Weber County Building Permit Application

Incomplete applications will not be accepted or held. All required items shall be submitted.

Date Submitted /Completed	Fees *(Office Use)*	Receipt Number *(Office Use)*

Property Owner Contact Information

Name of Property Owner(s)			
WOLF CREEK UTAH			
Phone	Email (Required)		
	kevine wolfcreek resort.com		

Authorized Representative Contact Information

Name of Person Authorized to Represent the Property Owner(s)		
KEVIN HILL		
Phone	Email(Required)	
801 430-8998	kevinewolfcreek resort.com	

Property Information

Property Address	Land Serial Number		Current Zoning
3900 N WOLF CREEK DR	22-016-0074		CV-2
Subdivision Name	Lot Number	Acreage	Frontage
	N/A	4,470 SF	N/A
Culinary Water Provider	Secondary Water Provider		Waste Water Provider
WCWSID	WCWSID		WCWSID

Detailed Description of Proposed Use/Structure

SEE ATTACHED DESIGN REVIEW NARRATIVE
8/25 Enclosure for Bistro & Engineering

Contractor Information

Architect or Engineer: BERTOLDI ARCHITECTS	Phone Number:
General Contractor: LEWIS HOMES	Contractor's Address: 3718 N WOLF CREEK DR. EDEN, UT 84310
Phone number: (801) 389-0040	State License: 322678-5501
Electrical Contractor: J & J ELECTRIC	Contractor's Address:
Phone number: (801) 458-3452	State License:
Plumbing Contractor: BROWN'S QUALITY PLUMBING	Contractor's Address:
Phone number: (801) 940-7711	State License:
Mechanical Contractor: MORRELL & SONS	Contractor's Address:
Phone number: (801) 430-3187	State License:

Signature of Property Owner or Authorized Representative

I (We), KEVIN D. HILL, depose and say that I (we) am (are) the owner(s) or authorized representative of the property identified in this application and that the statements herein contained, the information provided in the attached plans and other exhibits are in all respects true and correct to the best of my (our) knowledge.

Signature of Property Owner

Or

Signature of Authorized Representative

This permit becomes null and void if work or construction authorized is not commenced within 180 days, or if construction or work is suspended or abandoned for a period of 180 days at any time after work is commenced. I hereby certify that I have read and examined this application and know the same to true and correct. All provisions of laws and ordinances governing this type of work will be complied with whether specified herein or not the granting of a permit dose not presume to give authority to violate or cancel the provisions of any State or local law regulating construction or the performance of construction and I make this statement under penalty of perjury.

Submittal Requirements: *(Check all that apply)*

Parcel Information: *Check one*

- ☑ Lot of Record
- ☐ Lot within an approved subdivision meeting the applicable notes on the plat

Site plan with required information demonstrated on the site plan as outlined on the Submittal Checklist:

- **Site Access:** *Check One*
  - ☑ Across own front property line
  - ☐ Flag lot approval date: _____
  - ☐ Alternative Access approval date: _____
- **Setback Requirements:** *Check all that apply*
  - ☐ Meets setbacks per ordinance:
    Front: _____ Side: _____ Rear: _____ Side Street: _____
  - ☐ Meets additional setbacks per outlined "Site Restrictions"
- **Large Accessory Building:**
  - ☐ Located in the front or side of main dwelling with conforming architectural style and material as main building.
  - ☐ Located behind dwelling
- **Height Requirements:** *Check one*
  - ☑ Meets height requirements per Weber County Land Use Code
  - ☐ Height Variance approval date: _____

Site Restrictions: *Check all that apply*

- ☐ FEMA Flood Zone _____
- ☐ Buildable area recorded on the plat
- ☐ Lot identified as a "R" (restricted lot)
- ☐ Areas of slope greater than 25%
- ☐ Geologic Study Area
- ☐ Site Elevation below 4,218
- ☐ Wetlands as identified by the USGS
- Western Weber Stream Corridor:
  - ○ Year-Round stream; or
  - ○ Ephemeral stream
- Ogden Valley Sensitive Lands:
  - ○ Scenic Corridor
  - ○ Ridgeline
  - ○ Historic/Prehistoric and/or Cultural Resources
- Ogden Valley Stream corridor setbacks:
  - ○ North Fork, South Fork & Middle Fork of the Ogden River: 100' setback from high water mark
  - ○ Year Round: 75' setback from high water mark
  - ○ Ephemeral: 50' setback from high water mark

The following submittal items are considered required items and shall be submitted with the building permit application. Incomplete applications will not be accepted or held.

- Geology:
  - ○ Geologic Unit: _____ as determined by Weber County staff member: _____
    - ☐ Geologic site reconnaissance required? Y/N
    - ☑ If yes, Geologic site reconnaissance clearing the lot for development will be required to be submitted or if an engineering geologist deems additional studies are necessary, the final geotechnical and geologic reports will be required to be stamped, signed and dated by an approved Utah State Engineering Geologist and Geotechnical Engineer.
- Two complete and identical sets of the plans with scale including the following information:
  - ○ Site Plan including existing site contours and lot/parcel boundary lines
  - ○ Building elevations (renderings) reflecting natural grade
  - ○ Structure Footing\Foundation plan including all decks/porches/covered patios
  - ○ Floor plans (all levels)
  - ○ Section\Details
  - ○ Structural calculations from the Engineer of record
  - ○ Energy Conservation Compliance Method (REScheck or other method)
  - ○ SWPPP Storm Water Pollution Prevention Plan (State/Local)
  - ○ Receipt for payment of Fire District Impact Fee
  - ○ All engineered, architectural and site plans are to be combined into one PDF file format to scale
- The following items will not be required at the initial submittal stage, however these items will be required to be submitted to our office prior to the issuance of the building permit:
  - ☑ Proof of approved wastewater disposal system
  - ☑ Proof of approved culinary water system

-Project Narrative-

The proposed project ("Project") is a new golf clubhouse/pro shop with associated food service contained in a single structure located on a portion of the Wolf Creek Golf & Club as depicted on the attached **Exhibits A 1 – 3**. The Project is a new facility which replaces the existing use in the adjacent building owned by The Summit Group. The Project is necessary to the operation of the golf course as The Summit Group has terminated the lease for the existing use and requires vacating the current pro shop by November 15, 2017.

The area proposed for the Project disturbs less than one acre as depicted on **Exhibit B** and the combined facilities are less than 10,000 square feet as further detailed below. The area in which the Project will be constructed is currently zoned CV-2.

The following are key components of the Project:

Structures:

- The proposed building will contain approximately 5,514 SF total on two levels.

-The first floor is approximately 4,351 SF and contains the pro shop, snack bar and event space;

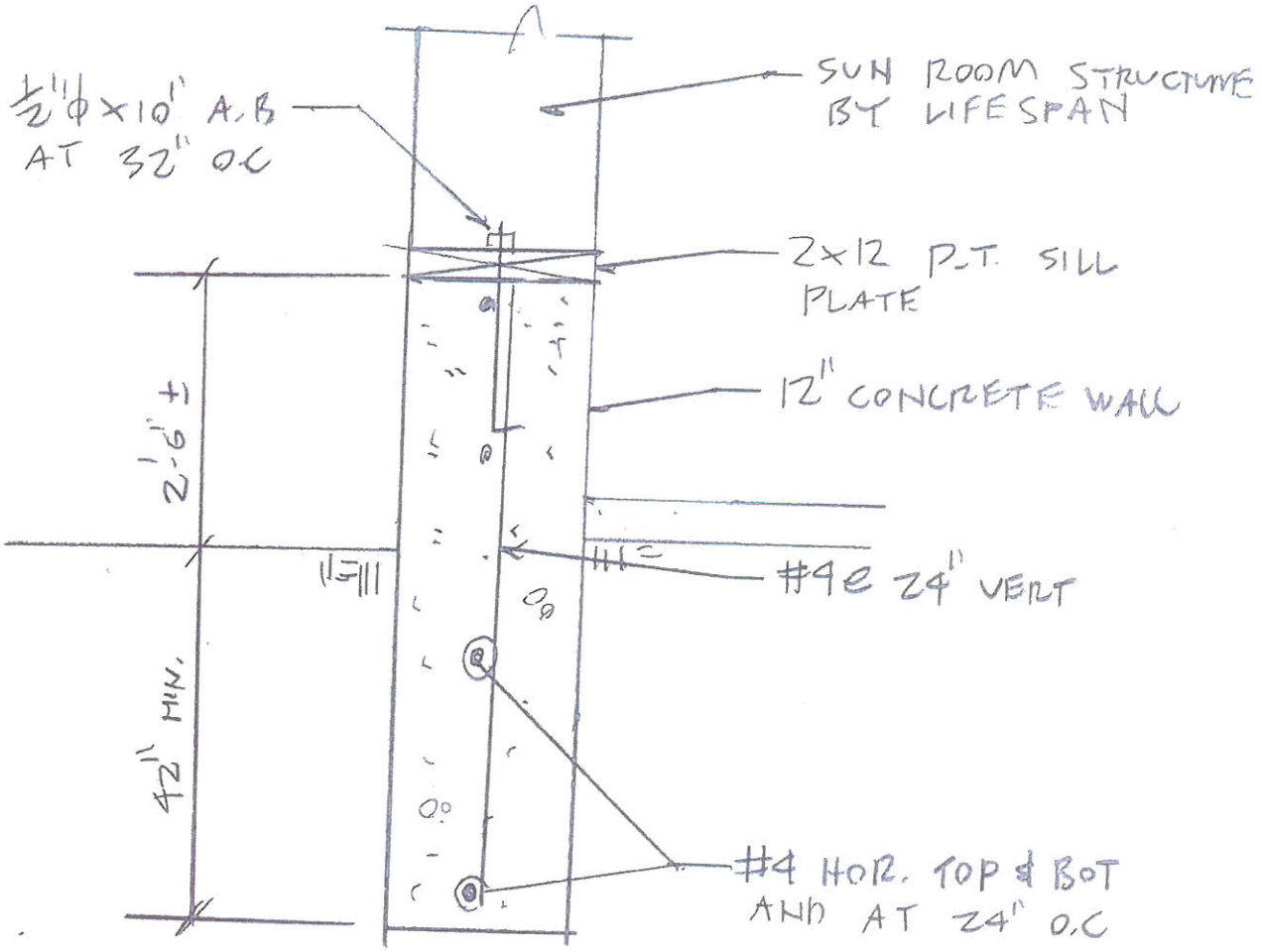
-The second floor is approximately 1,163 SF and contains a member's lounge and related ancillary uses.

-Design Review Fees have been calculated in accordance with the applicable square footages as attached on **Exhibit C**

Architecture and Finishes:

Both architectural and exterior finishes will be designed to match and blend with existing surrounding structures. Plentiful use of cultured stone, wood timbers and horizontal lap siding will be incorporated into exterior features. As depicted on the renderings, colors will consist of earth tones and subtle brown hues that

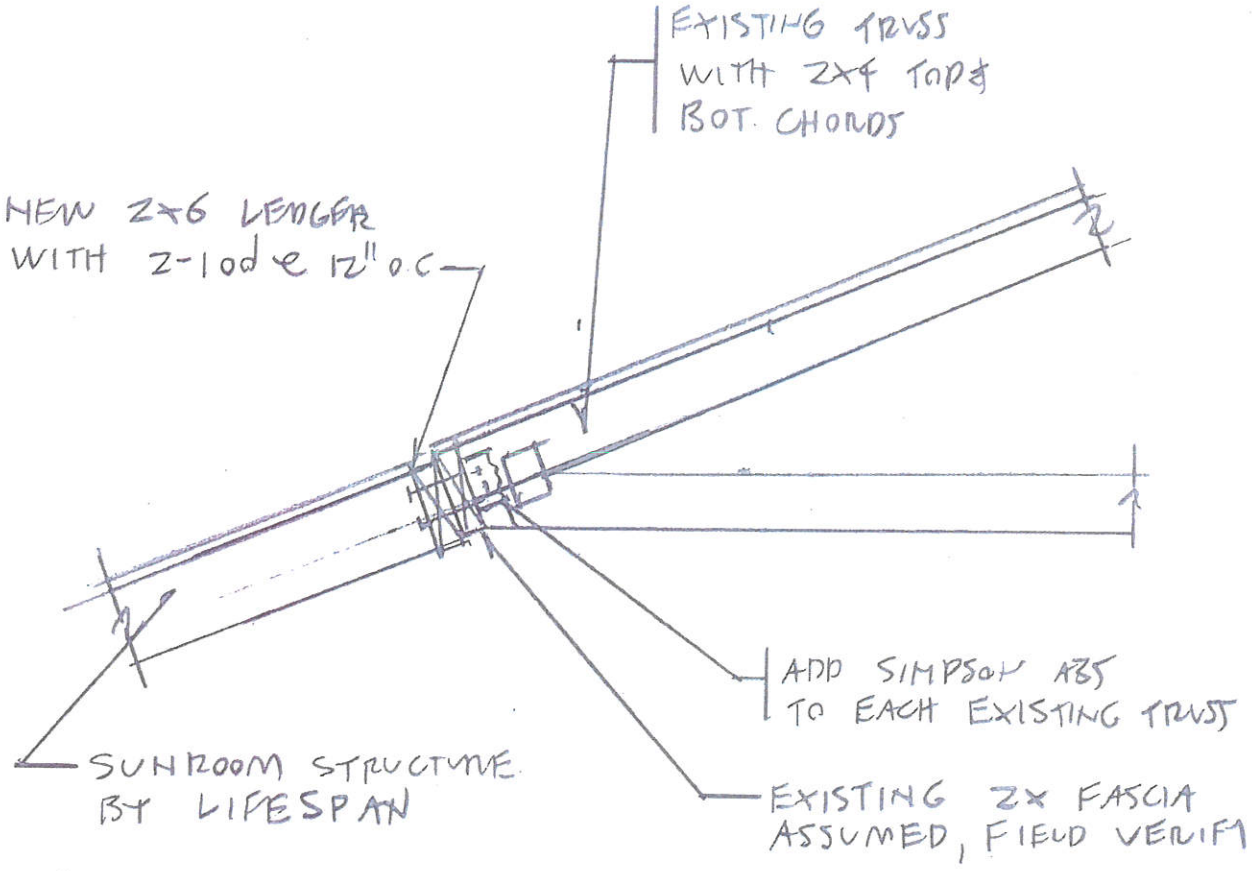
Jarratt Engineering Inc. Structural Engineering Consultant 8830 N. Upper Lando Lane, · Park City, Utah 84098, (435) 655-9557 pjarratt@qwestoffice.net	JOB TITLE WOLF CREEK	BY: PNJ DATE: 10-18 SHEET: 1 OF: 3
SUBJECT: SUNROOM ADDITION		



A/1 TTPICAL SUNROOM FOUNDATION DETAIL

PROFESSIONAL STRUCTURAL ENGINEER
No 166330
PETER N. JARRATT
STATE OF UTAH
10-23-18

Jarratt Engineering Inc. Structural Engineering Consultant 8830 N. Upper Lando Lane, Park City, Utah 84098, (435) 655-9557 pjarratt@qwestoffice.net	JOB TITLE WOLF CREEK	BY: PNJ DATE: 10-18 SHEET: 2 OF: 3
SUBJECT: SUN ROOM ADDITION		

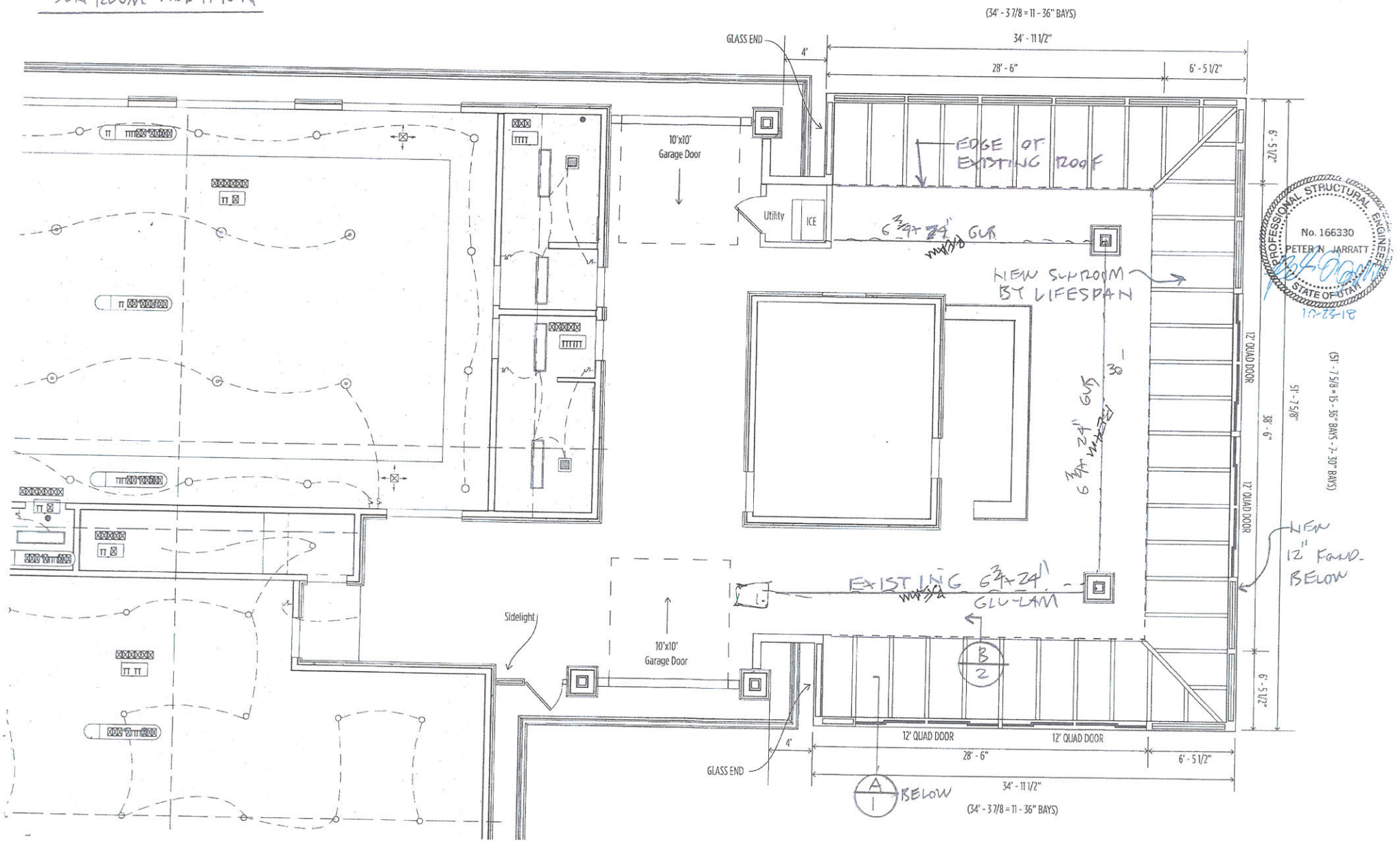


B/2 SUN ROOM TO EXISTING ROOF DETAIL

PROFESSIONAL STRUCTURAL ENGINEER
No. 166330
PETER N. JARRATT
STATE OF UTAH
10-23-18

WOLF CREEK
SUN ROOM ADDITION

3/3



PROFESSIONAL STRUCTURAL ENGINEER
No. 166330
PETER N. JARRATT
STATE OF UTAH
10-23-18

Jarratt Engineering Inc.
Structural Engineering Consultant
8830 N. Upper Lando Lane, · Park City, Utah 84098, (435) 655-9557
pjarratt@qwestoffice.net

JOB TITLE: WOLF GUESEK

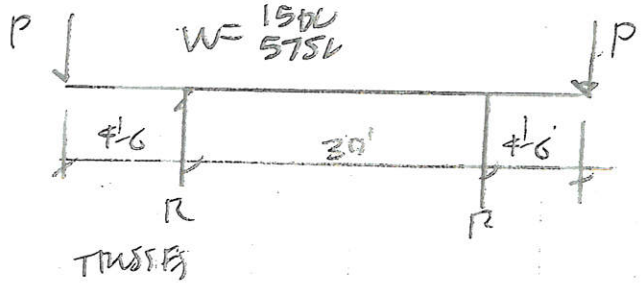
BY: PNJ DATE: 10-18

SHEET: 1 OF: 2

SUBJECT: SUNROOM ADDITION

ROOF LOADS: DL = 10 PSF
SL = 57 PSF

CHECK EXISTING BEAM & TRUSSES



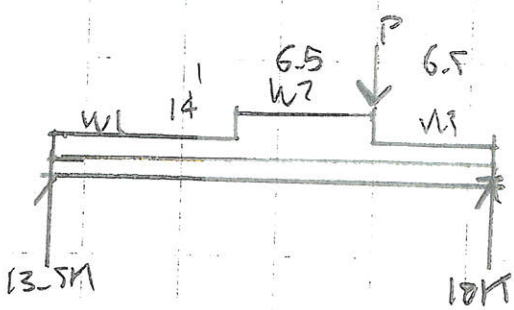
$P = 35\left(\begin{matrix}10DL\\57SL\end{matrix}\right) = \begin{matrix}350L\\2005L\end{matrix}$

$R = \begin{matrix}330DL\\1300SL\end{matrix}$

BEAM SPAN = 27'

6 3/4 + 24" GLU-LAM

$W = \begin{matrix}330DL\\1300SL\end{matrix}$



	EXIST.	NEW	
$W_1 =$	105 DL 400SL	+35 +200	= 140DL 600SL
$W_2 =$	290DL 1100	+35 200	= 325DL 1300SL
$W_3 =$	120DL 450SL	+35 200	= 155DL 650SL

$P = \begin{matrix}1.1\,DL\\4.5SL\end{matrix}$

Wood Beam

Description : Wolf Creek beam

CODE REFERENCES

Calculations per NDS 2015, IBC 2015, CBC 2016, ASCE 7-10
Load Combination Set : ASCE 7-10

Material Properties

Analysis Method : Allowable Stress Design
Load Combination : ASCE 7-10

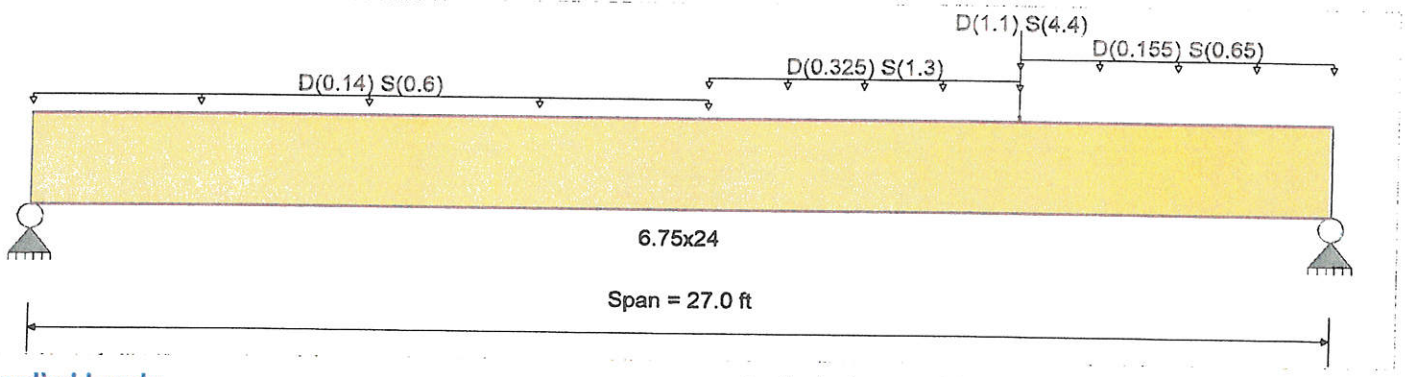
Wood Species : DF/DF
Wood Grade : 24F - V4

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb +	2,400.0 psi
Fb -	1,850.0 psi
Fc - Prll	1,650.0 psi
Fc - Perp	650.0 psi
Fv	265.0 psi
Ft	1,100.0 psi

E : Modulus of Elasticity

Ebend- xx	1,800.0 ksi
Eminbend - xx	950.0 ksi
Ebend- yy	1,600.0 ksi
Eminbend - yy	850.0 ksi
Density	31.20 pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Load for Span Number 1
Uniform Load : D = 0.140, S = 0.60 k/ft, Extent = 0.0 -->> 14.0 ft, Tributary Width = 1.0 ft
Uniform Load : D = 0.3250, S = 1.30 k/ft, Extent = 14.0 -->> 20.50 ft, Tributary Width = 1.0 ft
Uniform Load : D = 0.1550, S = 0.650 k/ft, Extent = 20.50 -->> 27.0 ft, Tributary Width = 1.0 ft
Point Load : D = 1.10, S = 4.40 k @ 20.50 ft

DESIGN SUMMARY

OK L 590

Design N.G.

