010000 GENERAL

1. CONFORM TO THE REQUIREMENTS OF THE BUILDING CODE OF IBC 2015, LATEST EDITION, AND ALL OTHER APPLICABLE LOCAL CODES AND REGULATIONS OF AGENCIES HAVING JURISDICTION.
2. READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH THE SPECIFICATIONS AND ALL OTHER CONTRACT DOCUMENTS.
3. BEFORE PROCEEDING WITH WORK, CHECK ALL THE DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND REPORT DISCREPANCIES TO THE CONSULTANT.
4. REFER TO THE ARCHITECTURAL AND OTHER DRAWINGS FOR LOCATIONS AND DIMENSIONING OF OPENINGS AND SLEEVES NOT SHOWN ON THE STRUCTURAL DRAWINGS. HOWEVER, OBTAIN THE CONSULTANT'S PRIOR APPROVAL BEFORE INSTALLING OPENINGS, SLEEVES, ETC. WHICH ARE NOT SHOWN ON STRUCTURAL DRAWINGS.
5. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS OF PITS, BASES, SUMPS, TRENCHES, DEPRESSIONS, GROOVES, CURBS, CHAMFERS AND SLOPES NOT SHOWN ON STRUCTURAL DRAWINGS.
6. HORIZONTAL AND VERTICAL DESIGN LOADS ARE NOTED. THEY SHALL NOT BE EXCEEDED DURING CONSTRUCTION.
7. TYPICAL STRUCTURAL DETAILS SHALL GOVERN THE WORK. IF DETAILS DIFFER ON THE DRAWINGS, THE MOST STRINGENT SHALL GOVERN.
8. ALL TEMPORARY WORKS INCLUDING SHORING ARE TO BE PROVIDED BY THE CONTRACTOR.

010001 DESIGN NOTES

1. ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS AND REQUIREMENTS OF THE FOLLOWING CODES:
2. THE IBC 2015, AND ALL OTHER APPLICABLE LOCAL CODES AND REGULATIONS HAVING JURISDICTION.
3. AMERICAN SOCIETY OF CIVIL ENGINEERS: ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.
4. AMERICAN CONCRETE INSTITUTE (ACI): ACI-318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
5. AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC): AISC-325 AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL 14TH EDITION.
6. AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC): AISC 360-10 SPECIFICATIONS FOR STRUCTURAL STEEL STRUCTURES.
7. AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC): AISC-341-10 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS.
8. AMERICAN WOOD COUNCIL (AWC): NDS-2015 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION COMMENTARY - WITH SUPPLEMENT 2015 EDITION.
9. AMERICAN WOOD COUNCIL (AWC): SDPWS-2015 SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC.
10. FORCES ON STRUCTURAL FRAME:

A. LIVE: VARIES REFER TO NOTES UNDER PLANS
B. DEAD: VARIES REFER TO NOTES UNDER PLANS
C. SNOW:
   - EXPOSURE FACTOR (CE) = 1.0
   - THERMAL FACTOR (CT) = 1.0
   - IMPORTANCE FACTOR (I) = 1
   - ROOF SLOPE FACTOR (CS) = 1
   - GROUND SNOW LOAD (PG): = 270psf
   - FLAT ROOF SNOW LOAD (PF): = 189psf
   - SLOPED ROOF SNOW LOAD (PS): = 189psf
   - FROST DEPTH: = 40in
D. WIND:
   - BASIC WIND SPEED (V): =115mph
   - WIND IMPORTANCE FACTOR (I): = 1
   - EXPOSURE FACTOR: = C

11. SEISMIC ANALYSIS:

E. SEISMIC IMPORTANCE FACTOR (I): = 1
F. RISK CATEGORY: = II

G. SPECTRAL RESPONSE ACCEL (Ss): = 0.831g
H. SPECTRAL RESPONSE ACCEL (S1): = 0.277g
I. SITE CLASSIFICATION: = C
J. DESIGN SPECTRAL RESPONSE (SDS): = 0.592g
K. DESIGN SPECTRAL RESPONSE (SD1): = 0.281g
L. SEISMIC DESIGN CATEGORY: = D

12. LATERAL LOAD RESISTING SYSTEMS

M. THE LATERAL FORCES ARE RESISTED BY:
   I) LATERAL SYSTEM: STEEL SPECIAL CONCENTRICALLY BRACED FRAMES - ABOVE L3
   - RESPONSE MOD. COEFFICIENT(R): 6
   - OVERSTRENGTH FACTOR(Ω): 2
   - DEFLECTION MODIFICATION FACTOR(Cd): 5

   II) LATERAL SYSTEM: STEEL ORDINARY CONCENTRICALLY BRACED FRAMES - BELOW L3
   - RESPONSE MOD. COEFFICTIENT(R): 3.25
   - OVERSTRENGTH FACTOR(Ω): 2
   - DEFLECTION MODIFICATION FACTOR(Cd): 3.25

   III) LATERAL SYSTEM: ORDINARY CONCRETE SHEAR WALLS BELOW L3
   - RESPONSE MOD. COEFFICTIENT(R): 4
   - OVERSTRENGTH FACTOR(Ω): 2.5
   - DEFLECTION MODIFICATION FACTOR(Cd): 4

N. SEISMIC ANALYSIS PROCEDURE: MODAL RESPONSE SPECRUM
O. ANALYSIS SOFTWARE: ETABS

13. LATERAL LOAD ON FOUNDATIONS

P. A GEOTEHCNICAL REPORT "GEOTCHNICAL AND GEOLOGIC HAZARD INVESTIGATION: LOT 14R OF SUMMIT EDEN PHASE 1A SUMMIT POWDER MOUNTAIN RESORT WEBER COUNTY, UTAH" PROJECT NUMBER 02693-001, DATED AUGUST 8TH, 2018 HAS BEEN PREPARED BY IGES INC. THE CONTRACTOR IS TO READ THE REPORT AND BE FAMILIAR WITH IT'S CONTENTS.
Q. FOUNDATION WALLS ARE DESIGNED ASSUMING THERE IS FREE-DRAINING BACKFILL OR THAT OTHER PROVISIONS HAVE BEEN MADE, SUCH THAT THE WALLS ARE NOT SUBJECT TO HYDROSTATIC PRESSURE.

010003 NOTABLE SUBMITTALS

1. GENERAL REVIEW BY COMPONENT ENGINEERS
   A. COMPONENT ENGINEERS ARE RESPONSIBLE FOR GENERAL REVIEW OF THE CONSTRUCTION FOR THE PORTION OF THE WORK PREPARED UNDER THEIR PROFESSIONAL SEALS. THEY SHALL PROVIDE:
      I) REPORTS FOR EACH SITE VISIT
      II) A PROJECT COMPLETION NOTICE
A. ENGINEERED COMPONENTS INCLUDE; PRECAST CONCRETE, OPEN WEB STEEL JOISTS, METAL DECK, PRE-ENGINEERED WOOD TRUSSES, DEEP FOUNDATIONS, MISCELLANEOUS METALS, STRUCTURAL GLASS, GLASS CONNECTIONS, CURTAINWALL, HELICAL PEIRS, GEOPIERS, MICROPILES.

010004 SUBMITTALS

1. GEOMETRY
   a). SUBMIT SURVEY RECORDS CONFIRMING THAT THE BUILT GEOMETRY MATCHES THE DESIGN GEOMETRY.
2. CONCRETE AND REINFORCEMENT
   a). SUBMIT REINFORCING PLACING DRAWINGS AND BAR LISTS FOR REVIEW BY THE CONSULTANT.
   b). PROVIDE TEST CYLINDERS IN ACCORDANCE WITH THE REQUIREMENTS OF THE ACI BUILDING CODE, ACI-318, BUT A MINIMUM OF 3 CYLINDERS FROM EACH LOAD OF CONCRETE, TO BE TESTED; 1 AT 7 DAYS AND 2 AT 28 DAYS.
3. STRUCTURAL STEEL
   a). DETAIL CONNECTIONS AND THE LIKE IN ACCORDANCE WITH THE AMERICAN SOCIETY OF CIVIL ENGINEERS: ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES FOR THE FORCES SHOWN ON THE DRAWINGS..
   b). SUBMIT SHOP, ERECTION, AND SETTING DRAWINGS FOR REVIEW BY THE CONSULTANT.
4. STEEL JOISTS
   a). DESIGN STEEL JOISTS, BRIDGING, AND THE LIKE IN ACCORDANCE WITH THE BUILDING CODE OF IBC 2015 FOR THE FORCES SHOWN ON THE DRAWINGS.
   b). SUBMIT SHOP DETAILS AND ERECTION DRAWINGS FOR REVIEW BY THE CONSULTANT.
   c). SUBMIT DRAWINGS STAMPED AND SIGNED BY QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF UTAH.
5. ROD AND CABLE SYSTEMS
   a). DESIGN ROD AND CABLE SYSTEMS INCLUDING RODS, CABLES, END FITTINGS, PRETENSIONING DEVICES, FITTINGS, AND THE LIKE IN ACCORDANCE WITH THE BUILDING CODE OF IBC 2015 FOR THE FORCES SHOWN ON THE DRAWINGS.
   b). SUBMIT SHOP DETAILS AND ERECTION DRAWINGS FOR REVIEW BY THE CONSULTANT.
   c). SUBMIT DRAWINGS STAMPED AND SIGNED BY QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF UTAH.
6. METAL DECK
   a). DESIGN DECK IN CONFORMANCE WITH THE REQUIREMENTS OF AISC 360-16, FOR THE FORCES SHOWN ON THE DRAWINGS.
   b). SUBMIT SHOP DRAWINGS STAMPED AND SIGNED BY QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF UTAH.
7. ENGINEERED WOOD FRAMING
   a). DESIGN FRAMING MEMBERS FOR THE FORCES SHOWN ON THE DRAWINGS IN ACCORDANCE WITH THE CODES, RULES, & REGULATIONS OF THE STATE OF UTAH AND THE REQUIREMENTS OF:
   - THE BUILDING CODE OF IBC 2015.
   - AMERICAN WOOD COUNCIL (AWC): NDS-2015 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION COMMENTARY - WITH SUPPLEMENT 2015 EDITION.
   b). DESIGN CONNECTIONS AND THE LIKE IN ACCORDANCE WITH THE CODES, RULES, & REGULATIONS OF THE STATE OF UTAH AND THE REQUIREMENTS OF:
   - THE BUILDING CODE OF IBC 2015.
   - AMERICAN WOOD COUNCIL (AWC): NDS-2015 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION COMMENTARY - WITH SUPPLEMENT 2015 EDITION.
   - AMERICAN WOOD COUNCIL (AWC): SDPWS-2015 SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC.
   c). SUBMIT SHOP DRAWINGS FOR REVIEW BY THE CONSULTANT, INCLUDING GRADES, FINISHES, SHOP AND ERECTION DETAILS, CONNECTIONS AND CAMBER.
   d). SUBMIT CALCULATIONS STAMPED AND SIGNED BY QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF UTAH.
8. PRE-ENGINEERED WOOD TRUSSES
   a). DESIGN TRUSSES IN ACCORDANCE WITH THE CODES, RULES, & REGULATIONS OF THE STATE OF UTAH AND THE REQUIREMENTS OF:
   - THE BUILDING CODE OF IBC 2015.
   - AMERICAN WOOD COUNCIL (AWC): NDS-2015 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION COMMENTARY - WITH SUPPLEMENT 2015 EDITION.
   - AMERICAN WOOD COUNCIL (AWC): SDPWS-2015 SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC.
   b). SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR PRE-ENGINEERED WOOD TRUSSES FOR REVIEW BY THE CONSULTANT.

030000 CONCRETE

1. NOMINAL MAXIMUM SIZE OF AGGREGATE SHALL BE 3/4". USE SMALLER AGGREGATES AS APPROPRIATE IN AREAS OF CONGESTED REINFORCING STEEL OR TO IMPROVE WORKABILITY. MODIFY MIX DESIGNS TO SUIT.

