	×
ELEC. METER	E	E B	TREE (SHRUB)	0	0
ELEC. TRANS.	Ē	Ē	TREE	E A	()
JUNCTION BOX	J	J	TREE		Carried Street
GUY WIRE	Ť	T	TEST HOLE	œ	œ
POWER STUB	©	Œ)	WELL	ŵ	ŵ
POWER POLE	+	-	WELL (MONITORING)	M	I X I
STREET LIGHT	❖	*	SURVEY		
STREET LIGHT WITH ARM	⇔¤	+-×	CAP	•	
TRAFFIC SIGNAL POLE	(CTRL PT	<u> </u>	
			NAIL	٥	٥
			BOLT	•	
			REBAR	0	•

DESCRIPTION	EXIST.	PROP
STORM DRAIN	sp	SD
DRAIN LINE	DL	DL
SANITARY SEWER	ss	ss
NATER	w	w-
RRIGATION	IRR	IRR-
NATURAL GAS		G
OVERHEAD POWER	OHP	ОНР
JNDERGROUND POWER	UP	UP-
OVERHEAD TELEPHONE	OHT	——онт—
INDERGROUND TELEPHONE	TU	——ит—
FIBER OPTIC	F/O	F/0-
CABLE TELEVISION	ctv	стv_
FENCE	x	x_
DITCH		
MAJOR CONTOUR	2520 	2520-
INOR CONTOUR		
OP OF BANK	тов	тов –
TOE OF SLOPE	—— тое ———	—— тое –
PROPERTY LINE	P/L	P/L-
PROPERTY LINE (OPTIONAL)		
RIGHT OF WAY	R/W	R/W
EMPORARY EASEMENT	т/Е	T/E_
PERMANENT EASEMENT	P/E	——P/E-
ROAD SHOULDER		
OAD CENTERLINE		
ROAD ASPHALT		- \ \
OAD GRAVEL	EG	EG
ROAD DIRT		
CURB AND GUTTER		

SHEET NUMBERING

EXAMPLE: SHEET NUMBER: C1-101

DISCIPLINE DESIGNATOR COMPONENT CODE (OPTIONAL) SHEET TYPE DESIGNATOR - SHEET SEQUENCE NUMBER C1-101

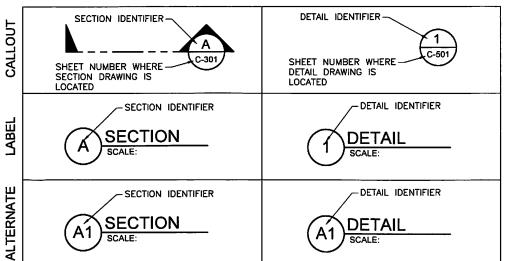
DISCIPLINE DESIGNATORS				
DISCIPLINE	DESIGNATOR	DESCRIPTION		
	G	ALL GENERAL		
GENERAL	GI	GENERAL INFORMATION		
GENERAL	GC	GENERAL CONTRACTUAL		
	GR	GENERAL RESOURCE		
SURVEY/MAPPING	٧	ALL SURVEY		
GEOTECHNICAL	В	ALL GEOTECHNICAL		
CIVIL	С	ALL CIVIL		
LANDSCAPE	L	ALL LANDSCAPE		
STRUCTURAL	S	ALL STRUCTURAL		
ARCHITECTURAL	A	ALL ARCHITECTURE		
EQUIPMENT	Q	ALL EQUIPMENT		
MECHANICAL	М	ALL MECHANICAL		
ELECTRICAL	E	ALL ELECTRICAL		
PLUMBING	Р	ALL PLUMBING		
PROCESS	D	ALL PROCESS		
RESOURCE	R	ALL RESOURCE		

	SHEET TYPE DESIGNATORS				
DESIGNATOR	SHEET TYPE				
0	GENERAL (SYMBOLS, LEGENDS, NOTES, ETC.)				
1	PLANS (HORIZONTAL VIEWS)				
2	ELEVATIONS, PROFILES, COMBINED PLAN & PROFILES				
3	SECTIONS (SECTIONAL VIEWS)				
4	LARGE-SCALE VIEWS (PLANS, ELEVATIONS, ETC.)				
5	DETAILS OR COMBINED DETAILS AND SECTIONS				
6	USER DEFINED				
7	USER DEFINED				
8	USER DEFINED				
9	3D REPRESENTATIONS (ISOMETRICS, PERSPECTIVES, PHOTOS)				

SECTION AND DETAIL IDENTIFIERS

SECTION IDENTIFICATION

DETAIL IDENTIFICATION



NOTE:
A DASH MAY BE PLACED IN THE LOWER PORTION OF THE IDENTIFIER IF
THE DETAIL DRAWING OR SECTION VIEW IS LOCATED ON THE SAME SHEET.

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AGENCY

WISHING WELL CONNECTION TO SYSTEM HUNTSVILLE TOWN CORPORATION LINE AND SYMBOL LEGENDS AND DETAIL KEY

FILE: 55-18-114_G-001X JUB PROJ. #: 55-18-114 DRAWN BY: JRH

DESIGN BY BRN

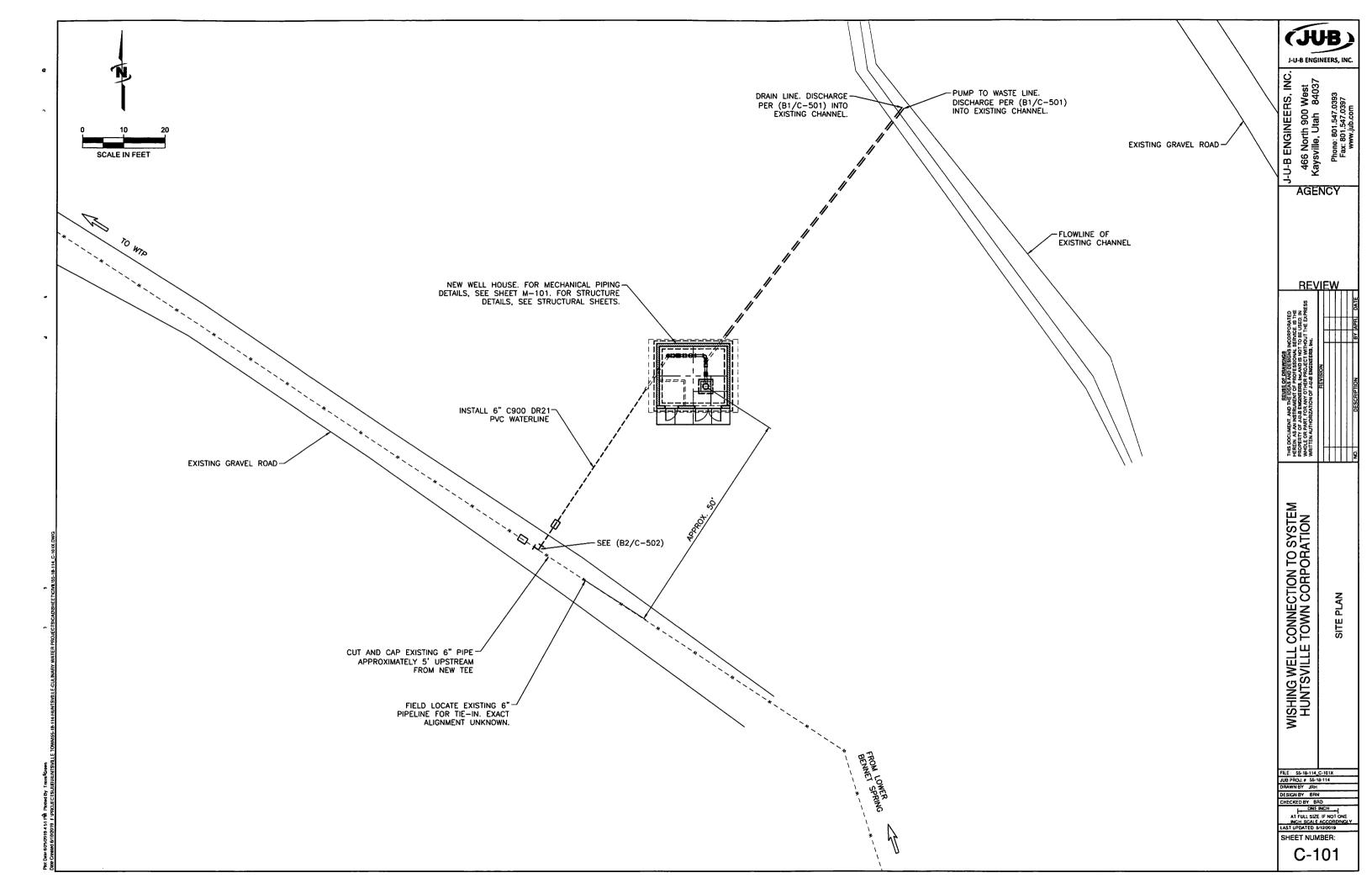
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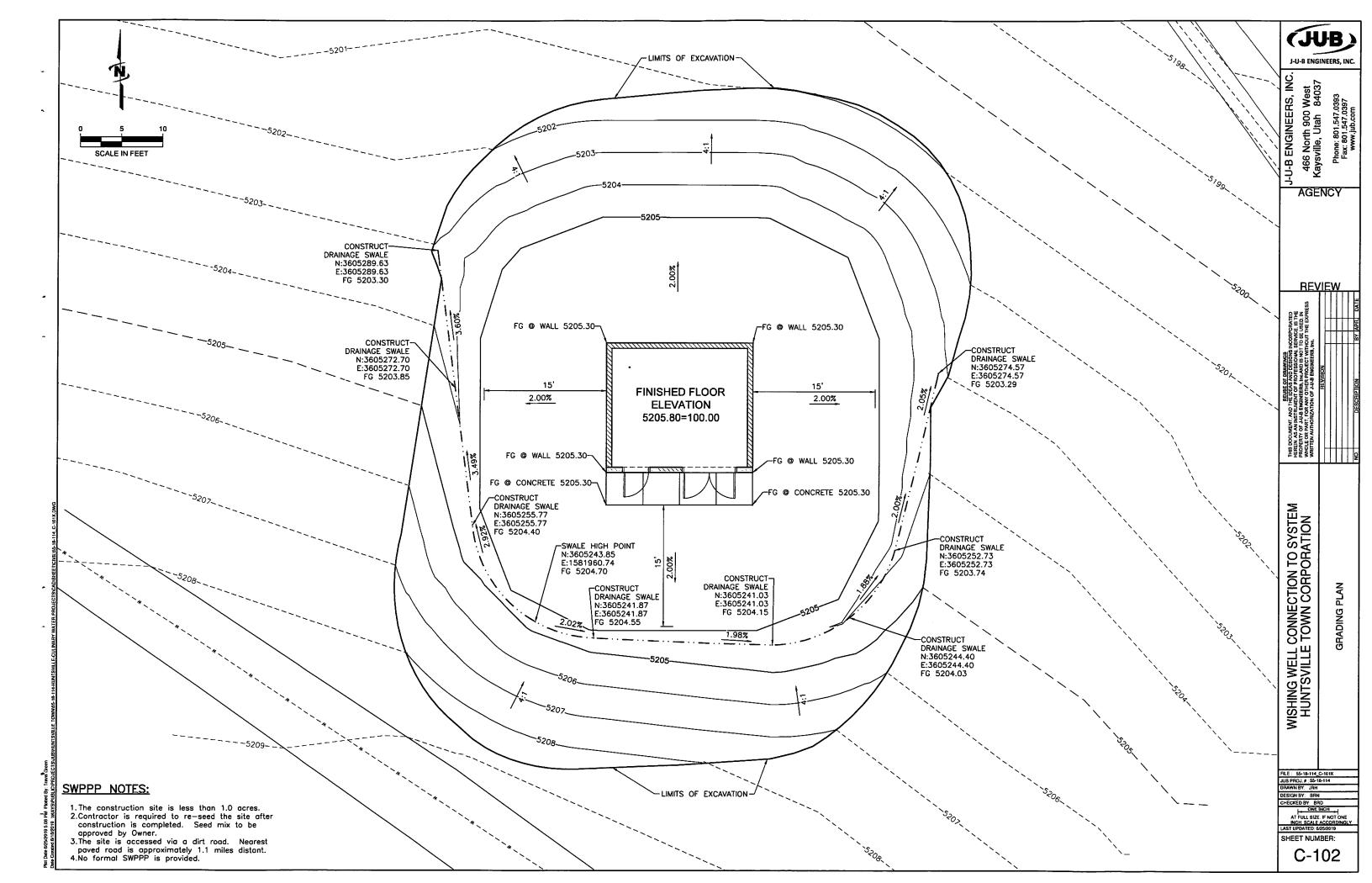
ONE INCH
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INCH SCALE ACCORDING.

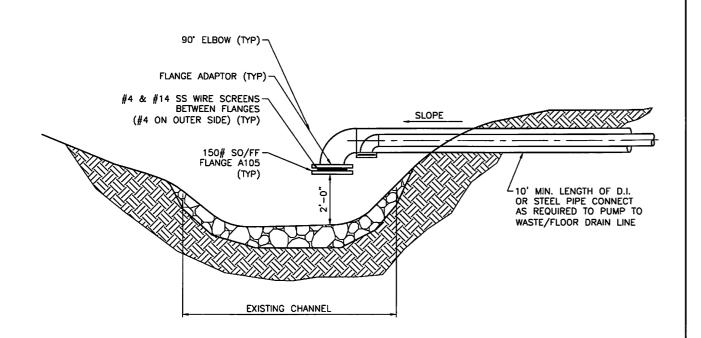
LAST UPDATED. 0/25/2019

SHEET NUMBER:

G-004







WASTE/FLOOR DRAIN OUTLET DETAIL

BEARING UNDISTURBED AREA EARTH (TYP) 75% OF 100% 100% OF 100% OF 90' TEE VALVE -PLUG OR CAP-

ALL MJ AND FLANGED FITTINGS TO BE WRAPPED WITH POLYETHYLENE WRAP PRIOR TO POURING THRUST BLOCK.

DETAIL NOTES: FIGURE (100%) AT THRUST BLOCK INDICATES PERCENT OF TOTAL THRUST TO BE APPLIED FOR BEARING AREA. ARROW (-INDICATES THRUST DIRECTION. CONCRETE FOR THRUST BLOCKS TO BE 3000 P.S.I.