Maximum Bending Stress Ratio	=	1.034 : 1	Maximum Shear Stress Ratio	=	0.581 : 1
Section used for this span		6.75x24	Section used for this span		6.75x24
fb : Actual	=	2,196.17 psi	fv : Actual	=	153.95 psi
FB : Allowable	=	2,124.38 psi	Fv : Allowable	=	265.00 psi
Load Combination		+D+S	Load Combination		+D+S
Location of maximum on span	=	15.865 ft	Location of maximum on span	=	25.029 ft
Span # where maximum occurs	=	Span # 1	Span # where maximum occurs	=	Span # 1

Maximum Deflection

Max Downward Transient Deflection	0.874 in	Ratio =	370 >=360
Max Upward Transient Deflection	0.000 in	Ratio =	0 <360
Max Downward Total Deflection	1.088 in	Ratio =	297 >=180
Max Upward Total Deflection	0.000 in	Ratio =	0 <180

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+S	1	1.0875	14.091		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1 Values in KIPS

Load Combination	Support 1	Support 2
Overall MAXimum	13.442	18.213
Overall MINimum	10.841	14.634
D Only	2.601	3.579
+D+S	13.442	18.213
+D+0.750S	10.732	14.554

LEI
ENGINEERS
SURVEYORS
PLANNERS

November 12, 2018

John Lewis
Wolf Creek Resort
john@wolfcreekresort.com

Re: BA Wolf Creek Pro Shop (LEI #2018-2913)

To Whom It May Concern:

We understand that there is a concern regarding the Wolf Creek Pro Shop located at Eden, Utah. The covered patio is to be enclosed with a glass atrium structure designed by others and new non-bearing 2x6 stud walls. We are providing plans, calculations, and details for the gravity and lateral support of the atrium structure to the existing structure. Below is a summary of what is required.

1. New steel beams are to be added outside of the existing fascia to support the vertical reaction of the new atrium.
2. The new steel beams are to be supported by wood posts and wood beams.
3. The atrium structure is to bear on a new concrete foundation wall and footing bearing at frost level.
4. The new 2x6 non-bearing stud walls are to bear on a new concrete foundation wall and footing bearing at frost level.
5. The existing shear walls and holdowns are structurally adequate to resist the increased wind load due to the addition of the atrium walls. The lateral force transfer is to occur through a connection at the fascia to the atrium. This connection is for lateral force transfer only.

Refer to the attached partial plans and details for additional information.

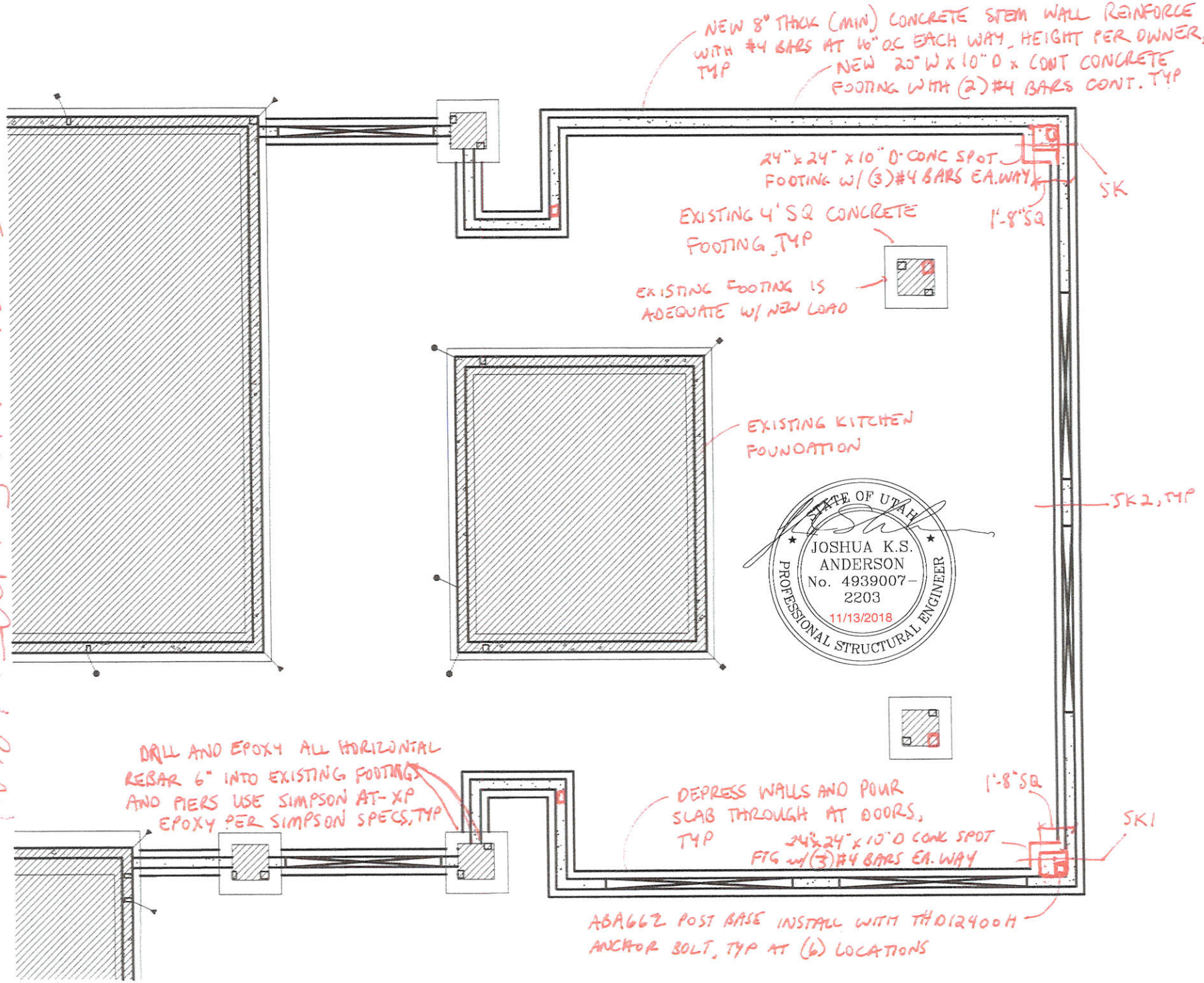
Please call if you have any questions or concerns. Thank you.

Sincerely,

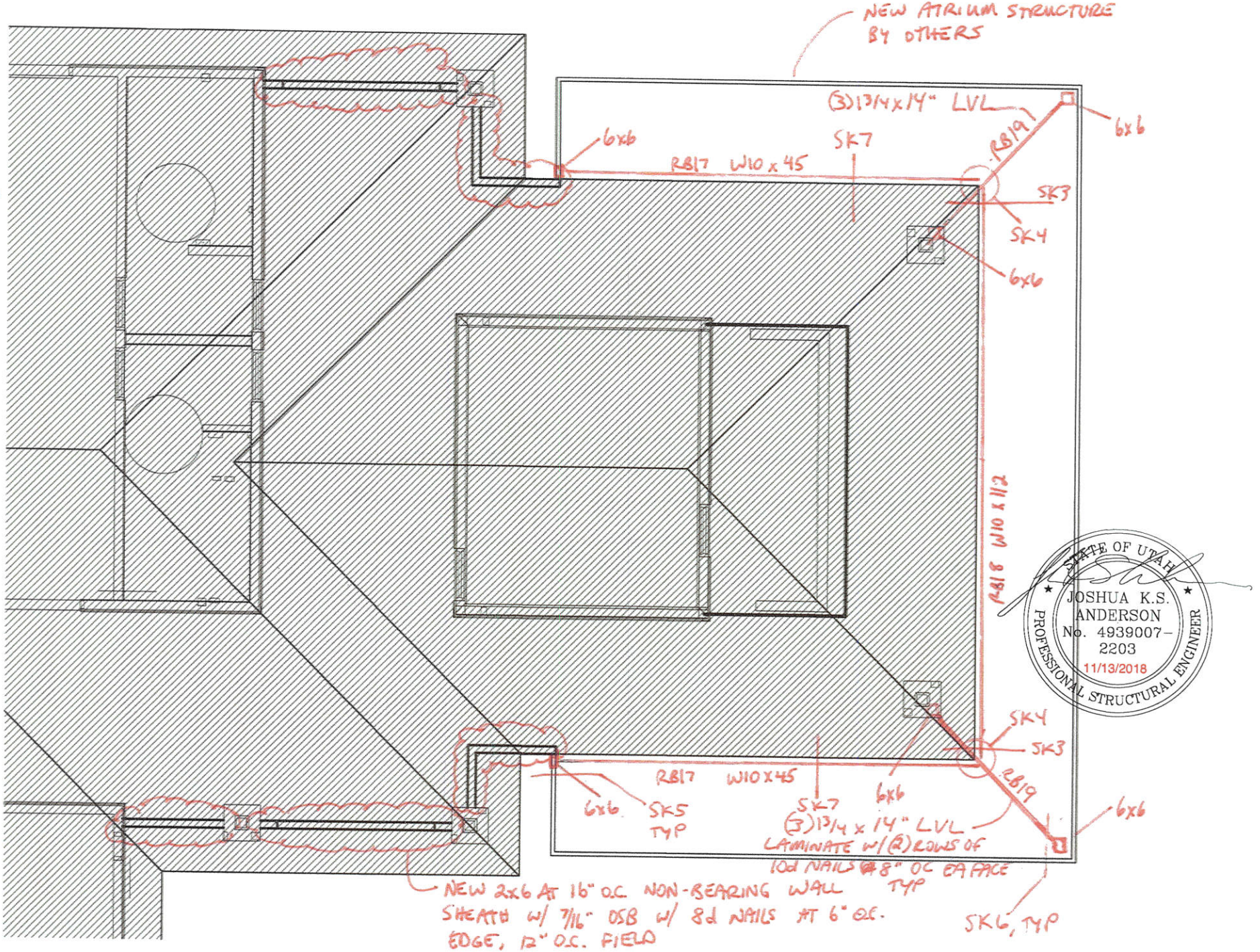
Joshua K.S. Anderson, S.E.
Principal Engineer

Attachment

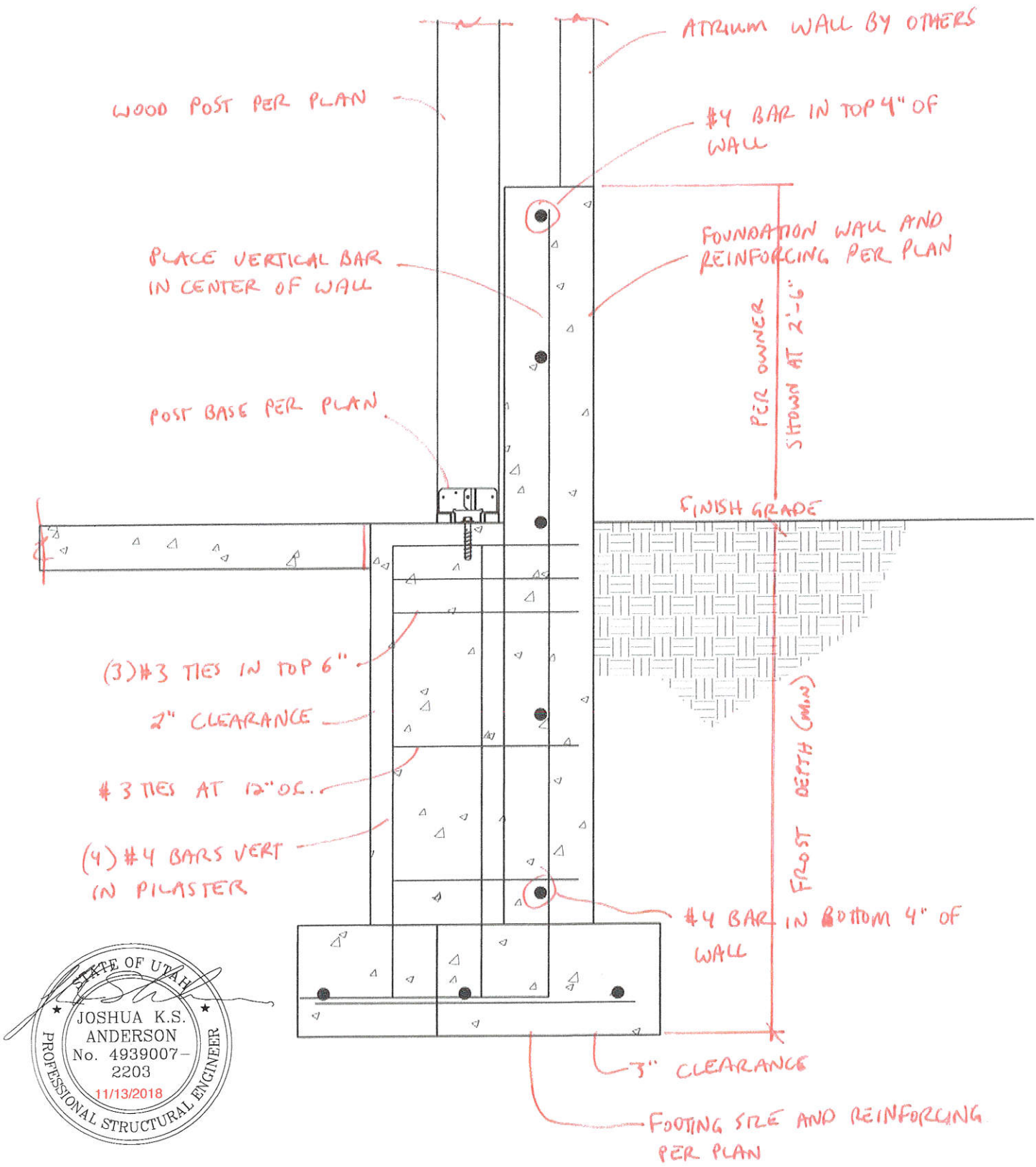
FOOTING AND FOUNDATION PLAN



ROOF FRAMING PLAN



STATE OF UTAH
JOSHUA K.S. ANDERSON
No. 4939007-2203
11/13/2018
PROFESSIONAL STRUCTURAL ENGINEER



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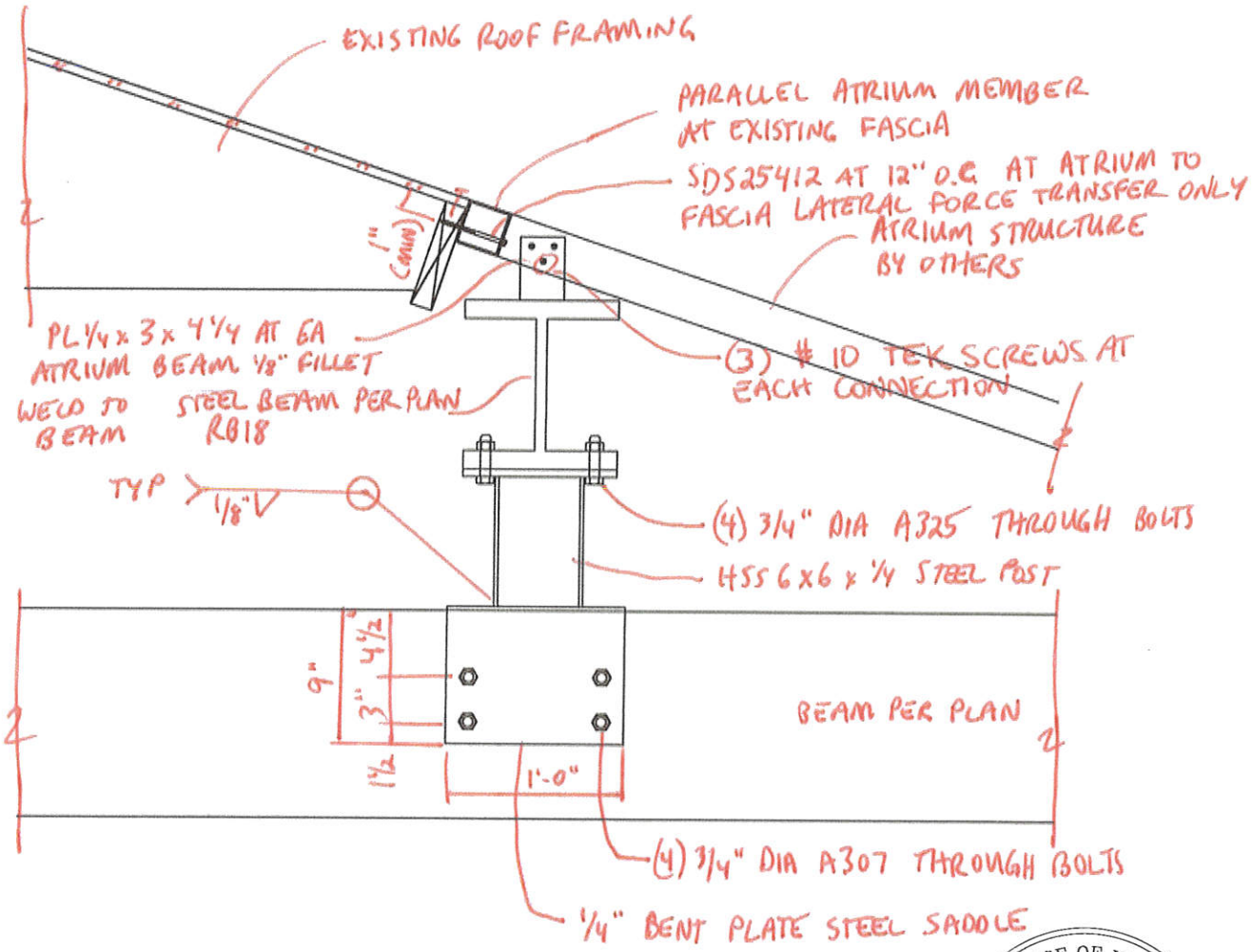
No. 4939007-2203
PROFESSIONAL STRUCTURAL ENGINEER
11/13/2018

SK1 WOOD POST AT FOUNDATION



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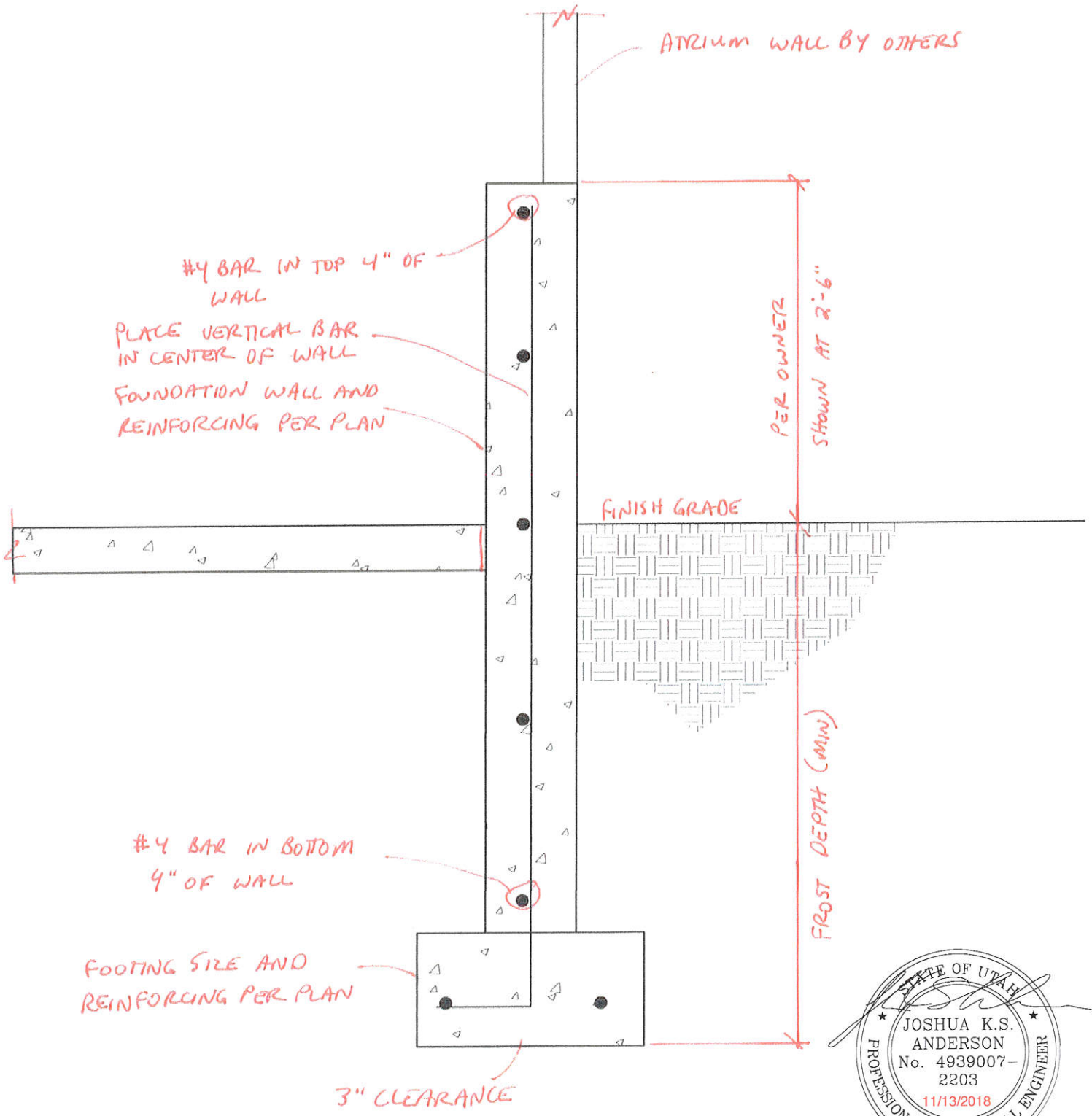
No. 4939007-2203
11/13/2018
PROFESSIONAL STRUCTURAL ENGINEER