CATEGORY	DESCRIPTION	EXPOSURE CLASS PER A23.1	CONCRETE STRENGTH f'c (psi)	MAX W/C RATIO	AIR CONTENT[1]	SCOPE
CM1	FOUNDATION MIX		3500		5-8%	FOOTING AND CAPS
CM2	SLAB ON GRADE MIX		3000			SLABS ON GRADE
CM3	SLAB AND BEAM MIX		4500			FRAMED SLABS AND BEAMS
CM4	COLUMN AND WALL MIX		4500			CONC. COLUMNS AND WALLS NOT EXPOSED TO FREEZE THAW OR DE-ICING CHEMICALS
CM5	TOPPING MIX		3000			TOPPINGS ON CONCRETE
CM6	COMPOSITE DECK MIX		3000			SLABS ON METAL DECKS
CM7	PARKING SLAB AND BEAM MIX	C-1[2]	5000	0.40	5-8%	FOUNDATION WALLS ADJACENT TO PAVING FRAMED SLABS AND BEAMS EXPOSED TO DE-ICING CHEMICALS
CM8	PAVING MIX	C-2	4700	0.45	5-8%	EXTERIOR PAVING AND SIDEWALKS
CM9	PARKING MIX	C-4	3500	0.55	4-7%	SLAB ON GRADE IN PARKING GARAGE EXPOSED TO DE-ICING CHEMICALS BUT NOT TO FREEZE THAW
CM10	INTENTIONALLY LEFT BLANK					
CM11	EXTERIOR WALL MIX	F-2	3500	0.55	4-7%	FOUNDATION WALLS AND OTHER WALLS AND OTHER WALLS EXPOSED TO FREEZE THAW BUT NOT EXPOSEDTO DE-ICING CHEMICALS

1. WHERE AGGREGATES SMALLER THAN 14 mm ARE USED, INCREASE AIR CONTENT BY 1%
2. REINFORCED CONCRETE EXPOSED TO DE-ICING CHEMICALS TO HAVE DCI CORROSION INHIBITOR @ 11L/cu.m. DOSAGE OR APPROVED EQUIVALENT

2. REINFORCEMENT:
   CONFORM TO THE REQUIREMENTS OF ASTM A615 AND ASTM A706 IF WELDABLE REINFORCEMENT IS USED.
A. REINFORCING BARS SHALL BE MINIMUM ASTM A615 GRADE 60 AND WELDED WIRE FABRIC SHALL BE MINUMUM ASTM A185, SUPPLY IN FLAT SHEETS.

3. SLAB ON GRADE:
A. PLACE SLABS ON GRADE ON MATERIAL CAPABLE OF OS SUSTAINING 500psf WITHOUT SETTLEMENT RELATIVE TO BUILDING FOOTING.
B. BEFORE PLACING SLAB, PLACE MINIMUM 6" OF 3/4" MAXIMUM SIZE CLEAR CRUSHED STONE OVER THE SUB GRADE. THOROUGHLY ROLL AND CONSOLIDATE TO THE LINES AND LEVELS REQUIRED.
4. CONCRETE AND REINFORCEMENT:
A. PROVIDE DOWELS TO WALLS AND COLUMNS SIMILAR IN NUMBER, SIZE, AND SPACING TO VERTICAL STEEL IN THE WALL OR COLUMN EXCEPT WHEN NOTED OTHERWISE.
B. PROVIDE 1.5"x2.5" KEYS AT ALL CONSTRUCTION JOINTS UNLESS NOTED OTHERWISE.
C. CONCRETE COVER TO REINFORCEMENT TO CONFORM TO THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE AND ACI 318 AND THE FOLLOWING COVER REQUIREMENTS:

REINFORCING TYPE:	
SLABS NOT EXPOSED TO WEATHER AND INTERIOR WALL SURFACES	3/4in
EXTERIOR WALL SURFACES, SLABS EXPOSED TO WEATHER #5 AND SMALLER	1 1/2in
EXTERIOR WALL SURFACES, SLABS EXPOSED TO WEATHER LARGER THAN #5	2in
COLUMN AND BEAM TIES	1 1/2in
CLEAR DISTANCE BETWEEN BARS	2in
FORMED DIRECTLY AGAINST EARTH	3in

D. SECURELY TIE IN PLACE AND ADEQUATELY SUPPORT ALL REINFORCEMENT. LAP ALL BARS MARKED 'CONTINUOUS JOINTS' (CONT.) MINIMUM 40db.
E. WHERE CHEMICAL ANCHORS ARE QEQUIRED, USE HILTI HIT HY 200 EPOXY OR APPROVED EQUAL.

050000 STRUCTURAL STEEL:

1. ALL STRUCTURAL STEEL AND MISCELANEOUS METAL SHALL BE DETAILED, FABRICATED AND ERECTED IN CONFORMANCE WITH AISC 325.
2. MATERIALS:
   ALL STRUCTURAL STEEL SHALL CONFORM TO THE NOTED ASTM STANDARDS UNO.

A. W-SHAPES — A992
B. HSS (RECTANGULAR AND SQUARE) — A500 (GRADE C, Fy = 50ksi)
C. HSS (CIRCULAR) — A500 (Fy = 42ksi)
D. ANGLES/C-CHANNELS/MC-CHANNELS — A36
E. ALL OTHER STEEL PLATES — A36

3. WHERE SPECIFIED, GALVANIZED STEEL IS TO BE COMPLETED IN ACCORDANCE WITH ASTM A123 HOT DIP PROCESS.
4. ALL TEMPORARY BRACING, SHORING, AND ERECTION CLIPS REQUIRED BY THE CONTRACTOR ARE NOT SHOWN. WORK IS TO CONFORM TO OSHA REQUIREMENTS.
5. SHOP DRAWINGS ARE TO BE SUBMITTED TO CONSULTANTS FOR REVIEW PRIOR TO FABRICATION.
6. TESTING AND INSPECTION AGENCIES SHALL SEND STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE CONSULTANT.
7. CONNECTIONS
   F. ALL STEEL-TO -STEEL BOLTED CONNECTIONS TO BE MADE WITH HIGH STRENGTH BOLTS AS PER 'SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS"
   G. UNLESS NOTED BOLTS IN CONNECTIONS SHALL BE BEARING TYPE WITH THREADS EXCLUDED FROM THE SHEAR PLANE. USE ASTM A325 BOLTS UNLESS NOTED.
   H. STEEL WASHERS CONFORM TO A436. NUTS TO CONFORM TO A563
   I. ANCHOR BOLTS AND ANCHOR RODS TO CONFORM TO ASTM F1554 GRADE 36.
   J. ALL WELDED CONNECTIONS TO BE COMPLETED IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE - STEEL (AWS-01.1) AND HAVE A MINIMUM TENSILE STRENGTH OF 70ksi FOR ALL ELECTRODES.
   K. ALL WELDERS ARE TO BE QUALIFIED IN ACCORDANCE WITH AWS.01.1 FOR ALL WELDS THEY WILL BE COMPLETING.
   L. WELD LENGTHS CALLED FOR ON STRUCTURAL DRAWINGS ARE NET EFFECTIVE LENGTH. IF NO LENGTH IS SPECIFIED USE THE MINIMUM SIZE AS SPECIFIED IN AISC 360, SECTION J2.2B.
   M. ALL WELDING TO BE PERFORMED IN ACCORDANCE WITH A WRITTEN WELDING PROCEDURE SPECIFICATION (WPS). SUBMIT ALL WPS TO CONSULTANT WHICH OUTLINES ALL PROCEDURES, ELECTRODE SPECIFICATIONS, DATA SHEETS AND LIMITATIONS.
   N. RUN-OFF TABS PER AWS D1.1 ARE REQUIRED FOR ALL COMPLETE JOINT PENETRATION WELDS. START AND COMPLETE ALL WELDS ON RUN-OFF TABS. WELDS ARE NOT TO BE COMPLETED AT COPE HOLE LOCATIONS.
   O. COMPLETE PENETRATION AND PARTIAL PENETRATION WELDS SHALL BE INSPECTED AND EXAMINED BY ULTRASONIC TESTING. ALL TESTING AND INSPECTION SHALL CONFORM TO IBC REQUIREMENTS.
   P. DEMAND CRITICAL WELDS SHALL BE MADE WITH FILLER METALS MEETING THE REQUIREMENTS SPECIFIED IN AWS D1.8/D1.8M CLAUSE 6.3
8. ALL HEADED STUDS WELDED TO BEAMS OR CONCRETE CONNECTIONS SHALL BE NELSON STUDS OR APPROVED EQUAL.
9. HEADED STUDS SHALL BE AUTOMATICALLY WELDED IN SHOP OR FIELD WELDED WITH EQUIPMENT APPROVED BY THE MANUFACTURER OF THE STUDS.

310000 FOUNDATIONS

1. A GEOTEHCNICAL REPORT "GEOTCHNICAL AND GEOLOGIC HAZARD INVESTIGATION: LOT 14R OF SUMMIT EDEN PHASE 1A SUMMIT POWDER MOUNTAIN RESORT WEBER COUNTY, UTAH" PROJECT NUMBER 02693-001, DATED AUGUST 8TH, 2018 HAS BEEN PREPARED BY IGES INC.. READ THIS REPORT, AND BE THOROUGHLY FAMILIARIZED WITH THEIR FINDINGS.
2. FOUND ALL FOOTINGS ON NATURALLY CONSOLIDATED UNDISTURBED SOIL CAPABLE OF SAFELY SUSTAINING AN ALLOWABLE BEARING VALUE OF 2900 PSF.
3. FOUND FOOTINGS EXPOSED TO FREEZING BELOW THE LEVEL AT WHICH POTENTIAL DAMAGE RESULTING FROM FROST ACTION CAN OCCUR, BUT A MINIMUM OF 42in BELOW FINISHED GRADE IF NOT NOTED TO BE FOUNDED LOWER.
4. THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR EXCAVATIONS OR ALONG STEPPED FOOTINGS SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10.
5. DO NOT PLACE BACKFILL AGAINST WALLS RETAINING EARTH (OTHER THAN CANTILEVER WALLS) UNTIL THE FLOOR CONSTRUCTION AT TOP AND BOTTOM OF THE WALLS IS POURED AND HAS ATTAINED 70% OF ITS SPECIFIED STRENGTH.
6. CARRY OUT BACKFILLING AGAINST FOUNDATION WALLS WHERE THERE IS GRADE ON BOTH SIDES IN SUCH A MANNER THAT THE LEVEL OF BACKFILLING ON ONE SIDE OF THE WALL IS NEVER MORE THAN 1'-8" DIFFERENT FROM THE LEVEL ON THE OTHER SIDE OF THE WALL.

060000 WOOD:

1. FRAMING LUMBER SHALL BE DOUGLAS FIR-LARCH AND MEET THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE.

   - 2x6 STUDS, SILLS AND PLATES — No2
   - 2x_ JOISTS & BLOCKING — No2
   - 6x6 AND LARGER — No1

2. ENGINEERED FRAMING BEAMS AND MATERIAL SHALL MEET THE FOLLOWING MINUMUM REQUIREMENTS UNLESS NOTED OTHERWISE.

   "PSL" PARALLEL STRAND LUMBER
   - BENDING STRESS (EDGE LOADED) Fb = 2,900psi
   - SHEAR STRESS (EDGE LOADED) Fv = 290psi
   - COMPRESSIVE STRESS (PERP TO GRAIN) Fc = 750psi
   - COMPRESSIVE STRESS (PARA TO GRAN) Fc = 2,900psi
   - MODULUS OF ELASTICITY E = 2,000ksi

   "LVL" LAMINATED VENEER LUMBER
   - BENDING STRESS (EDGE LOADED) Fb = 2,600psi
   - SHEAR STRESS (EDGE LOADED) Fv = 285psi
   - COMPRESSIVE STRESS (PERP TO GRAIN) Fc = 750psi
   - COMPRESSIVE STRESS (PARA TO GRAN) Fc = 2,510psi
   - MODULUS OF ELASTICITY E = 1,800ksi

   "LSL" LAMINATED STRAND LUMBER
   - BENDING STRESS (EDGE LOADED) Fb = 2,325psi
   - SHEAR STRESS (EDGE LOADED) Fv = 310psi
   - COMPRESSIVE STRESS (PERP TO GRAIN) Fc = 900psi
   - COMPRESSIVE STRESS (PARA TO GRAN) Fc = 2,170psi
   - MODULUS OF ELASTICITY E = 1,550ksi

3. ALL LOAD BEARING STUD WALLS NOT INDICATED AS SHEARWALLS ON PLANS TO BE SHEATHED WITH PLYWOOD OR OSB PANELS. BLOCK ALL PANEL EDGES. TYPICAL NAILING TO BE 10d @ 6" c/c AT ALL SUPPORTED EDGES AND 10d @ 12" c/c AT INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE.
4. ENGINEERED FLOOR JOISTS TO BE MANUFACTURED FLOOR JOIST SYSTEM BY REDBUILT ENGINEERED WOOD PRODUCTS. PROVIDE SEALED ENGINEERED FLOOR LAYOUTS FROM MANUFACTURER PRIOR TO FABRICATION OF ELEMENTS.
5. ALL BLOCKING IN ENGINEERED FLOOR SYSTEM TO BE FULL DEPTH LVL MATERIAL.
6. SUBSTITUTION OF FLOOR SYSTEM CAN BE MADE WITH THE SUBMISISON OF EQUIVALENCY REPORT FROM ALTERNATE SUPPLIER.
7. ALL WOOD-TO-WOOD CONNECTIONS ARE TO BE BY SIMPSON STRONG TIE OR APPROVED EQUIVALENT. ALL HANGERS TO BE RATED FOR MINUIMUM CONNECTION FORCES NOTED ON PLANS.
8. EXECUTION:
   A. ALL SILL PLATES TO BE STAMPED "KD" WHICH INDICATES KILN DRIED WITH A MOISTURE CONTENT NOT EXCEEDING 13%.
   B. PROVIDE SOLID BLOCKING, INCLUDING SQUASH BLOCKS, BELOW ALL POINT LOADS, EXTENDING DOWN TO THE TOP OF FOUNDATIONS.
   C. PROVIDE BRIDGING IN FLOOR AND ROOF ASSEMBLIES AT 8'-0" c/c MAXIMUM UNLESS SPECIFICALLY DETAILED OTHERWISE BY THE ENGINEERED FLOOR SUPPLIER.
   D. REFER TO TYPICAL DETAILS FOR STANDARD FRAMING REQUIREMENTS AT WOOD TO STEEL, WOOD TO FOUNDATION AND WOOD TO WOOD FLOOR ASSEMBLIES.
   PROTECT ALL WOOD PRODUCTS FROM DAMAGE AND STAINING DUE TO WETTING AND MOISTURE.
   E. RE-TIGHTEN ALL ANCHORS JUST PRIOR TO COVERING THE WALL FRAMING.