GRAV	TY THRUS	T_BLOCK	SIZES		
PIPE SIZE	GRAVITY	BLOCK SI	ZE (CY)		
	11.25' BEND	22.5° BEND	45° BEND		
4"	0.2	0.5	0.9		
6"	0.5	0.9	1.8		
8"	8.0	1.6	3.2		
10"	1.2	2.4	4.8		
12"	1.7	3.4	6.7		
14"	2.3	4.6	9.0		
DESIGN PRESSURE = 200 PSI SOIL BEARING CAPACITY = 2000 LB/SF WEIGHT OF CONCRETE TO RESIST 100% OF TOTAL THRUST					

1. IN USING THE CURVE THRUST BLOCKING TABLE, USE THE MAXIMUM INTERNAL PRESSURE ANTICIPATED (i.e. HYDROSTATIC TEST PRESSURE, POSSIBLE SURGE PRESSURE DUE TO PUMP SHUT-OFF, ETC.)

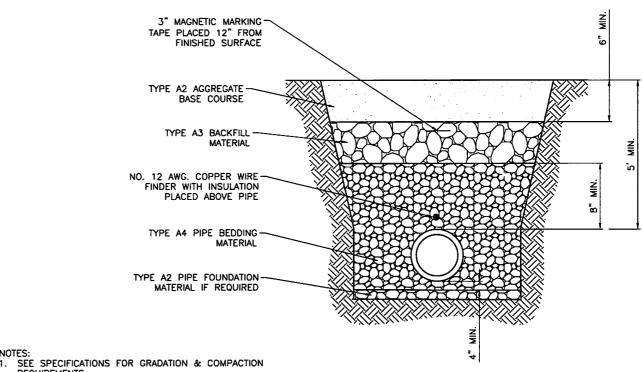
TRUST BLOCK DETAIL

SEE SOILS REPORT FOR BEARING STRENGTH OF SOIL IN THE ABSENCE OF A SOILS REPORT, AND AVERAGE SOIL (SPADABLE MEDIUM CLAY) CAN BE ASSUMED TO HAVE A BEARING STRENGTH OF 2000 P.S.F.

#5 EPOXY COATED ¾" TIE RODS LENGTH **REBAR WITH 12"** 5° MAX. DEFLECTION MIN EMBEDMENT JOINT FOR PIPES 3"-12" AND 3" MAX. FOR 14"-24" 50% 50% 50% 50% **CURVE THRUST BLOCKING**

	THRUST ON FITTINGS IN LBS 9 1 PSI OF WATER PRESSURE *					
	PIPE DEAD END 90° 45° 22½°					
SIZE	OR TEE	ELBOW	ELBOW	ELBOW		
4"	25	35	20	10		
6"	51	72	39	20		
8"	88	123	64	34		
10"	142	201	110	56		
12"	202	284	155	80		
14"	273	385	210	107		
16"	354	498	272	142		
18"	351	494	269	137		
20"	565	795	433	220		
24"	810	1142	622	318		
	* (SF=1.5)					

EXAMPLE: 8-INCH 90° ELBOW, PRESSURE = 200 Ib./SQ. IN. FROM TABLE: THRUST = $94 \times 200 = 18,800$ lb. ASSUME BEARING STRENGTH OF SOIL = 2000 lb./SQ. FT. $\frac{18800}{2000}$ = 9.4 SQ. FT. = AREA OF BEARING REQUIRED FOR THRUST BLOCK.



SEE SPECIFICATIONS FOR GRADATION & COMPACTION REQUIREMENTS.

TRENCH WIDTH SHALL BE O.D. + 12".

BACKFILL TO BE COMPACTED TO 95% MODIFIED PROCTOR PER ASTM D1557 IN ROADWAYS AND 93% IN LANDSCAPED AREAS.

GRAVEL TRENCH SECTION

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REVIEW

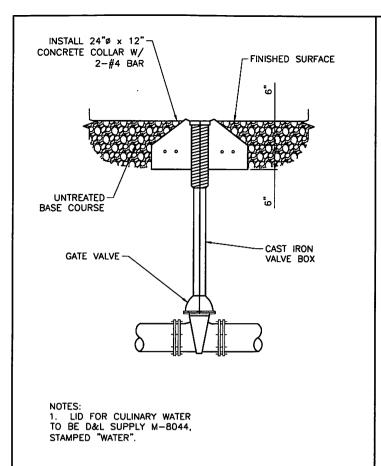
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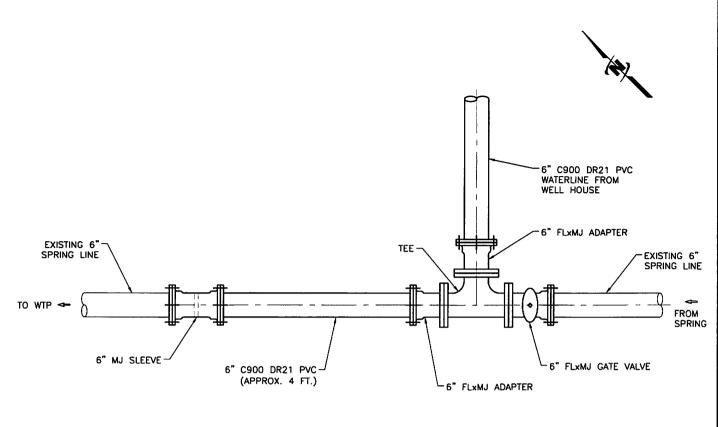
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C-501



VALVE DETAIL SCALE: N.T.S.



B2 CONNECTION DETAIL

SCALE: 1"=1"-0"

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J-U-B ENGINEERS, INC.

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Kaysville, Utah 84037
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WISHING WELL CONNECTION TO SYSTEM HUNTSVILLE TOWN CORPORATION

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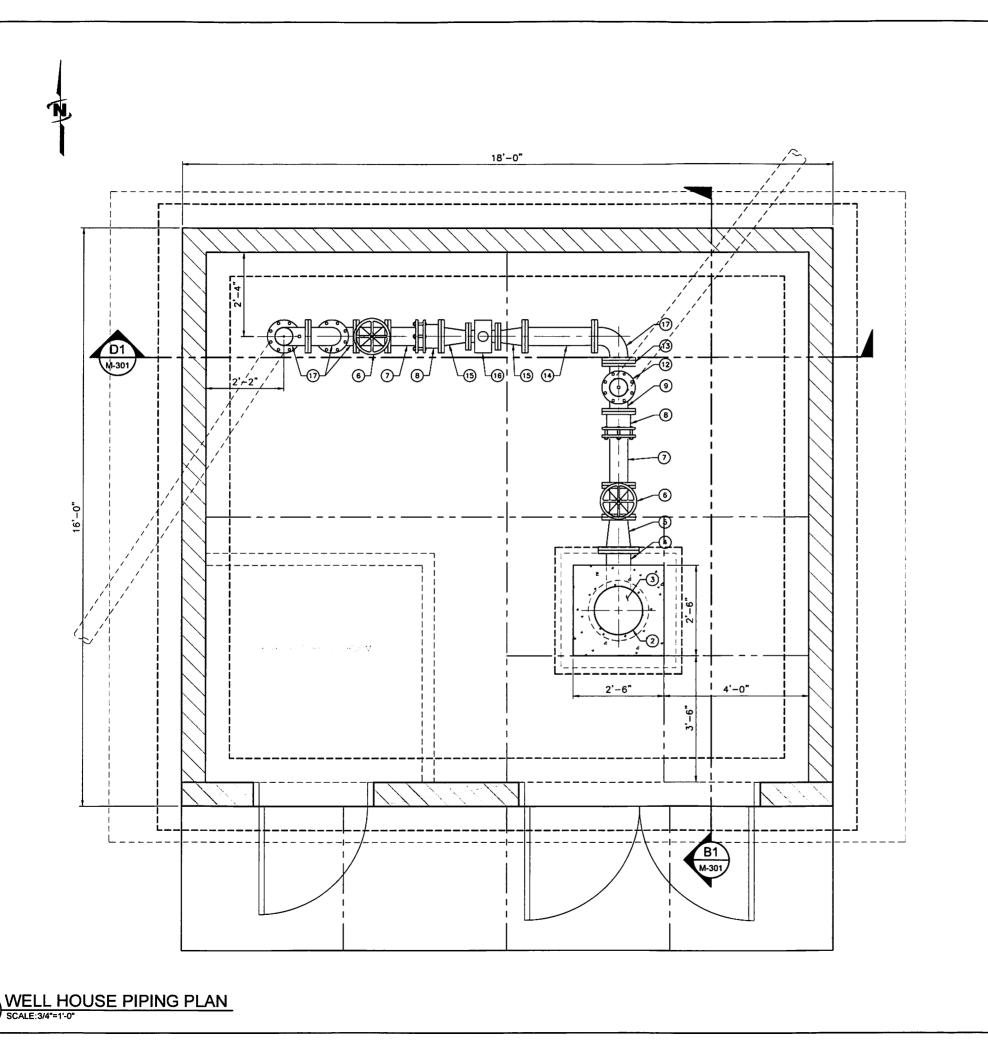
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DRAWN BY JRH

DESIGN BY BRN

SHEET NUMBER:

C-502



KEYED NOTES

- 1 20" WELL CASING.
- 2 16" WELL CASING.
- 3 WELL CASING CAP.
- 4 8" WELL OUTLET PIPE W/ FLANGE.
- 5 8"x6" CONC. REDUCER, FLxFL.
- 6 6" GATE VALVE, FLxFL.
- 7 6" SPOOL, FLxPE.
- 8 6" FLANGE COUPLING ADAPTOR.
- 9 6" CROSS, FLxFL.
- 10 COMBINATION AIR/VACUUM VALVE. PROVIDE WITH ISOLATION VALVE AND DISCHARGE PIPE WITHIN 12"-18" OFF FLOOR.
- 11 SMOOTH NOSE SAMPLING TAP.
- 12 6" BLIND FLANGE.
- (3) 6" TIDE FLEX CHECK VALVE.
- 6" SPOOL, FLXFL.
- (5) 6"x4" REDUCER, FLxFL.
- 16 4" FLOW METER, FLxFL.
- (17) 6" 90° BEND, FLxFL.
- (18) 6" 90° BEND, MJxMJ.
- 19 PIPE SUPPORT.
- 20 4" FLOOR DRAIN.

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WISHING WELL CONNECTION TO SYSTEM HUNTSVILLE TOWN CORPORATION

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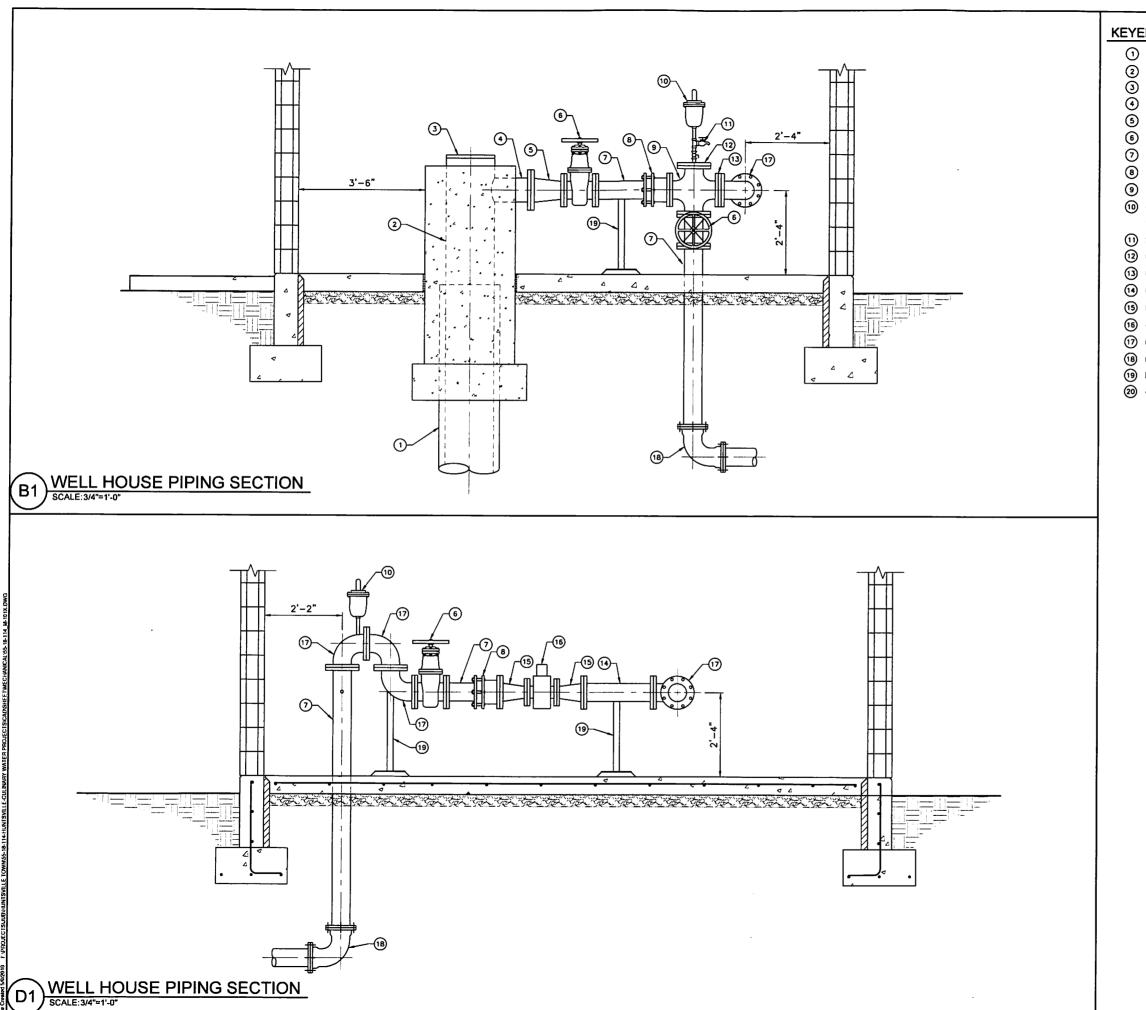
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LAST UPDATED 6/12/2019

SHEET NUMBER:

M-101



KEYED NOTES

- 1 20" WELL CASING.
- (2) 16" WELL CASING.
- 3 WELL CASING CAP.
- (4) 8" WELL OUTLET PIPE W/ FLANGE.
- (5) 8"x6" CONC. REDUCER, FLxFL.
- 6 6" GATE VALVE, FLxFL.
- 7) 6" SPOOL, FLxPE.
- (8) 6" FLANGE COUPLING ADAPTOR.
- 9 6" CROSS, FLxFL.
- (10) COMBINATION AIR/VACUUM VALVE. PROVIDE WITH ISOLATION VALVE AND DISCHARGE PIPE WITHIN 12"-18" OFF FLOOR.
- (1) SMOOTH NOSE SAMPLING TAP.
- 12 6" BLIND FLANGE.
- (3) 6" TIDE FLEX CHECK VALVE.
- 6" SPOOL, FLXFL.
- 15 6"x4" REDUCER, FLxFL.
- 16 4" FLOW METER, FLxFL.
- (17) 6" 90" BEND, FLxFL.
- (B) 6" 90" BEND, MJXMJ.
- 19 PIPE SUPPORT.
- 20 4" FLOOR DRAIN.