SK3 STEEL BEAM TO WOOD BEAM



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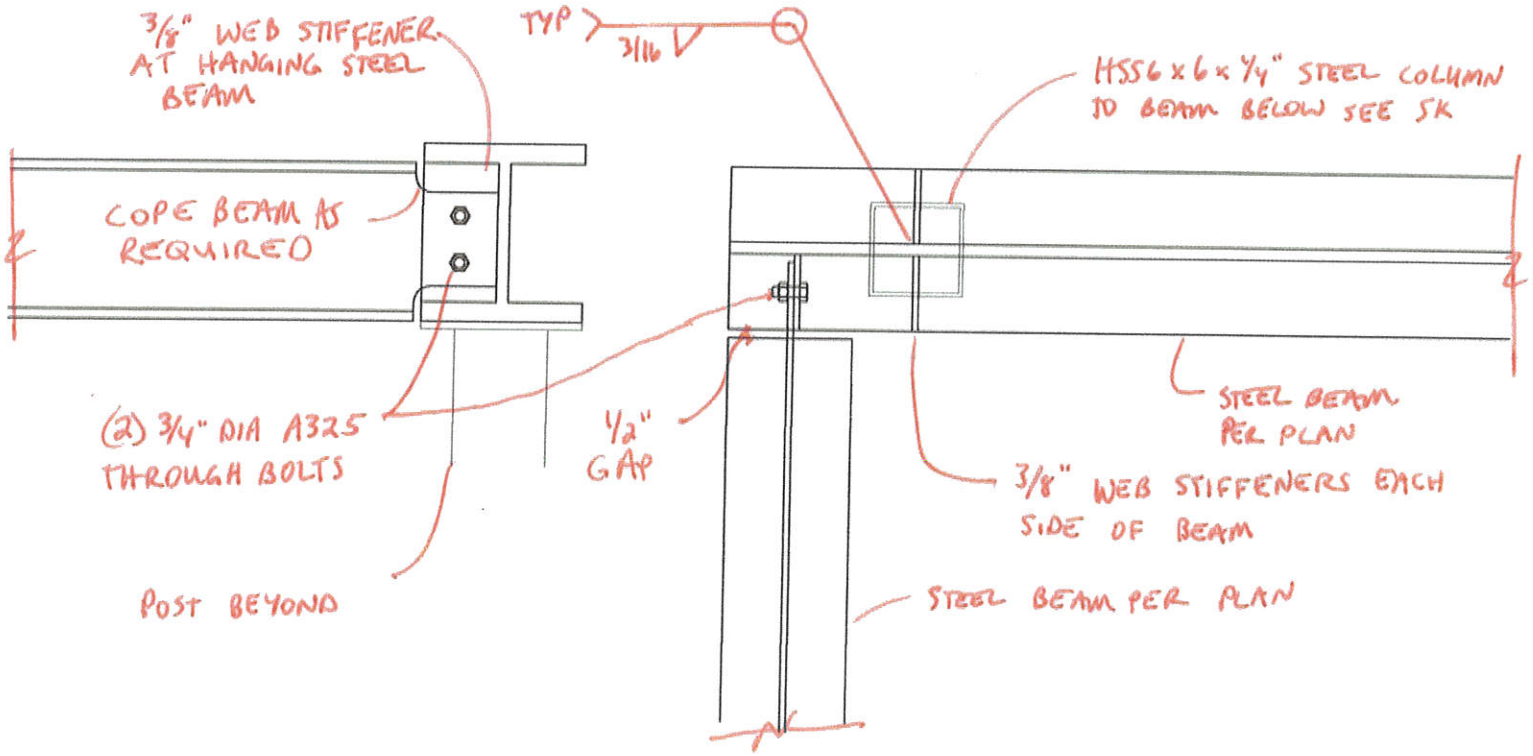
PROFESSIONAL STRUCTURAL ENGINEER

S1K2 ATRIUM WALL AT FOUNDATION



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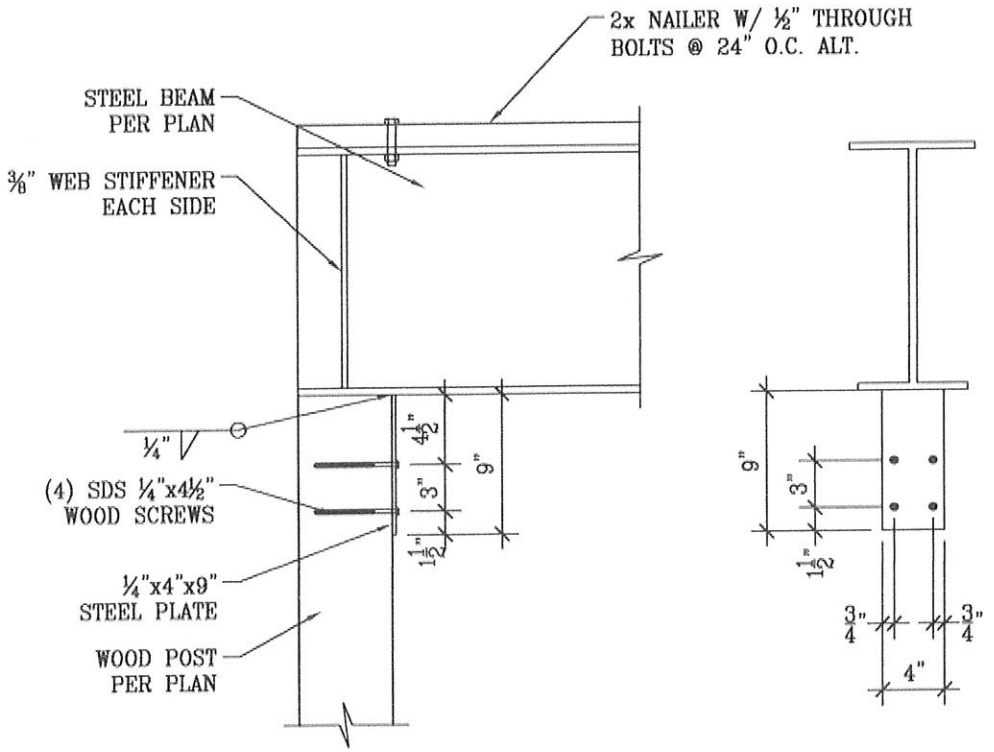
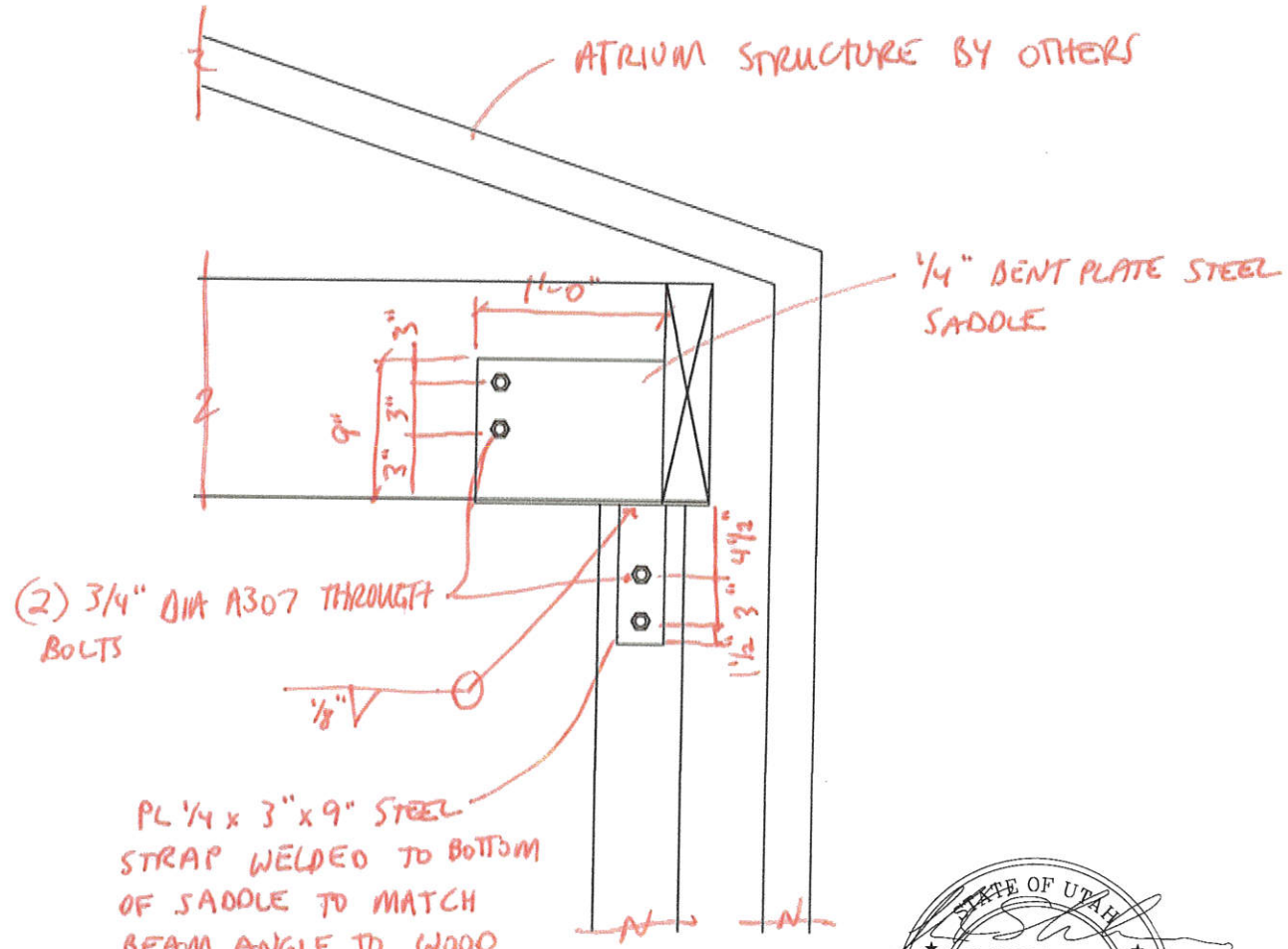
JOSHUA K.S. ANDERSON
No. 4939007-2203
11/13/2018
PROFESSIONAL STRUCTURAL ENGINEER

SK4 STEEL BEAM TO STEEL BEAM



SK5 **STEEL BEAM TO WOOD POST**



STATE OF UTAH
JOSHUA K.S. ANDERSON
No. 4939007-2203
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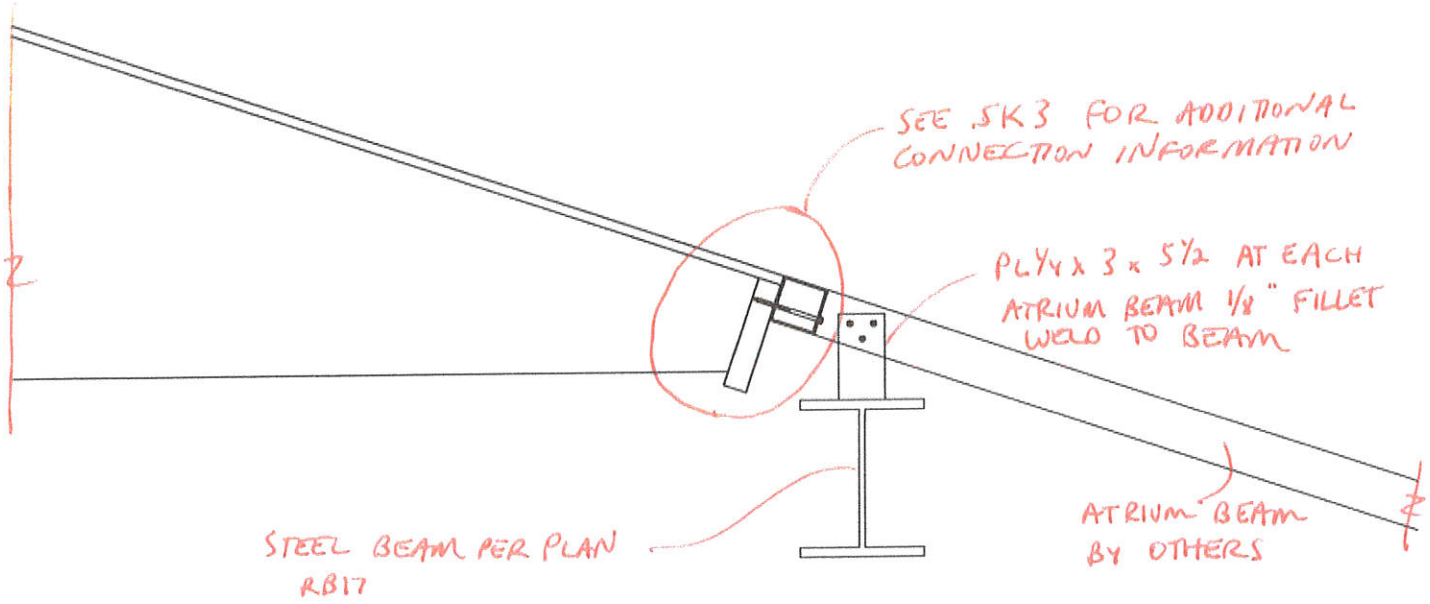
SK6 WOOD BEAM TO WOOD POST 45° SKEW



STATE OF UTAH
JOSHUA K.S. ANDERSON
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11/13/2018
PROFESSIONAL STRUCTURAL ENGINEER

SK 7 ATRIUM ROOF TO STEEL BEAM

Project: 2018-2913

Location: RB17
Multi-Loaded Multi-Span Beam
[2015 International Building Code(AISC 14th Ed ASD)]
A992-50 W10x45 x 28.5 FT
Section Adequate By: 12.7%
Controlling Factor: Deflection

StruCalc 9.0

page of

StruCalc Version 10.0.1.6 11/9/2018 11:09:25 AM

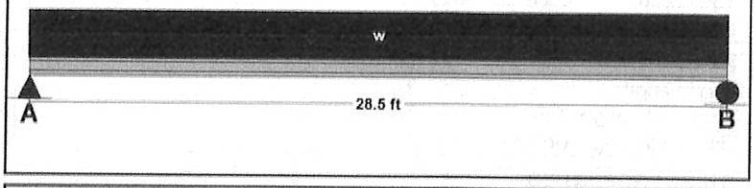
DEFLECTIONS	Center	
Live Load	0.38	IN L/896
Dead Load	0.12	in
Total Load	0.51	IN L/676

Live Load Deflection Criteria: L/600 Total Load Deflection Criteria: L/600

REACTIONS	A	B
Live Load	2636 lb	2636 lb
Dead Load	855 lb	855 lb
Total Load	3491 lb	3491 lb
Bearing Length	1.12 in	1.12 in

BEAM DATA	Center
Span Length	28.5 ft
Unbraced Length-Top	0 ft
Unbraced Length-Bottom	28.5 ft

LOADING DIAGRAM



UNIFORM LOADS	Center	
Uniform Live Load	185	plf
Uniform Dead Load	15	plf
Beam Self Weight	45	plf
Total Uniform Load	245	plf

STEEL PROPERTIES
W10x45 - A992-50

Properties:

Yield Stress:	Fy =	50 ksi
Modulus of Elasticity:	E =	29000 ksi
Depth:	d =	10.1 in
Web Thickness:	tw =	0.35 in
Flange Width:	bf =	8.02 in
Flange Thickness:	tf =	0.62 in
Distance to Web Toe of Fillet:	k =	1.12 in
Moment of Inertia About X-X Axis:	Ix =	248 in4
Section Modulus About X-X Axis:	Sx =	49.1 in3
Plastic Section Modulus About X-X Axis:	Zx =	54.9 in3

Design Properties per AISC 14th Edition Steel Manual:

Flange Buckling Ratio:	FBR =	6.47
Allowable Flange Buckling Ratio:	AFBR =	9.15
Web Buckling Ratio:	WBR =	22.46
Allowable Web Buckling Ratio:	AWBR =	90.55
Controlling Unbraced Length:	Lb =	0 ft
Limiting Unbraced Length - for lateral-torsional buckling:	Lp =	7.1 ft
Nominal Flexural Strength w/ safety factor:	Mn =	136976 ft-lb
Controlling Equation:	F2-1	
Web height to thickness ratio:	h/tw =	22.46
Limiting height to thickness ratio for eqn. G2-2:	h/tw-limit =	53.95
Cv Factor:	Cv =	1
Controlling Equation:	G2-2	
Nominal Shear Strength w/ safety factor:	Vn =	70700 lb

STATE OF UTAH
JOSHUA K.S. ANDERSON
No. 4939007-2203
11/13/2018
PROFESSIONAL STRUCTURAL ENGINEER

Controlling Moment: 24875 ft-lb
14.25 Ft from left support of span 2 (Center Span)
Created by combining all dead loads and live loads on span(s) 2
Controlling Shear: 3491 lb
At left support of span 2 (Center Span)
Created by combining all dead loads and live loads on span(s

Comparisons with required sections:	Req'd	Provided
Moment of Inertia (deflection):	219.98 in4	248 in4
Moment:	24875 ft-lb	136976 ft-lb
Shear:	3491 lb	70700 lb

Project: 2018-2913

Location: RB18
Multi-Loaded Multi-Span Beam
[2015 International Building Code(AISC 14th Ed ASD)]
A992-50 W10x112 x 38.5 FT
Section Adequate By: 3.7%
Controlling Factor: Deflection

StruCalc 9.0

StruCalc Version 10.0.1.6 11/7/2018 4:45:20 PM

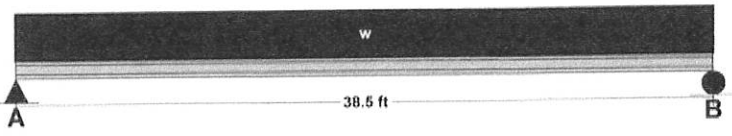
DEFLECTIONS	Center	
Live Load	0.44	IN L/1049
Dead Load	0.30	in
Total Load	0.74	IN L/622

Live Load Deflection Criteria: L/600 Total Load Deflection Criteria: L/600

REACTIONS	A	B
Live Load	3561 lb	3561 lb
Dead Load	2445 lb	2445 lb
Total Load	6006 lb	6006 lb
Bearing Length	1.75 in	1.75 in

BEAM DATA	Center
Span Length	38.5 ft
Unbraced Length-Top	0 ft
Unbraced Length-Bottom	38.5 ft

LOADING DIAGRAM



UNIFORM LOADS	Center
Uniform Live Load	185 plf
Uniform Dead Load	15 plf
Beam Self Weight	112 plf
Total Uniform Load	312 plf

STEEL PROPERTIES
W10x112 - A992-50

Properties:		
Yield Stress:	Fy =	50 ksi
Modulus of Elasticity:	E =	29000 ksi
Depth:	d =	11.4 in
Web Thickness:	tw =	0.76 in
Flange Width:	bf =	10.4 in
Flange Thickness:	tf =	1.25 in
Distance to Web Toe of Fillet:	k =	1.75 in
Moment of Inertia About X-X Axis:	Ix =	716 in4
Section Modulus About X-X Axis:	Sx =	126 in3
Plastic Section Modulus About X-X Axis:	Zx =	147 in3
Design Properties per AISC 14th Edition Steel Manual:		
Flange Buckling Ratio:	FBR =	4.16
Allowable Flange Buckling Ratio:	AFBR =	9.15
Web Buckling Ratio:	WBR =	10.46
Allowable Web Buckling Ratio:	AWBR =	90.55
Controlling Unbraced Length:	Lb =	0 ft
Limiting Unbraced Length - for lateral-torsional buckling:	Lp =	9.47 ft
Nominal Flexural Strength w/ safety factor:	Mn =	366767 ft-lb
Controlling Equation:	F2-1	
Web height to thickness ratio:	h/tw =	10.46
Limiting height to thickness ratio for eqn. G2-2:	h/tw-limit =	53.95
Cv Factor:	Cv =	1
Controlling Equation:	G2-2	
Nominal Shear Strength w/ safety factor:	Vn =	172140 lb

Controlling Moment: 57808 ft-lb
19.25 Ft from left support of span 2 (Center Span)
Created by combining all dead loads and live loads on span(s) 2
Controlling Shear: -6006 lb
38.0 Ft from left support of span 2 (Center Span)
Created by combining all dead loads and live loads on span(s

Comparisons with required sections:	Req'd	Provided
Moment of Inertia (deflection):	690.59 in4	716 in4
Moment:	57808 ft-lb	366767 ft-lb
Shear:	-6006 lb	172140 lb

STATE OF UTAH
JOSHUA K.S. ANDERSON
No. 4939007-2203
11/13/2018
PROFESSIONAL STRUCTURAL ENGINEER

Project: 2018-2913

Location: RB19
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(3) 1.75 IN x 14.0 IN x 14.0 FT
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 23.2%
Controlling Factor: Moment

StruCalc 9.0

page of

StruCalc Version 10.0.1.6 11/7/2018 4:45:25 PM

CAUTIONS
* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS	Center
Live Load	0.24 IN L/699
Dead Load	0.17 in
Total Load	0.41 IN L/406

Live Load Deflection Criteria: L/240 Total Load Deflection Criteria: L/180

REACTIONS	A	B
Live Load	3984 lb	2213 lb
Dead Load	2896 lb	1680 lb
Total Load	6880 lb	3893 lb
Bearing Length	1.75 in	0.99 in

BEAM DATA	Center
Span Length	14 ft
Unbraced Length-Top	0 ft
Unbraced Length-Bottom	14 ft
Live Load Duration Factor	1.15
Notch Depth	0.00

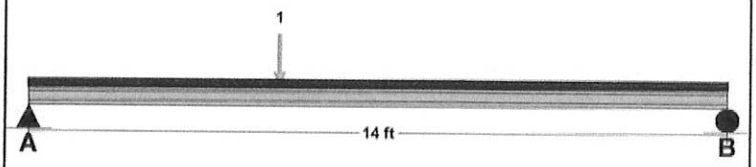
MATERIAL PROPERTIES
1.9E Microllam - iLevel Trus Joist

	Base Values	Adjusted
Bending Stress:	Fb = 2600 psi	Fb' = 2928 psi
	Cd=1.15 CF=0.98	
Shear Stress:	Fv = 285 psi	Fv' = 328 psi
	Cd=1.15	
Modulus of Elasticity:	E = 1900 ksi	E' = 1900 ksi
Comp. ⊥ to Grain:	Fc - ⊥ = 750 psi	Fc - ⊥' = 750 psi

Controlling Moment: 33965 ft-lb
5.04 Ft from left support of span 2 (Center Span)
Created by combining all dead loads and live loads on span(s) 2
Controlling Shear: 6854 lb
At a distance d from left support of span 2 (Center Span)
Created by combining all dead loads and live loads on span(s) 2

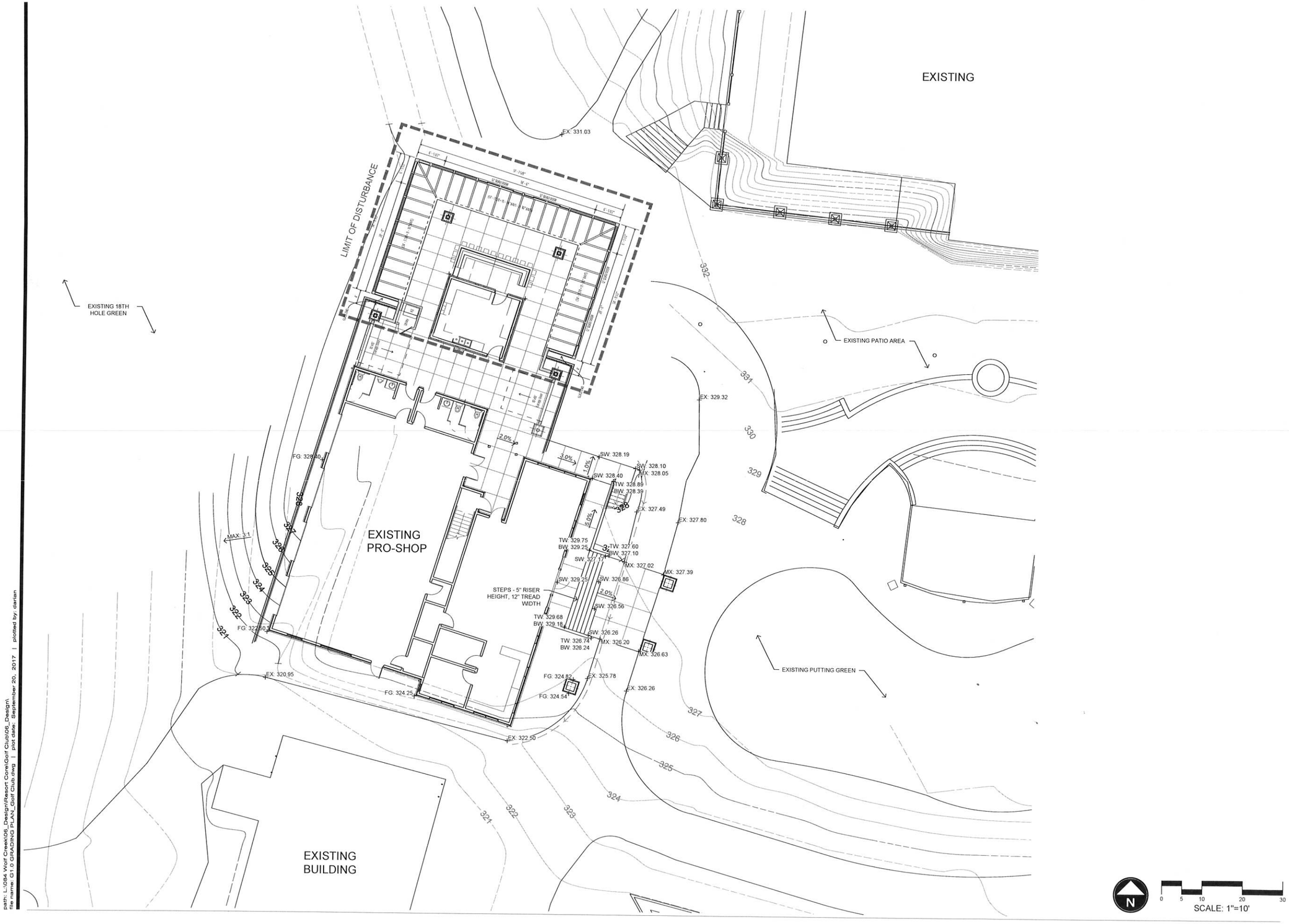
Comparisons with required sections:	Req'd	Provided
Section Modulus:	139.2 in3	171.5 in3
Area (Shear):	31.37 in2	73.5 in2
Moment of Inertia (deflection):	532.78 in4	1200.5 in4
Moment:	33965 ft-lb	41845 ft-lb
Shear:	6854 lb	16060 lb