053100 STEEL DECKING:

1. MATERIALS
   a). STEEL DECKING PER PLAN AND CONFORMING TO AISC 360-16 AND THE FOLLOWING;
      i. AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS (2012)
      ii. SDI DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS (2017)
      iii. SDI SHORT FORM SPECIFICATIONS (2017)
   b). MINIMUM ZINC COATING OF Z275 FOR EXTERIOR DECKING AND DECKING EXPOSED TO VIEW WITHOUT PAINTED WITHOUT PAINTED FINISH.
   c). MINIMUM ZINC COATING OF ZF75 FOR INTERIOR DECKING NOT EXPOSED TO VIEW AND INTERIOR DECKING WITH FIELD APPLIED PAINT SYSTEM.
   d). MINIMUM 18 GAUGE STEEL CONFORMING TO ABOVE STANDARDS FOR COVER PLATES, CELL CLOSURES, WEB STIFFENERS, EDGE STRIPS AND FLASHINGS.
   e). FORM ROOF DECK WITH INTEGRAL RIBS OF A SHAPE TO MATCH EXISTING DECK WHERE REPAIR/REPLACEMENT OF EXISTING DECK IS REQUIRED.
2. EXECUTION
   a). DESIGN DECK IN ACCORDANCE WITH THE REQUIREMENTS OF THE BUILDING CODE OF IBC 2015.
   b). DESIGN AND CONNECT METAL EDGE AND CLOSURE STRIPS, METAL SCREEDS, FLASHINGS AND THE LIKE.
   c). DESIGN FRAMING FOR 18" OR SMALLER OPENINGS IN ROOF DECK, AND 12" OR SMALLER OPENINGS IN FLOOR DECK. REINFORCE OPENINGS OVER 6", AS REQUIRED.
   d). PLACE SHEETS IN MINIMUM 3 SPAN LENGTHS. BEAR ENDS MINIMUM 2".
   e). LAP ENDS OF NON-COMPOSITE DECK UNITS A MINIMUM OF 2" AND ONLY OVER SUPPORTING MEMBERS.
   f). AS A MINIMUM, WELD DECK TO SUPPORTS AND PERIMETER ELEMENTS WITH 3/4" PUDDLE WELDS AT MAXIMUM 16" o/c OR EVERY SECOND FLUTE, WHICHEVER IS LESS.
   g). AS A MINIMUM, FASTEN SIDE JOINTS OF DECK UNITS BETWEEN SUPPORTS BY CLINCHING AT 24" INTERVALS OR WITH 1" LONG WELDS AT 40" INTERVALS.
   h). PAINT WELDS AND REPAIR DAMAGED COATING WITH GALVACON COATING.
   i). DO THE FOLLOWING WHERE DECKING IS EXPOSED TO VIEW;
      i. LAP ENDS OF DECK UNITS ONLY OVER SUPPORTING MEMBERS. NO SEAMS ARE PERMITTED WITHIN SPANS.
      ii. KEEP DECK FREE OF DIRT, SCALE, FOREIGN MATTER, DENTS OR DEFORMATIONS.
      iii. KEEP FUSION WELDS WELL WITHIN THE BEARING WIDTH OF SUPPORTING MEMBERS.
      iv. AVOID WELD DAMAGE TO THE DECK OR ITS SUPPORTS.

FRAMING PLAN LEGEND

- REPEAT FRAMING ELEMENTS (SPAN)
- REPEAT FRAMING ELEMENTS (EXTENT)
- BEAM MEMBERS
- EXTERIOR STUD & LOAD BEARING STUDS
- WALLS (NON-LOAD BEARING)
- STUD WALL ABOVE
- CONCRETE WALLS
- COLUMN (HSS)
- COLUMN (WOOD)
- COLUMN ABOVE
- CANTILEVERS — CANT.
- MOMENT CONNECTIONS
- EXTENT OF FINISHES
- EXTENT OF ROOF
- EXTENT OF ELEMENTS BELOW

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	DETAILED INSTRUCTIONS AND FREQUENCIES
REINFORCED CONCRETE (IBC 1705.3 & 1705.12.1)			
REINFORCING STEEL		X	VERIFY PRIOR TO PLACING CONCRETE THAT REINFORCING IS OF SPECIFIED TYPE, GRADE AND SIZE; THAT IT IS FREE OF OIL, DIRT AND RUST; THAT IT IS LOCATED AND SPACED PROPERLY; THAT HOOKS, BENDS, TIES, STIRRUPS, AND SUPPLEMENTAL REINFORCEMENT ARE PLACED CORRECTLY; THAT TAP LENGTHS, STAGGER AND OFFSETS ARE PROVIDED; AND THAT ALL MECHANICAL CONNECTIONS ARE INSTALLED PER THE MANUFACTURER'S INSTRUCTIONS AND/OR EVALUATION REPORT.
ANCHORAGE		X	INSPECTION OF ANCHORS CAST IN CONCRETE.
USE OF REQUIRED MIX DESIGN		X	VERIFY THAT ALL MIXTURES USED COMPLY WITH THE APPROVED CONSTRUCTION DOCUMENTS; ACI 318: Ch. 4, 5.2-5.4; AND IBC 1904.3, 1913.2, 1913.3.
CONCRETE SAMPLING FOR STRENGTH TESTS, SLUMP, AIR CONTENT, AND TEMPERATURE	X		
CONCRETE PLACEMENT	X		
CURING TEMPERATURE AND TECHNIQUES		X	VERIFY THAT AMBIENT TEMPERATURE FOR CONCRETE IS KEPT > 50°F FOR AT LEAST 7 DAYS AFTER PLACEMENT. HIGH-EARLY-STRENGTH CONCRETE SHALL BE KEPT > 50°F FOR AT LEAST 3 DAYS. ACCELERATED CURING METHODS MAY BE USED (SEE ACI 318:5.11.3). ALL CONCRETE MATERIALS, REINFORCEMENT, FORMS, FILLERS, AND GROUND SHALL BE FREE FROM FROST. IN HOT WEATHER CONDITIONS ENSURE THAT APPROPRIATE MEASURES ARE TAKEN TO AVOID PLASTIC SHRINKAGE CRACKING AND THAT THE SPECIFIED WATER/CEMENT RATIO IS NOT EXCEEDED.
STRENGTH VERIFICATION		X	VERIFY THAT ADEQUATE STRENGTH HAS BEEN ACHIEVED PRIOR TO THE REMOVAL OF FORMS.
FORMWORK		X	VERIFY THAT FORMS ARE PLACED PLUMB AND CONFORM TO THE SHAPES, LINES, AND DIMENSIONS OF THE MEMBERS AS REQUIRED BY THE APPROVED CONSTRUCTION DOCUMENTS.
STRUCTURAL STEEL - PRIOR TO WELDING (TABLE N5.4-1, AISC 360-10)			
VERIFY WELDING PROCEDURES (WPS) AND CONSUMABLE CERTIFICATES	X		
MATERIAL IDENTIFICATION		X	VERIFY TYPE AND GRADE OF MATERIAL.
WELDER IDENTIFICATION		X	A SYSTEM SHALL BE MAINTAINED BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED.
FIT-UP GROOVE WELDS		X	VERIFY JOINT PENETRATION, DIMENSIONS, CLEANLINESS, TACKING, AND BACKING.
ACCESS HOLES		X	VERIFY CONFIGURATION AND FINISH.
FIT-UP FILLET WELDS		X	VERIFY ALIGNMENT, GAPS AT ROOT, CLEANLINESS OF STEEL SURFACES, AND TACK WELD QUALITY AND LOCATION.
STRUCTURAL STEEL - DURING WELDING (TABLE N5.4-2, AISC 360-10)			
USE OF QUALIFIED WELDERS		X	VERIFY THAT WELDERS ARE APPROPRIATELY QUALIFIED.
CONTROL AND HANDLING OF WELDING CONSUMABLES		X	VERIFY PACKAGING AND EXPOSURE CONTROL.
CRACKED TACK WELDS		X	VERIFY THAT WELDING DOES NOT OCCUR OVER CRACKED TACK WELDING.
ENVIRONMENTAL CONDITIONS		X	VERIFY THAT WIND SPEED, PRECIPITATION, AND TEMPERATURE ARE WITHIN LIMITS.
WPS FOLLOWED		X	VERIFY ITEMS SUCH AS SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED, AND PROPER POSITION.
WPS FOLLOWED		X	VERIFY ITEMS SUCH AS SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED, AND PROPER POSITION.
WELDING TECHNIQUES		X	VERIFY INTERPASS AND FINAL CLEANING, EACH PASS IS WITHIN PROFILE LIMITATIONS, AND QUALITY OF EACH PASS.
STRUCTURAL STEEL - AFTER WELDING (TABLE N5.4-3, AISC 360-10)			
WELDS CLEANED		X	VERIFY THAT WELDS HAVE BEEN PROPERLY CLEANED.
SIZE, LENGTH, AND LOCATION OF WELDS	X		
WELDS MEET VISUAL ACCEPTANCE CRITERIA	X		
ARC STRIKES	X		
K-AREA	X		
BACKING AND WELD TABS REMOVED	X		
REPAIR ACTIVITIES	X		
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT/MEMBER	X		
NON-DISTRUCTIVE TESTING (SECTION N5.5, AISC 360-10)			
CJP WELDS		X	ULTRASONIC TESTING SHALL BE PERFORMED ON 10% OF CJP GROOVE WELDS IN BUTT, T- AND CORNER JOINTS SUBJECTED TO TRANSVERSELY APPLIED TENSION LOADING IN MATERIALS 5/16" THICK OR GREATER. TESTING RATE MUST BE INCREASED IF >5% OF WELDS TESTED HAVE UNACCEPTABLE DEFECTS.
ACCESS HOLES (FLANGE > 2")	X		
WELD JOINTS SUBJECT TO FATIGUE	X		
OTHER STEEL INSPECTIONS (SECTION N5.7, AISC 360-10; TABLES J8-1 & J10-1, AISC 341-10)			
STRUCTURAL STEEL DETAILS		X	ALL FABRICATED STEEL OR STEEL FRAMES SHALL BE INSPECTED TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN IN THE CONSTRUCTION DOCUMENTS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS, AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.
ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL		X	SHALL BE ON THE PREMISES DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH CONSTRUCTION DOCUMENTS. VERIFY THE DIAMETER, GRADE, TYPE, AND LENGTH OF THE ANCHOR ROD OR EMBEDMENT ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT PRIOR TO PLACEMENT OF CONCRETE.
WOOD CONSTRUCTION (IBC 1705.10.1 & 1705.11.2)			
HIGH-LOAD DIAPHRAGMS		X	VERIFY THICKNESS AND GRADE OF SHEATHING, SIZE OF FRAMING MEMBERS AT PANEL EDGES, NAIL/STAPLE DIAMETERS AND LENGTH, AND THE NUMBER OF FASTENER LINES AND FASTENER SPACING PER APPROVED PLANS. *PERFORMED BY CODE INSPECTION FIRM.*
STRUCTURAL WOOD		X	WHERE FASTENER SPACING IS < 4" o.c.: VERIFY PROPER NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF SHEAR WALLS, DIAPHRAGMS, BRACES, AND HOLDOWNS. *PERFORMED BY CODE INSPECTION FIRM.*
SOILS (IBC 1705.6)			
VERIFY SUBGRADE IS ADEQUATE TO ACHIEVE DESIGN BEARING CAPACITY		X	PRIOR TO PLACEMENT OF CONCRETE.
VERIFY EXCAVATIONS EXTEND TO PROPER DEPTH AND MATERIAL		X	PROIR TO PLACEMENT OF COMPACTED FILL OR CONCRETE.
VERIFY THAT SUBGRADE HAS BEEN APPROPRIATELY PREPARED PRIOR TO PLACING COMPACTED FILL		X	PROIR TO PLACEMENT OF COMPACTED FILL.
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS		X	ALL MATERIALS SHALL BE CHECKED AT EACH LIFT FOR PROPER CLASSIFICATIONS AND GRADATIONS NOT LESS THAN ONCE FOR EACH 10,000 SQ.FT. OF SURFACE AREA.
VERIFY PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION.	X		ALL MATERIALS SHALL BE CHECKED AT EACH LIFT FOR PROPER CLASSIFICATIONS AND GRADATIONS NOT LESS THAN ONCE FOR EACH 10,000 SQ.FT. OF SURFACE AREA.