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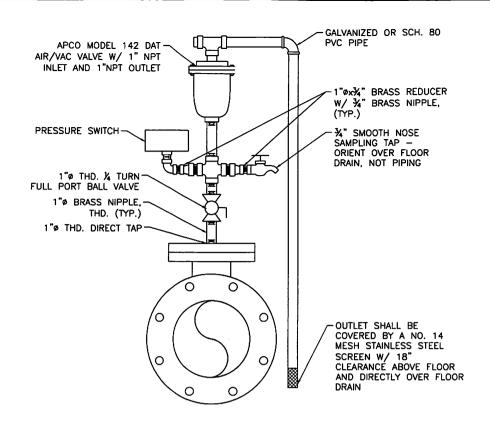
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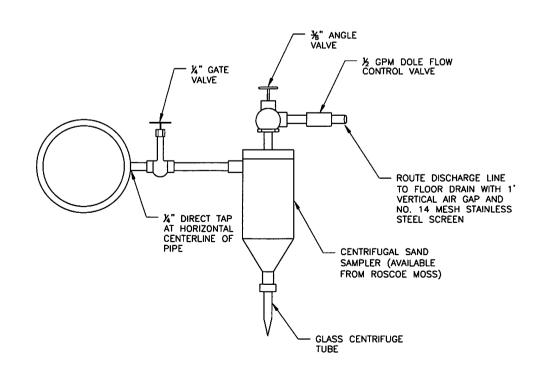
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SHEET NUMBER:

M-301



DIRECT TAP W/ AIR-VAC, SAMPLING TAP, AND PRESSURE SWITCH SCALE: N.T.S.



D1) CENTRIFUGAL SAND SAMPLER SCALE: N.T.S.

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GENERAL STRUCTURAL NOTES

1. GENERAL

These general structural notes and specifications supplement the project written technical specifications and the project structural drawings.

The contractor is responsible for all construction bracing, temporary shoring, and other site safety controls required during construction in accordance with all applicable local, state and federal regulations, to insure the stability and safety of all construction until it is completed and self-supporting.

The contractor is responsible for all water, both above and below ground, runoff and other environmental contractors required during construction in insure the site is

other environmental controls required during construction to insure the site is maintained in compliance with all applicable local, state and federal regulations.

Details on these plans are intended to depict the general construction details and methods for this structure. connection details and conditions not specifically shown that are similar in nature to those that are specified shall be assumed one and the same. If questions regarding the application of details are encountered, notify the engineer for clarification or instruction.

Prior to implementing any changes to these plans, the engineer shall be notified in writing for their written approval. changes implemented without the engineers written approval shall relieve the engineer of any claim or liability resulting from that portion of the structure changed or affected by the change.

2. CONTRACTOR RESPONSIBILITY FOR COORDINATION

A. It is the Contractors Prime responsibility to coordinate the work shown on all of the Project Drawings, general, special and technical specifications.
 B. The Contractor is responsible to verify all existing construction material types

dimensions, elevations and conditions

almensions, elevations and conditions.

The Contractor shall verify and coordinate the dimensions among all drawings and in the field prior to proceeding with any work or fabrication, any discrepancy shall be immediately reported to the Engineer.

It is the Contractor's responsibility to carefully study and coordinate the construction requirements shown on the Architectural, Civil, Mechanical, Electrical, and the Structural Drawings. When conflicts or discrepancies are found between these plan sets and/or within these drawings, the Contractor shall report them immediately to the project Engineer for direction and/or clarification.

Any construction work done by the Contractor before obtaining such clarification from the Project Engineer shall be at the Contractor's own risk and cost. Furthermore; any work required to correct, replace and/or restore the work as directed by the Engineer shall be at the Contractor's own risk and cost.

3. CODES

International Building Code, IBC 2015 Edition.

A. Soils. By Geotechnical Engineer.

Minimum Design Loads for Buildings and Other Structures, ASCE 7; current edition. American Concrete Institute, ACI 318, Building Code Requirements for Structural

Concrete; current edition. American Concrete Institute, ACI 530, Building Code Requirements and Specifications for

Masonry Structures; current edition.

American Concrete Institute, ACI 301, Specifications for Structural Concrete.

National Design Specifications, NDS For Wood Construction; current edition.

Frequency

SPECIAL INSPECTIONS. Special Inspections per IBC Chapter 17 are required for the following items: C indicates Continuous, P indicates Periodic

		A.1.	Site preparation:	Р
		A.2.	Fill material verification:	PCCC
			Fill placement and compaction:	Č
		A.4.		Č
£	3.	Concre	te.	-
			Reinforcement placement:	Ρ
		B.2.	Placement of cast-in-place anchors:	P P
		B.3.	Verification of use of required mix:	P
			Concrete placement:	Ċ
		B.5.	Verification of in-situ concrete prior to removal	-
			of forms and shores from elevated slabs:	Ρ
(2.	Post In	stalled Concrete Anchors.	•
		C.1.	Installation:	С
0).	Structu	ral Masonry.	_
			Verification of site proportioned mortar & grout:	Р
			Observation of prism preparation:	Ċ
		D.3.	Placement of masonry units & mortar joints:	C P P
			Verification of size and location of structural elements:	P
		D.5.		
			diaphragms including type, size and location of anchors	P
		D.6.	Type, grade and size of reinforcing steel:	Ρ
		D.7.	Reinforcing steel and connector placement:	P
		D.8.	Cold/Hot weather masonry protection:	PPPPCC
		D.9.	Verify use of grout mix design:	Р
			Verify grout space is clean prior to grouting:	С
		D.11.		Ċ
Е		Wood.	•	
		E.1.	Fabrication of pre-fabricated structural elements:	Р
		E.2.	Material verification of structural panels and nails for	
			diaphragms and shear walls with edge nailing:	Ρ
		E.3.	Verification of framing size at diaphragm and shear wall	

5. SUBMITTALS

A. Submit required copies, four (4) minimum, of product or material design information to the Engineer for review for the following items:

Concrete mix designs and admixtures.

Non-shrink grout.

Expansion bolts.

panel edges with edge nailing less than or equal to 4": P
F. All special inspection shall be performed by ICBO certified inspectors.

A.3. Expansion botts.
A.4. Epoxy Anchors.
A.5. Structural masonry grout and mortar mix designs.
The following items to be designed by others are considered "Deferred Submittals".
Deferred submittals shall be accompanied by design drawings, shop drawings and structural calculations, stamped and signed by a Professional Structural Engineer currently registered in the State of Utah.
B.1. Pre-engineered and shop fabricated wood joists and trusses.

C. Submit required copies of shop drawings, four (4) minimum, to the Engineer for review prior to fabrication of the following items:
C.1. Reinforcing steel for all concrete.
C.2. Reinforcing steel for masonry walls.

6. DESIGN CRITERIA

A.	Live Load	
	A.1. Floor Live Load	150 psf
	A.2. Roof Live Load A.3. Skylight Live Load	20 psf 200 lb
В.	Roof Snow Load	200 10
	B.1. Ground Snow Load	Pg = 62 psf
	B.2. Flat Roof Snow Load	Pf = 47 psf ls = 1.1
	B.3. Importance Factor B.4. Snow Exposure Factor	Ce = 0.9
	B.5. Thermal Factor	Ct = 1.1
C.	Wind Load	
	C.1. Basic Wind Speed C.2. Wind Importance Factor	V = 120 mph lw = 1.0
	C.3. Wind Exposure	C C
D.	Seismic Load	-
	D.1. Occupancy Category	
	D.2. Seismic Importance Factor D.3. Mapped Spectral Response Acceleration	leq = 1.25
	D.3.1. Short Period Acceleration	Ss = 0.76
	D.3.2. 1-Second Acceleration	S1 = 0.25
	D.4. Site Class (Soil Profile) D.5. Spectral Response Coefficients	D
	D.5.1. Short Period Acceleration	Sds = 0.60
	D.5.2. 1—Second Acceleration	Sd1 = 0.32
	D.6. Seismic Design Category	D
	D.7. Basic Seismic Force Resisting System D.7.1. Response Modification Coef.	Special Reinforced Masonry Shear Walls R = 5.0
	D.7.2. System Overstrength Factor	Ω o = 2.5
	D.7.3. Deflection Amplification Factor	Cd = 3.5
	D.B. Analysis Procedure	Equivalent Lateral Force
EOI	INDATIONS	

7 FOUNDATIONS

A. All footings to be placed on 12" minimum compacted structural fill extending to firm, undisturbed, inorganic material. Proof roll sub-grade prior to placing structural fill where the material has been disturbed by the excavating equipment.
 B. All piers and footings outside or at the perimeter of the structure, or in other unheated areas shall be set to a depth of at least 40" below finish grade, unless

other wise noted on the plans.

other wise noted on the plans.

Net allowable bearing pressure Qa = 1,500 psf.

Local areas of soft and/or unacceptable material encountered at bottom of footing elevations indicated on the plans must be over-excavated and brought up to design grade with compacted "structural fill" or "lean concrete fill".

See specifications for structural fill requirements. Design for the mitigation of subsurface water shall be the responsibility of the

The Engineer shall be notified in writing if any clay type soils, debris or unconsolidated materials are encountered during excavations for foundations.

8. CONCRETE

A. GENERAL. Concrete shall be proportioned to provide an average compressive strength, fc, as prescribed in ACI 318 Section 26.4.3 and shall satisfy the durability criteria of ACI 318 Chapter 19.

PROJECT CONCRETE MIX TYPES: Concrete shall be proportioned and furnished for the

various project uses as indicated on the plans and as follows:

B.1. M1: Footings: fc = 3,000 psi, Absolute water-cement ratio by weight = 0.45,

Air Content = 4-6%.

B.2. M2: Foundation Walls, slab on grade, and all other miscellaneous concrete:

fc = 4,000 psi, Absolute water-cement ratio by weight = 0.45, Air Content = 4-6%

C. CONCRETE MIX COMPONENTS.

A water—reducing admixture conforming to ASTM C494, used in strict conformance with the manufacturer's instructions, shall be incorporated in all conformance with the manufacturers instructions, shall be incorporated in all concrete mix designs. At Contractor's option, a high-range water-reducing (HRWR) admixture conforming to ASTM C494, Type F or G, may be used provided the total slump is less than 10°. Fly—ash conforming to ASTM C618 Type F or C, may replace up to 20% of the cement content, provided that the mix strength is substantiated by test data.

Cement: ASTM C150 Type II. Water: Clean & Potable.

Air entraining agent: ASTM C260. Except where noted non-air entrained. Aggregate: 0.75-inch Maximum aggregate per ASTM C33. Unless noted

otherwise.

C.7. Mix Proportioning: ACI 211.1
D. CONCRETE ACCESSORIES:

REINFORCING STEEL: Reinforcing steel shall conform to ASTM A615 Grade 60;

#3 bars may be Grade 40. EXPANSION BOLTS: Bolts noted on the plans as Expansion Bolts shall be HILTI

Kwik Bolt-II, stud anchors; size and embedment as noted on the drowings, installed per the manufacturers recommendations; or an approved equal. EPOXY SET BOLTS & REBAR: Bolts and reinforcing steel bars noted on the plans as Epoxy or Construction Adhesive Set Bolts or Rebar shall be set in place utilizing the SIMPSON SET High Strength Epoxy system; size and

embedment as noted on the drawings, installed per the manufacturers recommendations; or an approved equal.

E. CONCRETE PROPORTIONS. Concrete mix proportioning shall be in accordance with ACI 211.1; Standard Practice for Selecting Proportions for Normal, Heavyweight, and Mass

CONCRETE MIX VERIFICATION: Concrete mix designs shall be verified by standard

28-day cylinder tests per ASTM C39.

EVALUATION AND ACCEPTANCE OF CONCRETE. Concrete shall be tested in accordance with the requirements of ACI 318 Section 26.12.

condition for at least 7 days after placement, except when cured in accordance with

MIXING & PLACING CONCRETE. Concrete shall be prepared, mixed, placed and consolidated in accordance with ACI 318 Section 26.5 and as follows:

H.1. ACI 304; Guide for Measuring, Mixing, Transporting, and Placing Concrete.

H.2. ACI 309; Guide for Consolidation of Concrete.

CONCRETE CURING. Concrete shall be maintained above 50—degrees F and in a moist

ACI 318 Section 26.5.3

AUI 318 Section 25.3.3.

1.1. Curing of concrete shall be per the recommendations given in ACI 308; Guide to Curing Concrete.

J. COLD WEATHER REQUIREMENTS. Adequate equipment shall be provided for heating concrete materials and protecting concrete during freezing or near-freezing weather. The recommended procedures listed in ACI 306; Cold Weather Concreting shall be

Cold weather is defined as a period when, for more than 3 consecutive days,

J.1. Cold weather is defined as a period when, for more than 3 consecutive days, the following conditions exist:

 J.1.1. The average daily air temperature is less than 40-degrees F and J.1.2. The air temperature is not greater than 50-degrees F for more than one-half of any 24-hour period.

 K. HOT WEATHER REQUIREMENTS. During hot weather, proper attention shall be given to ingredients, production methods, handling, placing, protection, and curing to prevent excessive concrete temperatures or water evaporation that could impair required

excessive concrete temperatures of water exploitation that could impoin required strength or serviceobility of the member or structure. The recommended procedures listed in ACI 305; Hot Weather Concreting shall be followed.

K.1. Hot weather is any combination of the following conditions that tends to impoin the quality of freshly mixed or hardened concrete by accelerating the rate of moisture loss and rate of cement hydration, or otherwise causing detrimental

K.1.1. High ambient temperature.

K.1.2. High concrete temperature. K.1.3. Low relative humidity.

K.1.4. Wind speed. K 1.5. Solar radiation

9. FORMWORK AND FINISHING

A. Forms shall result in a final structure that conforms to shapes, lines, and dimensions of the members as required by the design drawings and specifications.

A.1. Design of formwork shall be in accordance with ACI 318 Section 26.11.1.

A.2. Formwork shall be in accordance with ACI 347; Guide to Formwork for

Concrete.