LOADING DIAGRAM



UNIFORM LOADS	Center
Uniform Live Load	0 plf
Uniform Dead Load	0 plf
Beam Self Weight	23 plf
Total Uniform Load	23 plf

POINT LOADS - CENTER SPAN		
Load Number	One	Two
Live Load	2636 lb	3561 lb
Dead Load	1810 lb	2445 lb
Location	5 ft	5 ft



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EST. 2006
LDG
SALT LAKE CITY
LANDSCAPE ARCHITECTURE

WOLF CREEK RESORT

WOLF CREEK PRO-SHOP

WEBER COUNTY, UTAH

PREPARED FOR:
WOLF CREEK GOLF COURSE
3900 NORTH WOLF CREEK DR.
EDEN, UT 84310

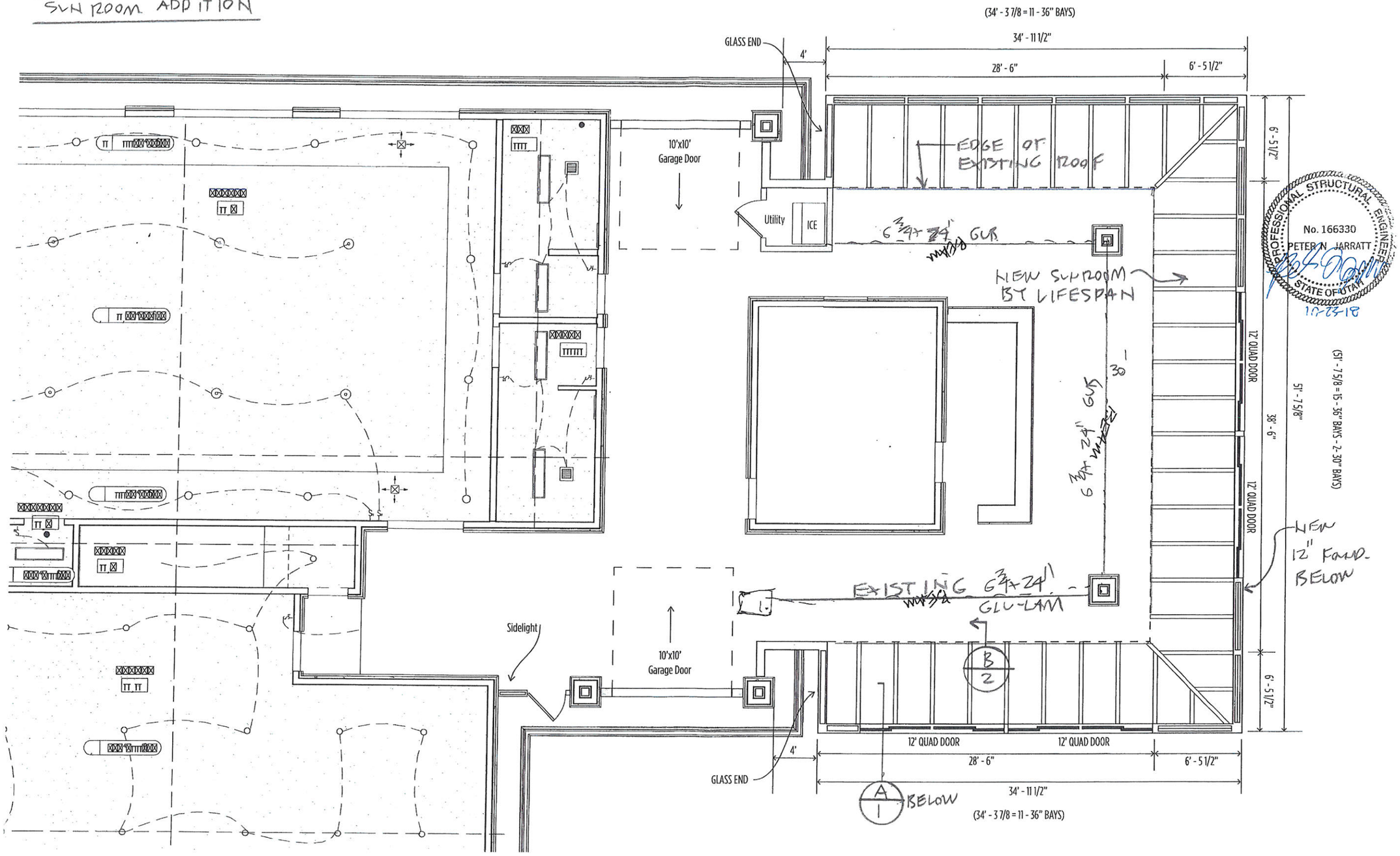
DATE:	SEPTEMBER 2017
PROJECT:	000 0000 84
DRAWN BY:	DW
REVIEW BY:	EL
VERSION:	DESIGN REVIEW
REVISIONS:	

SHEET TITLE:
SITE PLAN

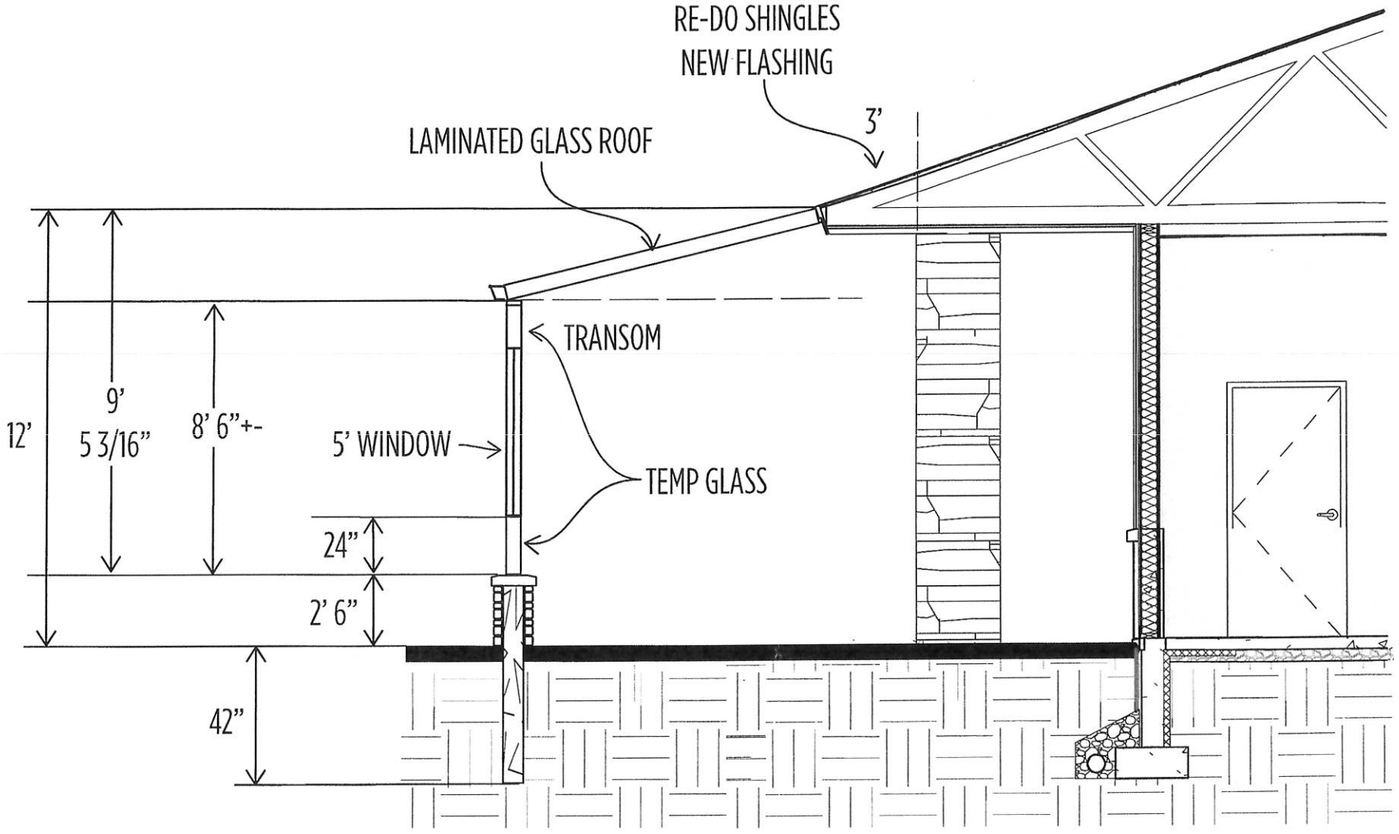
SHEET NUMBER:
G1.0

WOLF CREEK
SUN ROOM ADDITION

3/3



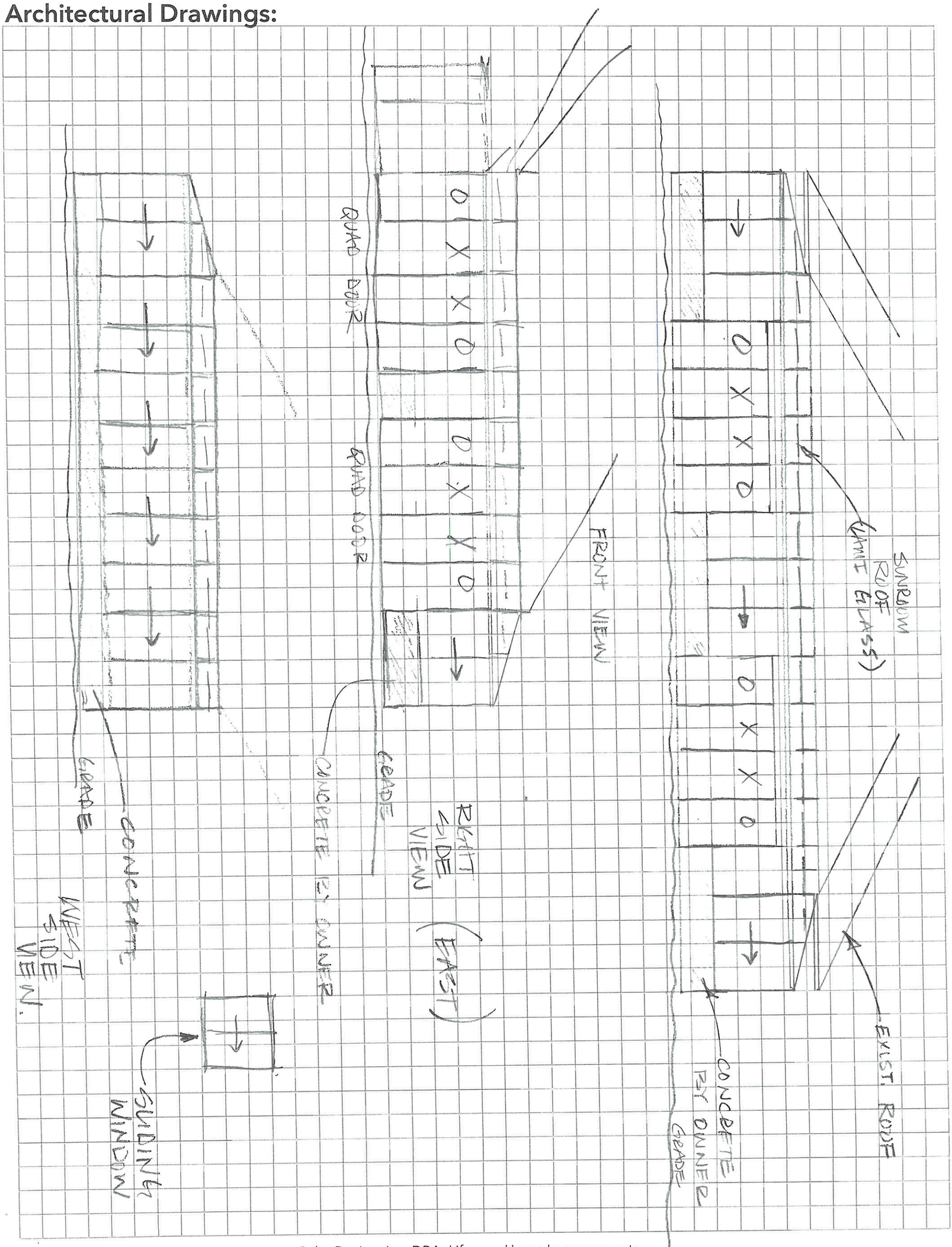
PROFESSIONAL STRUCTURAL ENGINEER
No. 166330
PETER N. JARRATT
STATE OF UTAH
10-23-18


RE-DO SHINGLES
NEW FLASHING
3'
LAMINATED GLASS ROOF
TRANSOM
12'
9'
5 3/16"
8' 6"+-
5' WINDOW
TEMP GLASS
24"
2' 6"
42"

LIFESPAN
HOME IMPROVEMENTS
BATHS • SUNROOMS • WINDOWS

OWNER: WOLF CREEK RESORT	PROJECT: DINING ROOM	PROJECT CONTACT: RUSS
ADDRESS:	DRAWN BY: R TYCHSEN	PROJECT CONTACT PHONE:
CITY, STATE, ZIP: EDEN UT	SCALE: NONE	PROJECT CONTACT EMAIL:
PHONE:	SECTION: VIEWS	801-949-4191

Architectural Drawings:



FOUR SEASONS SUNROOMS & WINDOWS

230 SUN & STARS ROOMS: STRAIGHT EAVE (2 in 12 ROOF PITCH) ENGINEERING AND STRUCTURAL LOADING INFORMATION

EFFECTIVE DATE 1-18
REVISION: C

ROOM MODEL	GLAZING BAR O.C. SPACING	RAFTER TYPE	ROOF LIVE LOAD (psf)	EXPOSURE B WIND LOAD (mph)
S*M-6DH (5'-5 3/4")	2'-6 5/8"	5LB3	149	140
	3-0 5/8"	5LB3	133	125
S*M-7DH (6'-5 1/2")	2'-6 5/8"	5LB3	104	140
	3-0 5/8"	5LB3	87	125
S*M-9DH (8'-7 1/2")	2'-6 5/8"	5LB3	43	140
	3-0 5/8"	5LB3	36	125
	2'-6 5/8"	5HB3	87	140
	3-0 5/8"	5HB3	73	125
S*M-10DH (9'-8 1/8")	2'-6 5/8"	5LB3	31	140
	3-0 5/8"	5LB3	26	125
	2'-6 5/8"	5HB3	61	140
	3-0 5/8"	5HB3	51	125
	2'-6 5/8"	5LB5	64	140
	3-0 5/8"	5LB5	53	125
	2'-6 5/8"	5CB5	88	140
	3-0 5/8"	5CB5	73	125
S*M-11DH (10'-8 5/8")	2'-6 5/8"	5LB3	22	140
	3-0 5/8"	5LB3	19	125
	2'-6 5/8"	5HB3	45	140
	3-0 5/8"	5HB3	38	125
	2'-6 5/8"	5LB5	47	140
	3-0 5/8"	5LB5	39	125
	2'-6 5/8"	5CB5	74	140
	3-0 5/8"	5CB5	60	125
S*M-12DH (11'-10 1/8")	2'-6 5/8"	5LB3	16	125
	3-0 5/8"	5LB3	14	119
	2'-6 5/8"	5HB3	33	135
	3-0 5/8"	5HB3	28	125
	2'-6 5/8"	5LB5	35	135
	3-0 5/8"	5LB5	29	125
	2'-6 5/8"	5CB5	66	135
	3-0 5/8"	5CB5	54	125
S*M-13DH (12'-8 3/4")	2'-6 5/8"	5LB3	13	119
	3-0 5/8"	5LB3	11	119
	2'-6 5/8"	5HB3	27	125
S*M-13DH (CONT.)	3-0 5/8"	5HB3	22	125
	2'-6 5/8"	5LB5	28	125
	3-0 5/8"	5LB5	23	125
	2'-6 5/8"	5CB5	53	125
	3-0 5/8"	5CB5	44	125
S*M-15DH (14'-9 1/4")	2'-6 5/8"	5HB3	17	115
	3-0 5/8"	5HB3	14	115
	2'-6 5/8"	5LB5	18	115
	3-0 5/8"	5LB5	15	115
	2'-6 5/8"	5CB5	34	115
	3-0 5/8"	5CB5	28	115
	2'-6 5/8"	5HB7	52	115
	3-0 5/8"	5HB7	42	115
S*M-16DH (15'-11 1/2")	2'-6 5/8"	5HB3	13	115
	3-0 5/8"	5HB3	11	115
	2'-6 5/8"	5LB5	14	115
	3-0 5/8"	5LB5	11	115
	2'-6 5/8"	5CB5	26	115
	3-0 5/8"	5CB5	22	115
	2'-6 5/8"	5HB7	48	115
	3-0 5/8"	5HB7	38	115
S*M-17DH (16'-10 1/4")	2'-6 5/8"	5LB5	12	105
	3-0 5/8"	5LB5	10	105
	2'-6 5/8"	5CB5	22	105
	3-0 5/8"	5CB5	19	105
	2'-6 5/8"	5HB7	45	105
	3-0 5/8"	5HB7	36	105
S*M-18DH (17'-10")	2'-6 5/8"	5CB5	19	100
	3-0 5/8"	5CB5	16	100
	2'-6 5/8"	5HB7	42	100
	3-0 5/8"	5HB7	34	100
S*M-19DH (18'-10")	2'-6 5/8"	5CB5	16	100
	3-0 5/8"	5CB5	13	100
	2'-6 5/8"	5HB7	40	100
	3-0 5/8"	5HB7	32	100

*WIND LOADS ARE BASED ON ACTUAL CONDITIONS, 120 MPH AND OVER ARE BASED ON A PARTIALLY ENCLOSED DESIGN. THERE IS NO NEED TO "SUBTRACT" OVER 120 MPH

TABLE A - BASIC WIND SPEED CONVERSION

1	2009 EQUIVALENT BASIC WIND SPEED	87	91	95	103	111	119	126	134	142
2	2012 & 2015 BASIC WIND SPEED	110	115	120	130	140	150	160	170	180

NOTE: 1. WIND SPEED SHOWN IN CHART ABOVE IS BASED ON 2009 IBC
2. TO DETERMINE IF ROOM MEETS 2012 & 2015 WIND SPEED REQUIREMENTS SELECT APPROPRIATE DESIGN WIND SPEED FROM 2012 & 2015 IBC/IRC AND THEN USING **TABLE A** ABOVE SELECT 2012 VALUE FROM LINE 2 AND THEN DETERMINE EQUIVALENT 2009 BASIC WIND SPEED FROM LINE 1
3. WIND SPEED VALUE IN CHART MUST EXCEED VALUE DETERMINED FROM LINE 1 IN **TABLE A**
4. LINE 1 AND LINE 2 COMPARE WIND SPEED MAP VALUES FROM 2009 AND 2012/2015 CODES - THESE MAPS ARE BASED ON EXPOSURE C

NOTES:

1) 5LB3 = 3" LITE BAR, 5HB3 = 3" HEAVY BAR, 5LB5 = 5" LITE BAR
5CB5 = 5" HEAVY BAR, 5HB7 = 5" HEAVY BAR W/4RSB RAFTER STIFFENER
2) ALUMINUM ALLOY FOR GLAZING BARS IS 6005-T5.
3) DEAD LOAD OF ROOF SYSTEM IS 5 PSF.
4) ALL ROOMS ARE ACCEPTABLE FOR CONSTRUCTION IN SEISMIC AREAS WITH A SPECTRAL RESPONSE ACCELERATION, Ss, LESS THAN OR EQUAL TO 141%g. OTHER SEISMIC LOADS MUST BE EVALUATED ON AN INDIVIDUAL BASIS.
5) DEFLECTION ARE BASED ON L/120 DEAD + LIVE CRITERIA, L/180 LIVE CRITERIA.
6) WINDS ARE BASED ON AN ENCLOSED STRUCTURE, EXCEPT WHEN 120 MPH AND GREATER, THEN WINDS ARE BASED ON AN PARTIALLY ENCLOSED STRUCTURE.
7) LOADS REPRESENT ALLOWABLE VALUES UP TO A 8'-0" EAVE HEIGHT, AND A 15' ROOM WIDTH. OTHER CONFIGURATIONS MUST BE EVALUATED ON AN INDIVIDUAL BASIS.
8)THIS SUMMARY PERTAINS TO THE STRUCTURAL INTEGRITY OF OUR UNIT UP TO, BUT NOT INCLUDING, THE CONNECTIONS TO THE EXISTING STRUCTURE AND/OR ANY NEW CONSTRUCTION. ALL SUBSTRUCTURE DESIGN REQUIREMENTS AND CONNECTIONS TO THE EXISTING STRUCTURE ARE NOT INCLUDED IN THE SCOPE OF WORK FOR THE FOUR SEASONS PRODUCT, AND MUST BE EVALUATED BY OTHERS.
9) THE ENGINEERING DESIGN SCOPE FOR THE FOUR SEASONS PRODUCT DOES NOT ACCOUNT FOR SPECIAL LOAD CONDITIONS CREATED BY ATTACHMENT TO THE EXISTING STRUCTURE. THESE MAY INCLUDE SNOW DRIFTING OR UNBALANCE SNOW LOADING. ANY SPECIAL LOADING CONDITIONS MUST BE EVALUATED BY OTHERS.
10) ENGINEERS CERTIFICATION: I CERTIFY THAT THESE ENGINEERING SPECIFICATIONS HAVE BEEN PREPARED UNDER MY DIRECT SUPERVISION AND THAT I AM A REGISTERED PROFESSIONAL ENGINEER IN THE STATES SHOWN.

REFERENCE NUMBER 1200

SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

GENERAL NOTES

A. FOUNDATIONS
1. ALL SUBSTRUCTURES INCLUDING BUT NOT LIMITED TO FOUNDATIONS & DECKS, SHALL BE DESIGNED BY OTHERS.
2. CONNECTION DETAILS SHOWN ON DRAWINGS INDICATE MINIMUM REQUIREMENTS BASED ON CAPACITY OF FOURS SEASONS COMPONENTS. THE ACTUAL CONNECTIONS TO SUBSTRUCTURE SHALL BE DESIGNED BY OTHERS.