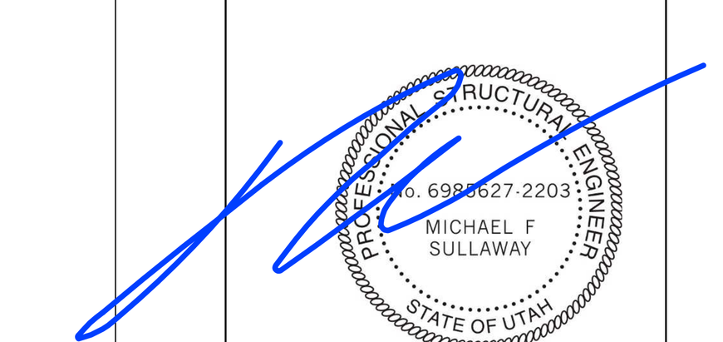
1. SPECIAL INSPECTORS SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO PERFORMING ANY DUTIES.
2. SPECIAL INSPECTORS SHALL PROVIDE PROOF OF LICENSURE BY THE STATE OF UTAH FOR EACH TYPE OF INSPECTION.
3. SPECIAL INSPECTIONS AND TESTING SHALL BE PERFORMED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, THIS STATEMENT, AND THE IBC SECTIONS 1704 AND 1705.
4. INSPECTION REPORTS WILL BE SUBMITTED TO THE CODE CONSULTANT, THE ARCHITECT, AND THE STATE OF UTAH BUILDING OFFICIAL WITHIN 48 HOURS OF PERFORMING INSPECTIONS.
5. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS, TESTING AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS AND A STATEMENT INDICATING THAT THE STRUCTURE IS IN COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND APPLICABLE CODES SHALL BE SUBMITTED.

PLAN REVIEW ACCEPTANCE
FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW.
[X] BUILDING [X] STRUCTURAL
[X] MECHANICAL [X] PLUMBING
[X] ELECTRICAL [X] ENERGY
[ ] ACCESSIBILITY [ ] FIRE
PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS.
BY: MEM DATE: 08/22/18
WEST COAST CODE CONSULTANTS, INC.

Lot 14 Mountain House
Summit Powder Mountain
Eden, Utah

MacKay-Lyons Sweetapple
Architects Limited
2188 Gottingen St.
Halifax, Nova Scotia
Canada B3K 3B4
ph: (902) 429.1867
fax: (902) 429.6276

Blackwell
Toronto | Waterloo | Victoria | Halifax
416.593.5300 | blackwell.ca



No.	Description	Date
05	Issued for Building Permit	2018.08.15
04	Issued for SSI No 01	2018.07.27
03	Issued for Building Permit	2018.07.13
02	Issued for Review	2018.06.13
01	Issued for Costing	2018.05.30

Revision:

NOTES:

ARCHITECT'S REQUIREMENTS AND APPROVALS:
It is the Builder's responsibility to notify MacKay-Lyons Sweetapple Architects Ltd. and to seek prior written approval for materials and workmanship which deviates from instructions provided by the Architect.

ENGINEER'S REQUIREMENTS AND APPROVALS:
It is the Builder's responsibility to notify MacKay-Lyons Sweetapple Architects Ltd. and to seek prior written approval for materials and workmanship which deviates from instructions provided by the Engineer.

AUTHORITIES' REQUIREMENTS AND APPROVALS:
All materials and workmanship must comply with the requirements of all authorities having jurisdiction over the work. It is the Builder's responsibility to gain necessary approval from all relevant Authorities.

DIMENSIONS:
All dimensions must be verified on site. Do not scale off drawings. Plans take precedent over elevations. In the absence of dimensions, or if discrepancies exist, consult Architect. All minimum dimensions are to comply with the National Building Code of Canada.

SHOP DRAWINGS:
Submit shop drawings to the Architect and Engineer for approval prior to manufacture of prefabricated elements of the building.

General Notes

scale: NOT TO SCALE
date: 2018-05-20
drawn: DP
chk'd: TJ

S001

PLOT DATE: July 27, 2018
FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg

ABBREVIATIONS 0001

A.BOLT = ANCHOR BOLT
ADJ. = ADJUSTABLE
ALT. = ALTERNATE
ARCH. = ARCHITECTURAL

B = BOTTOM
BLL = BOTTOM LOWER LAYER
BUL = BOTTOM UPPER LAYER
BLDG. = BUILDING
BM. = BEAM
BP. = BASE OR BEARING PLATE
BSMT. = BASEMENT

CA = COLUMN ABOVE
C/C = CENTRE TO CENTRE
℄ = CENTRE LINE
CANT. = CANTILEVER
COL. = COLUMN
CONC. = CONCRETE
CONSTR. = CONSTRUCTION
CONT. = CONTINUOUS
c/w = COMPLETE WITH

DET. = DETAIL
DIAG. = DIAGONAL
DIA. = DIAMETER
Ø = DIAMETER, BAR DIAMETER
DIM. = DIMENSION
D.J. = DOUBLE JOIST
DO. = DITTO
D.L. = DEAD LOAD
DWG. = DRAWING
DWL. = DOWEL

EA. = EACH
EA.F. = EACH FACE
EA.W. = EACH WAY
EL. = ELEVATION
ELECT. = ELECTRICAL
ELEV. = ELEVATOR
E-W = EAST-WEST
EQ. = EQUAL
EXIST. = EXISTING
EXP.J. = EXPANSION JOINT
EXT. = EXTERIOR

F.F. = FAR FACE
FDN. = FOUNDATION
FIN. = FINISHED
FL. = FLOOR
FTG. = FOOTING

GA. = GAUGE
GALV. = GALVANIZED
GEN. = GENERAL

H. HOR. = HORIZONTAL
HH = HOOKED EACH END

INT. = INTERIOR

JT. = JOINT

kN = KILONEWTON
kg = KILOGRAM
kN.m = KILONEWTON METRES
kN/sq.m = KILONEWTON PER SQUARE METRE
kN/m = KILONEWTON PER METRE

L.L. = LIVE LOAD
LG. = LONG
LLV. = LONG LEG VERTICAL
LLH. = LONG LEG HORIZONTAL

MAX. = MAXIMUM
MECH. = MECHANICAL
MEZZ. = MEZZANINE
MIN. = MINIMUM
MISC. = MISCELLANEOUS
ML = MIDDLE LAYER
mm = MILLIMETRE
MOM. = MOMENT
m = METRIC,METRE
MPa = MEGAPASCAL
Mf = FACTORED MOMENT

N = NEWTONS
N.F. = NEAR FACE
N-S = NORTH-SOUTH
NTS. = NOT TO SCALE

OWSJ = OPEN WEB STEEL JOISTS
OPEN = OPENING

PL. = PLATE
P.C. = PRECAST
PROJ. = PROJECTION

R = REACTION
RAD = RADIUS
REF. = REFERENCE
REINF. = REINFORCING,REINFORCEMENT
REQ'D = REQUIRED
REV. = REVISION,REVISED
r/w = REINFORCED WITH

SECT. = SECTION
SDF = STEP DOWN FOOTING
SL. = SLAB
SPEC'S. = SPECIFICATIONS
STD. = STANDARD
SQ. = SQUARE
STRUCT. = STRUCTURAL

T = TOP
T.J. = TIE JOIST
TLL = TOP LOWER LAYER
TUL = TOP UPPER LAYER
TEMP. = TEMPERATURE
TYP. = TYPICAL

U/N = UNLESS OTHERWISE NOTED
U/S = UNDERSIDE

Vf = FACTORED SHEAR FORCE
V. VERT. = VERTICAL

WWF = WELDED WIRE FABRIC
w/ = WITH
wD; wL = UNIFORMLY DISTRIBUTED LOADS

REINFORCEMENT DEVELOPMENT LENGTHS 0301

TABLE 1 - TENSION DEVELOPMENT LENGTH (in)

BAR SIZE	f'c 2900psi	3626psi	4352psi	5077psi	5802psi
4	12.6	11.8	11.8	11.8	11.8
5	18.9	16.9	15.4	14.6	13.4
6	25.2	22.8	20.9	19.3	18.1
8	39.8	35.4	32.3	29.9	28.0
9	47.6	42.5	39.0	35.8	33.5
11	55.5	49.6	45.3	41.7	39.4
14	71.7	63.8	58.3	53.9	50.8
18	87.4	78.0	71.3	66.1	61.8

TABLE 2 - TENSION LAP SPLICE (CLASS B) LENGTH (in)

BAR SIZE	f'c 2900psi	3626psi	4352psi	5077psi	5802psi
4	16.5	15.0	13.4	12.4	11.8
5	24.8	22.0	20.1	19.1	17.5
6	33.1	29.7	27.2	25.2	23.6
8	51.8	46.1	42.1	39.0	36.4
9	54.1	55.3	50.8	46.5	43.5
11	72.4	64.6	58.9	55.3	51.2
14	LAP SPLICES NOT PERMITTED				
18					

TABLE 3 - DEVELOPMENT LENGTH FOR STD HOOKS (in)

BAR SIZE	f'c 2900psi	3626psi	4352psi	5077psi	5802psi
4	6.1	5.9	5.9	5.9	5.9
5	9.4	8.3	7.5	6.9	6.7
6	12.4	11.0	10.2	9.4	8.9
8	15.4	13.8	12.6	11.6	11.0
9	18.5	16.5	15.2	14.2	13.0
11	21.7	16.9	17.7	16.3	15.2
14	38.5	34.4	31.4	29.1	27.2
18	49.6	44.4	40.6	37.5	35.1

TABLE 4 - COMPRESSION DEVELOPMENT LENGTH (in)

BAR SIZE	f'c=2900psi	f'c=3626psi	f'c≥4352psi
4	8.3	7.9	7.9
5	12.6	11.4	10.2
6	16.9	15.0	13.8
8	21.3	18.9	17.3
9	25.2	22.8	20.9
11	29.5	26.4	24.4
14	38.2	33.9	31.1
18	46.5	41.7	38.2

TABLE 5 - COMPRESSION LAP SPLICE LENGTH (in)

BAR SIZE	USUAL CONFINEMENT
4	11.8
5	17.3
6	22.8
8	28.7
9	34.6
11	40.2

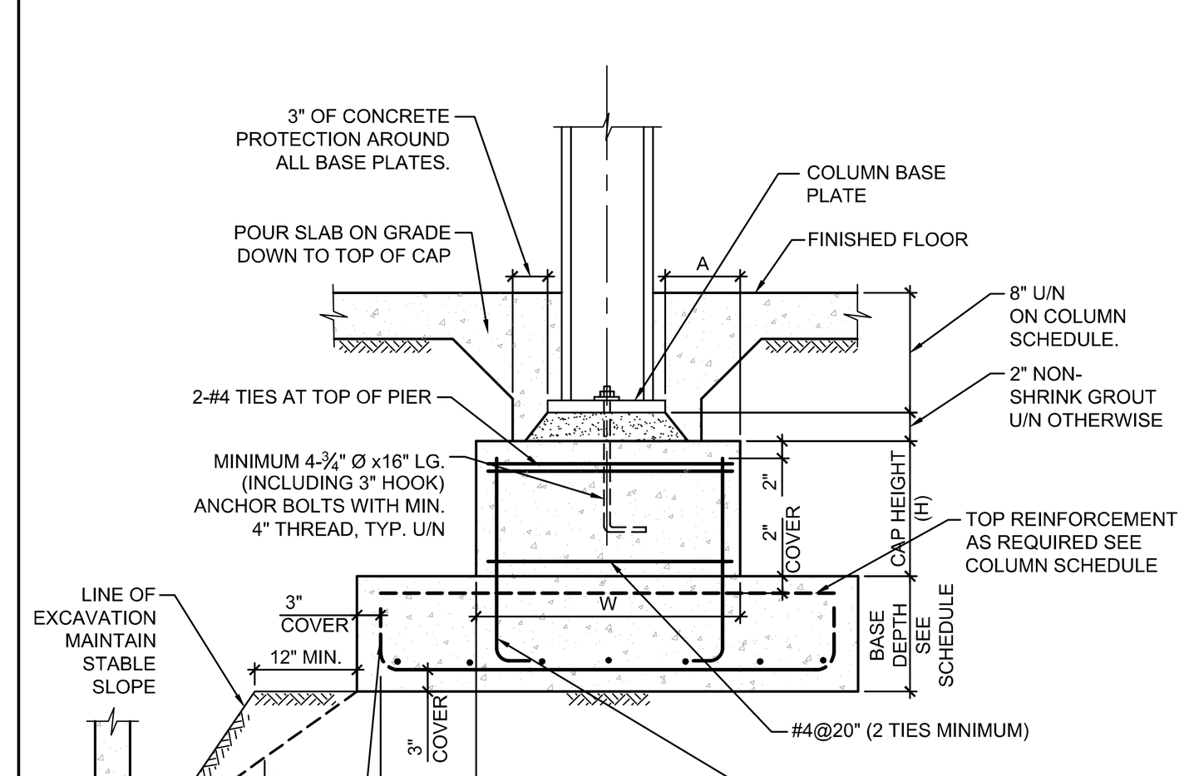
NOTE: #14 AND #18 BARS SHALL BE SPLICED WITH MECHANICAL CONNECTORS

TABLE 6 - STANDARD HOOK DIMENSION FOR BLACK REINFORCING

BAR SIZE	400R OR 500R 90° HOOK (in)	400R OR 500R 180° HOOK (in)	400W OR 500W 90° HOOK (in)	400W OR 500W 180° HOOK (in)
4	7.1	5.5	7.1	5.1
5	10.2	7.1	9.8	6.7
6	12.2	8.7	11.8	7.9
8	15.7	11.0	15.7	11.0
9	20.1	15.7	19.3	13.8
11	24.0	18.9	23.2	16.9
14	31.1	26.8	30.3	24.4
18	40.6	35.4	39.8	32.7