Tolerances for finished concrete surfaces shall meet the following requirements from ACI 117, class of surface is per section 4.8.3: Footings: Class C

Foundation walls: Class B
Above grade concrete not visible to sight: Class B

B.4. Above-grade concrete visible to sight: Class A C. REMOVAL OF FORMS.

AL OF FORMS.

Concrete forms shall not be removed until the retained concrete has reached the following minimum percentage of the required 28 day compressive strength:

C.1.1. Footings and base slabs on grade: 50% of f'c.

C.1.2. Foundation walls and columns: 67% of f'c.

Where concrete cylinder tests are not available for strength verification the following guide may be used when permitted by the Project Engineer:

C.2.1. Footings and base slabs on grade: 12 hours.

C.2.2. Foundation walls and columns: 24 hours.

D. EMBEDMENTS IN CONCRETE. Conduits, pipes, and sleeves of any material not harmful to concrete and within limitations of ACI 318 Sections 20.7 and 26.8 shall be permitted to be embedded in concrete with approval of the Project Engineer, provided they are

not considered to replace structurally the displaced concrete.

Conduits and pipes of aluminum shall not be embedded in structural concrete unless effectively coated or covered to prevent aluminum—concrete reaction or

electrolytic action between aluminum and steel. E. CONSTRUCTION JOINTS.

Construction joints shall only be placed where indicated on the project drawings or as approved by the Project Engineer.

Construction joints shall be constructed in accordance with ACI 318 Section

E.3. Sawed contraction joints. Conform to ACI 301 Section 5.3.5.
CONCRETE FINISHING. All concrete surfaces shall be finished in accordance with ACI

G. Formed Concrete Surfaces. After removal of forms, give each formed surface one or more of the following finishes:
 G.1. Non-liquid Retaining Concrete Structures:

G.1.1. Concrete footings and foundations not exposed to view. Provide a surface finish SF-1.0 per Section 5.3.3.3a.
G.1.2. Foundation wall and other surfaces below grade and not exposed to view. Provide a surface finish SF-2.0 per Section 5.3.3.3.b.

G.1.3. Interior, exterior and top surfaces exposed to view to 6-inches below grade. Provide a surface finish SF-3.0 per Section 5.3.3.3.c. Unformed Concrete Surfaces. Unformed concrete surfaces including the top surface of all concrete floor slabs shall be finished in accordance with ACI 301 Section 5.3.4 and ACI 302 Chapter B.

G.2.1. For the top surfaces of walls, provide a "Scratched finish" per Section 53420

G.2.2. Interior floor surfaces shall receive a Troweled finish per Section

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FILE 55-18-114 S-001X JUB PROJ. # 55-18-11-DRAWNBY JRH DESIGN BY: BRN CHECKED BY: BR AT FULL SIZE, IF NOT ON

LAST UPDATED 6/12/2019 SHEET NUMBER:

GENERAL STRUCTURAL NOTES CONTINUED

10. DETAILS OF REINFORCEMENT

- Placement of all reinforcing steel within concrete structures shall be in conformance with ACI 318 Chapter 25.
- with ACI 318 Chapter 25.
 Reinforcing steel hooks, bends, ties, splices and other reinforcement details shall be in accordance with ACI 315; Details and Detailing of Concrete Reinforcement.

 Spacing limits for reinforcement shall be in conformance with ACI 318 Section 25.2.
 Concrete protection for reinforcement. Unless noted elsewhere on the drawings, all reinforcing steel shall have the following concrete cover:

 D.1. For non-liquid containing concrete structures; per ACI 318 Section 20.6.1.3:

 D.1.1. Concrete cast against earth: 3.00-inch
 D.1.2. Concrete exposed to earth, or weather:

 D.1.2.1. No. 5 or smaller bars: 1.50-inch
 D.1.2.2. No. 6 or larger bars: 2.00-inch
 D.1.3. Concrete not exposed to earth or weather;
 D.1.3.1. No. 11 or smaller bars: 0.75-inch
 D.1.3.2. No. 14 or larger bars: 1.50-inch
 D.1.3.3. No. 14 or larger bars: 1.50-inch
- - D.1.4. Beams and columns:
- D.1.4. Beams and columns:

 D.1.4.1. Primary reinforcement, ties, stirrups or spirals: 1.50-inch

 E. Concrete blocks or plastic-coated bar chairs shall be provided for support of all slab reinforcing steel, sufficient in number to prevent settlement or sagging, but in no case shall such support be continuous. Metal clips or supports shall not be placed in contact with the forms or the sub-grade.

 F. Dowels and anchor botts shall be wired or otherwise held in correct position prior to placing concrete. Care shall be taken to insure that dowels and anchor botts remain plum after concrete is poured and vibrated. In no case shall dowels or anchor botts be stabled into freshly poured concrete!
- be stabbed into freshly poured and violated.

 Provide dowels in footings and at construction joints to match vertical reinforcing bar size and spacing, unless otherwise noted on the drawings.

 Coordinate placement of dowels into masonry or brick walls with the masonry shop
- where drilled in anchors are to be post—installed into concrete surfaces take care to locate reinforcing steel so that it will not interfere with the drilling operations. Move bars plus or minus 1 to 2 inches in order to avoid drilling conflicts. All bar bends, hooks, splices and other reinforcing steel details shall conform to the requirements of ACI 315.
- Unless otherwise noted on the plans all bars shall be spliced with a minimum Class A lap splice; lap splices of deformed bars and deformed wire in tension zones shall be
- At all corners and wall intersections provide bent bars to match the horizontal
- reinforcing steel and in accordance with the typical corner reinforcing details.

 Chamfer all exposed corners and fillet entrant angles 3/4" unless otherwise noted on

11. STRUCTURAL MASONRY REQUIREMENTS

- A. GENERAL. All structural masonry construction shall be in accordance with ACI 530.1, Specifications for Masonry Structures; current edition.
- MASONRY: The masonry assemblage shall have a minimum 28 day compressive strength of 1,500 psi. Assembly shall be verified per IBC standards.

 STRUCTURAL MASONRY UNITS:

 C.1. CONCRETE MASONRY UNITS: All concrete masonry units (CMU) shall conform to
 - ASTM C-90, Grade N, with a minimum net area compressive strength of 1,900
 - All block shall be laid up with a standard running bond unless specifically noted otherwise on the drawings.
 - Place masonry units in accordance with ACI 530.1 Section 3.3, Masonry
- - All mortar for use with structural masonry units shall conform to ASTM C270, Class S and have a minimum 28 day compressive strength of 1,800 psi. Mortar shall be in accordance with ACI 530.1, Section 2.1 Mortar materials.
- E.1.
- All grout for use with structural mosonry units shall conform to ASTM C476 and have a minimum 28 day compressive strength of 2,500 psi. Grout shall be in accordance with ACI 530.1, Section 2.2 Grout materials. Place grout in accordance with ACI 530.1, Section 3.5 Grout placement. Grout Pour Height. Do not exceed the maximum grout pour height listed in ACI 530.1, Table 7.
- Grout Lift Height. Do not exceed the maximum grout lift heights as defined by ACI 530.1, Section 3.5 D.
 Fill all cells containing reinforcing steel and as directed on the drawings solid
- CELLS:
- full height with grout.

 BOND BEAMS: All bond beams shall be grouted solid to a minimum height of
- BOND BEAMS: All bond beans shall be grouted sold to thimbel.

 B-inches.

 LINTELS: All mosonry lintels (units over wall openings greater than 8-inches in length) shall be grouted solid from the bottom of the lintel to a total structural depth as indicated on the plans, or 16" minimum. Extend the length of solid grouting past the edge of each opening as indicated on the plans or 8" minimum.

 REINFORCING:

 1.1. REINFORCING STEEL: Reinforcing steel shall conform to ASTM A615 Grade 60;
- #3 bars may be Grade 40.
 - Fabricate bars used in masonry reinforcement in accordance with the fabricating tolerances of ACI 315, and in accordance with ACI 530.1, Section
- Place reinforcement in accordance with ACI 530.1, Section 3.4 B.
- Place reinforcement in accordance with ACI 530.1, Section 3.4 B. All reinforcing steel shall be in place and secured against displacement prior to grouting with wire ties, spacers or other suitable devices at tops and bottoms and intervals not exceeding 192 bar diameters nor 10-feet.

 BAR PLACEMENT: Where one vertical bar is called for in each vertical core the bar is to be placed in the center of the masonry core. Where two vertical bars are called for they shall be placed near each wall face with 1/5-inch of clearance for fine grout and 1/2' of clearance for course grout.

 LAPS: Lab all masonry reinforcing per bar size as follows:

 1.6.1. Required lap lengths for single bars centered in each cell:

 #3 = 16"
 #4 = 22"
 #7 = 60"
 #8 = 72"

 1.6.2. Required lap lengths for flush wall pilaster/column. 2 bars per cell with
- - 1.6.2. Required lap lengths for flush wall pilaster/column, 2 bars per cell with 5" cover: #3 = 16" #4 = 22" #5 = 32"
- #7 = 63" #8 = 72" J. ANCHOR BOLTS: Anchor bolts shall be accurately set with templates or by approved equivalent means and held in place to prevent movement. Conform to ACI 530.1,

- K. WALL TIES: Install wall ties in accordance with ACI 530.1, Section 3.4 C.
 L. FOUNDATION DOWELS: It is the Contractor's responsibility to coordinate placement of dowels projecting from concrete foundations into reinforced masonry or brick walls.
 M. Bond beams with one (1) #5 bars horizontally shall be provided at all floor and roof lines and at the top of walls. Provide a bond beam with one (1) #5 bars horizontally above and below all openings, and extend these bars 2'-0" past the opening edge. Provide full height vertical reinforcement, matching typical vertical reinforcing, each side
- of openings, at wall ends and intersections.

 N. COLD-WEATHER CONSTRUCTION. When ambient air temperature is below 40-degrees F, implement Cold Weather procedures in accordance with ACI 530.1, Section 1.8 C.

 O. FIELD QUALITY CONTROL: Provide special inspection and verification in accordance with
- P. CLEANING: Clean all exposed masonry surfaces in accordance with ACI 530.1, Section

12. WOOD

- LUMBER: Grading shall be to the Standard Grading Rules of the WWPA. Typical structural lumber shall be Number 2 Douglas—Fir/Larch or better. Members noted as wood beams, posts or columns shall be Number 1 Douglas—Fir/Larch or better. Studs for interior non-bearing walls may be stud grade lumber. Lumber to be left exposed, without other finish and lumber in contact with concrete shall be pressure
- treated.

 B. TREATED LUMBER: Lumber, including wood sheathing, to be left exposed without other finish, located within 8" of finish grade, or in contact with concrete shall be pressure treated material. Contractor shall coordinate and verify that all steel items in contact with the treated material, including steel hangars, connectors and fasteners have a galvanized finish of sufficient thickness, or other type of protection, that is compatible with the specific treatment type selected.

 C. BOLTS & LAG SCREWS FOR WOOD CONSTRUCTION: Conform to ANSI/ASME Standards B18.2.1-1981 and the National Design Specification for Wood Construction (NDS) 1991 Edition Part VIII for Bolts and Part IX for Screws.

 D. WOOD SCREWS: Conform to ANSI/ASME Standards B18.6.1-1981 and the National Design Specification for Wood Construction (NDS) 1991 Edition Part XI.

 E. NAILS & SPIKES: Conform to Federal Specification FF-N-105B and the National Design Specification (NDS) 1991 Edition Part XII.

 F. NAILING: Where not otherwise specified on the plans, nailing shall conform to IBC Table 2304.9.1. Fastening Schedule. All nails shall be common wire nails or

- NAILING: Where not otherwise specified on the plans, nailing shall conform to IBC Table 2304.9.1, Fastening Schedule. All nails shall be common wire nails or pneumatically driven nails with an equivalent cross—section and penetration, unless
- LUMBER HARDWARE: Wood construction connectors shall be as manufactured by Simpson Strong—Tie Company; current catalog, or an approved equal. Hardware exposed to weather or view, in unheated portions of the structure, or as indicated on the drawings or in the specifications shall be hot—dipped galvanized with galvanized
- ROOF SHEATHING: All roof sheathing shall be 15/32" nominal, Exterior APA rated Sheathing {32/16} installed with ply—clips.

 EXTERIOR WALL SHEATHING: All exterior wall sheathing shall be 1/2" nominal APA rated
- Exterior sheathing.

 All wood framing, blocking and nailing shall conform to the current local building code. All rafters, trusses and joists shall have full depth blocking, unless noted otherwise on the plans and details, at bearing supports, shear transfer supports, intermediate and cantilever supports and at mid-span, and as required by the building code or product
- L. All framing hardware including column caps and bases, joist hangers, truss anchors, straps, etc. shall be approved (i.e. Simpson Co. or equivalent) or custom fabricated specifically as detailed on the plans. They shall be installed with nails, screws or bolts exactly as called for by the manufacturer or as noted on the plans.

 M. WOOD SHEATHED ROOF DIAPHRAGMS:
- Unless otherwise noted on the drawings, orient roof sheathing with face—grain perpendicular to supporting members, with joints in adjacent rows staggered 1/2 panel length.
 - Provide 2x4 flat blocking at unsupported panel edges in areas noted as
- **Blocked Roof Diaphragm*.

 M.3. Nail sheathing per roof sheathing schedule shown on drawings.

 N. WOOD SHEATHED SHEAR WALLS:

 N.1. Shear wall sheathing to be oriented vertically.

 N.2. All unsupported edges to be backed with 2x solid blocking.

- Nail sheathing as shown on drawings.

 Minimum nailing where not noted otherwise shall be 10d nails @ 6" o.c. to all panel edges and 12" o.c. at intermediate supporting members.

13. PRE-ENGINEERED/FABRICATED WOOD TRUSSES

- A. All pre-engineered/fabricated wood (PFT) trusses indicated on the drawings shall be All pre-engineered/tobricated wood (PF) trusses indicated on the drawings shall be metal press-plate connected wood trusses designed by a Professional Engineer registered in the State of Utah per these notes and the specifications.

 B. Design PFT trusses to the following deflection limits:

 B.1. Roof Dead + Live Load: Span/240 or 1 in maximum

 B.2. Roof Live Load only: Span/360 or 1/2-in maximum

 C. Design PFT trusses to support the concentrated and other distributed loads as shown on the plans in addition to the following loads:

- - Dead Load (Top Chord) =
 Dead Load (Bottom Chord) =
 Snow Load* (Bottom Chord) =
 Live Load** (Bottom Chord) = 10 psf 10 psf
 - 41 psf
 - - 10 psf 61 psf Max Total Load
- Does not include loads caused by drifting, unblanced or sliding snow
 Does not occur concurrently with top chord live load
 D. Design all PFT trusses and bearing attachments for wind uplift, assuming a dead load
- of 8 psf to resist uplift.
- of 8 psr to resist uplift.