B. EXISTING STRUCTURES
1. THE CAPACITY OF THE EXISTING OR NEW STRUCTURE TO RESIST ALL LOADS IMPOSED BY THE FOUR SEASONS ROOMS SHALL BE EVALUATED BY OTHERS.
2. CONNECTION DETAILS SHOWN ON DRAWINGS INDICATE MINIMUM REQUIREMENTS BASED ON CAPACITY OF FOUR SEASONS COMPONENTS. THE ACTUAL CONNECTIONS TO EXISTING OR NEW STRUCTURES SHALL BE DESIGNED BY OTHERS.

C. STRUCTURAL STEEL
1. ALL STRUCTURAL STEEL CONFORMS TO ASTM A36 OR ASTM A572 GRADE 50.

D. ALUMINUM
1. ALL STRUCTURAL ALUMINUM CONFORMS TO THE MINIMUM REQUIREMENTS OF 6005-T5 FOR ALLOY AND TEMPER EXCEPT AS NOTED BELOW:
   GREAT ROOM 12 RIDGE BEAM.....6063-T6
   CORNER COLUMN.....6063-T6
   H-COLUMN.....6105-T5
2. ALL STRUCTURAL ALUMINUM WORK CONFORMS TO "PART I-A - SPECIFICATIONS FOR ALUMINUM STRUCTURES - ALLOWABLE STRESS DESIGN" OR "PART I-B - SPECIFICATIONS FOR ALUMINUM STRUCTURES - BUILDING LOAD AND RESISTANCE FACTOR DESIGN" OF THE ALUMINUM ASSOCIATION, INC. SEVENTH EDITION, EFFECTIVE JANUARY 2000.
3. IN ALL INSTANCES WHERE ALUMINUM COMES INTO CONTACT WITH STEEL, PROVIDE DIELETRIC SEPERATION.
4. ALL EXPOSED ALUMINIUM RECEIVES ONE COAT OF PAINT. COLOR TO IS COORDINATED WITH MODEL AVAILABILITY.

E. FASTENERS
1. ALL LAG BOLTS SHALL CONFORM TO ASTM A36.
2. ALL LAG BOLTS SHALL HAVE A MINIMUM EMBEDMENT OF 8x BOLT DIAMETER INTO STRUCTURAL FRAMING (G=.45 MIN.)
3. LAG BOLTS AND SCREWS INTO WOOD FRAMING SHALL BE PROVIDED WITH PILOT HOLES HAVING A DIAMETER NOT GREATER THAN 70 PERCENT OF THE THREAD DIAMETER OF THE BOLT OR SCREW. ALL LAG BOLTS AND SCREWS SHALL BE INSERTED IN PILOT HOLES BY TURNING AND UNDER NO CIRCUMSTANCES BY DRIVING WITH A HAMMER.
4. ALL EXPANSION ANCHORS SHALL BE DESIGNED (BY OTHERS) IN ACCORDANCE WITH THE SPECIFIC MANUFACTURER'S REQUIREMENTS AND ALLOWABLE LOADS AND SHALL ONLY BE APPLIED IN CONDITIONS ACCEPTABLE TO MANUFACTURER.
5. ALL FASTENERS CONNECTING ALUMIMUM COMPONENTS ARE STAINLESS STEEL TYPE 300 18-8 UNLESS OTHERWISE NOTED ON PLANS.

F. GLASS
1. GLASS UNITS CONSISTS OF TWO PANES OF 1/8" THICK TEMPERED GLASS WITH A 5/8" STAINLESS STEEL SPACER BETWEEN PANES WITH AN ARGON FILL.
2. GLASS CONFORMS TO ASTM E1300.
3. ALL MC-16 CLEAR ARGON ROOF GLASS HAS THE FOLLOWING MINIMUM PROPERTIES:
   VISIBILITY TRANSMITTANCE = 16%
   SOLAR TRANSMITTANCE = 10%
   ULTRAVIOLET TRANSMITANCE = 7%
   VISIBLE OUTSIDE REFLECTIVITY = 11%
   VISIBLE INSIDE REFLECTIVITY = 25%
   SHADING COEFFICIENT = .18
   SOLAR HEAT GAIN COEFFICIENT = .15
   RELATIVE HEAT GAIN = 39
   ASHRAE WINTER U VALUE = .25
   ASHRAE WINTER R VALUE = 4.0
4. ALL MC-56 CLEAR ARGON WALL GLASS HAS THE FOLLOWING MINIMUM PROPERTIES:
   VISIBILITY TRANSMITTANCE = 56
   SOLAR TRANSMITTANCE = 29
   ULTRAVIOLET TRANSMITANCE = 13
   VISIBLE OUTSIDE REFLECTIVITY = 10
   VISIBLE INSIDE REFLECTIVITY = 17
   SHADING COEFFICIENT = .38
   SOLAR HEAT GAIN COEFFICIENT = .33
   RELATIVE HEAT GAIN = 79
   ASHRAE WINTER U VALUE = .25
   ASHRAE WINTER R VALUE = 4.0

G. SEALANT
1. ALL SEALANT CONFORMS TO TT-S-001543-A, TT-S-002306, ASTM C-920 TYPE S, GRADE NS, CLASS 25.

H. GASKETS
1. ALL GASKETS ARE CO-EXTRUDED AND ARE NON-MIGRATORY.

I. ROOM SPECIFICS

REFERENCE STANDARDS;
ASTM E 119
ASTM E 1300
ASCE 7-98

a) WALL GLASS CODE 74, R-VALUE; 4.0,
   ROOF GLASS CODE 78, R-VALUE; 4.0
b) WALL PANELS TO BE 3" INSULATED
c) (XX) LITE COLUMNS
d) (XX) UTILITY "H" COLUMNS (GABLE ENDS)
e) (XX) UTILITY "H" COLUMNS
f) (X) SWING DOOR
g) (X) SLIDER DOOR

J. DEFLECTION CRITERIA
1. ALL MEMBERS MEET OR EXCEEDS THE FOLLOWING MINIMUM DEFLECTION LIMITS:

	LL	DL + LL
a. STRUCTURAL ALUMINUM:	L/180	L/120
b. GLASS:	L/175	-----

K. DESIGN LIVE LOADS
STRUCTURAL MEMBERS HAVE BEEN DESIGNED FOR FULL DEAD LOADS AND THE FOLLOWING LIVE LOADS:

1. IMPORTANCE FACTORS: WIND (Iw) = 1.0
   SNOW (Is) = 1.0
   SEISMIC (Ie) = 1.0

2. LIVE LOADS SEE ENGINEERING TABLES FOR MAXIMUM ALLOWABLE ROOF LOAD

3. SNOW SEE ENGINEERING TABLES FOR MAXIMUM ALLOWABLE ROOF LOAD

4. WIND LOAD SEE ENGINEER'NG TABLES FOR MAXIMUM ALLOWABLE WIND SPEED

6. SEISMIC LOAD SEISMIC PARAMETERS ARE AS FOLLOWS:

UBC '97:
SEISMIC ZONE
BASIC STRUCTURAL SYSTEM: BUILDING FRAME SYSTEM - LIGHT FRAMED WALLS w/SHEAR PANELS (OTHER MATERIAL)
RESPONSE MODIFICATION FACTOR R = 5
SEISMIC FORCE AMPLIFICATION FACTOR = 2.8
BUILDING HEIGHT LIMIT, FEET H = 65'

2000 IBC OR 2003 IBC OR 2006 IRC:
SEISMIC SPECTRAL RESPONSE COEFFICIENT (Ss)=
BASIC STRUCTURAL SYSTEM: BUILDING FRAME SYSTEM - LIGHT FRAMED WALLS w/ SHEAR PANELS (OTHER MATERIAL)
RESPONSE MODIFICATION FACTOR R = 2.5
SEISMIC FORCE AMPLIFICATION FACTOR = 2.5
DEFLECTION AMPLIFICATION FACTOR Cd = 2.5
BUILDING HEIGHT LIMIT, FEET H = 35'

7. LATERAL DESIGN CONTROLLED BY WIND.

L. COORDINATION
1. FOUR SEASONS SUNROOMS IS NOT RESPONSIBLE FOR VERIFYING & COORDINATING THE INFORMATION BETWEEN THESE DRAWINGS & ANY OTHER DRAWINGS USED IN CONJUCTION WITH THESE DRAWINGS.

M. ABBREVIATIONS
THE FOLLOWING LIST OF ABBREVIATIONS IS NOT INTENDED TO REPRESENT ALL THOSE USED ON THESE DRAWINGS, BUT TO SUPPLEMENT THE MORE COMMON ABBREVIATIONS USED:

1. TYP. - TYPICAL
2. SIM. - SIMILIAR
3. UON - UNLESS OTHERWISE NOTED
4. CONT. - CONTINUOUS

N. CONSTRUCTION SAFETY
1. THESE DRAWINGS DO NOT CONTAIN NECESSARY COMPONENTS FOR SAFETY DURING CONSTRUCTIN.
2. THE INSTALLER SHALL PROVIDE ADEQUATE TEMPORARY BRACING, SHORING & GUYING OF FRAMING AGAINST WIND, CONSTRUCTION LOADS & OTHER TEMPORARY FORCES UNTIL NO LONGER REQUIRED FOR THE SUPPORT OF THE FRAMING.

O. PATIO COVER
1. WHEN USED AS A PATIO COVER THE ROOM COMFORMS TO ALL APPLICABLE PROVISIONS OF EITHER:
   a) APPENDIX H OF THE 2000 IRC OR 2003 IRC OR 2006 IRC, WHICHEVER IS APPLICABLE
   b) APPENDIX I OF THE 2000 IBC OR 2003 IBC OR 2006 IRC, WHICHEVER IS APPLICABLE.
   c) APPENDIX CHAPTER 31 OF THE 1997 UBC CODE.

E.C.O. #:	REV.	DESCRIPTION:	DATE:	APPROVED
ECN-514	A	INITIAL ENGINEERING RELEASE	08-22-05	AS
ECO-835	B	NEW GITTER/FASCIA RELEASE	07-10-07	AS

NOTE:
THE FRONT WALL LENGTH OF THE ROOMS DOES NOT TYPICALLY GOVERN THE STRUCTURAL DESIGN. THE FRAMING IS CONFIGURED AS A BAY AND THEREFORE IS REPETETIVE. CONSEQUENTLY, A FOUR BAY ROOM BEHAVES IN A SIMILAR MANNER AS A TWENTY BAY ROOM.

FASTENER SCHEDULE FOR STRAIGHT EAVE TO EXISTING STRUCTURES & FOUNDATIONS

COMPONENT	FASTENER (MIN.)	NO./SPACING FASTENERS
COLUMN/SILL	3/8" ø BOLT w/ 1 1/4" WASHER	2 (MIN.) @ EACH COLUMN AND 16" O.C. (MAX.)
GLAZING BAR	3/8" ø	2 (MIN.) @ EACH 3" GLAZING BAR 3 (MIN.) @ EACH 5" GLAZING BAR
WALL COLUMN	3/8" ø	2 IN TOP 6" (3" APART) 30" O.C. (MAX.) VERTICALLY

NOTE: ALL FASTENERS SHALL BE STAINLESS STEEL

DOCUMENTS ARE VALID ONLY WITH AN ORIGINAL STAMP AND DATED SIGNATURE

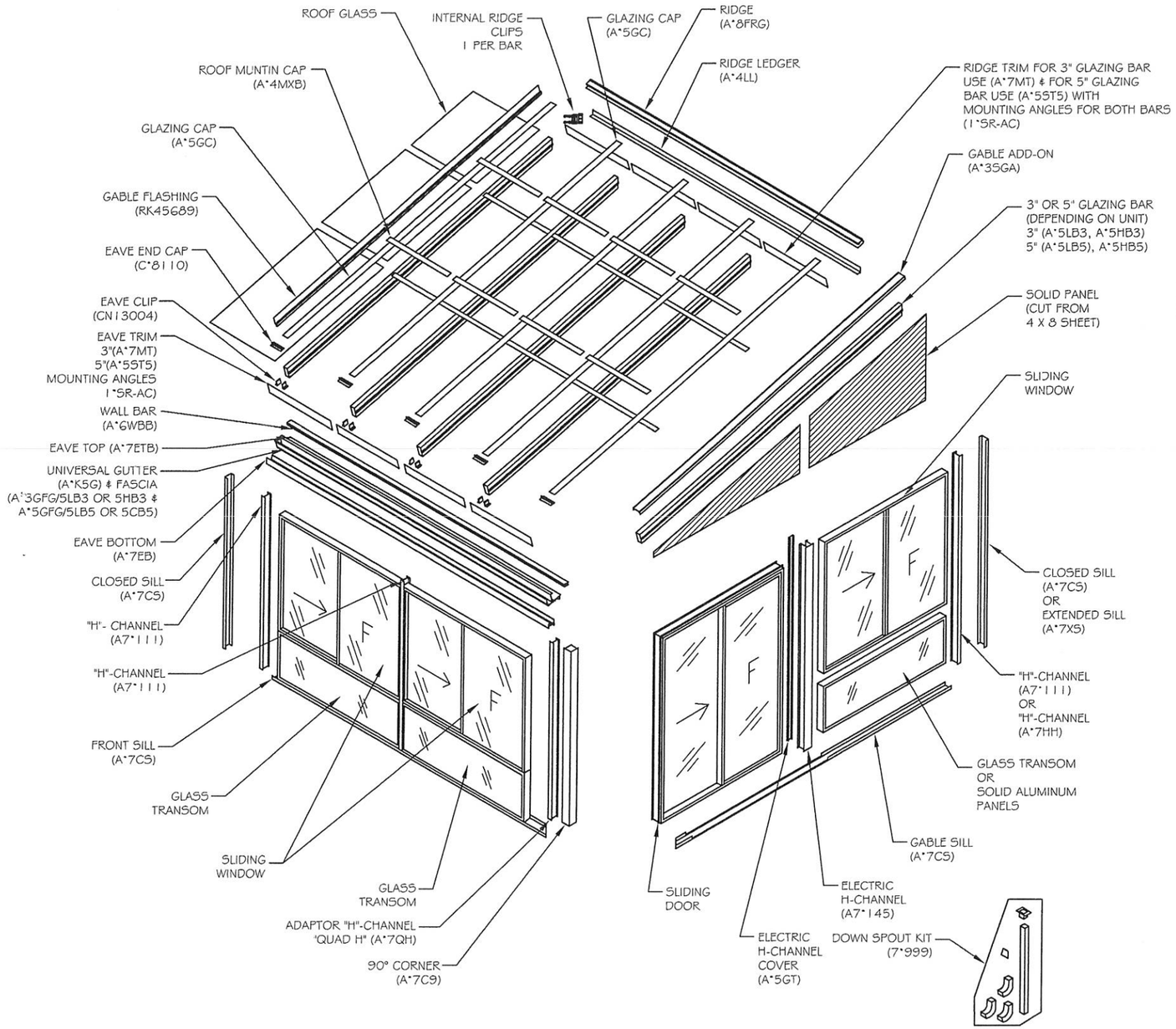
FOUR SEASONS SOLAR PRODUCTS, LLC.
5005 VETERANS MEMORIAL HIGHWAY
HOLBROOK, NEW YORK 11741
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

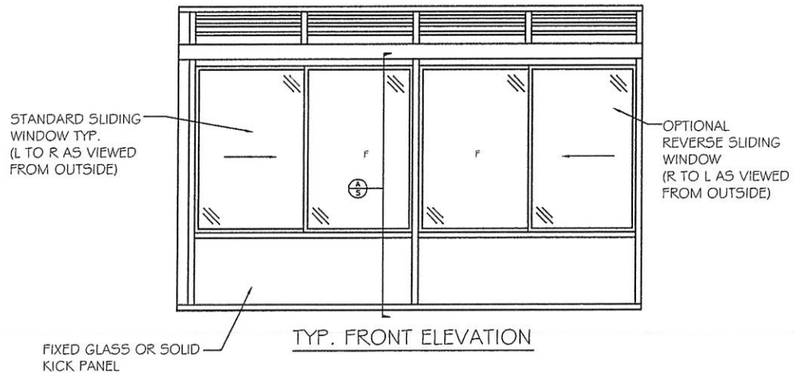
SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

DRAWN BY: TW
REVIEWED BY: AS
DATE: 4/15/04

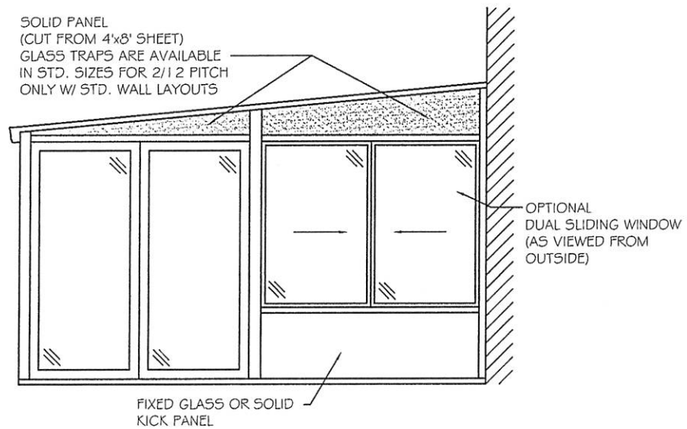
REVISION	DATE
B	07-10-07

DRAWING NAME:
230 ST STR EV-01





TYP. FRONT ELEVATION



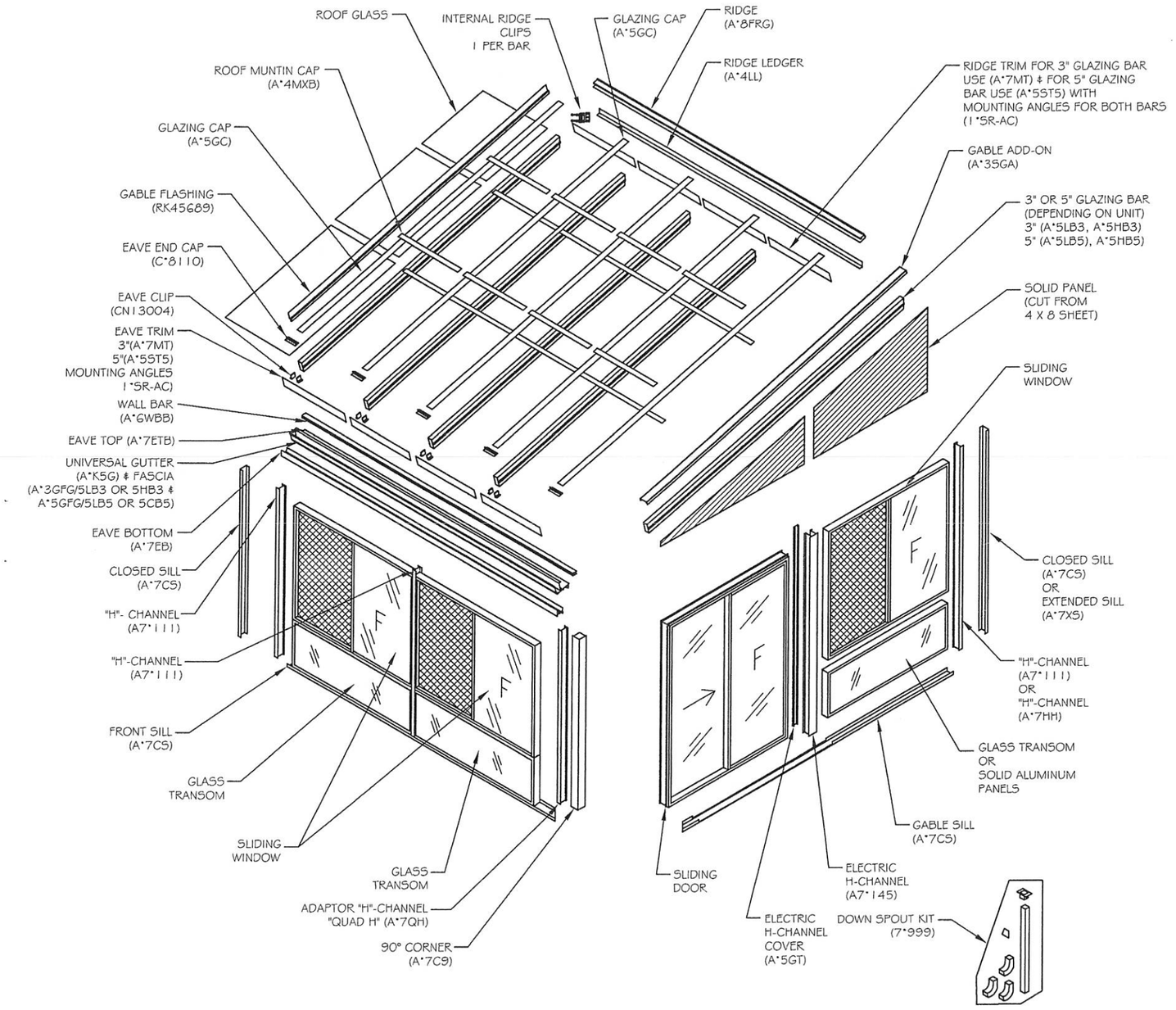
TYP. GABLE ELEVATION

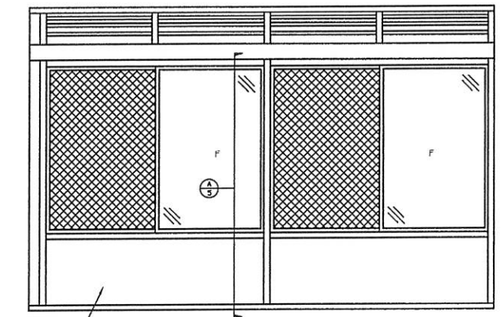
DOCUMENTS ARE VALID ONLY WITH AN ORIGINAL STAMP AND DATED SIGNATURE

FOUR SEASONS SOLAR PRODUCTS, LLC.
5005 VETERANS MEMORIAL HIGHWAY
HOLBROOK, NEW YORK 11741
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

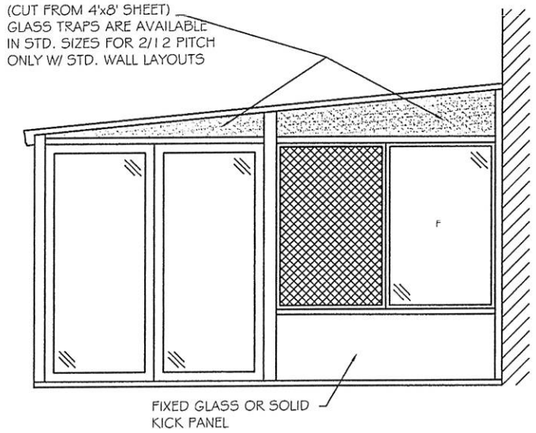
SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