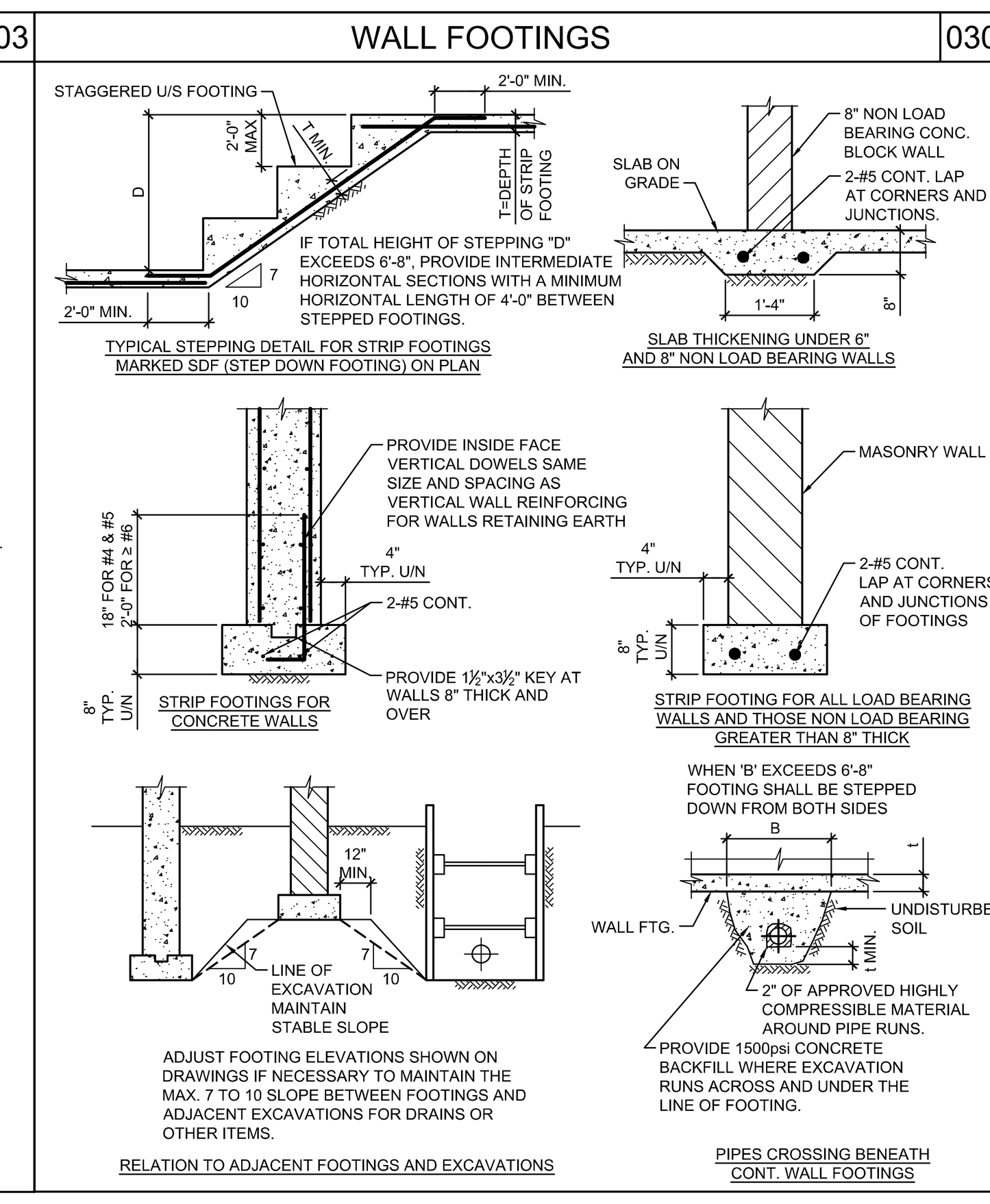
REFER TO REINFORCING STEEL MANUAL OF STANDARD PRACTICE FOR MORE INFORMATION

FOOTINGS AND BASES FOR STEEL COLUMNS 0303

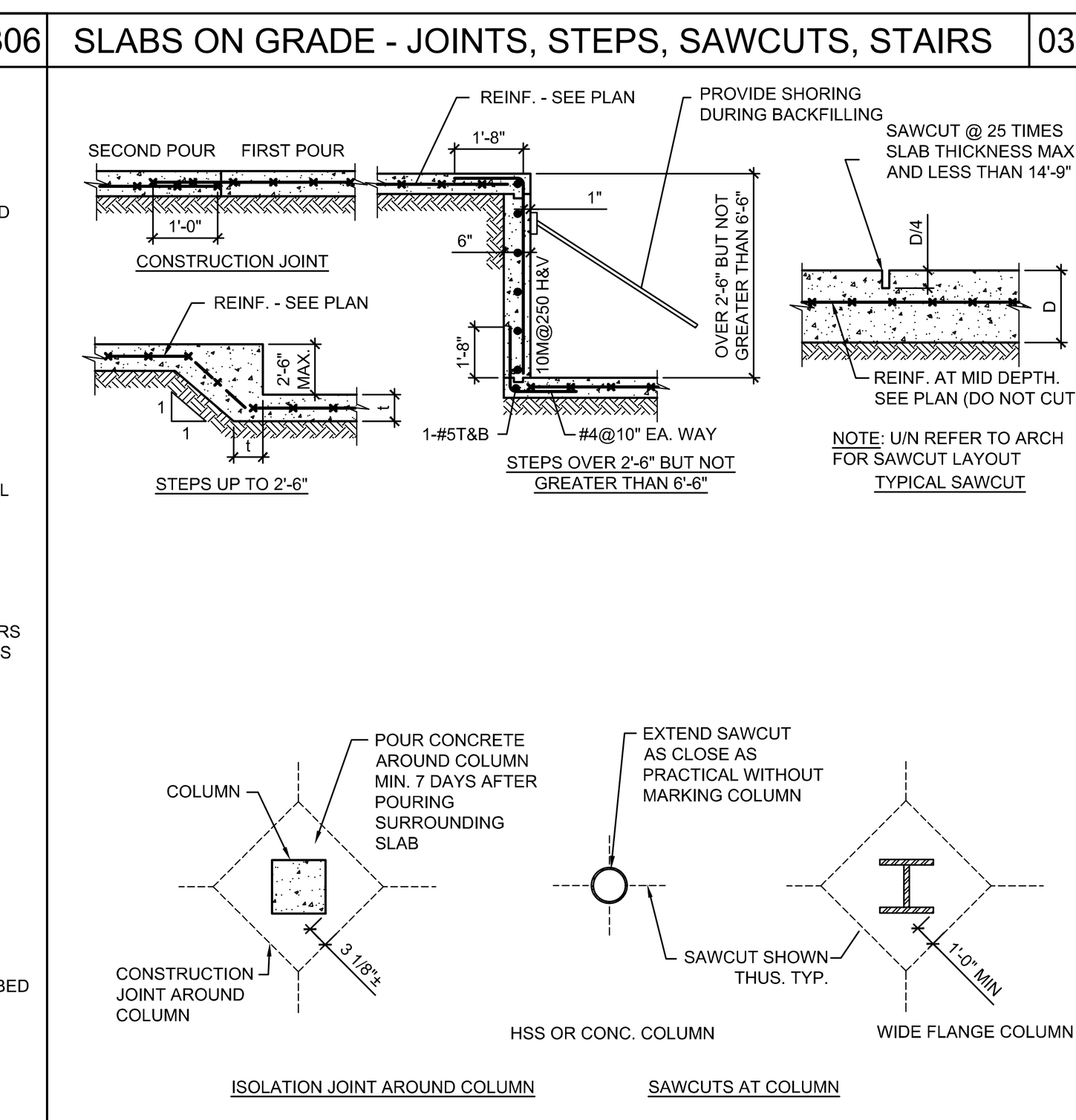


NOTE:
1. WHERE RATIO OF CAP HEIGHT TO CAP WIDTH (H/W) EXCEEDS 3, PROVIDE 0.5% VERTICAL STEEL (8-#6 MINIMUM) WITH #4 TIES SPACED AT THE LESSER OF 20" o/c OR THE LEAST CAP DIMENSION (2 TIES MINIMUM).
2. HEIGHT (H) OF CAP SHALL BE A MINIMUM OF 2 X PROJECTION (A) BUT NOT LESS THAN 8".
3. SEE COLUMN SCHEDULE FOR ADDITIONAL REQUIREMENTS.
4. IF D1 IS LESS THAN THE TENSION DEVELOPMENT LENGTH FOR THE SPECIFIED BAR (REFER TO TABLE 1, DETAIL 0301), PROVIDE STANDARD HOOK.
5. TIE ARRANGEMENT IN PIERS AS PER TYPICAL DETAIL 0329.