 Shop drawings and design calculations signed and stamped by the Design Engineer shall be submitted to the Engineer for review prior to fabrication.

 All necessary bridging, blocking, pre-notched or beveled plates, hangers, etc. shall be detailed or specified on the shop drawings and furnished by the truss manufacturer. Truss manufacturer shall verify all setbacks, dimensions, overhangs, vertical controls and dimensions prior to fabrication.
- Alteration of the truss layout shown on the plans may require supporting structural and foundation changes, therefore, prior approval by the Engineer is required for any
- proposed layout change.
 Trusses shall not be field modified without written authorization from the truss manufacturer's Engineer of Record.
- J. Trusses shall be handled, erected and braced as directed by the truss manufacturer and per the requirements of the Truss Plate Institute Manual HIB—91 or current



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ENGINEERS,

J-U-B

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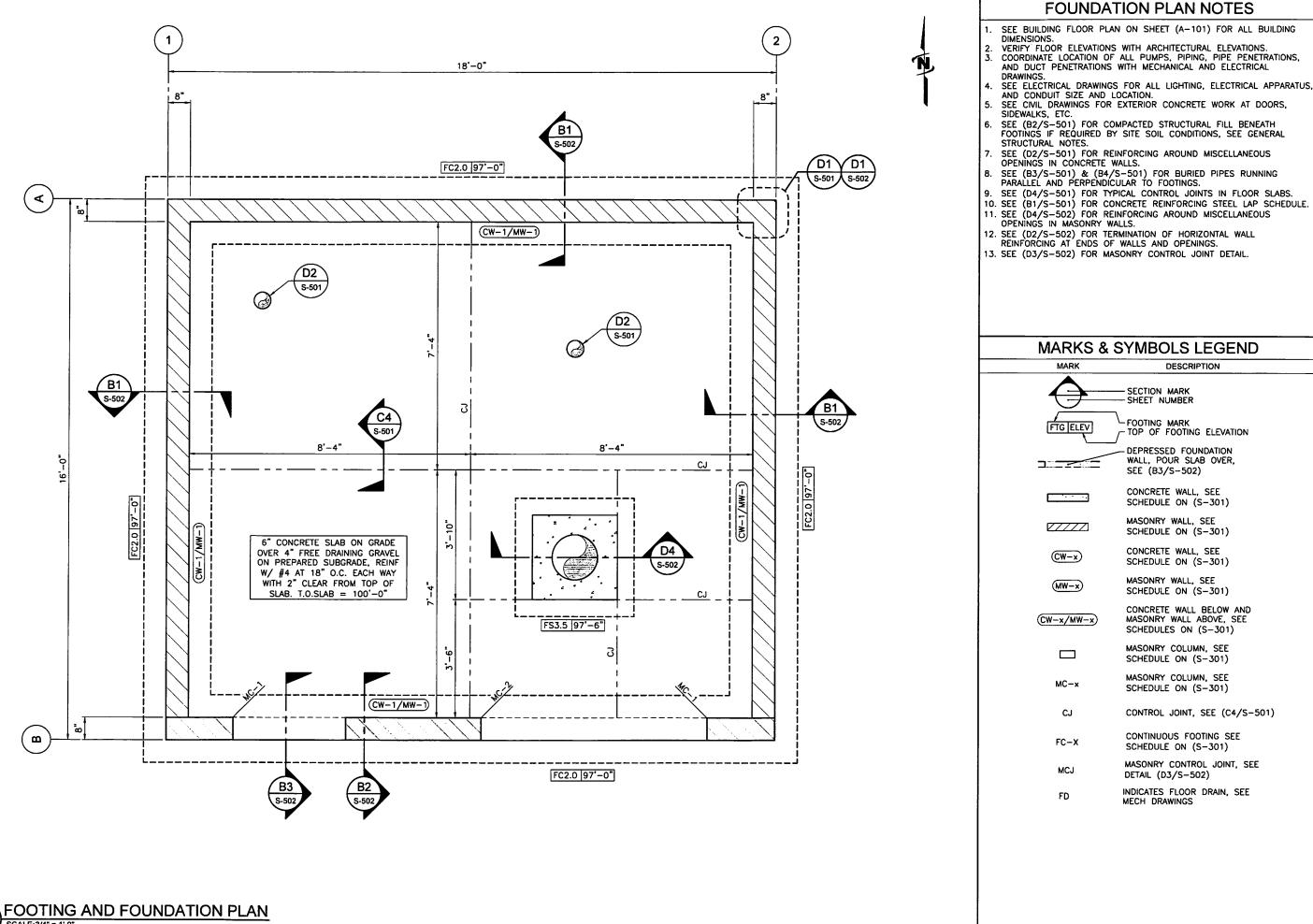
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DESIGN BY: BRN CHECKED BY: BR AT FULL SIZE. IF NOT ON INCH SCALE ACCOR LAST UPDATED 6/12/2019

SHEET NUMBER



FOUNDATION PLAN NOTES

SEE BUILDING FLOOR PLAN ON SHEET (A-101) FOR ALL BUILDING

VERIFY FLOOR ELEVATIONS WITH ARCHITECTURAL ELEVATIONS.

SEE CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS,

SEE (B2/S-501) FOR COMPACTED STRUCTURAL FILL BENEATH FOOTINGS IF REQUIRED BY SITE SOIL CONDITIONS, SEE GENERAL

SEE (D4/S-501) FOR TYPICAL CONTROL JOINTS IN FLOOR SLABS.

11. SEE (D4/S-502) FOR REINFORCING AROUND MISCELLANEOUS

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FOOTING AND FOUNDATION PLAN

SCHEDULES ON (S-301)

CONTROL JOINT, SEE (C4/S-501)

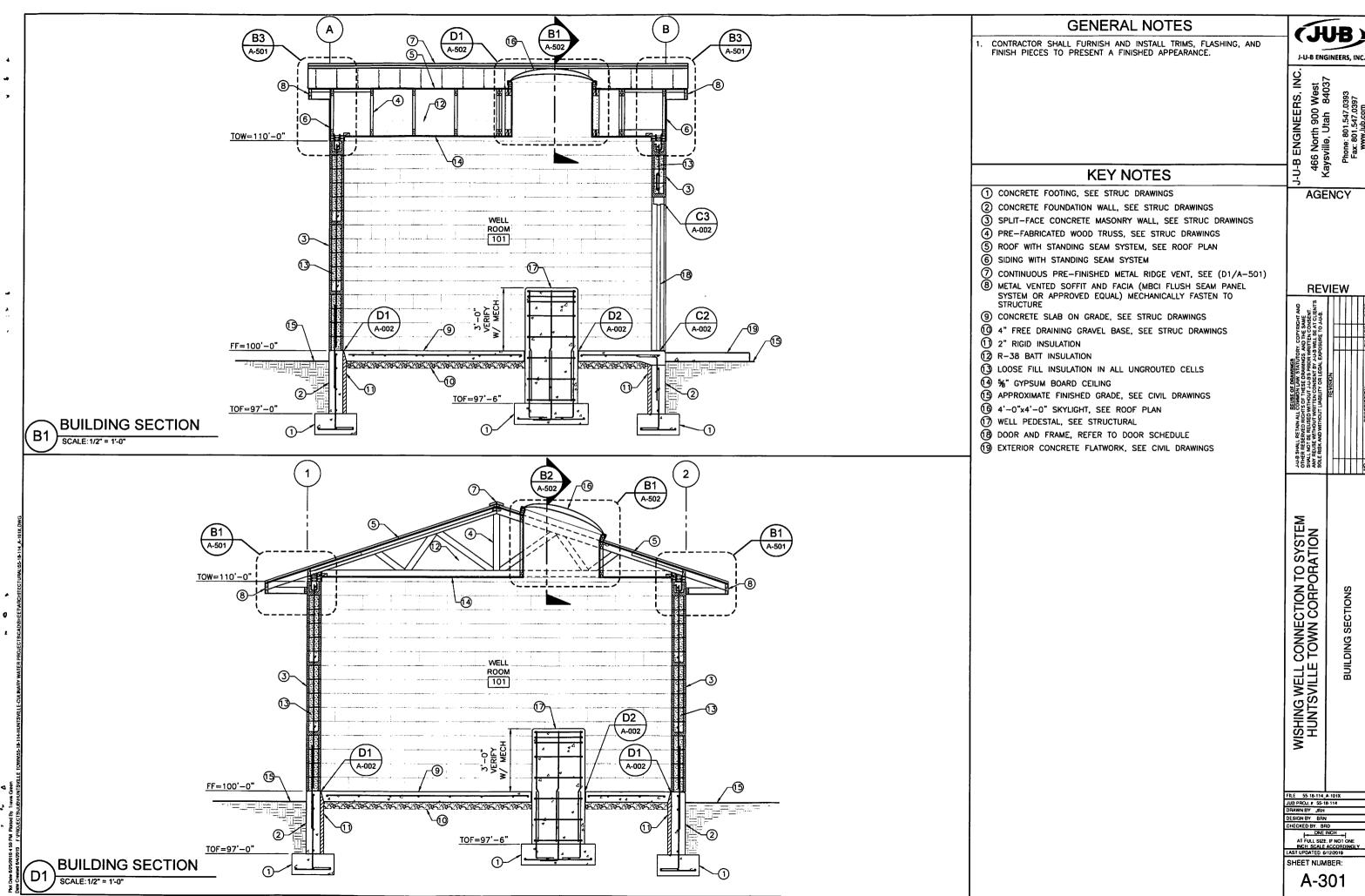
CONTINUOUS FOOTING SEE

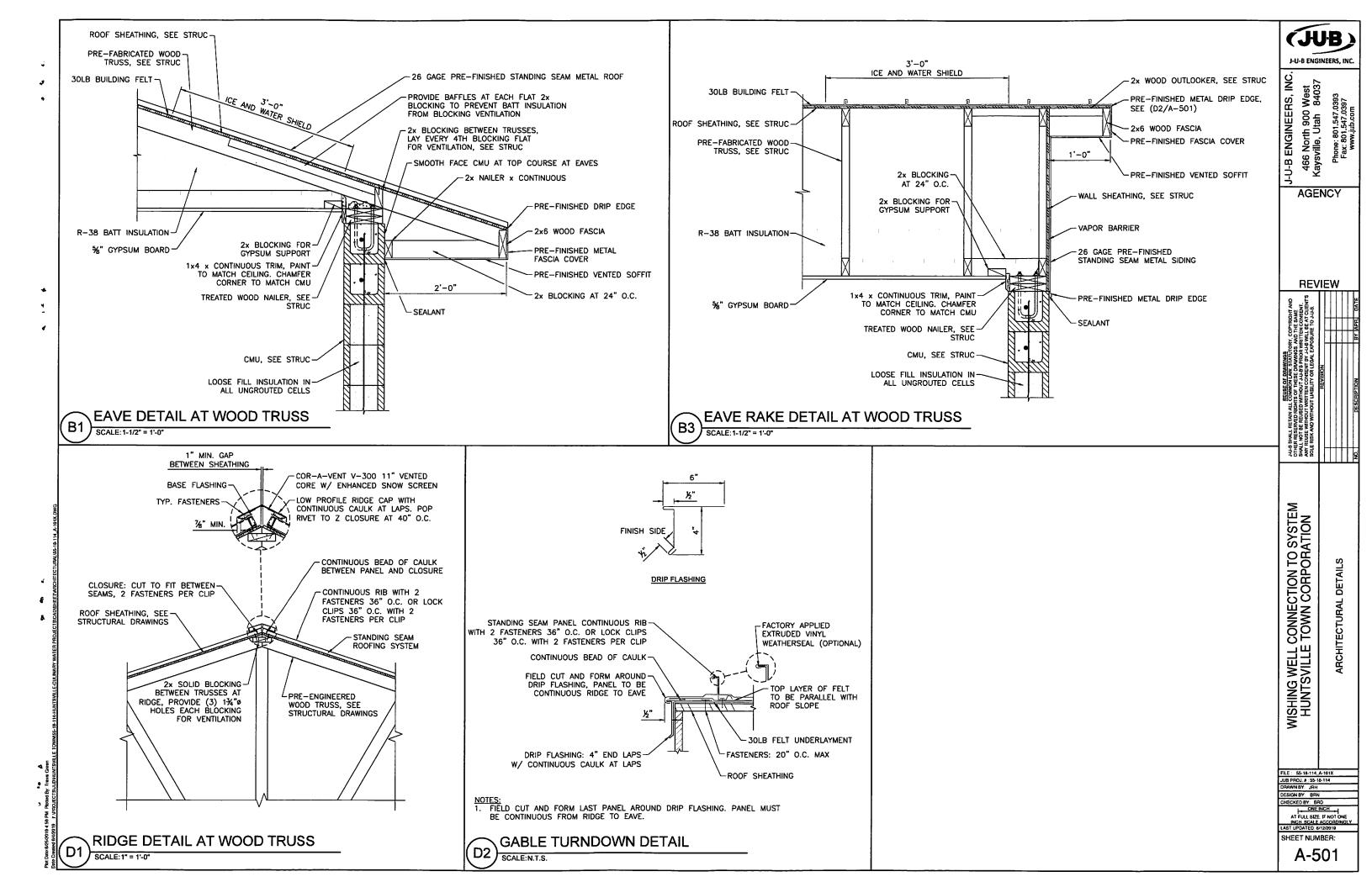
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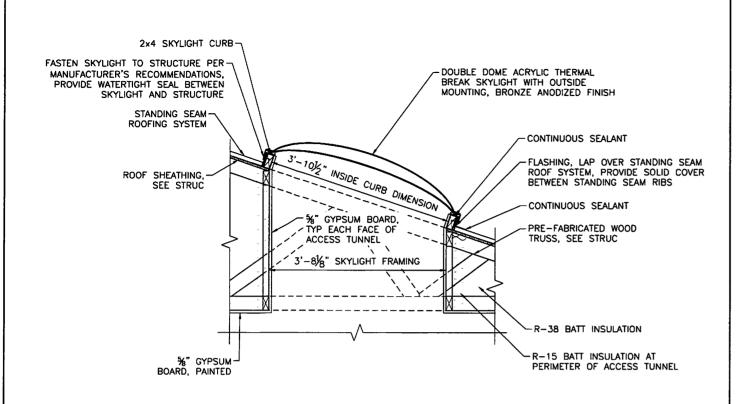
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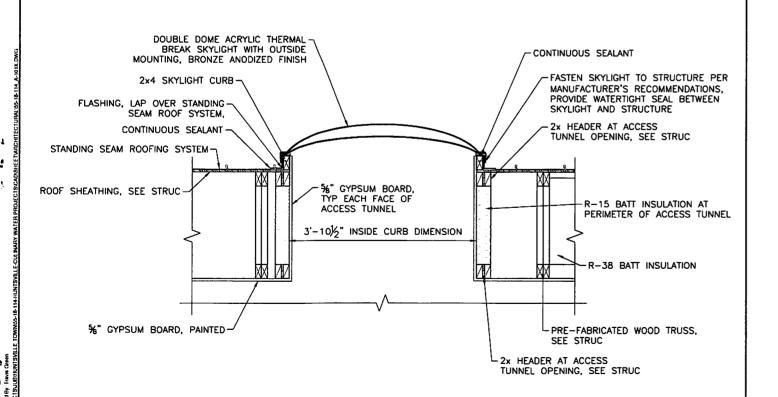
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ACCESS TUNNEL SECTION - PERPENDICULAR TO ROOF TRUSS **B**1 SCALE: 1" = 1'-0"