DRAWN BY: TW	
REVIEWED BY: AS	
DATE: 4/15/04	
REVISION	DATE
B	07-10-07
DRAWING NAME: 230 ST STR EV-02	





TYP. FRONT ELEVATION



TYP. GABLE ELEVATION

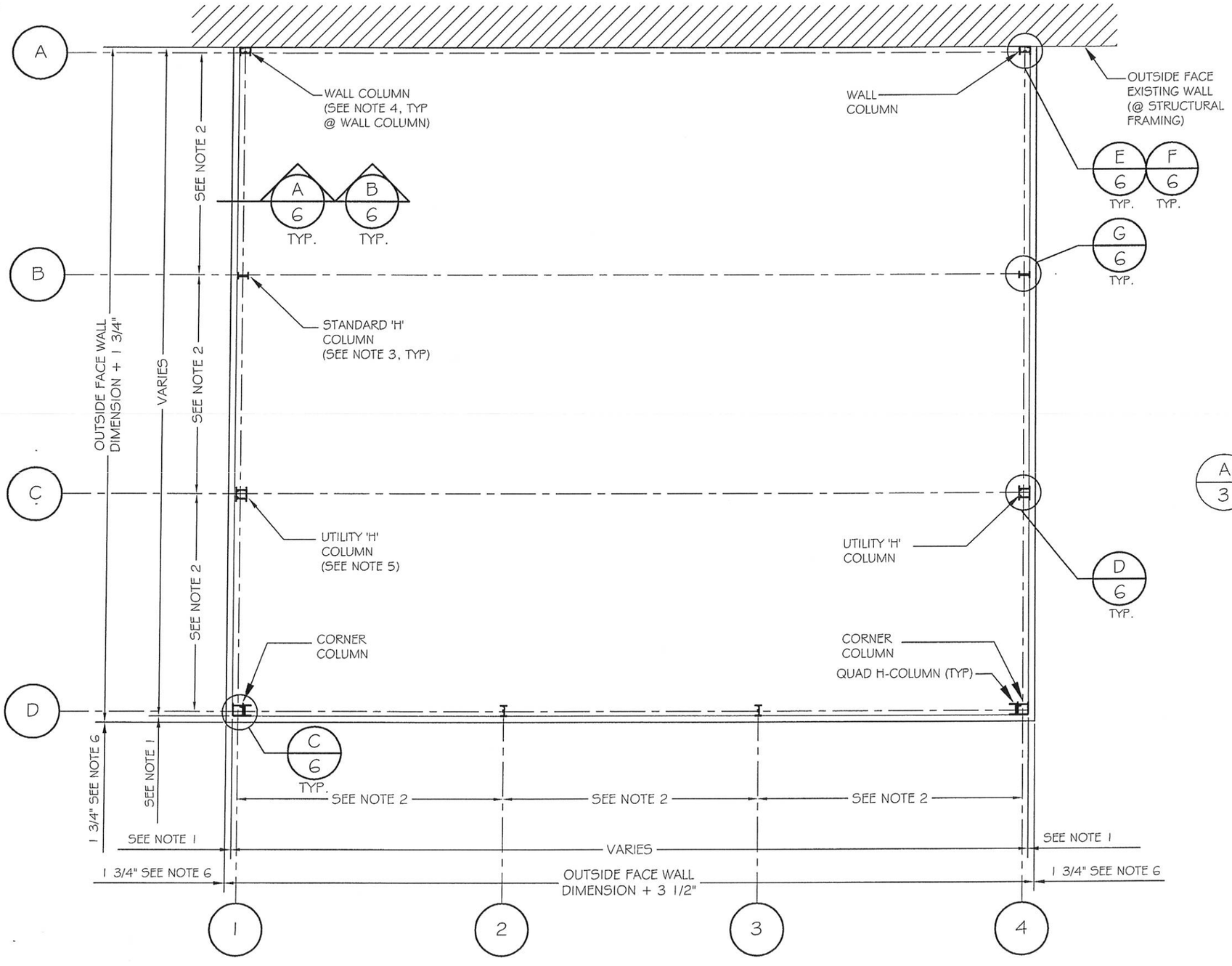
DOCUMENTS ARE VALID ONLY WITH AN ORIGINAL STAMP AND DATED SIGNATURE

FOUR SEASONS SOLAR PRODUCTS, LLC.
5005 VETERANS MEMORIAL HIGHWAY
HOLBROOK, NEW YORK 11741
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

DRAWN BY: TW	
REVIEWED BY: AS	
DATE: 4/15/04	
REVISION	DATE
B	07-10-07

DRAWING NAME: 230 ST STR EV-025



A/3 FLOOR PLAN SCALE=3/8"=1'-0"

NOTE:
1. DIM. TO OUTSIDE FACE OF WALL.
2. COLUMN SPACING VARIES DEPENDING UPON WINDOW & DOOR SIZES & CONFIGURATION. 6'-3 1/4" MAX. ON CENTER SPACING.
3. ACTUAL H- CHANNEL IS SLIGHTLY LESS THAN 1 3/4";USING 1 3/4" IN CALCULATIONS OF WALL DIMENSIONS HELPS TO ACCOUNT FOR MANUFACTURING TOLERANCES IN WINDOW AND DOOR DIMENSIONS.
4. IF H- CHANNEL IS OMITTED EXPOSED WIDTH OF CLOSED SILL IS 1 3/8" (THIS WILL NOT ALLOW FOR AN OUT OF PLUMBHOUSE WALL).
5. MINIMUM FILL WIDTH 1 1/8" (H-CHANNEL BUTTING TO CORNER OR WINDOW/DOOR DIRECTLY INTO CORNER).
6. MINIMUM FOUNDATION PROJECTION REQ'D TO ACHIEVE DESIRED ANCHOR BOLT CAPACITY (FOR CONCRETE FOUNDATION ONLY).

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5005 VETERANS MEMORIAL HIGHWAY
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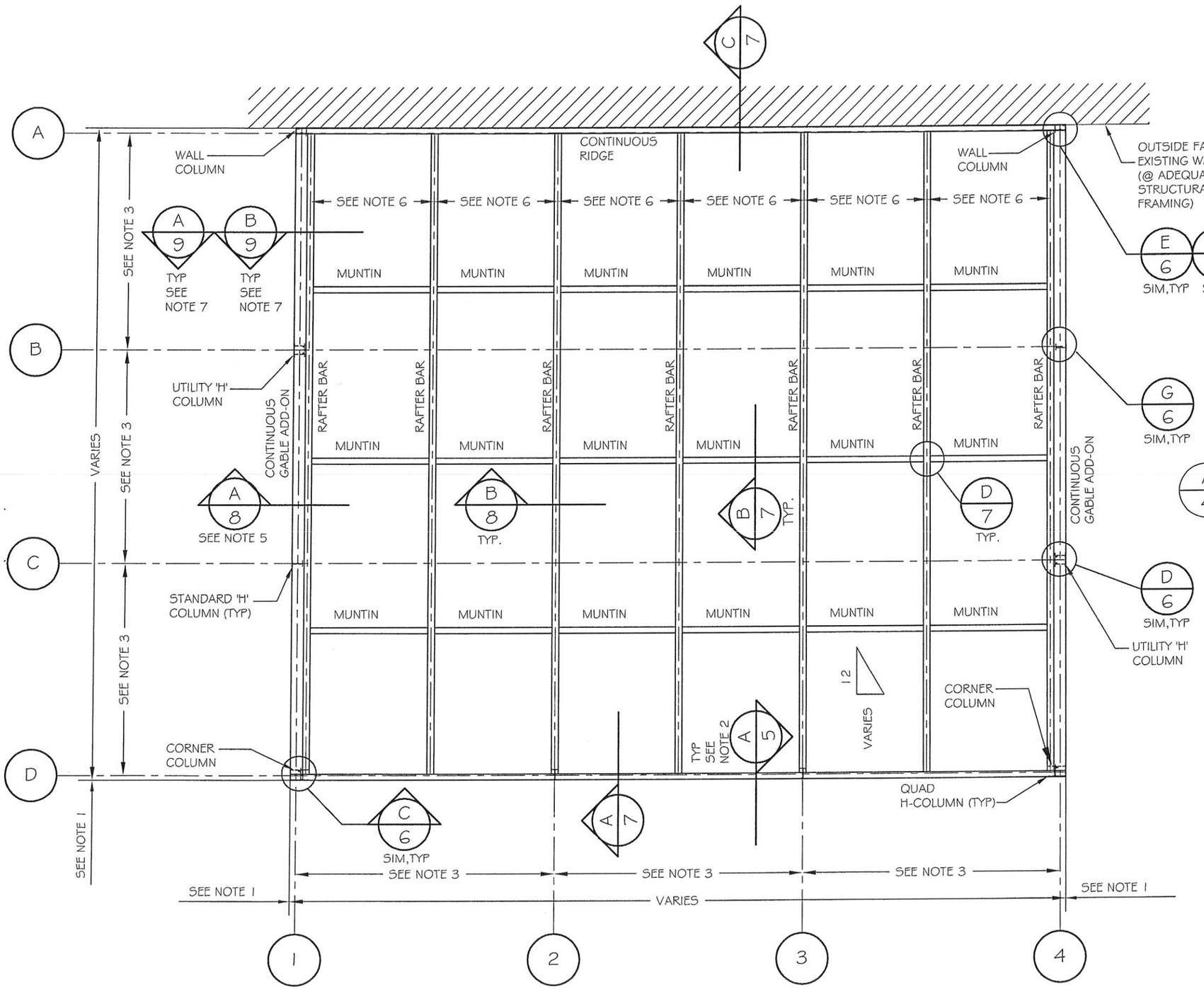
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

DRAWN BY: TW	
REVIEWED BY: AS	
DATE: 4/15/04	
REVISION	DATE
B	07-10-07

DRAWING NAME:
230 ST STR EV-03



A/4 ROOF FRAMING PLAN SCALE=3/8"=1'-0"

NOTE:
1. DIM. TO OUTSIDE FACE OF WALL.
2. WALL SECTIONS VARY DEPENDING UPON MODEL CONFIGURATION.
3. COLUMN SPACING VARIES DEPENDING UPON WINDOW & DOOR SIZES & CONFIGURATION. 6'-3 1/4" MAX. ON CENTER SPACING.
4. SEE PAGE 3 "FLOOR PLAN" FOR NOTES NOT SHOWN.
5. APPLIES ONLY WHEN AN EXISTING WALL IS PRESENT.
6. 30 5/8" & 36 5/8" BAY SPACING VARIES DEPENDING ON ROOM MODEL.
7. FOR ADDITIONAL OPTIONS SEE SHEET 9A.

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SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

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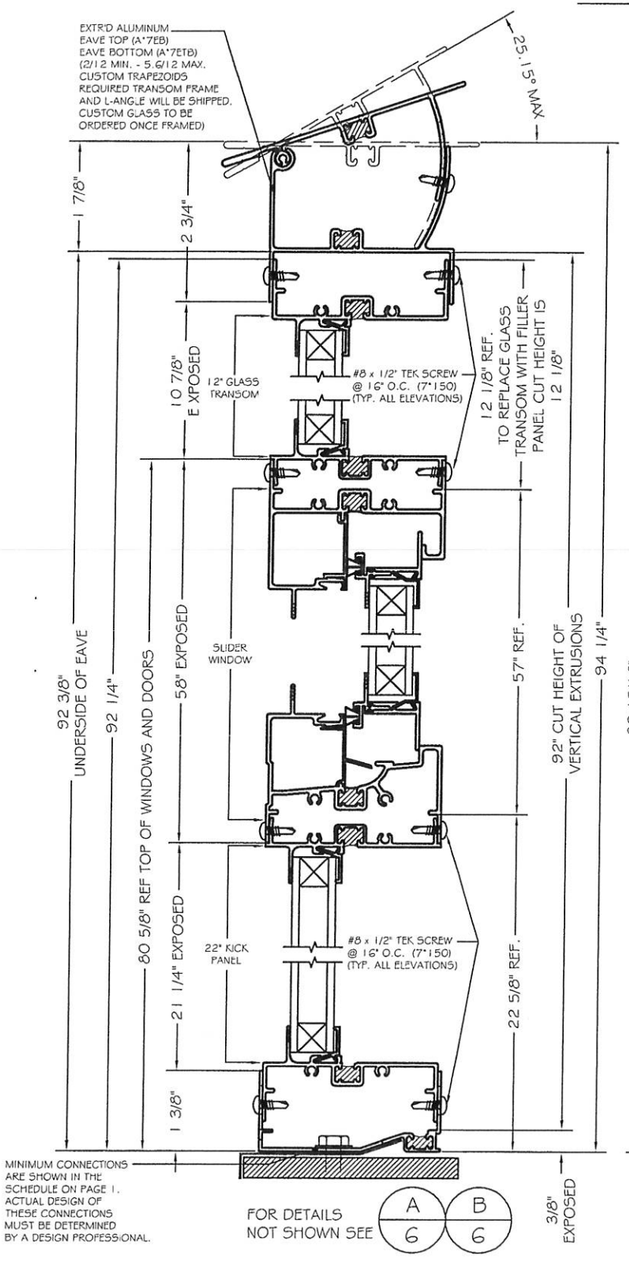
REVIEWED BY: AS	
DATE: 4/15/04	
REVISION	DATE
B	07-10-07

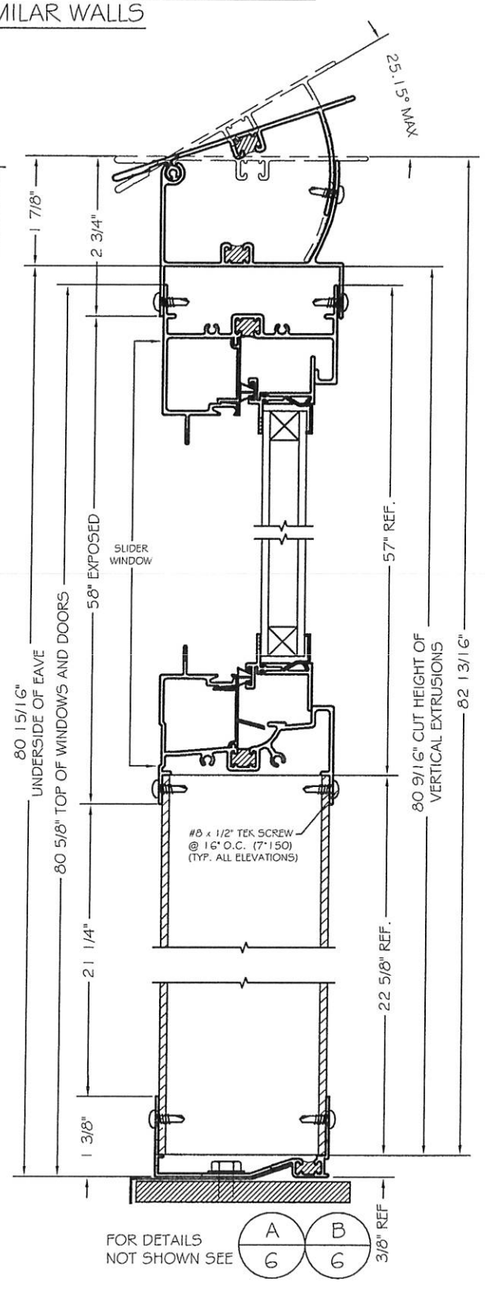
DRAWING NAME: 230 ST STR EV-04

DOCUMENTS ARE VALID ONLY WITH AN ORIGINAL STAMP AND DATED SIGNATURE

FRONT WALL SECTIONS GABLE-END WALLS SIMILAR WALLS



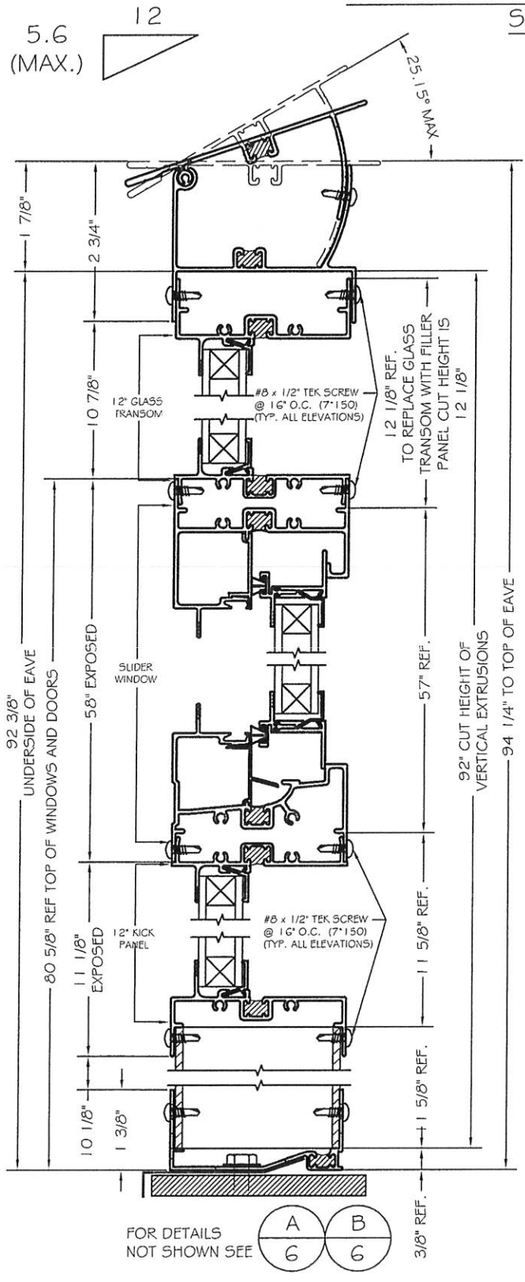
A/5 22" GLASS KICK PANEL-WINDOW NOT TO SCALE
12" GLASS TRANSOM w/ADJUSTABLE EAVE



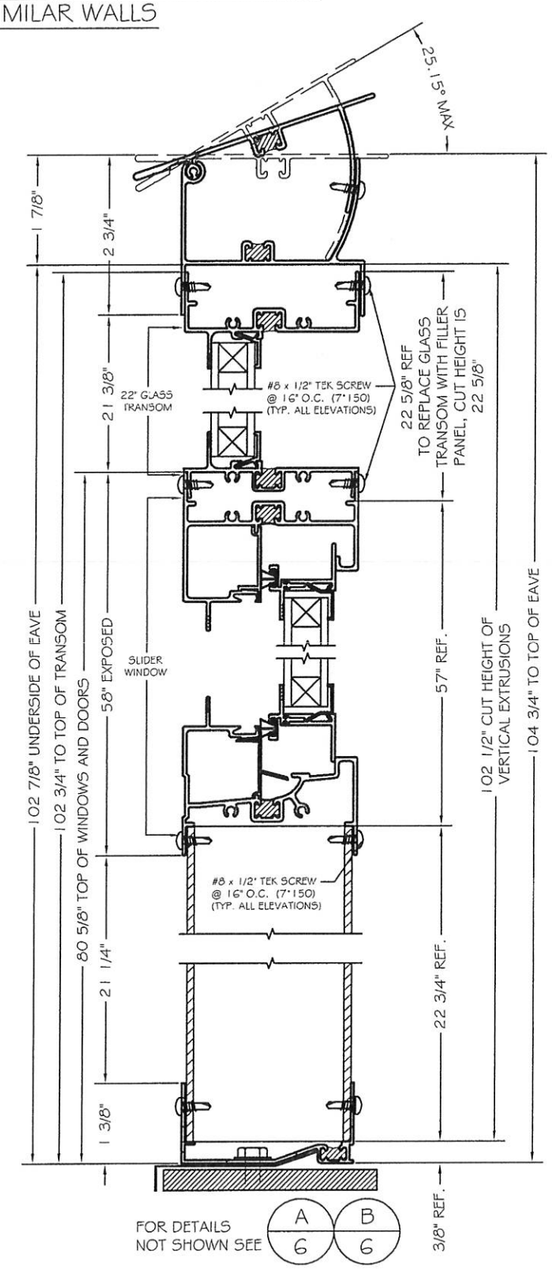
A/5 SOLID KICK PANEL-WINDOW NOT TO SCALE
NO TRANSOM w/ADJUSTABLE EAVE

FRONT WALL SECTIONS GABLE-END WALLS SIMILAR WALLS

5.6 (MAX.) 12



A/5 12" GLASS KICK PANEL-WINDOW NOT TO SCALE
12" GLASS TRANSOM w/ADJUSTABLE EAVE



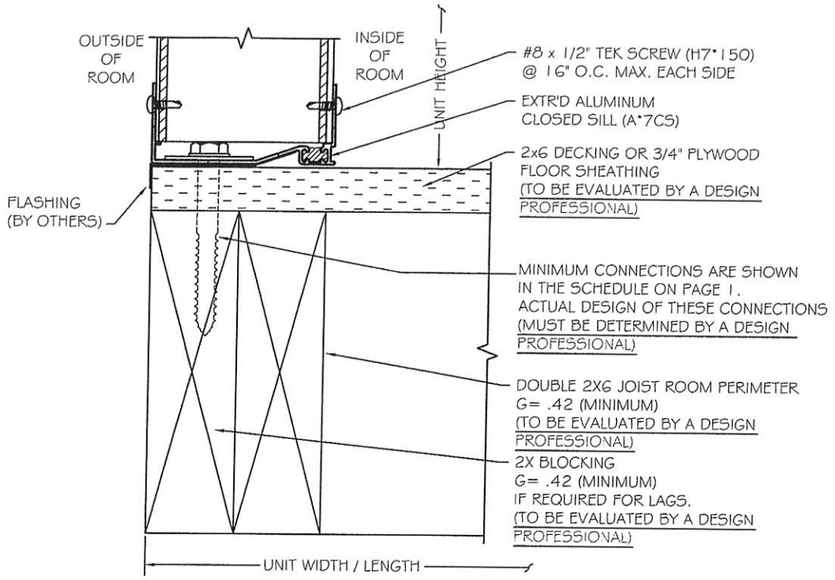
A/5 SOLID KICK PANEL-WINDOW NOT TO SCALE
22" GLASS TRANSOM w/ADJUSTABLE EAVE

FOUR SEASONS SOLAR PRODUCTS, LLC.
5005 VETERANS MEMORIAL HIGHWAY
HOLBROOK, NEW YORK 11741
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