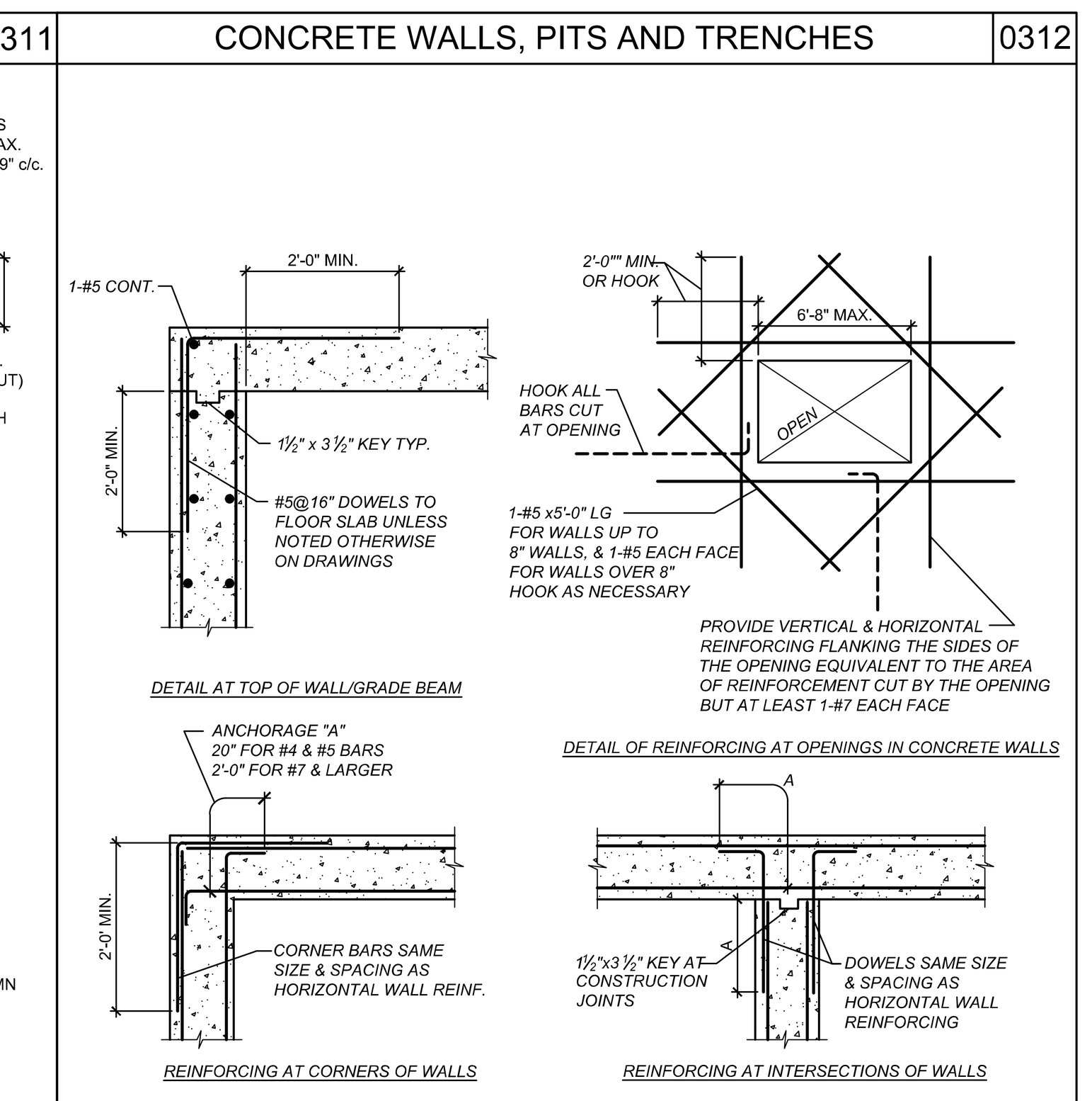
WALL FOOTINGS 0306



SLABS ON GRADE - JOINTS, STEPS, SAWCUTS, STAIRS 0311

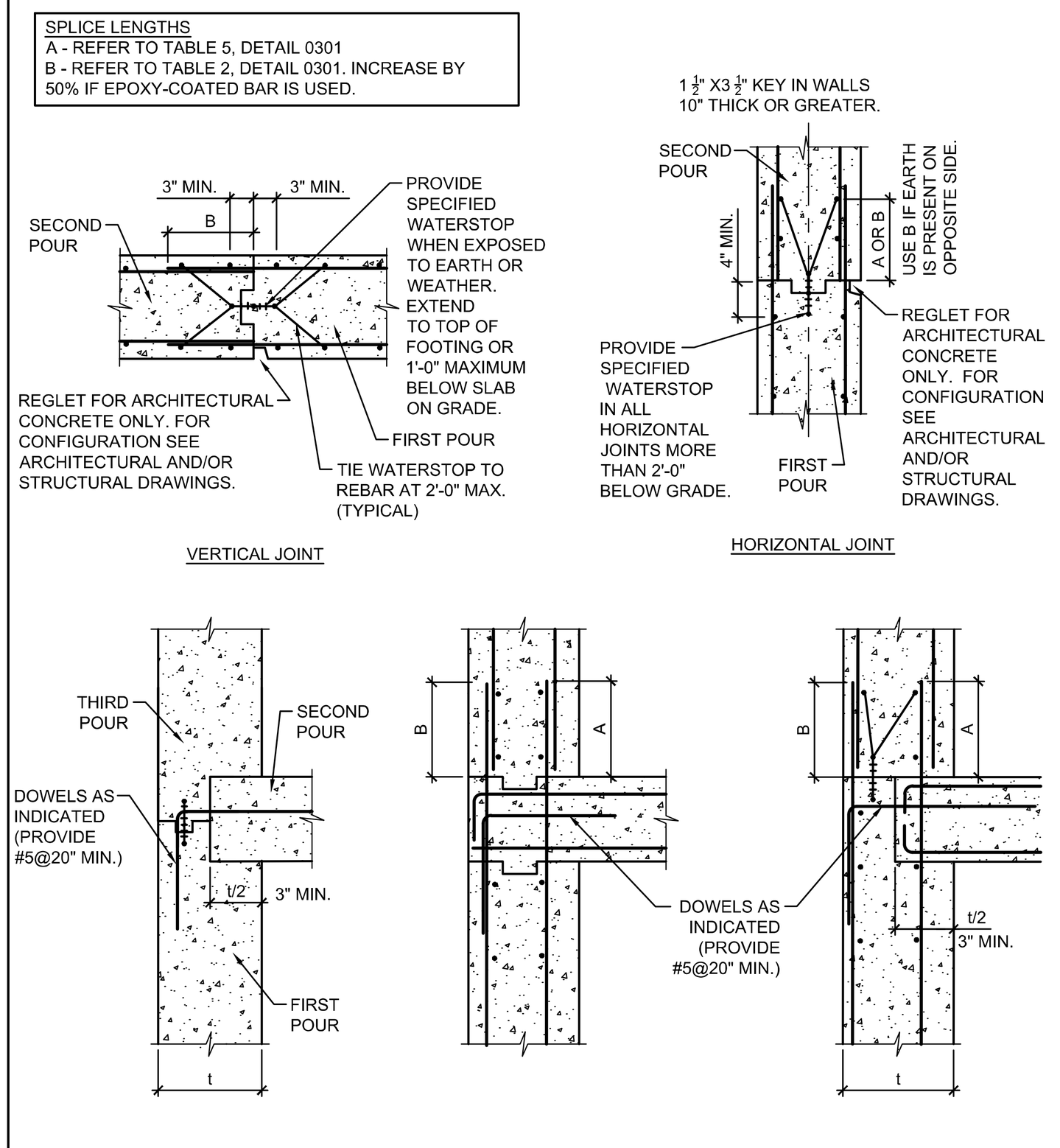


CONCRETE WALLS, PITS AND TRENCHES 0312

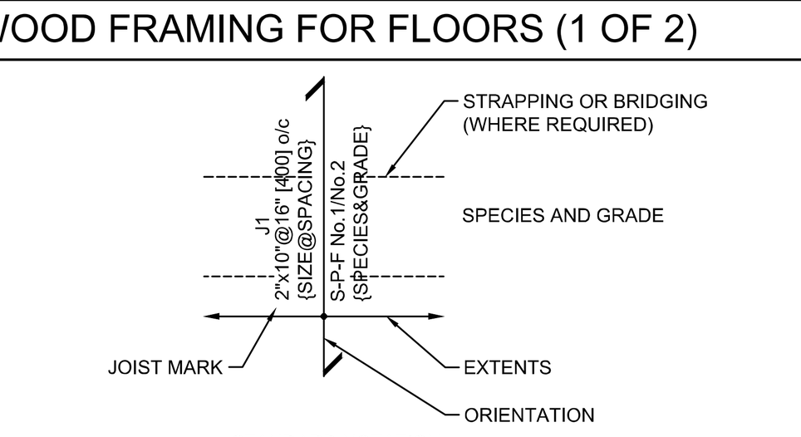


CONSTRUCTION JOINTS IN WALLS 0314

SPLICE LENGTHS
A - REFER TO TABLE 5, DETAIL 0301
B - REFER TO TABLE 2, DETAIL 0301. INCREASE BY 50% IF EPOXY-COATED BAR IS USED.