ACCESS TUNNEL SECTION - PARALLEL TO ROOF TRUSS

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ARCHITECTURAL DETAILS

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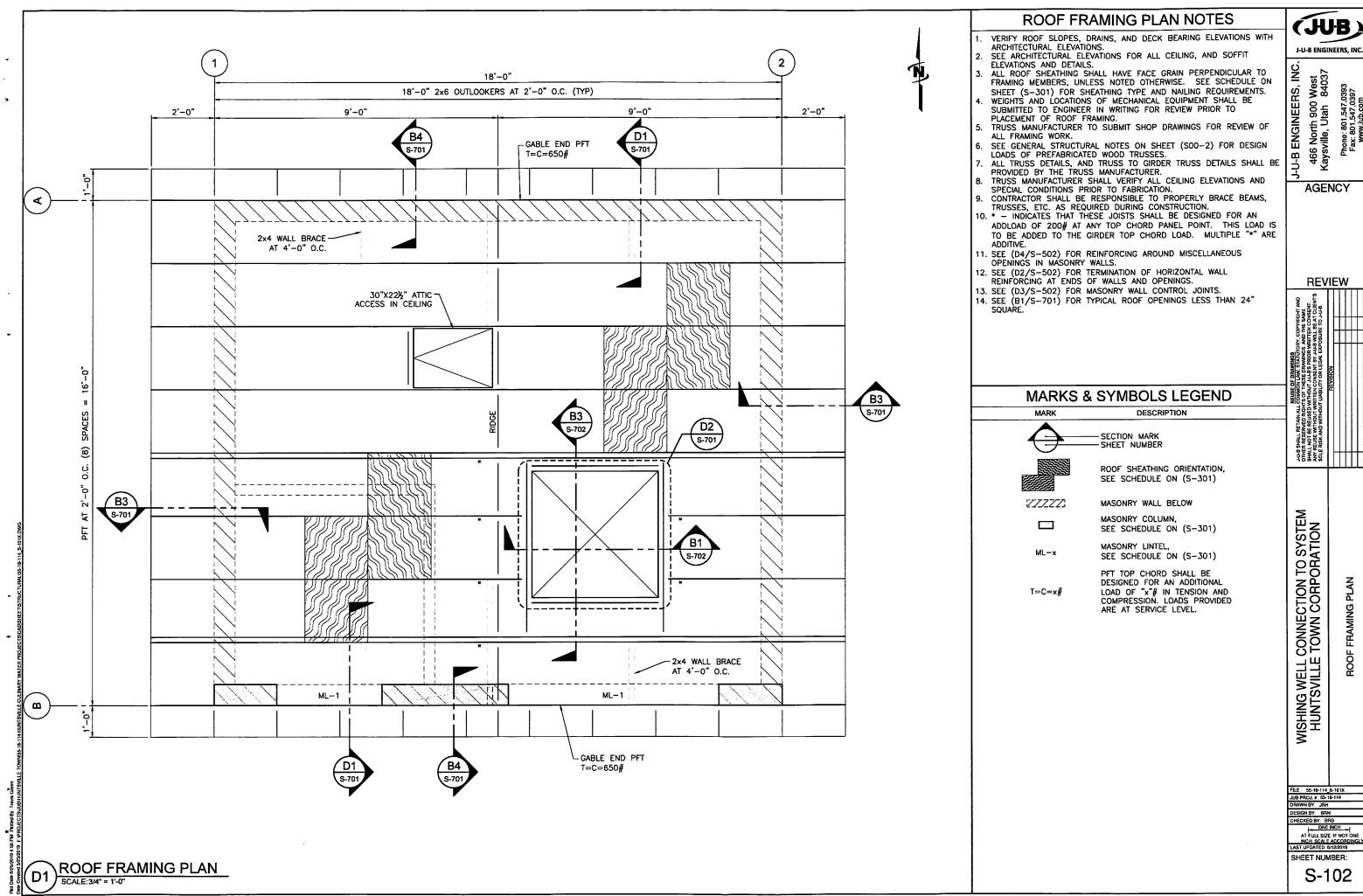
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LAST UPDATED 6/12/2019

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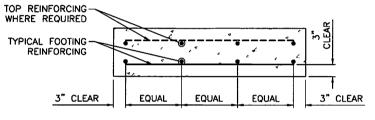
A-502



			CO	NC	RE	TE F	AITOC	1G	SC	HEDL	JLE		
FOOTING WIDTH LENGTH DEPTH REINFORCING CROSSWISE REINFORCING LENGTHWISE REMARKAN													
MARK	WIDTH	LENGIA	DEFIN	NO.	SIZE	LENGTH	SPACING	NO.	SIZE	LENGTH	SPACING	KEWAKKS	
FC2.0	2'-0"	CONT	12"		_	_	_	3	#4	CONT	EQ		
FC3.5	3'-6"	3'-6"	12"	4	#5	3'-0"	EQ	4	#5	3'-0"	EQ		

CONCRETE FOOTING NOTES:

- PLACE ALL FOOTING REINFORCING IN BOTTOM OF FOOTING WITH 3" CLEAR CONCRETE COVER UNLESS OTHERWISE NOTED.
- TOP REINFORCING, WHERE SPECIFIED, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
- IF FOOTINGS ARE EARTH FORMED, FOOTING WIDTH AND LENGTH SHALL BE 6" WIDER AND
- NOT ALL FOOTINGS ARE USED, SEE FOUNDATION PLAN FOR FOOTING MARKS
- RUN CONTINUOUS BARS IN "FC" FOOTING THROUGH INTERSECTED "FS" FOOTINGS.
- SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.
- CENTER SPOT FOOTINGS AT COLUMN LOCATIONS.



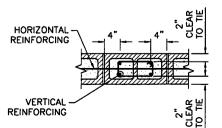
TYPICAL FOOTING SECTION

CONCRETE FOOTING SCHEDULE **B**1 SCALE: NOT TO SCALE

	MA	SONRY	COLUM	N SCHEDULE	
COLUMN	COLUMN				
MARK	SIZE	VERTICAL	TIES	CONFIGURATION	REMARKS
MC-1	8"x24"	(3) #5	#4 AT 8"oc	6	
MC-2	8"x48"	(6) #5	#4 AT 8"oc	• • • • • •	

MASONRY COLUMN NOTES:

- THE CENTERLINE OF VERTICAL BARS SHALL BE LOCATED 23" FROM THE FACE OF THE MASONRY. HORIZONTAL WALL REINFORCING SHALL BE LOCATED TO THE INSIDE OF THE VERTICAL BARS. UNLESS NOTED OTHERWISE, VERTICAL REINFORCING AND TIES SHALL EXTEND TO FULL WALL
- VERTICAL MASONRY COLUMN REINFORCING SHALL EXTEND INTO THE FOOTING AND TERMINATE
- WITH A STANDARD 90' HOOK.
 IN CONCRETE FOUNDATION WALLS, VERTICAL MASONRY COLUMN REINFORCING SHALL BE TIED WITH #3 TIES AT THE SAME SPACING AND CONFIGURATION AS MASONRY COLUMNS ABOVE.
 SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.



SCHEMATIC MASONRY COLUMN CONFIGURATION

MASONRY COLUMN SCHEDULE SCALE:NOT TO SCALE

	(CONCRETE	WALL SCHE	DULE	
WALL	THICKNESS	REINFO	ORCING	WALL	REMARKS
MARK	ITICKNESS	VERTICAL	HORIZONTAL	TYPE	KEWIAKKS
CW-1	8"	(1) #5 AT 16"oc	(1) #4 AT 12"oc	Α	
	_				

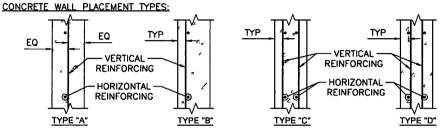
CONCRETE WALL NOTES:

SEE GENERAL STRUCTURAL NOTES FOR REQUIREMENTS NOT SHOWN IN SCHEDULE.
 CONCRETE WALLS NOT DESIGNATED ON THE PLANS SHALL BE REINFORCED AS FOLLOWS:

VERTICAL REINFORCING	HORIZONTAL REINFORCIN
#4 AT 18"oc	#4 AT 16"oc
#4 AT 18"oc	#4 AT 12"oc
#4 AT 16"oc	#5 AT 15 " oc
#4 AT 18"oc EACH FACE	#4 AT 16"oc EACH FAC
	#4 AT 18"oc #4 AT 18"oc #4 AT 16"oc

PLACE STEEL IN THE CENTER OF THE WALL (EXCEPT TYPE "B" AND RETAINING WALLS). WALLS THICKER THAN 10" SHALL HAVE TWO CURTAINS OF REINFORCEMENT (PLACED NEAR EACH FACE OF THE WALL) UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.

TYPICAL HORIZONTAL BAR SHALL BE PLACED AT THE BOTTOM OF THE WALL (NEAR THE FOOTING) AT EACH FLOOR LEVEL, AT THE ROOF LEVEL, AND AT THE TOP OF WALL.



CONCRETE WALL SCHEDULE

SCALE:NOT TO SCALE

	N	MASO	NRY LI	NTEL SCHED	ULE
l	LINTEL	LINTEL	LINTEL	REINFORC	ING
1	MARK	DEPTH	MAXIMUM	HORIZONTAL	STIRRUPS
	ML-1	1'-4"	7'-0"	(1) #5 BAR CONT TOP & BOTTOM	#4 AT 8" O.C.

MASONRY LINTEL NOTES:

- LINTEL WIDTH AND MATERIAL TYPE SHALL BE THE SAME AS THE WALL IN WHICH THE LINTEL IS CONSTRUCTED.

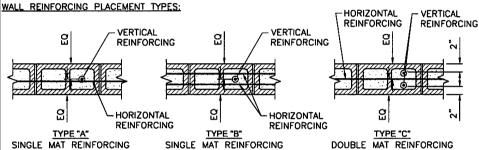
 GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL
- OR COLUMN AT EACH END.
- MASONRY LINTELS ML-1 SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. WHEN A LINTEL IS SPECIFIED ON THE PLANS, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SPECIFIED ON THE PLANS WHICH HAVE A SPAN GREATER THAN 7'-0".
- EXTEND ALL HORIZONTAL REINFORCING 48 BAR DIAMETERS MINIMUM BEYOND THE EDGE OF ALL OPENINGS. IF HORIZONTAL REINFORCING CANNOT EXTEND 48 BAR DIAMETERS BEYOND EDGE OF OPENING, PROVIDE 90° STANDARD HOOK.
- SPLICE TOP BARS AT MID-SPAN OF LINTEL ONLY AND BOTTOM BARS OVER SUPPORTS ONLY.
- HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING
- DOWEL VERTICAL REINFORCING OF WALL ABOVE LINTEL INTO THE FULL DEPTH OF LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.

MASONRY LINTEL SCHEDULE

		MA	ASONF	RY WALL SC	HEDULE		
WALL	THOMESO	ťm	SOLID		REINFORCING		SPECIAL
MARK	THICKNESS	(psi)	GROUT	VERTICAL	HORIZONTAL	TYPE	INSPECTION
MW-1	8"	1500	NO	(1) #5 AT 32"oc	(1) #5 AT 40"oc	Α	YES

MASONRY WALL NOTES:

- DO NOT SOLID GROUT WALLS UNLESS NOTED OTHERWISE.
- INSTALL LOOSE FILL INSULATION IN ALL UNGROUTED CELLS WHERE NOTED.
- ALL MASONRY BELOW GRADE SHALL BE SOLID GROUTED.
- VERTICAL REINFORCING SHALL BE CENTERED IN THE WALL UNLESS NOTED OTHERWISE. (1) VERTICAL BARS MINIMUM AT ALL CORNERS AND END OF WALLS.
- HÓRIZONTAL WALL REINFORCING SHALL BE PLACED BETWEEN VERTICAL MASONRY COLUMN REINFORCING BARS
- HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
- SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.

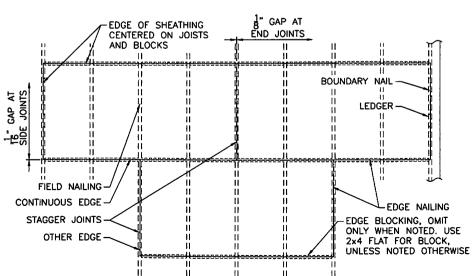


MASONRY WALL SCHEDULE SCALE: NOT TO SCALE

ROOF SHEATHING SCHEDULE **EDGE NAIL** WOOD SHEATHING FIELD **BOUNDARY** EDGE LOCATION NAIL SIZE NAIL CONT EDGE OTHER EDGE TYPICAL 15/32" (32/16) 8d 6"oc 6"oc 12"oc 6"oc

ROOF SHEATHING NOTES:

- MINIMUM NAIL PENETRATION INTO FRAMING: $8d-1\frac{1}{2}$ ", $10d-1\frac{1}{8}$ " USE COMMON NAILS (8d DIAMETER=0.131", 10d DIAMETER=0.148")
- SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.