DRAWN BY: TW	
REVIEWED BY: AS	
DATE: 4/15/04	
REVISION	DATE
B	07-10-07

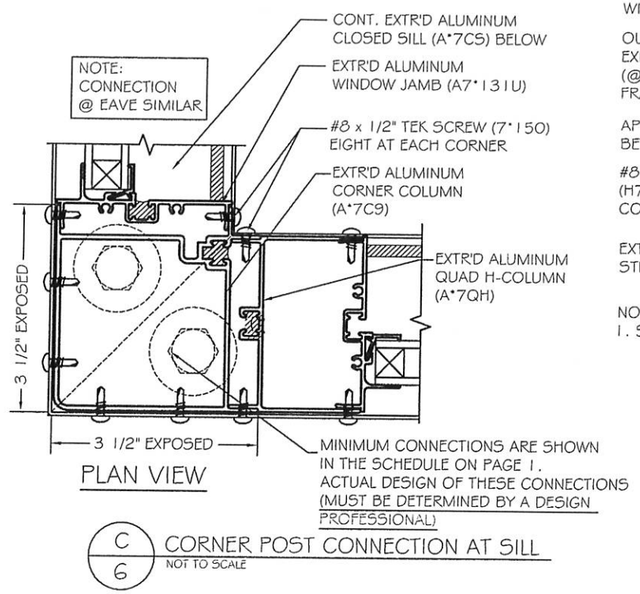
DRAWING NAME: 230 ST STR EV-05ADJ



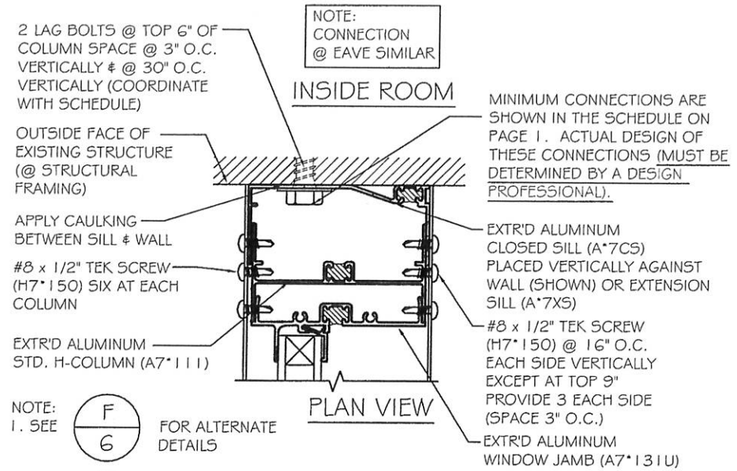
NOTE: 1. SEE B/6 FOR ALTERNATE DETAILS

2. CONNECTION TO FOUR SEASONS "CONSERVA DECK" SAME AS SHOWN

A/6 SILL TO DECK CONNECTION DETAIL
NOT TO SCALE

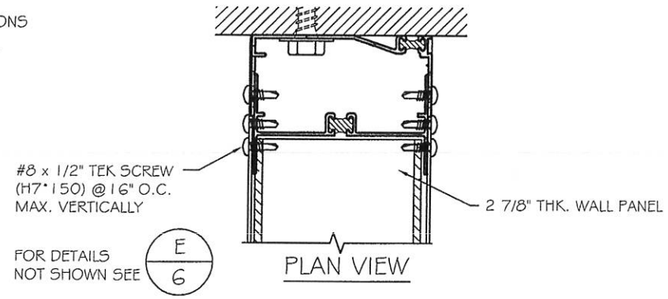


C/6 CORNER POST CONNECTION AT SILL
NOT TO SCALE



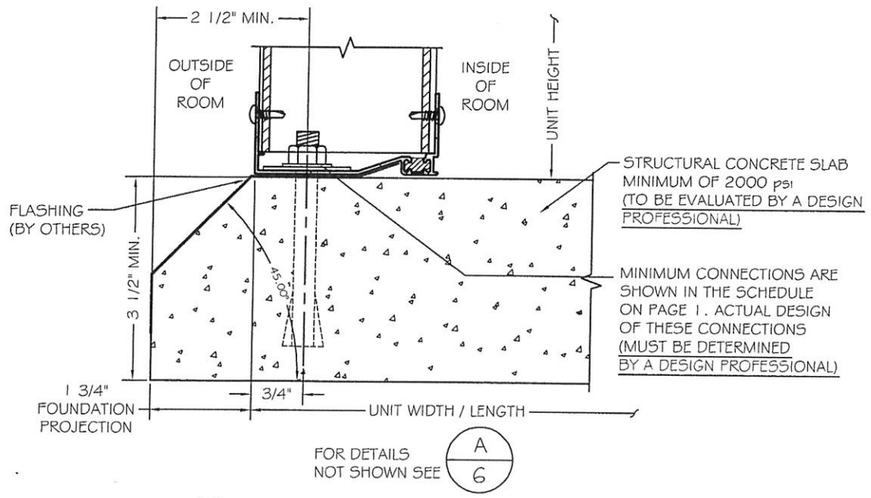
NOTE: 1. SEE F/6 FOR ALTERNATE DETAILS

E/6 GABLE ATTACHMENT AT HOUSEWALL w/WINDOW
NOT TO SCALE



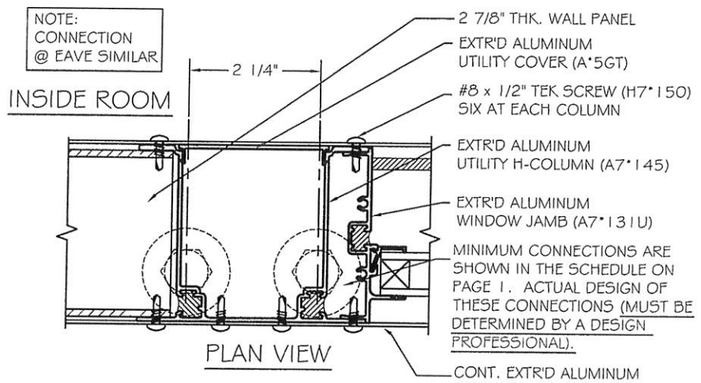
FOR DETAILS NOT SHOWN SEE E/6

F/6 GABLE ATTACHMENT AT HOUSEWALL w/SOLID PANEL
NOT TO SCALE

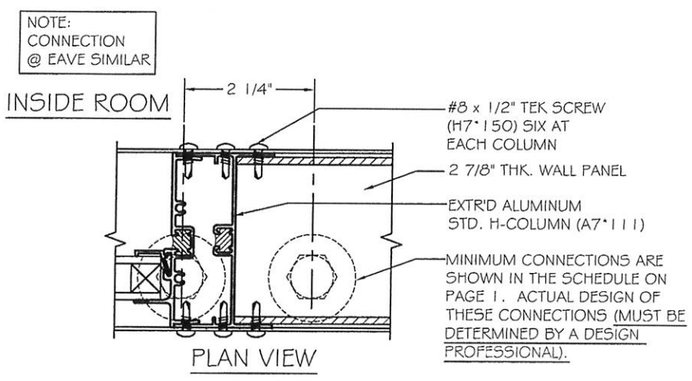


FOR DETAILS NOT SHOWN SEE A/6

B/6 SILL TO CONCRETE SLAB CONNECTION DETAIL
NOT TO SCALE



D/6 UTILITY H-CHANNEL CONNECTION AT SILL
NOT TO SCALE



G/6 H-CHANNEL CONNECTION AT SILL
NOT TO SCALE

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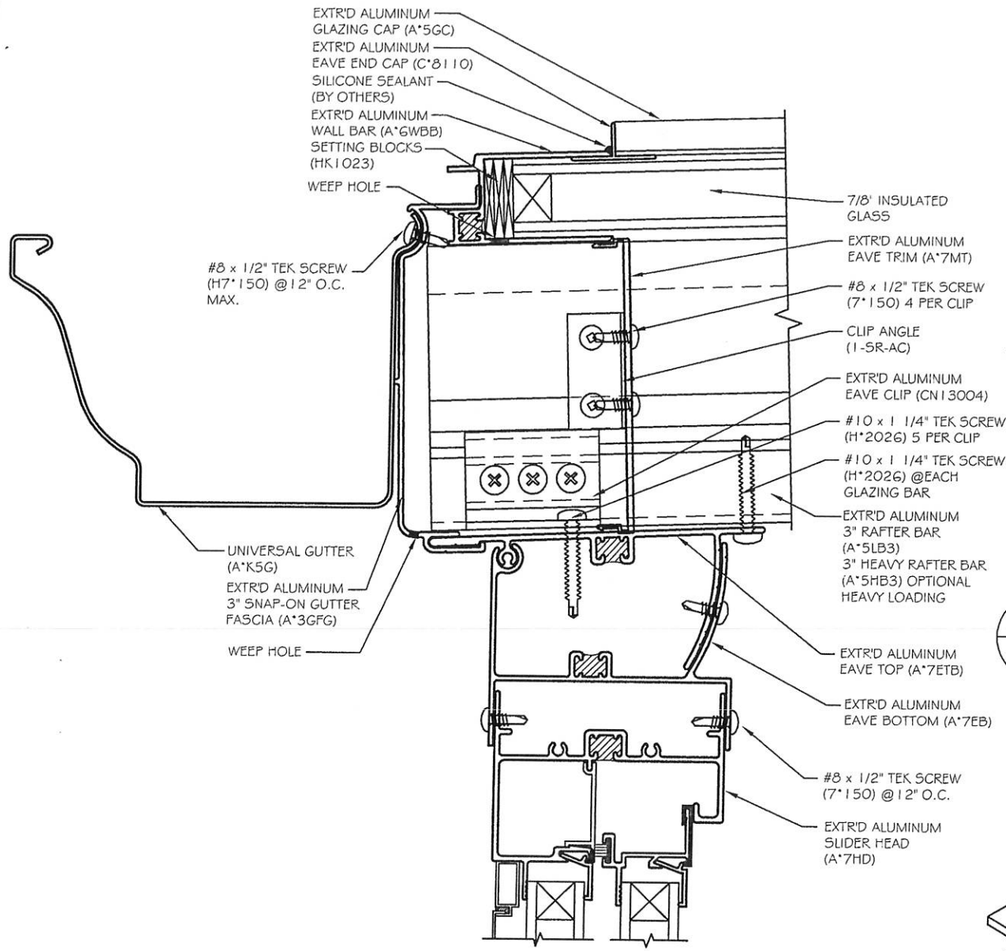
FOUR SEASONS SOLAR PRODUCTS, LLC.
5005 VETERANS MEMORIAL HIGHWAY
HOLBROOK, NEW YORK 11741
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

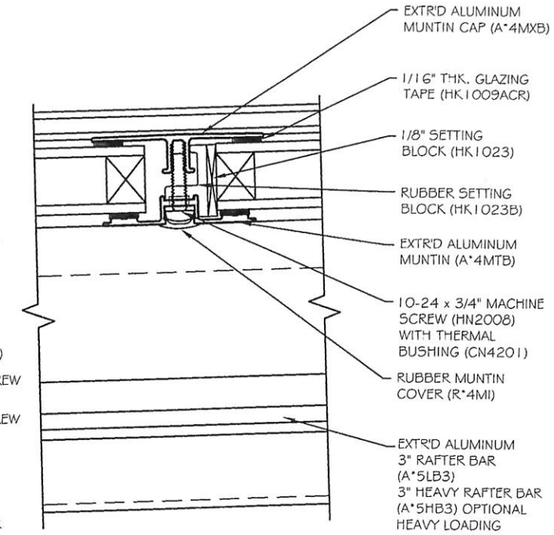
DRAWN BY: TW
REVIEWED BY: AS
DATE: 4/15/04

REVISION	DATE
B	07-10-07

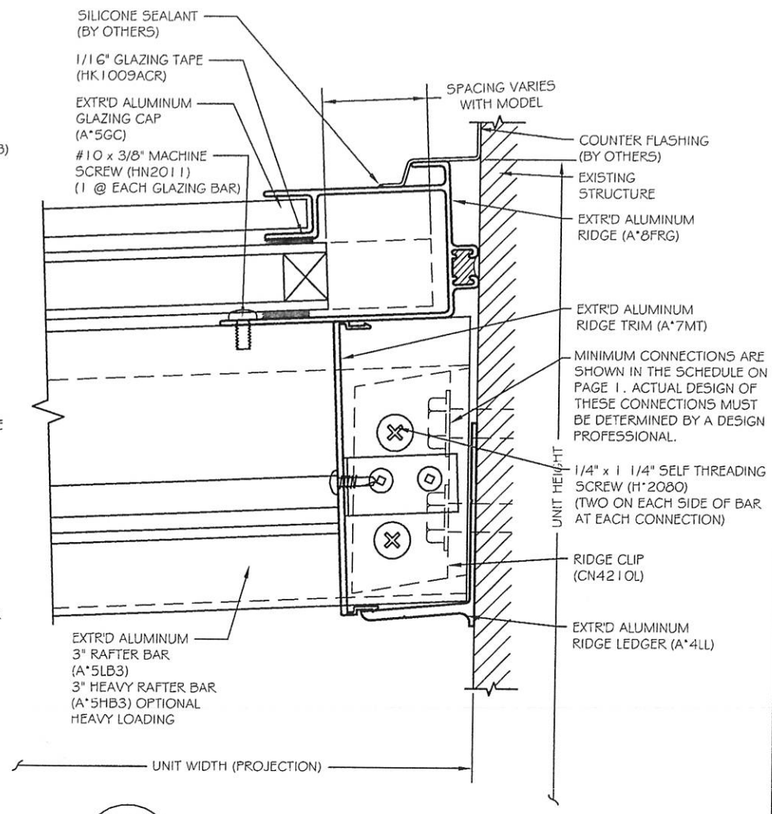
DRAWING NAME: 230 ST STR EV-06



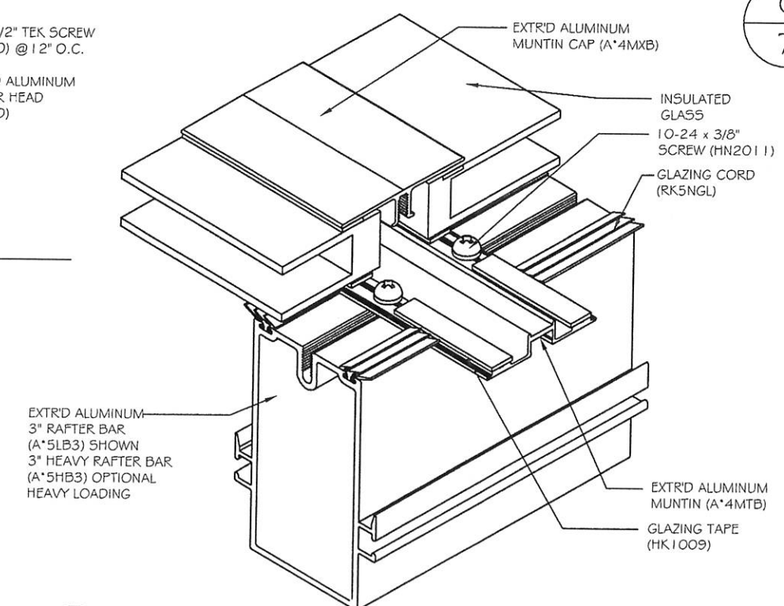
A/7 EAVE DETAIL
SCALE: 6" = 1'-0"



B/7 3" CROSS MUNTIN SECTION
SCALE: 6" = 1'-0"



C/7 3" RIDGE SECTION
SCALE: 6" = 1'-0"



D/7 CROSS MUNTIN & RAFTER BAR CONNECTION
SCALE: 6" = 1'-0"

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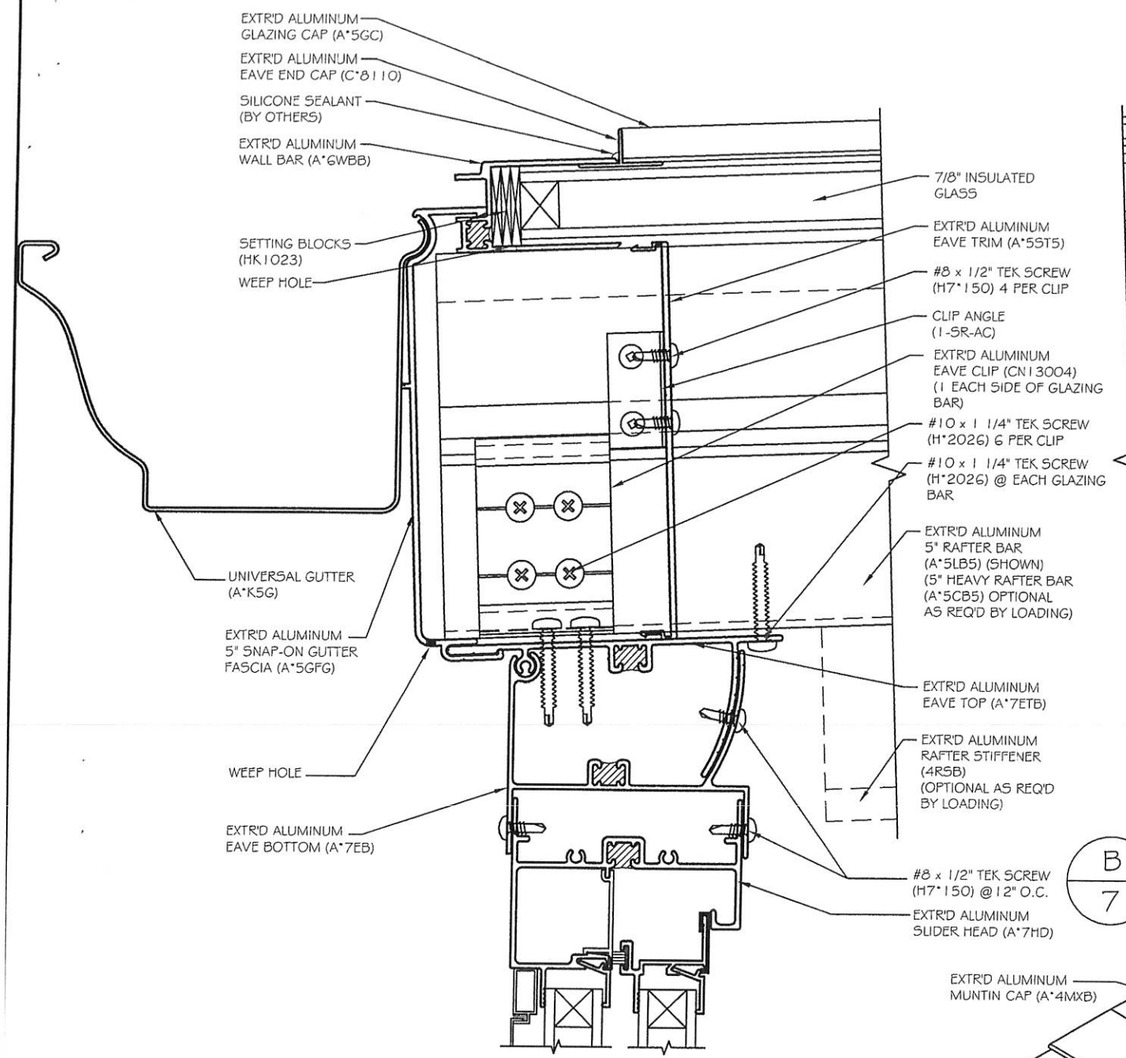
FOUR SEASONS SOLAR PRODUCTS, LLC.
5005 VETERANS MEMORIAL HIGHWAY
HOLBROOK, NEW YORK 11741
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

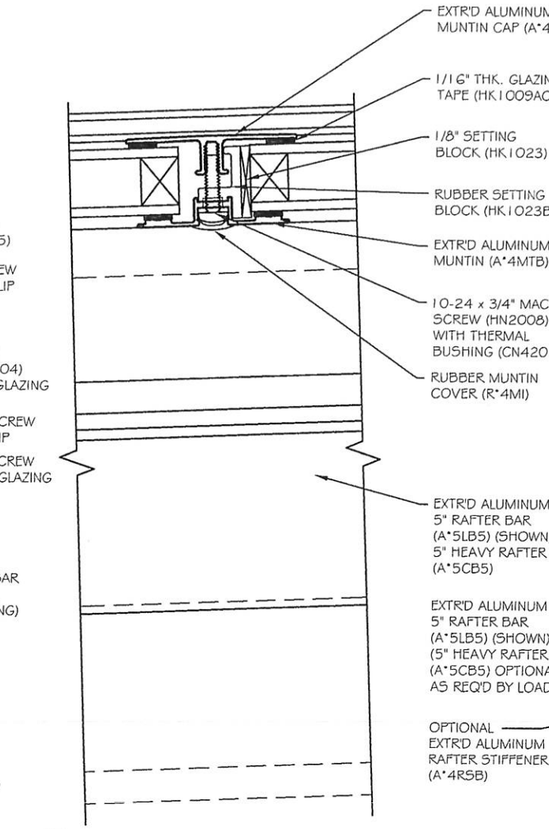
SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

DRAWN BY: TW	
REVIEWED BY: AS	
DATE: 4/15/04	
REVISION	DATE
B	07-10-07

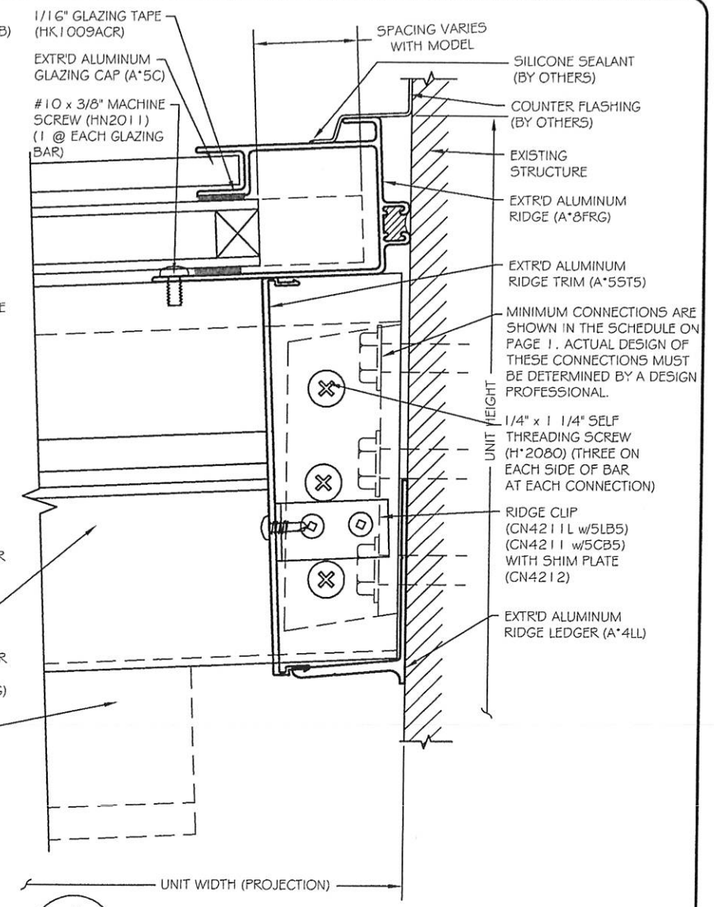
DRAWING NAME: 230 ST STR EV-07-3B



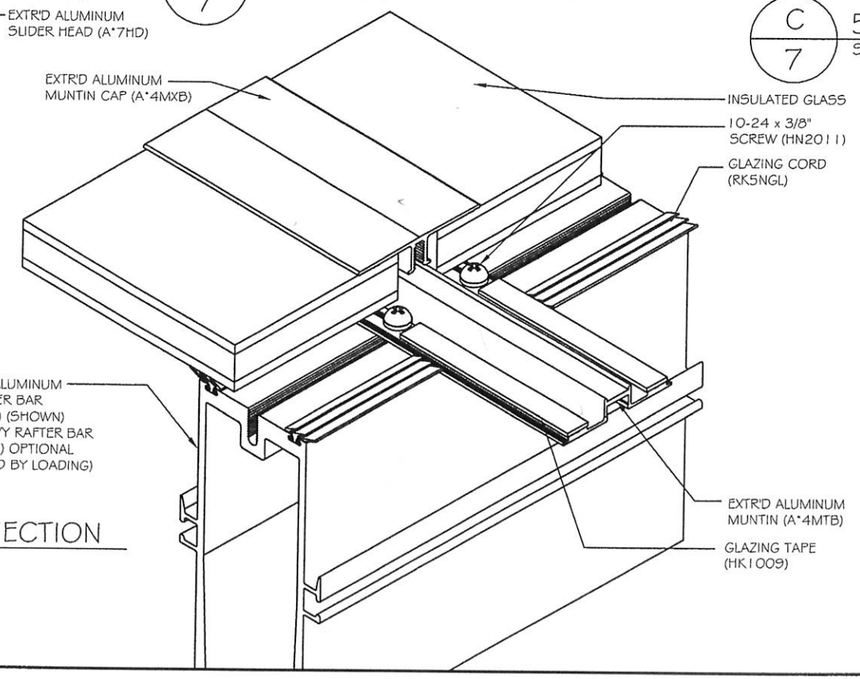
A/7 5" EAVE SECTION
SCALE: 6" = 1'-0"



B/7 5" CROSS MUNTIN SECTION
SCALE: 6" = 1'-0"



C/7 5" RIDGE SECTION
SCALE: 6" = 1'-0"



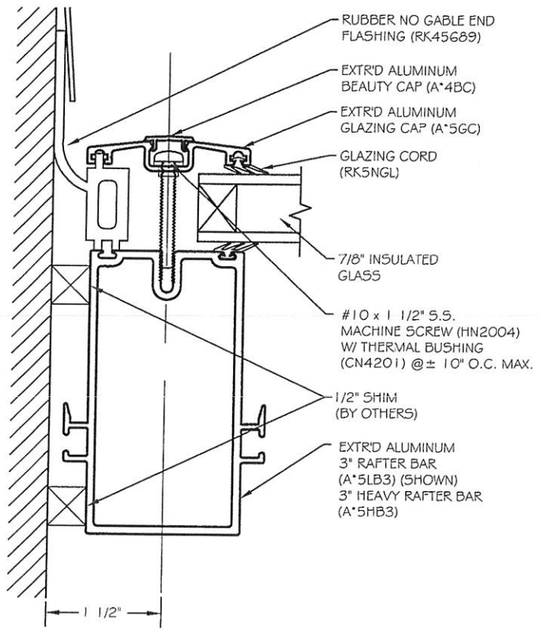
D/7 CROSS MUNTIN & RAFTER BAR CONNECTION
SCALE: 6" = 1'-0"