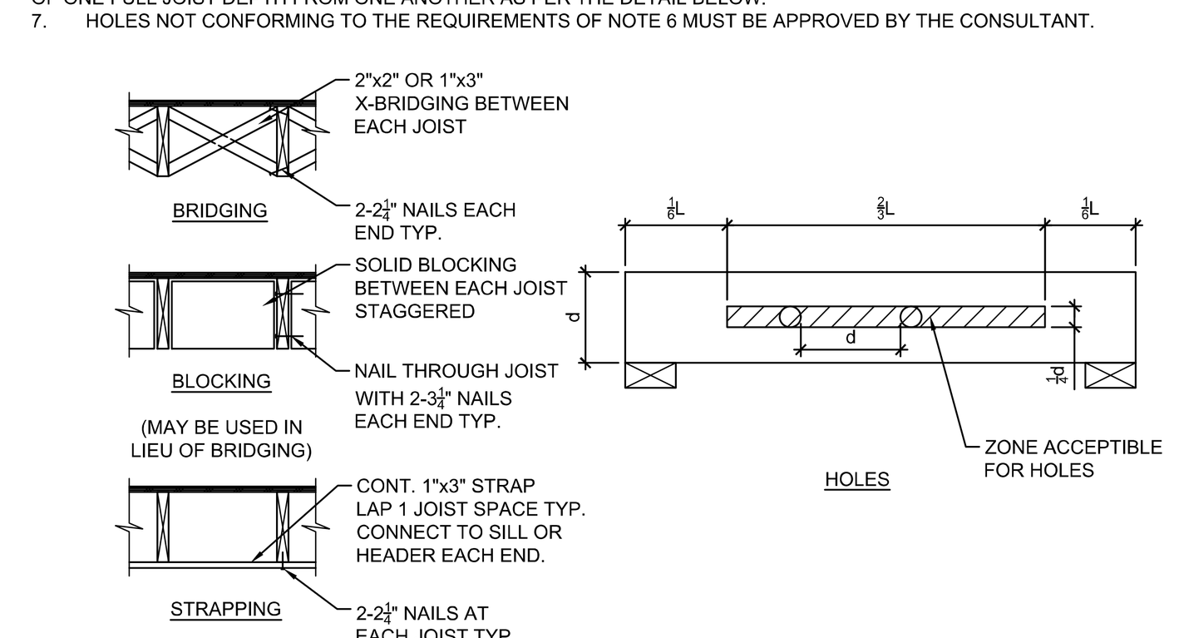


WOOD FRAMING FOR FLOORS (1 OF 2) 0601

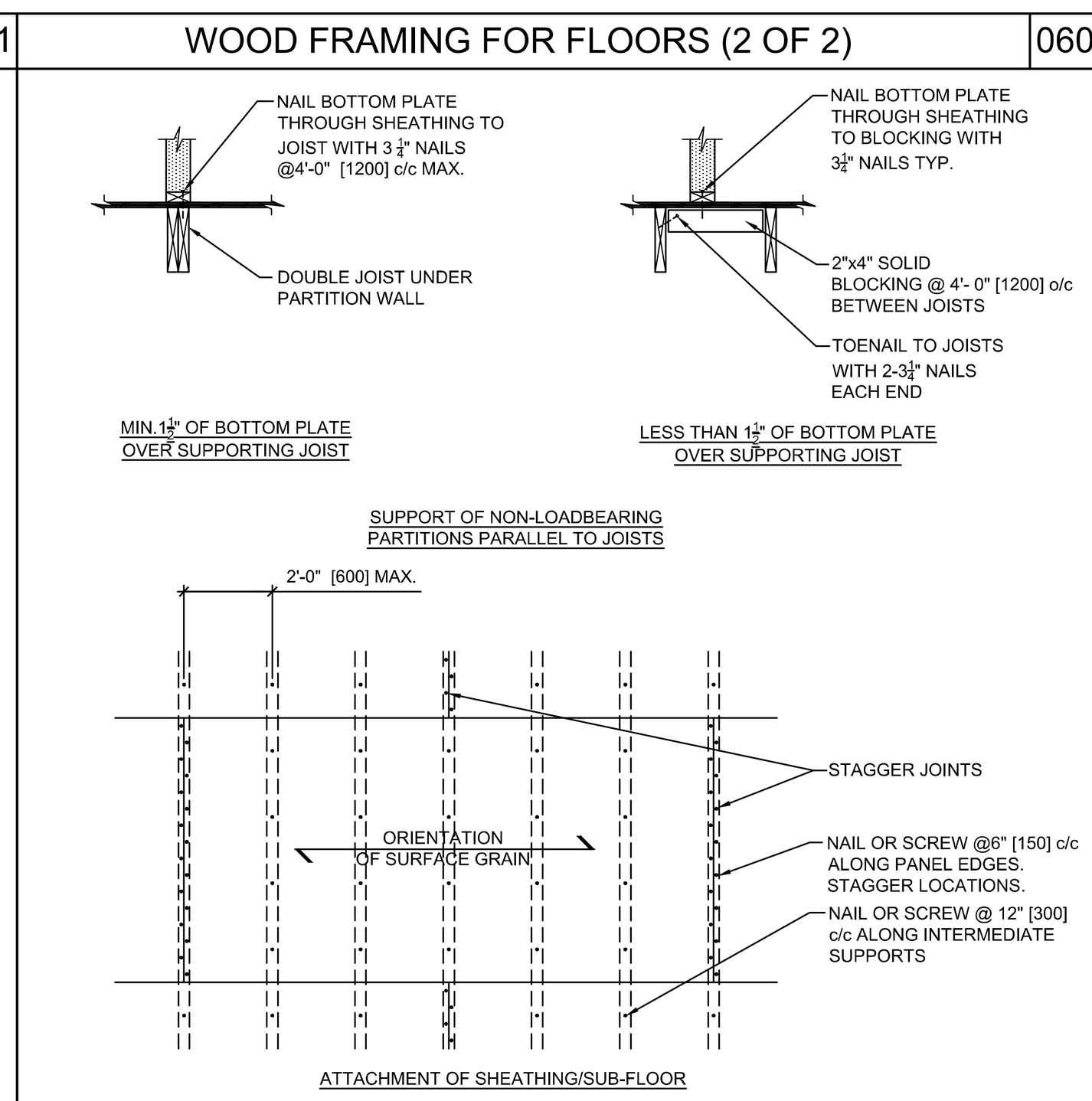


STANDARD NOTATION

NOTES:
1. SEE PLANS FOR JOIST SIZE, SPACING, SPECIES AND LUMBER GRADE. SPACING NOT TO EXCEED 16" [406].
2. UNLESS NOTED OTHERWISE ON THE DRAWINGS, PROVIDE BRIDGING AS PER THE DETAIL BELOW AND THE SPACING REQUIREMENTS OF NOTE 4.
3. WHERE SPECIFIED ON THE DRAWINGS, PROVIDE STRAPPING AS PER THE DETAIL BELOW AND THE SPACING REQUIREMENTS OF NOTE 4.
4. LINES OF BRIDGING AND/OR STRAPPING ARE TO BE LOCATED NOT MORE THAN 7'-0" FROM A SUPPORT AND SPACED AT NOT GREATER THAN 7'-0" o/c.
5. UNLESS NOTED OTHERWISE, JOISTS ARE NOT TO BE NOTCHED.
6. WHERE HOLES ARE REQUIRED IN UNIFORMLY LOADED JOISTS FOR WIRING OR PLUMBING, THEIR DIAMETER SHALL NOT EXCEED ONE QUARTER OF THE JOIST DEPTH. CENTRE HOLES AT THE MID-DEPTH OF THE JOISTS, WITHIN THE MIDDLE TWO-THIRDS OF THE SPAN. HOLES SHALL BE SPACED NO CLOSER THAN A CLEAR DISTANCE OF ONE FULL JOIST DEPTH FROM ONE ANOTHER AS PER THE DETAIL BELOW.
7. HOLES NOT CONFORMING TO THE REQUIREMENTS OF NOTE 6 MUST BE APPROVED BY THE CONSULTANT.

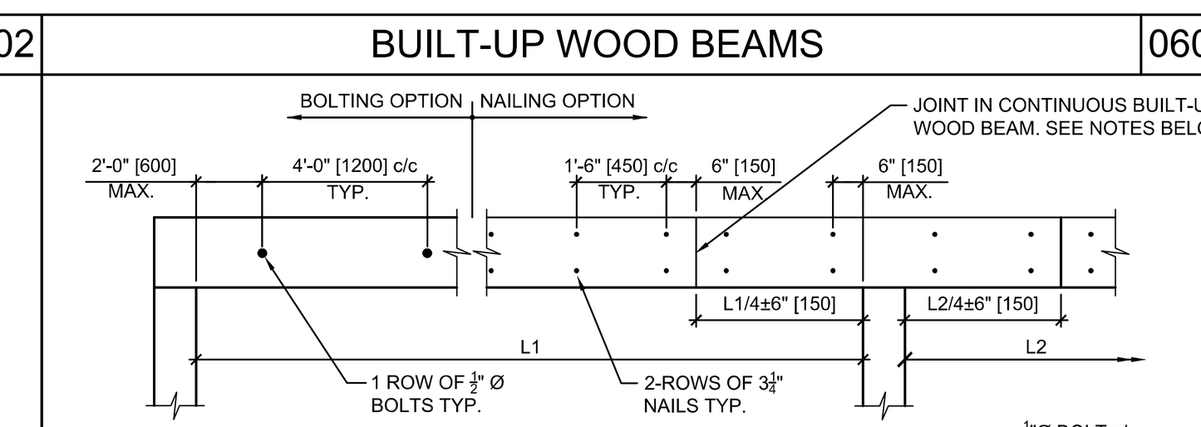


WOOD FRAMING FOR FLOORS (2 OF 2) 0602

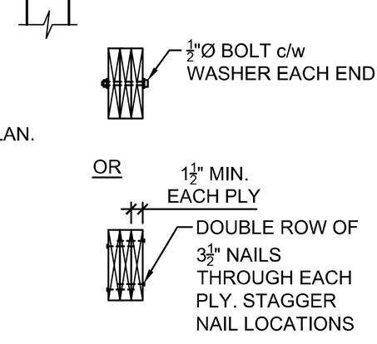


NOTE:
1. EDGES OF FLOOR SHEATHING TO BE TONGUE AND GROOVE.
2. USE 2" NAILS OR 1¾" SCREWS FOR SHEATHING LESS THAN ¾" THICK.
3. USE 2¼" NAILS OR 2" SCREWS FOR ¾" TO 1" SHEATHING
4. USE PRESSURE TREATED PLYWOOD FOR FLOORS OF KITCHENS AND BATHROOMS.
5. MINIMUM THICKNESS OF SUB-FLOOR SHALL BE ⅝" FOR JOIST SPACING UP TO 1'-8" [500] AND ¾" FOR JOIST SPACING OF 2'-0" [600]

BUILT-UP WOOD BEAMS 0604

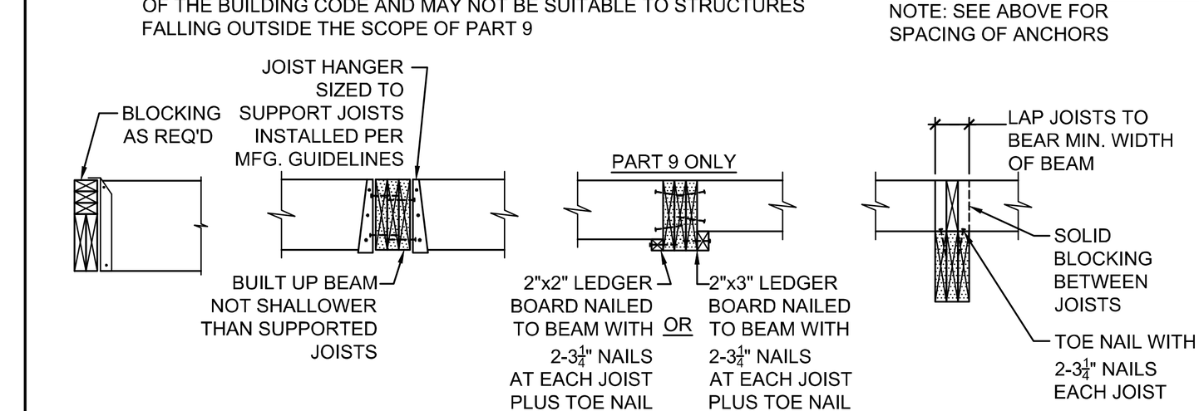


JOINTS SHALL BE MADE IN SUCH A WAY THAT:
1. JOINTS SHALL BE LOCATED AS SHOWN ABOVE.
2. JOINTS SHALL ONLY BE MADE IN MULTIPLE SPAN BEAMS WHICH ARE CONTINUOUS OVER THEIR SUPPORTS AS INDICATED ON PLAN. PLIES OF BEAMS INDICATED AS SINGLE SPAN BEAMS SHALL NOT HAVE JOINTS.
3. JOINTS SHALL NOT BE MADE NEAR END SUPPORTS.
4. NOT MORE THAN ONE JOINT SHALL OCCUR IN ANY INDIVIDUAL MEMBER WITHIN ANY ONE SPAN NOR WITHIN TWO ADJACENT SPANS.
5. JOINTS SHALL NOT BE MADE IN ADJACENT MEMBERS OF A BUILT-UP BEAM AT THE SAME LOCATION.
6. NOT MORE THAN HALF OF THE MEMBERS WITHIN A BUILT UP BEAM SHALL BE JOINED AT ANY ONE LOCATION.
7. THE ABOVE REQUIREMENTS ARE BASED ON THE PROVISIONS OF PART 9 OF THE BUILDING CODE AND MAY NOT BE SUITABLE TO STRUCTURES FALLING OUTSIDE THE SCOPE OF PART 9



FASTENING OF BEAMS

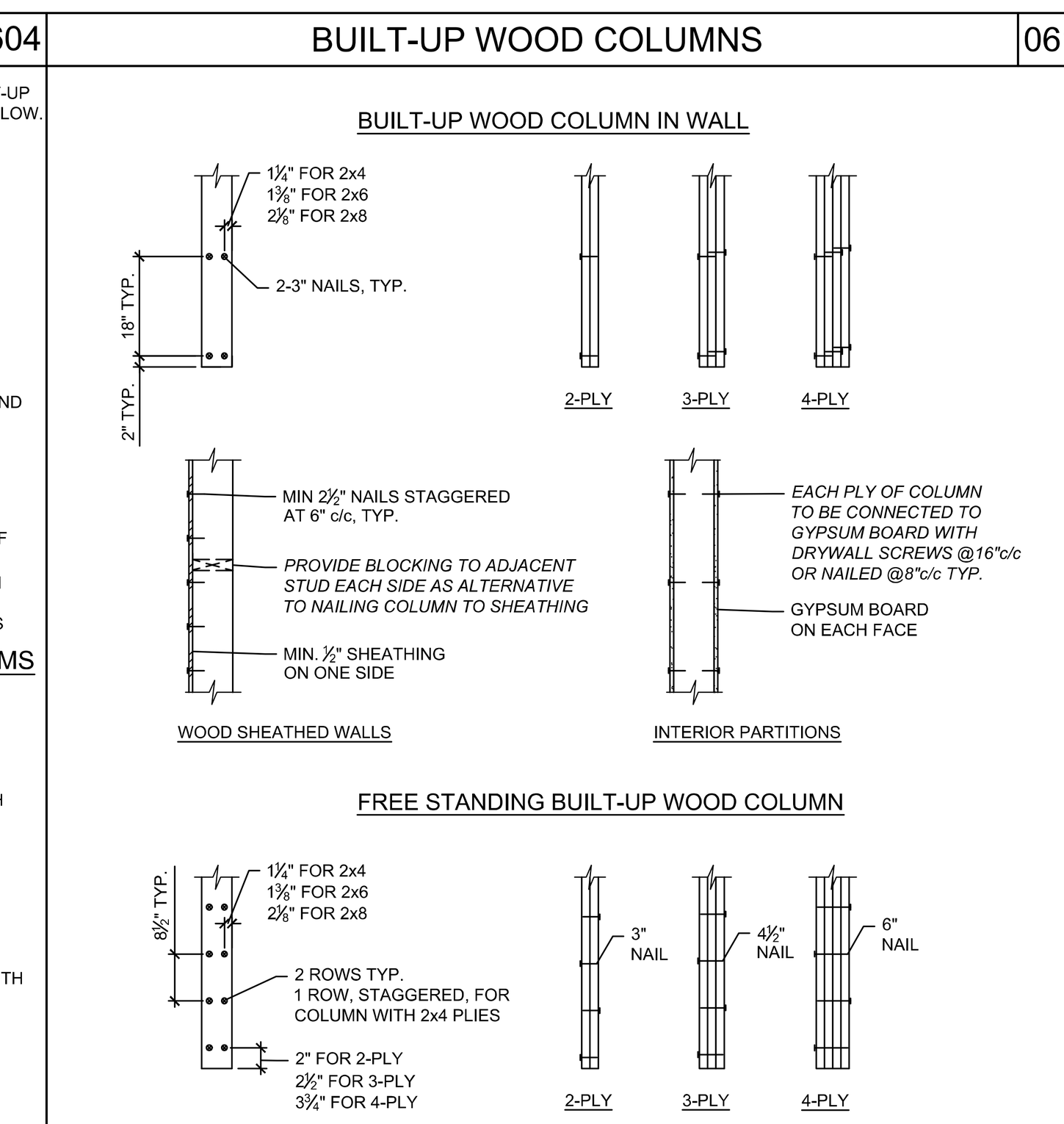
NOTE: SEE ABOVE FOR SPACING OF ANCHORS



SUPPORT OF JOISTS

NOTES:
1. THE PLIES OF 4- AND 5-PLY BEAMS ARE TO BE FASTENED USING BOLTS ONLY. 2- AND 3-PLY BEAMS MAY BE NAILED OR BOLTED.
2. 4- AND 5-PLY BEAMS ARE TO BE TOP LOADED WHENEVER POSSIBLE; THEY MAY BE SIDE LOADED PROVIDED THEY ARE LOADED EQUALLY FROM BOTH SIDES.