ROOF SHEATHING SCHEDULE

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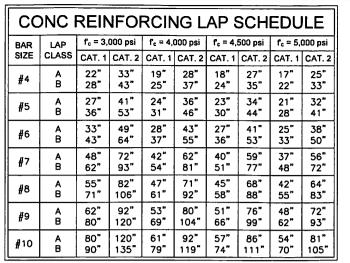
CONNECTION TO SYSTE TOWN CORPORATION

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JUB PROJ. # : 55-18-114 DRAWN BY: JRH DESIGN BY BRN

CHECKED BY BRD
ONE INCH
AT FULL SIZE. IF NOT ONE
INCH SCALE ACCORDING
LAST UPDATED 6/12/2019

SHEET NUMBER:



CONCRETE REINFORCING LAP NOTES: 1. FOR GRADE 60 REINFORCING BARS

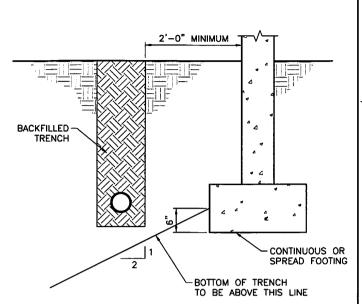
ALL LAP SPLICES SHALL BE CLASS B, UNLESS NOTED OTHERWISE CATEGORY 1: CLEAR COVER >= db & CLR. SPACING >= db, AND STIRRUPS OR TIES THROUGHOUT Ld ARE PROVIDED.

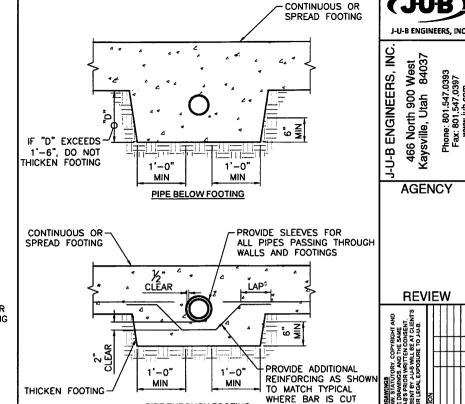
CATEGORY 1: CLEAR COVER >= db & CLR. SPACING >= 2db.

CATEGORY 2: CLEAR COVER < db OR CLR. SPACING < 2db.

FOR TOP BARS MULTIPLY LAP LENGTH LISTED BY 1.30. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST

"B" CONTINUOUS OF SPREAD FOOTING STRUCTURAL FILL SEE GENERAL STRUCTURAL NOTES "B"+"D" MINIMUM

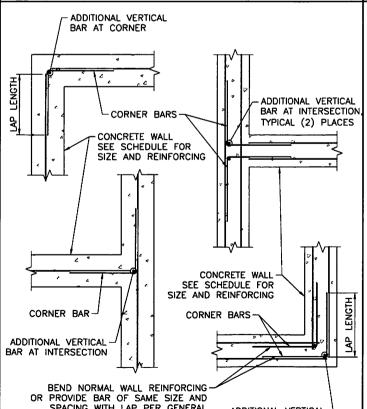




CONCRETE

REINFORCING LAP SCHEDULE

SCALE:NOT TO SCALE



CORNER REINFORCEMENT DETAIL FOR CONCRETE WALLS

SCALE: NOT TO SCALE

TYPICAL COMPACTED STRUCTURAL FILL **B2** SCALE:NOT TO SCALE

PIPE PARALLEL TO FOOTING DETAIL

STEEL REINFORCEMENT -2"-0"Ø MAX CUT BAND "B" 1) #5x4'-0" DIAGONAL AT EACH CORNER, EACH FACE (1) #5 HOOP DIAMETER OF ÓPËNING +8", EACH FACE REINFORCEMENT CUT BAND "A"

AREA OF BARS (OR OF BARS) EQUAL BAND "B" BARS CUT AREA OF BARS (OR # OF BARS) EQUAL BAND "A" BARS CUT

AREA OF BARS (OR # OF BARS) EQUAL BAND "A" BARS CUT

CIRCULAR OPENINGS

SQUARE/RECTANGULAR OPENINGS

MAX

- GENERAL NOTES:
 1. TYPICAL FOR ALL OPENINGS IN CONCRETE WALLS AND SLABS UNLESS INDICATED OTHERWISE ON PLANS.
- COORDINATE PLACEMENT OF ALL PIPING AND REINFORCING STEEL SO THAT NO CONTACT EXISTS BETWEEN TWO METALS.
- DO NOT WELD REINFORCEMENT TO PIPE SLEEVES, INSERTS OR EMBEDMENTS
- PROVIDE A MINIMUM OF TWO (2) "A" BARS AND TWO (2) "B" BARS EACH SIDE OF OPENING
- SPACE BARS AT 3 BAR DIAMETERS (OR 3" MIN) ON CENTER.
- IF OPENING REINFORCING TERMINATES AT THE EDGE OF THE SLAB PROVIDE A STANDARD HOOK ON THE "EDGE" SIDE OF THE REINFORCING.
- CONTINUE SPLICE REINFORCEMENT THROUGH ONE SPAN LENGTH PLUS REQUIRED EMBEDMENT
- 8. AT CIRCULAR OPENING PIPE PENETRATIONS, CONTRACTOR SHALL CAST PIPE IN SEEP RING.

TYPICAL OPENING REINFORCING DETAIL

INSTALL BACKER ROD IMMEDIATELY AFTER SAWCUTTING JOINTS BARS CONTINUOUS THRU PERFORM "T" QUICK JOINT -OR SAWCUT WITHIN 24 HOURS OF POUR, USE TOOLED JOINTS AT EXTERIOR SLABS

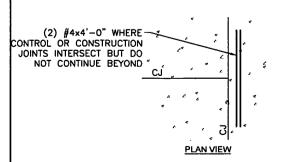
PIPE THROUGH FOOTING

PERPENDICULAR TO FOOTING DETAIL

PIPE

SCALE: NOT TO SCALE

TYPICAL CONTROL JOINT DETAIL



ADDITIONAL SLAB REINF LOCATION

ONE INCH AST UPDATED 6/12/2019

CONNECTION TO SYSTEI TOWN CORPORATION

JUB

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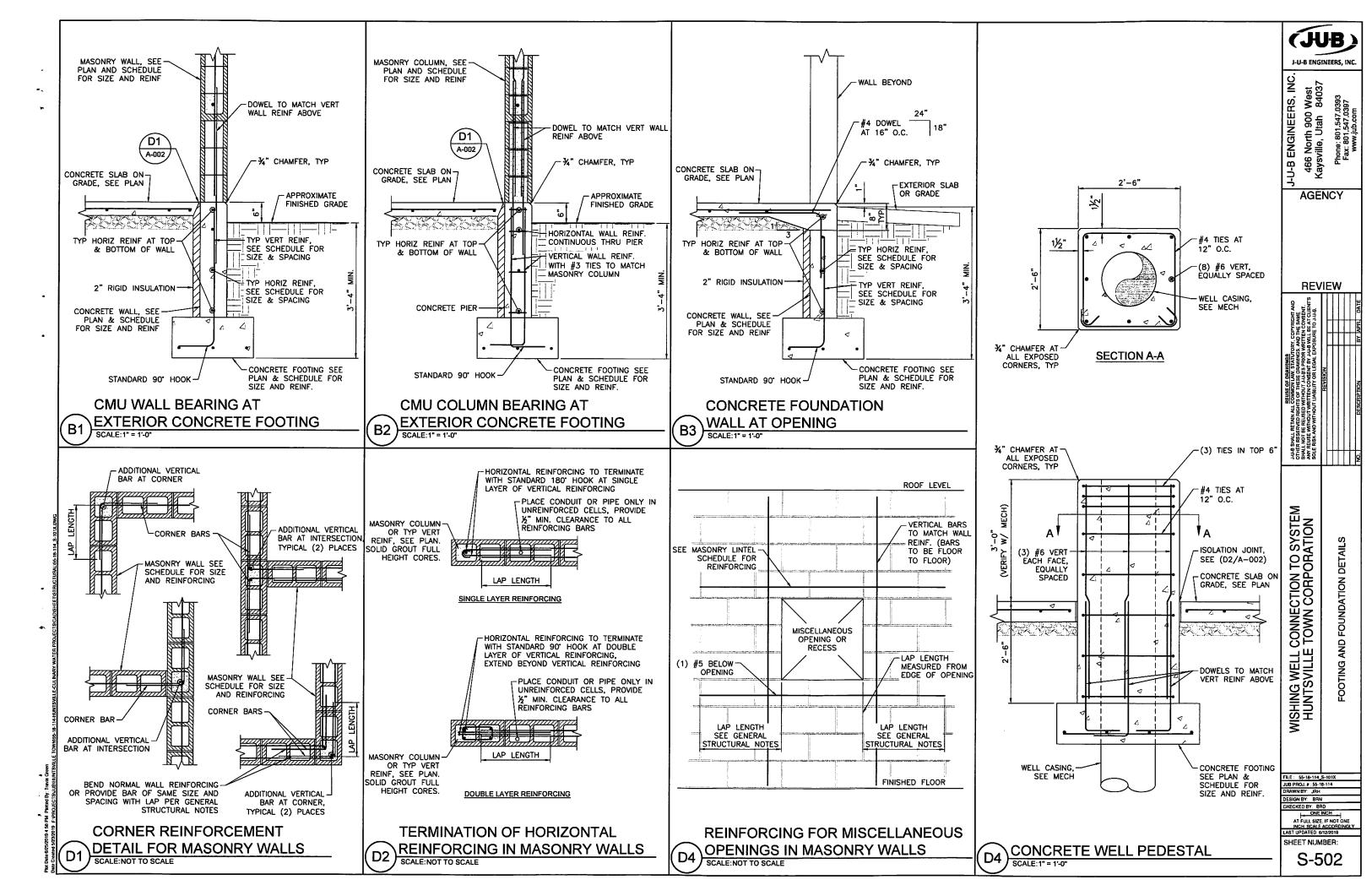
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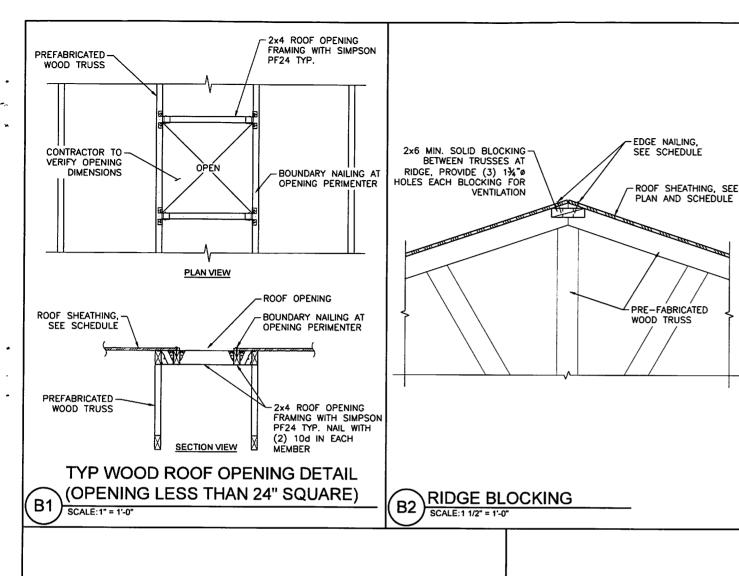
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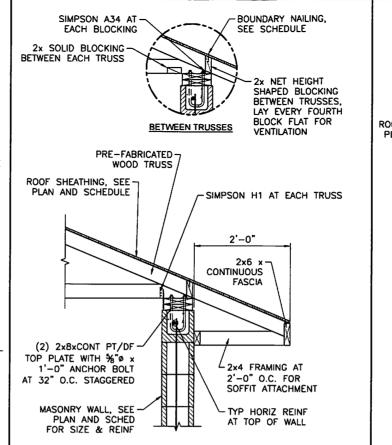
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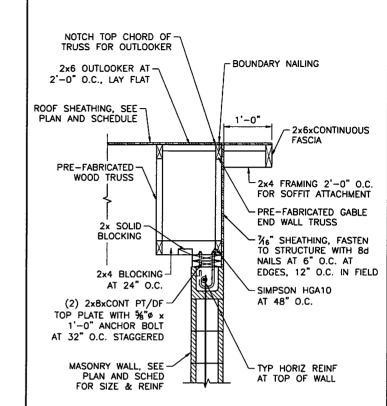
SPACING WITH LAP PER GENERAL ADDITIONAL VERTICAL -STRUCTURAL NOTES BAR AT CORNER, TYPICAL (2) PLACES



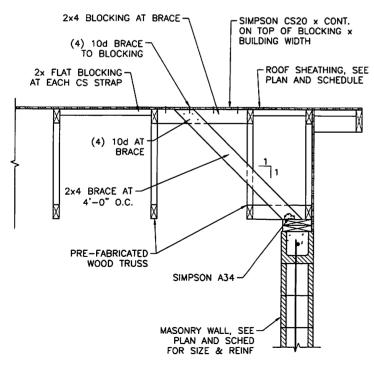


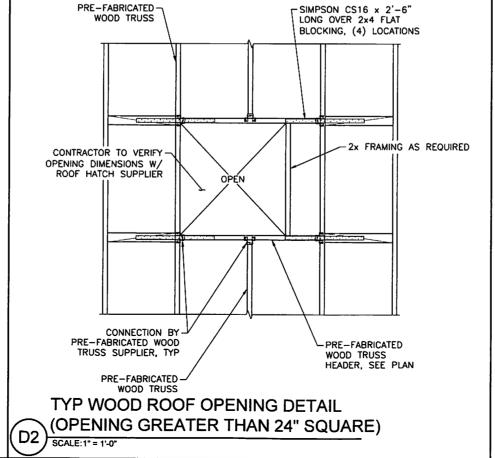


PFT BEARING AT CMU WALL



GABLE END PFT AT CMU WALL





MISHING WELL CONNECTION TO SYSTEM

WISHING WELL CONNECTION

WISHING WELL CONNECTION

WISHING WELL TOWN CORPORATION

WHO SYNTHMS BAN

THE ROLL BOWN

WHO SHE HAD NOT ONE WAY RELIEVE

WHO SHE SOLE RISK.

AND MAN RELIEVE

WHO SHE HAD NOT ONE WAY RELIEVE

WHO SHE HAD NOT ONE WAY RELIEVE

WHO SHE HAD NOT ONE WAY RELIEVE

SHEET NUMBER:

SOLE RISK.

TO 1

J-U-B ENGINEERS, INC.