DOCUMENTS ARE VALID ONLY WITH AN ORIGINAL STAMP AND DATED SIGNATURE

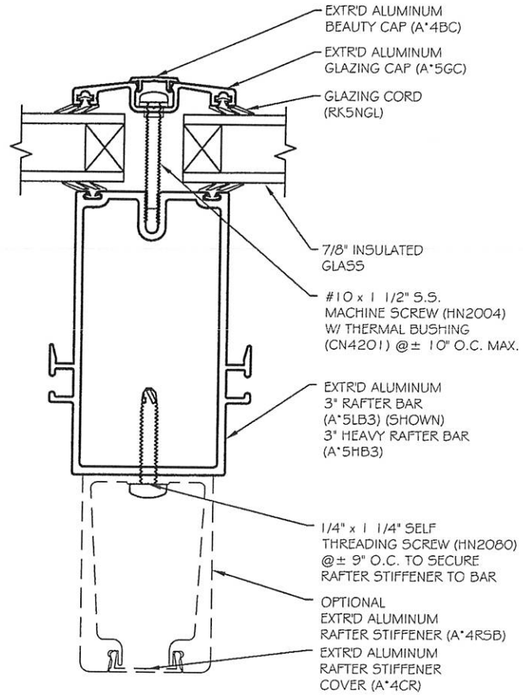
FOUR SEASONS SOLAR PRODUCTS, LLC.
5005 VETERANS MEMORIAL HIGHWAY
HOLBROOK, NEW YORK 11741
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

DRAWN BY: TW	
REVIEWED BY: AS	
DATE: 4/15/04	
REVISION	DATE
B	07-10-07
DRAWING NAME: 230 ST STR EV-07-5B	
PAGE 7 OF 9	



A/8 NO GABLE END SECTION
SCALE: 6" = 1'-0"



B/8 3" RAFTER BAR SECTION
SCALE: 6" = 1'-0"

DOCUMENTS ARE VALID ONLY WITH AN ORIGINAL STAMP AND DATED SIGNATURE

FOUR SEASONS SOLAR PRODUCTS, LLC.
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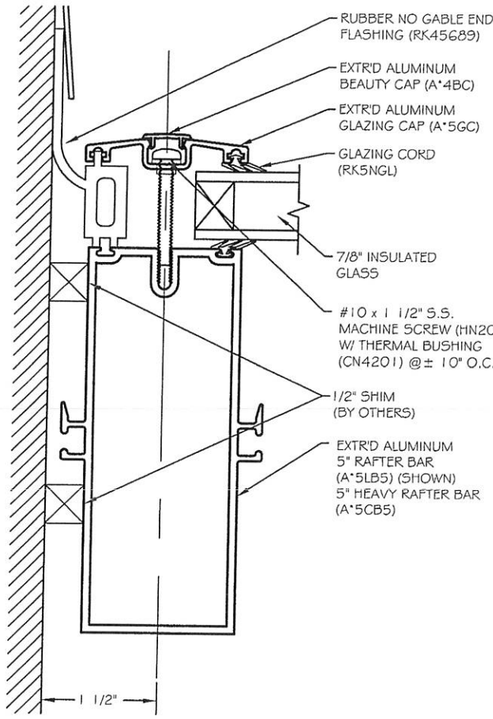
HOLBROOK, NEW YORK 11741
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

DRAWN BY: TW
REVIEWED BY: AS
DATE: 4/15/04

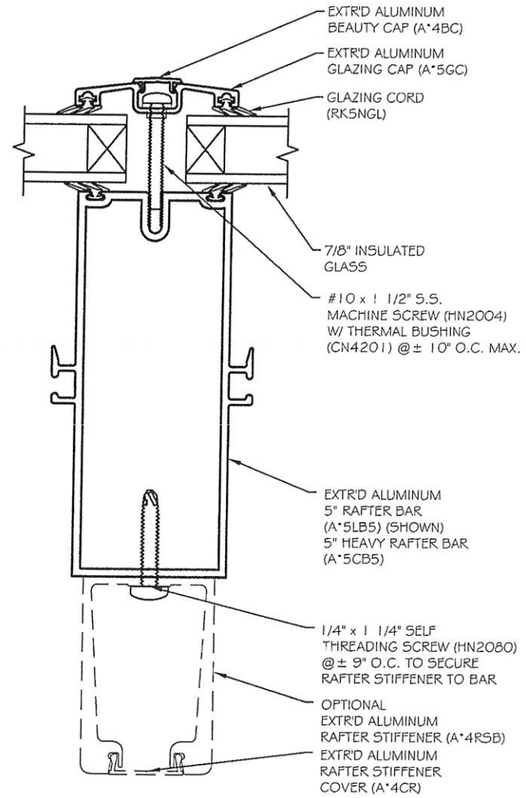
REVISION	DATE
B	07-10-07

DRAWING NAME: 230 ST STR EV-08-3B



A/8 NO GABLE END SECTION

SCALE: 6" = 1'-0"



B/8 5" RAFTER BAR SECTION

SCALE: 6" = 1'-0"

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FOUR SEASONS SOLAR PRODUCTS, LLC.

5005 VETERANS MEMORIAL HIGHWAY
HOLBROOK, NEW YORK 11741
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

SYSTEM TYPE:

SERIES 230 SUN & STARS

STRAIGHT EAVE DESIGN

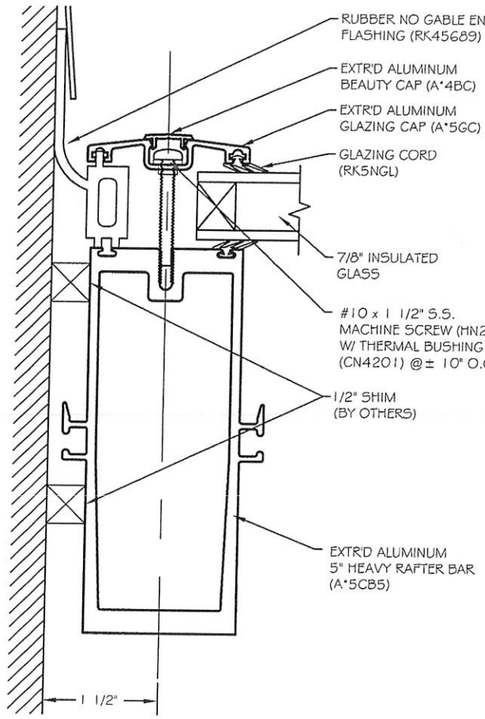
DRAWN BY: TW

REVIEWED BY: AS

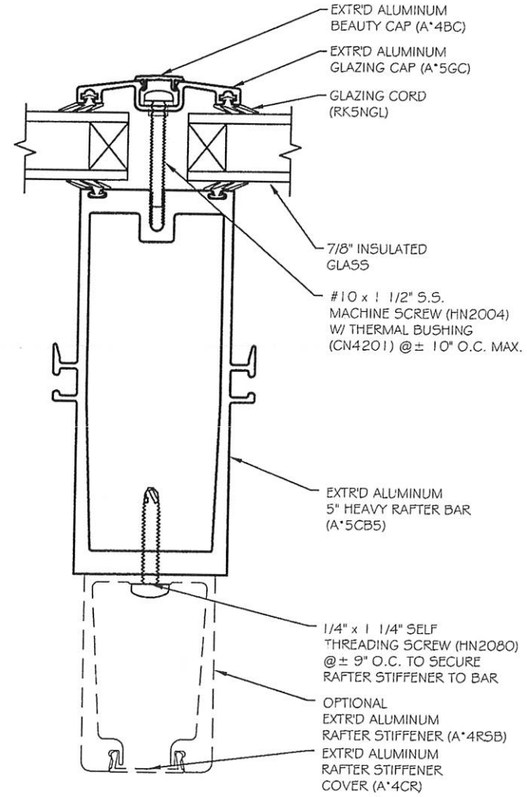
DATE: 4/15/04

REVISION	DATE
B	07-10-07

DRAWING NAME: 230 ST STR EV-08-5B



A/8 NO GABLE END SECTION
SCALE: 6" = 1'-0"



B/8 5" RAFTER BAR SECTION
SCALE: 6" = 1'-0"

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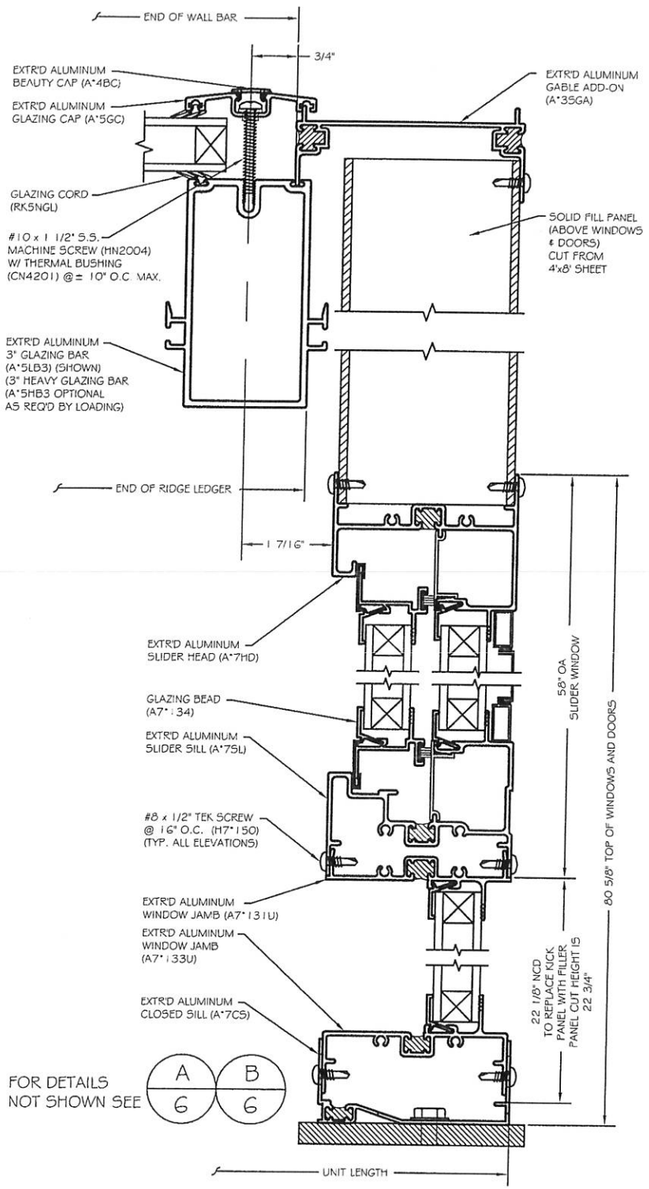
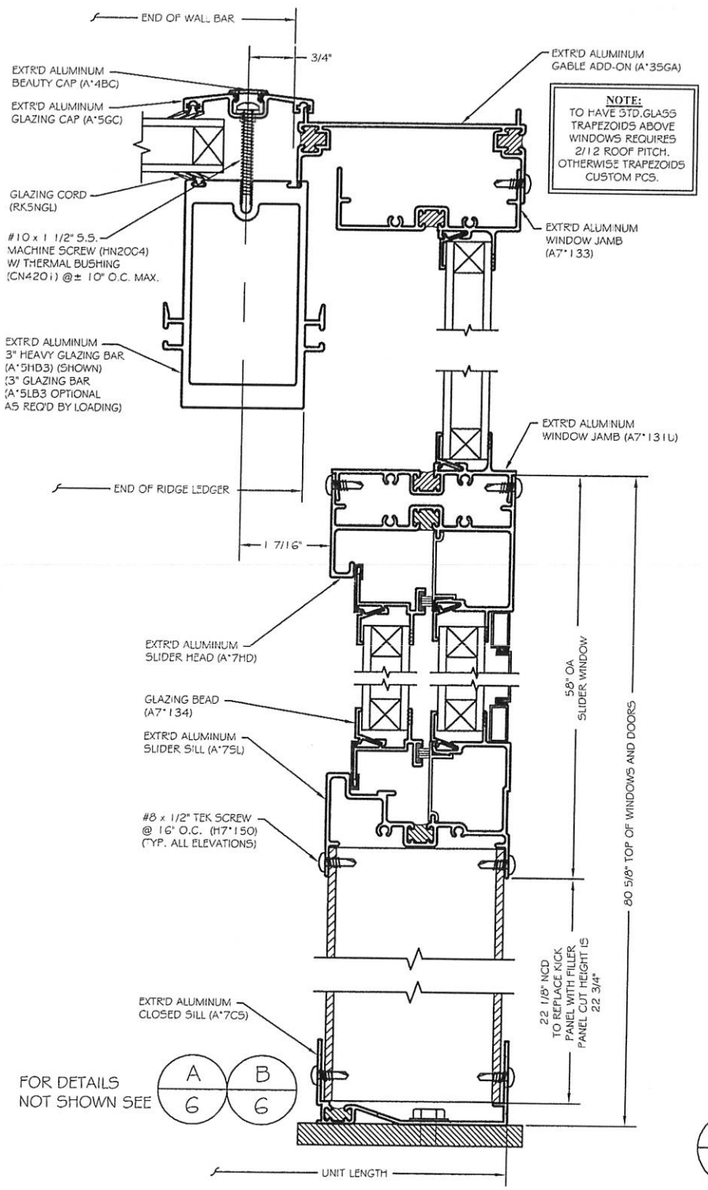
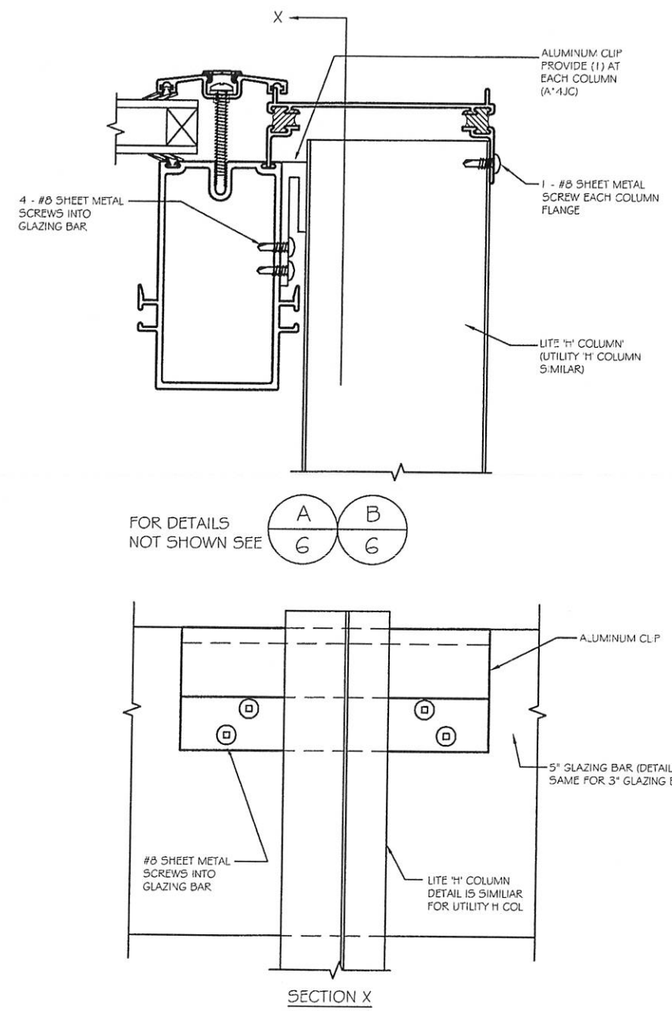
DRAWING NAME:
230 ST STR EV-08-5B



A/9 GABLE END WALL SECTION w/ SOLID TRAP
NOT TO SCALE



B/9 GABLE END WALL SECTION w/ STD. OR CUSTOM GLASS TRAP
NOT TO SCALE



C/9 STRAIGHT EAVE GABLE WALL SECTION @ COLUMN CONNECTION
NOT TO SCALE

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DRAWN BY: TW
REVIEWED BY: AS
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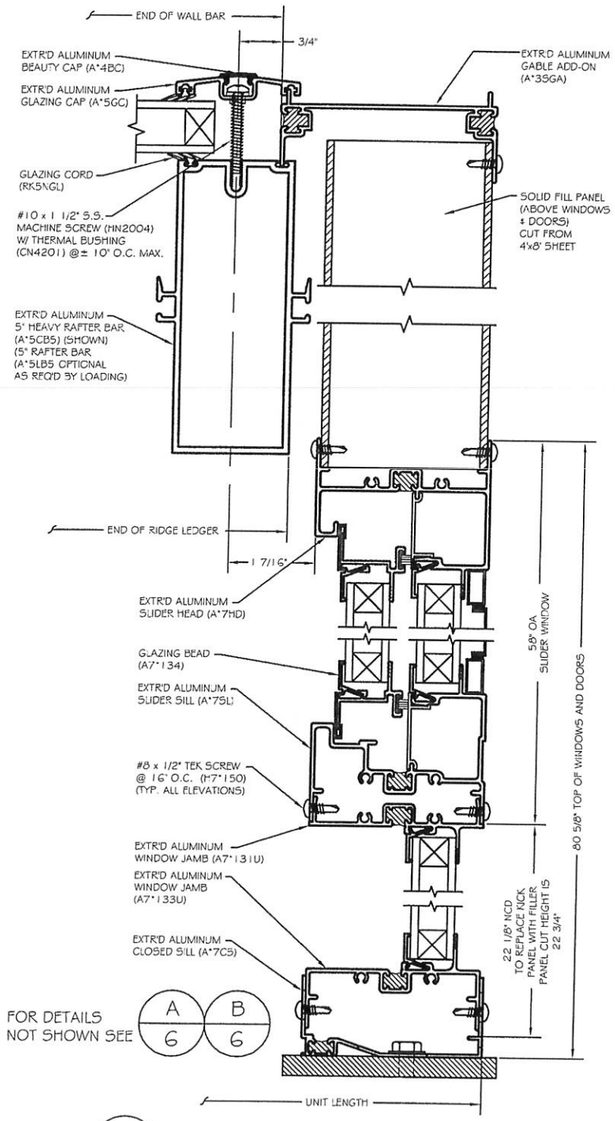
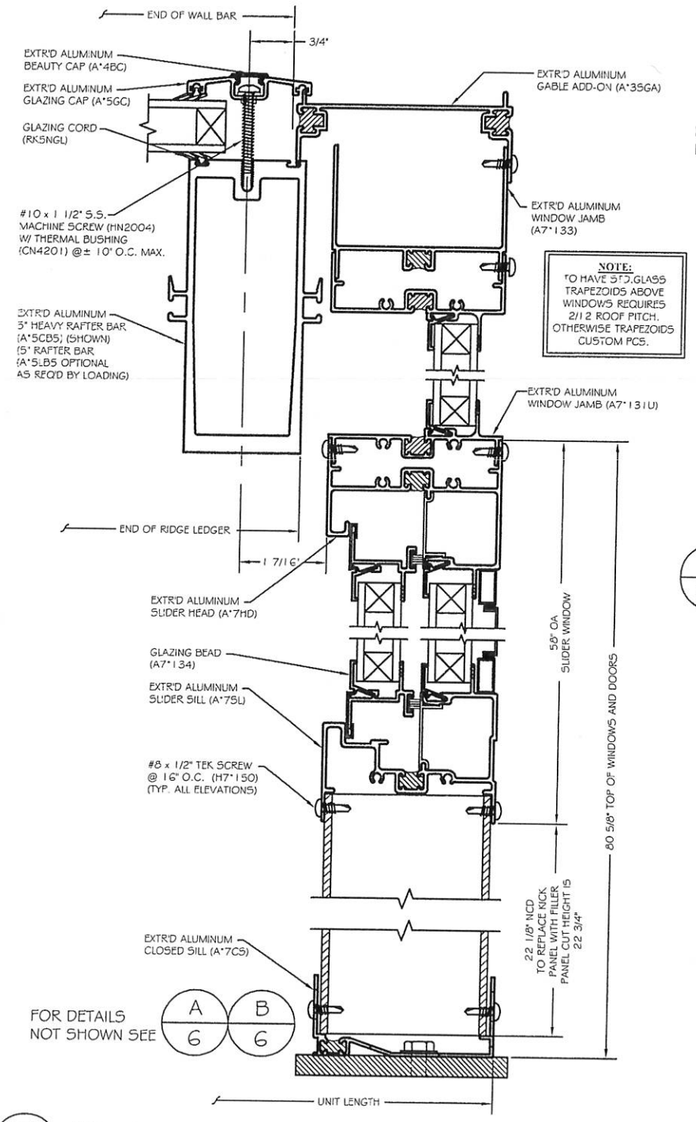
REVISION	DATE
B	07-10-07

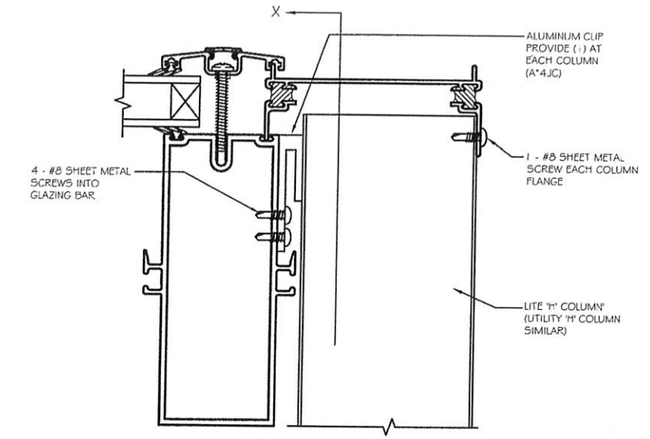
DRAWING NAME: 230 ST STR EV-09-3B



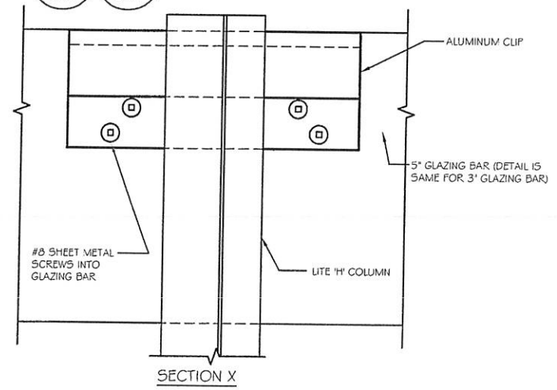
A 9A GABLE WALL SECTION w/ SOLID TRAP
SCALE: 1/2" = 1'-0"



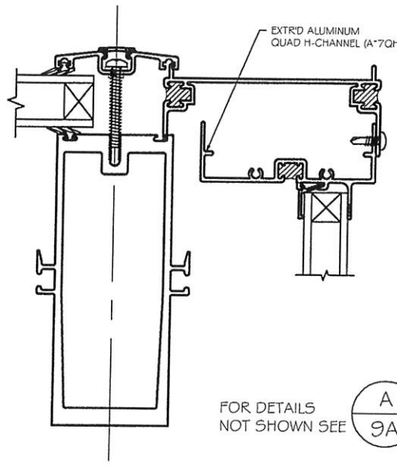
B 9A STD. GABLE WALL SECTION w/ STD. OR CUSTOM GLASS TRAP
SCALE: 1/2" = 1'-0"



FOR DETAILS NOT SHOWN SEE A 9A B 9A



D 9A STRAIGHT EAVE GABLE WALL SECTION @ COLUMN SUPPORT
NOT TO SCALE



FOR DETAILS NOT SHOWN SEE A 9A

C 9A OPTIONAL GABLE WALL SECTION w/ CUSTOM TRAP
SCALE: 1/2" = 1'-0"

FOUR SEASONS SOLAR PRODUCTS, LLC.

5005 VETERANS MEMORIAL HIGHWAY
HOLBROOK, NEW YORK 11741
DESIGNERS AND MANUFACTURER OF FOUR SEASONS SUNROOMS

SYSTEM TYPE:
SERIES 230 SUN & STARS
STRAIGHT EAVE DESIGN

DRAWN BY: TW	
REVIEWED BY: AS	
DATE: 4/15/04	
REVISION	DATE
B	07-10-07

DRAWING NAME: 230 ST STR EV-09-5B