466 North 900 West Kaysville, Utah 84037

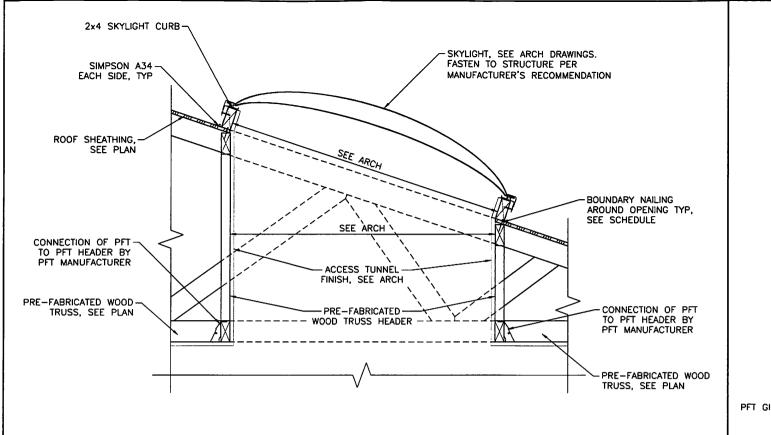
AGENCY

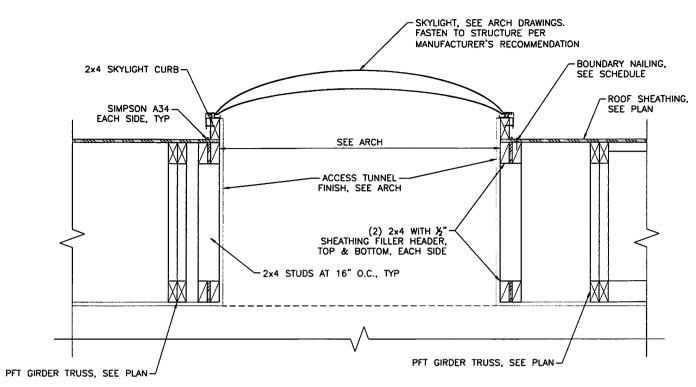
REVIEW

ENGINEERS,

.U-B

D1) CMU WALL BRACE AT GABLE END





ACCESS TUNNEL SECTION - PERPENDICULAR TO ROOF TRUSS (B1

ACCESS TUNNEL SECTION - PARALLEL TO ROOF TRUSS SCALE:1 1/2" = 1'-0"

WISHING WELL CONNECTION TO SYSTEM HUNTSVILLE TOWN CORPORATION

ROOF FRAMING DETAILS

J-U-B ENGINEERS, INC.

Phone: 801.547.0393 Fax: 801.547.0397 www.iub.com

J-U-B ENGINEERS, INC. 466 North 900 West Kaysville, Utah 84037

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REVIEW

REUSE OF DRAWINGS

JULS SHALL RETAIN ALL CORNORIN LAWS STATIONS, COPYRIGHT AND OTHER RESERVED RIGHTS OF THESE DRAWINGS AND THE SAME SHALL NOT BE REUSED WITHOUT LAUES PRIOR WATTER COASEEN. SAME, INTO TE RELESE WITHOUT LAUES THOUGHT BY JULS WILL BE AT CLUEN SOLE RISK AND WITHOUT LIABILITY OR LECAL EXPOSURE TO JULB.

REVISION

FILE: 55-18-114_S-101X JUB PROJ. #: 55-18-114 DRAWN BY: JRH DESIGN BY BRN CHECKED BY SHN

CHECKED BY SHD

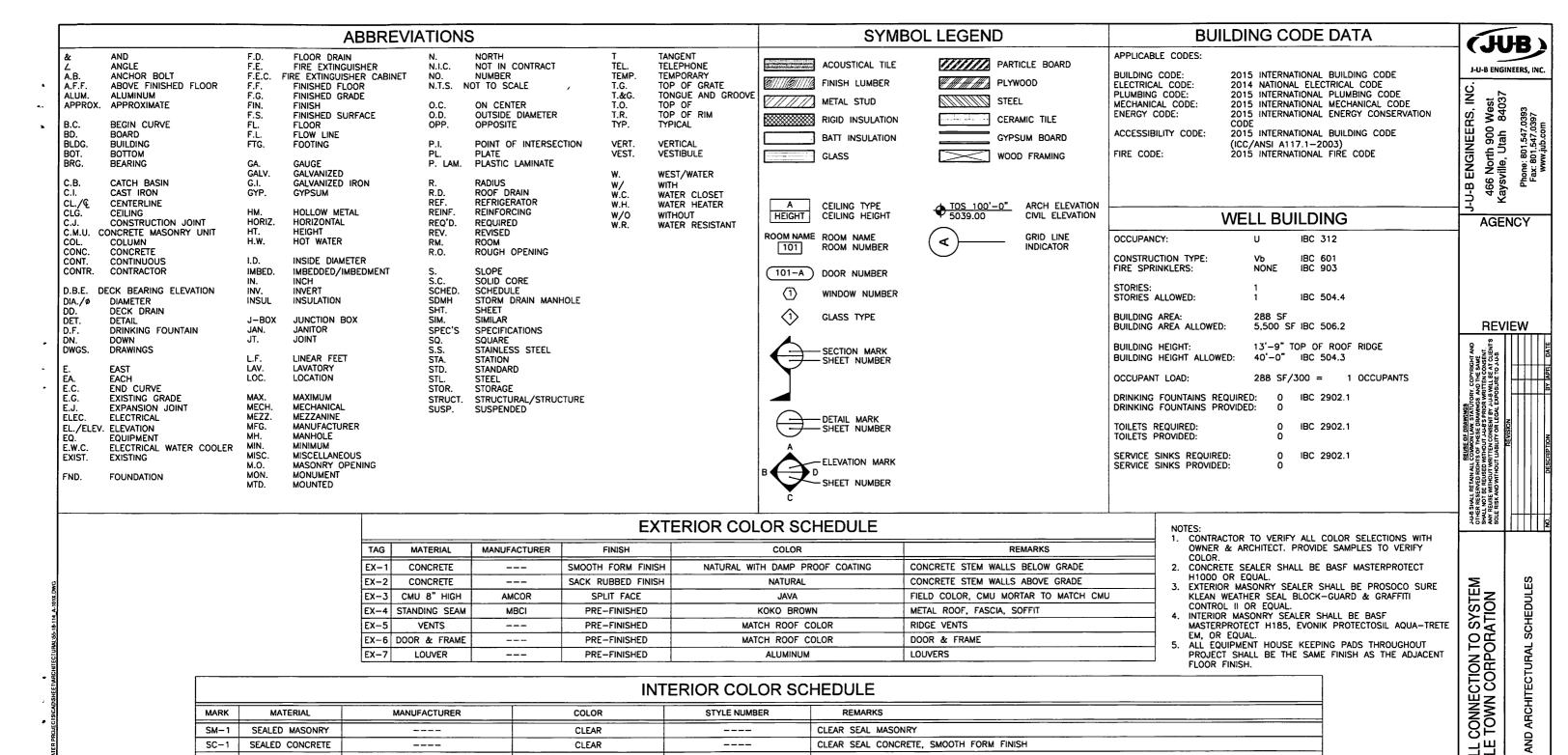
ONE INCH

AT FULL SIZE IF NOT ONE

INCH SCALE ACCORDING!

LAST UPDATED 6/12/2019

SHEET NUMBER:



		EX-7 LOUVER	PRE-FINISHED	ALUMI	NOW LUDVERS	FLOOR FINISH.
			INT	ERIOR COLOR S	SCHEDULE	
MARK	MATERIAL	MANUFACTURER	COLOR	STYLE NUMBER	REMARKS	
SM-1	SEALED MASONRY		CLEAR		CLEAR SEAL MASONRY	
SC-1	SEALED CONCRETE		CLEAR		CLEAR SEAL CONCRETE, SMOOTH FORM FINISH	
P1	PAINT	SHERWIN WILLIAMS	MATCH ROOF COLOR	SEMI-GLOSS	DOORS & JAMBS	
P2	PAINT	SHERWIN WILLIAMS	WESTHIGHLAND WHITE SW7566	SEMI-GLOSS	GYPSUM BOARD WALLS & CEILINGS	

WILE TO								FI	NISH AND	FLOOR S	SCHEDULE	:						
HUNTS	ROOM NUMBER									W	ALLS, WAINSCOTS, I	BASES, DOORS						
S	UMBER	ROOM NAME	FLOOR	NORTH				EAST			SOUTH			WEST		CEILING		DOOR
DECTS.	[000]			WALL	WAINSCOT	BASE	WALL	WAINSCOT	BASE	WALL	WAINSCOT	BASE	WALL	WAINSCOT	BASE	TYPE	HEIGHT	JAMBS
F P.B.	101	WELL ROOM	SC-1	SM-1	-	SM-1	SM-1	-	SM-1	SM-1		SM-1	SM-1	-	SM-1	GYP BD P2	10'-2"	P1
£ _		-																
ted 6/4/2019					1								l					

SHEET NUMBER A-001

IISHING WELL CHUNTSVILLE

₹

FILE 55-18-114 A-101X JUB PROJ. #: 55-18-11 DRAWN BY: JRH HECKED BY: BI AT FULL SIZE. IF NOT ON LAST UPDATED: 6/12/20

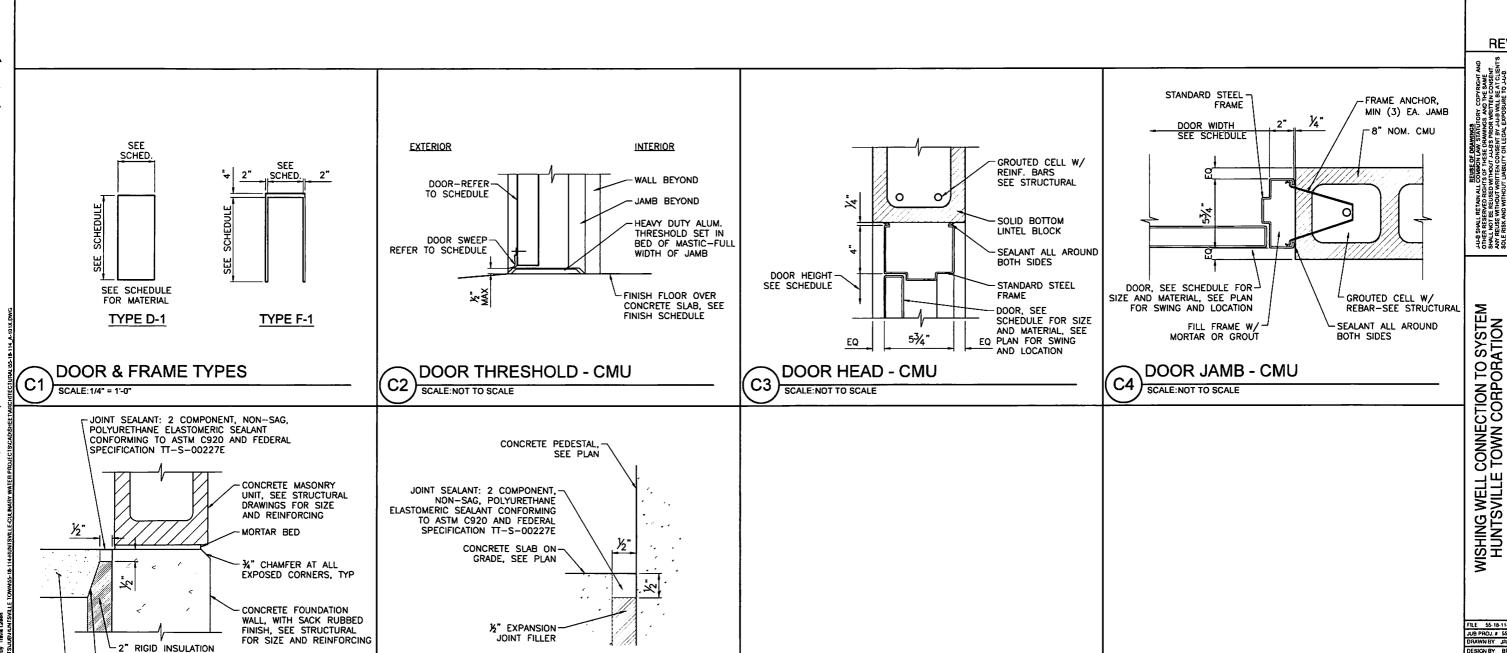
BUILDING CODE

											DO	OR S	CHEDU	LE										_									
		DOOR SIZ	<u></u>				T	OPENING	DETAILS			FRAME	DETAILS			T	Н	NGES			1	LOC	KSET	rs'			STO	PS	DOOR	SEAL	МІ	SCELLA	NEOUS
TAG 000-X	МПТН	НЕІСНТ	THICKNESS	DOOR TYPE	DOOR MATERIAL	DOOR FINISH	HEAD	RIGHT JAMB	LEFT JAMB	SILL THRESHOLD	TYPE	MATERIAL	FINISH	FIRE RATING	COMMENTS	NUMBER	[A Z	BRONZE / BRASS	STAINLESS	STEEL N.R. PINS	ENTRANCE	OFFICE	PRIVACY	STORE ROOM	DUST PROOF	SIRIKE	BOLTS WALL STOP	SMOKE STRIP	WEATHER STRIP	SWEEP	CLOSER	COORDINATOR	ARMOR PLATE
101-A	3'-2"	7'-0"	1 3/4"	D-1	INSULATED METAL	P1	C3/A-002	C4/A-002	C4/A-002	C2/A-002	F-1	НМ	P1	T	PAIR OF DOORS	6	X		X	×	TT			x x	X				X X	(x)	(x	x	X 3
102-B	3'-0"	7'-0"	1 3/4"	D-1	INSULATED METAL	P1	C3/A-002	C4/A-002	C4/A-002	C2/A-002	F-1	НМ	P1			3	Х		X	X				X X					x	x >	(X	$\perp \perp$	X)

- NOTES:

 1. SECURE ALL DOOR HARDWARE SCREWS WITH "LOC-TIGHT" COMPOUND.

 WATCH OWNERS MASTER KEYS AND CYLINDER CORE, COORDINATE WITH CONSTRUCTION MANAGER.



ISOLATION JOINT DETAIL

SCALE:6" = 1'-0"

J-U-B ENGINEERS, INC.

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AGENCY

REVIEW

DITER RESERVED RICHT OTHER RESERVED RICHT SHALL NOT BE REUSED WA ANY REUSE WITHOUT WA SOLE RISK AND WITHOUT

CONNECTION TO SYSTEI TOWN CORPORATION

DOOR SCHEDULE AND DETAILS

FILE SS-18-114 A-101X JUB PROJ. # 55-18-114 DRAWN BY JRH DESIGN BY: BRN CHECKED BY: BRD
ONE INCH
AT FULL SIZE. IF NOT ONE
INCH. SCALE ACCORDING

LAST UPDATED: 6/12/2019

SHEET NUMBER: A-002

- BEVEL INSULATION

CMU AT FOUNDATION WALL

CONCRETE SLAB ON GRADE

SCALE: NOT TO SCALE