BUILT-UP WOOD COLUMNS 0610

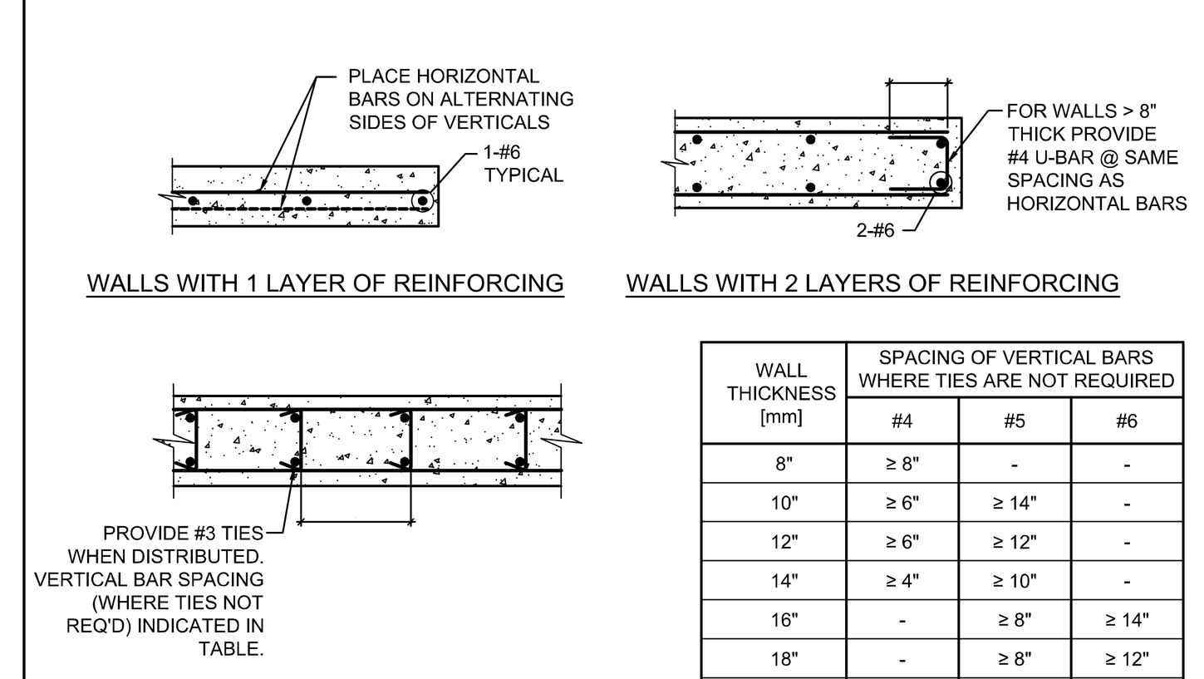


NOTES:
1. NO SPLICES SHALL BE MADE IN ANY BUILT-UP WOOD COLUMNS.
2. DETAILS ABOVE ONLY APPLY TO SPF COLUMNS.
3. ALL NAILS TO BE COMMON WIRE NAILS.

CONCRETE WALL REINFORCING 0313A

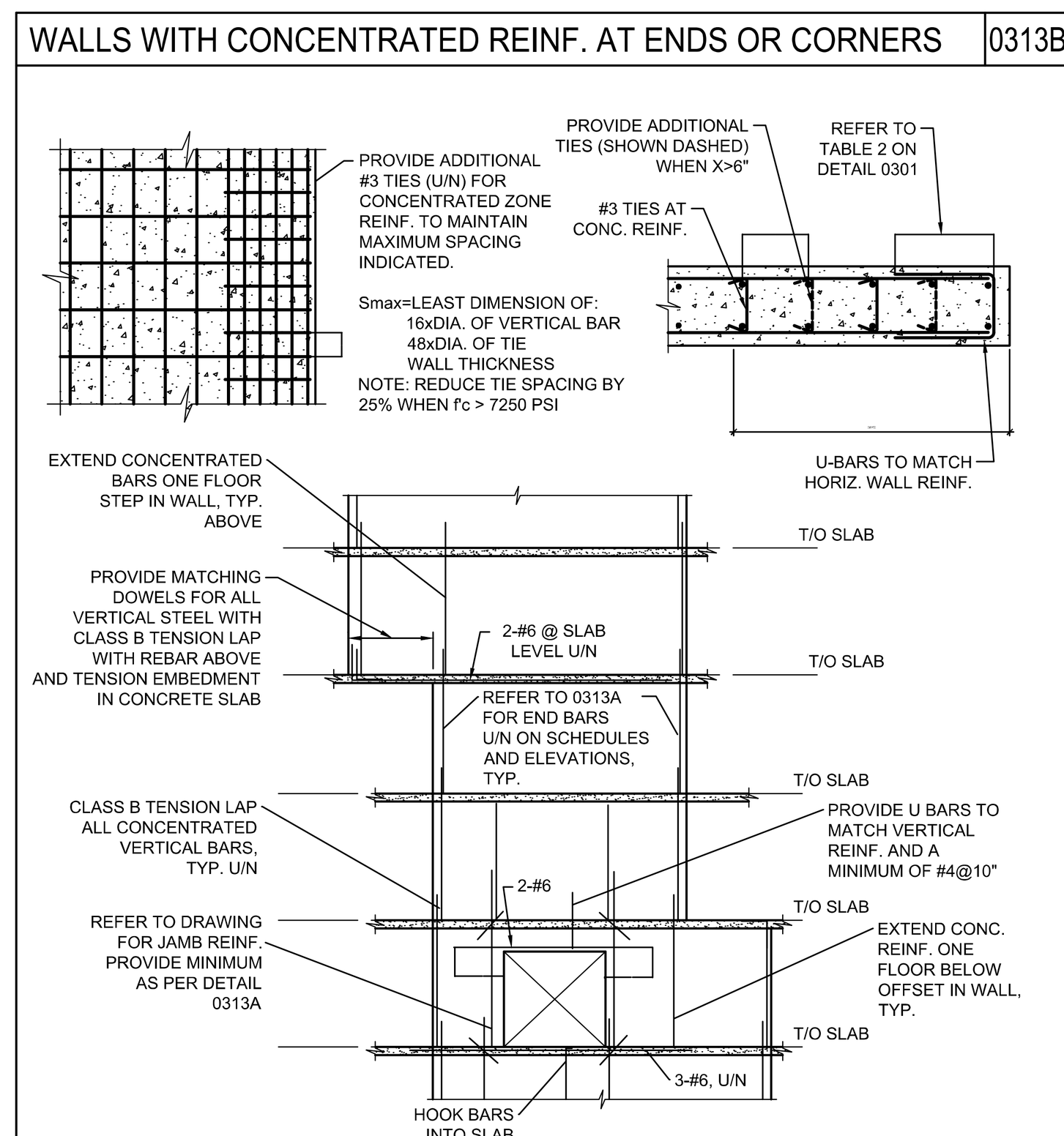
MARK	THICKNESS	VERTICAL BARS EACH FACE	HORIZONTAL BARS EACH FACE	REMARKS
W6	6"	#4@14"	#4@12"	1 LAYER
W8	8"	#5@20"	#4@20"	
W10	10"	#5@20"	#4@16"	
W12	12"	#5@20"	#4@12"	
W14	14"	#5@20"	#5@20"	
W16	16"	#5@20"	#5@20"	

NOTES:
1. UNLESS NOTED OTHERWISE ON THE DRAWINGS, PROVIDE THE REINFORCING SHOWN ABOVE IN ALL CONCRETE WALLS, EXCEPT FOR FOUNDATION WALLS RETAINING SOIL ON ONE SIDE.
2. SEE PLAN FOR WALL MARKS
3. CONFORM TO DETAILS BELOW FOR ADDITIONAL TIES.
4. FOR WALL THICKNESS BETWEEN THOSE SCHEDULED, PROVIDE REINFORCING AS FOR THE NEXT THICKER WALL.
5. #7 AT ENDS OF WALLS TO BE TENSION LAPPED WITH REBAR ABOVE.
5. SEE DETAIL 0313B FOR ADDITIONAL INFORMATION AT WALL OFFSETS.



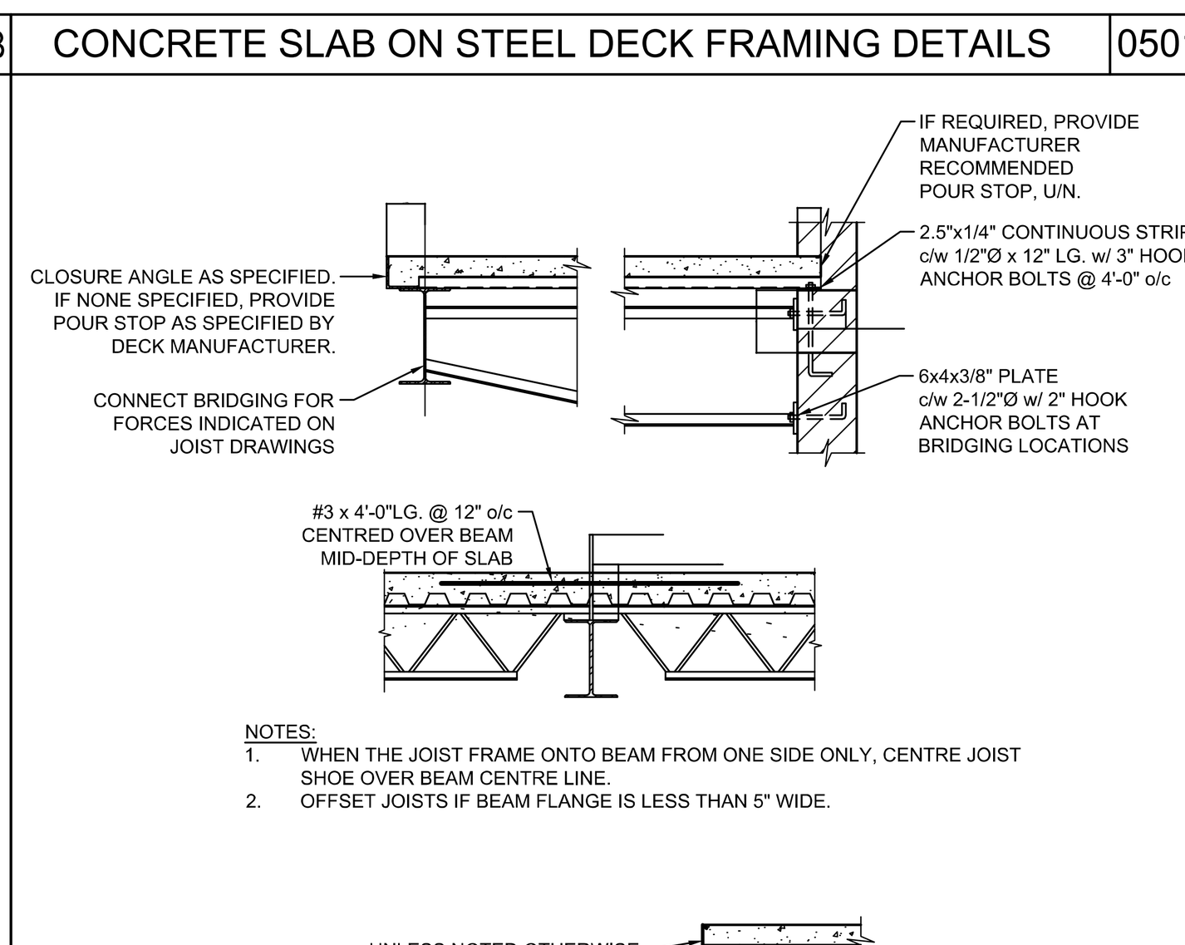
WALL THICKNESS [mm]	SPACING OF VERTICAL BARS WHERE TIES ARE NOT REQUIRED #4	#5	#6
8"	≥ 8"	-	-
10"	≥ 6"	≥ 14"	-
12"	≥ 6"	≥ 12"	-
14"	≥ 4"	≥ 10"	-
16"	-	≥ 8"	≥ 14"
18"	-	≥ 8"	≥ 12"
20"	-	≥ 6"	≥ 10"
24"	-	≥ 5"	≥ 8"
6" (2 LAYERS)	≥ 10"	-	-

WALLS WITH CONCENTRATED REINF. AT ENDS OR CORNERS 0313B

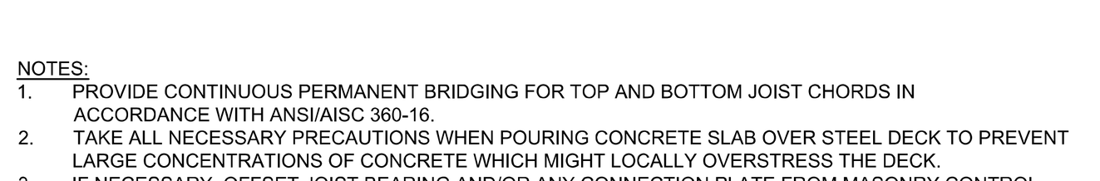


NOTES:
1. TIE ARRANGEMENT AT CONCENTRATED REINFORCING TO SAME AS COLUMNS, SEE TYPICAL DETAIL 0329.
2. CONFORM TO DETAILS ABOVE FOR ADDITIONAL TIES.
3. DO NOT CRANK VERTICAL BARS.

CONCRETE SLAB ON STEEL DECK FRAMING DETAILS 0501



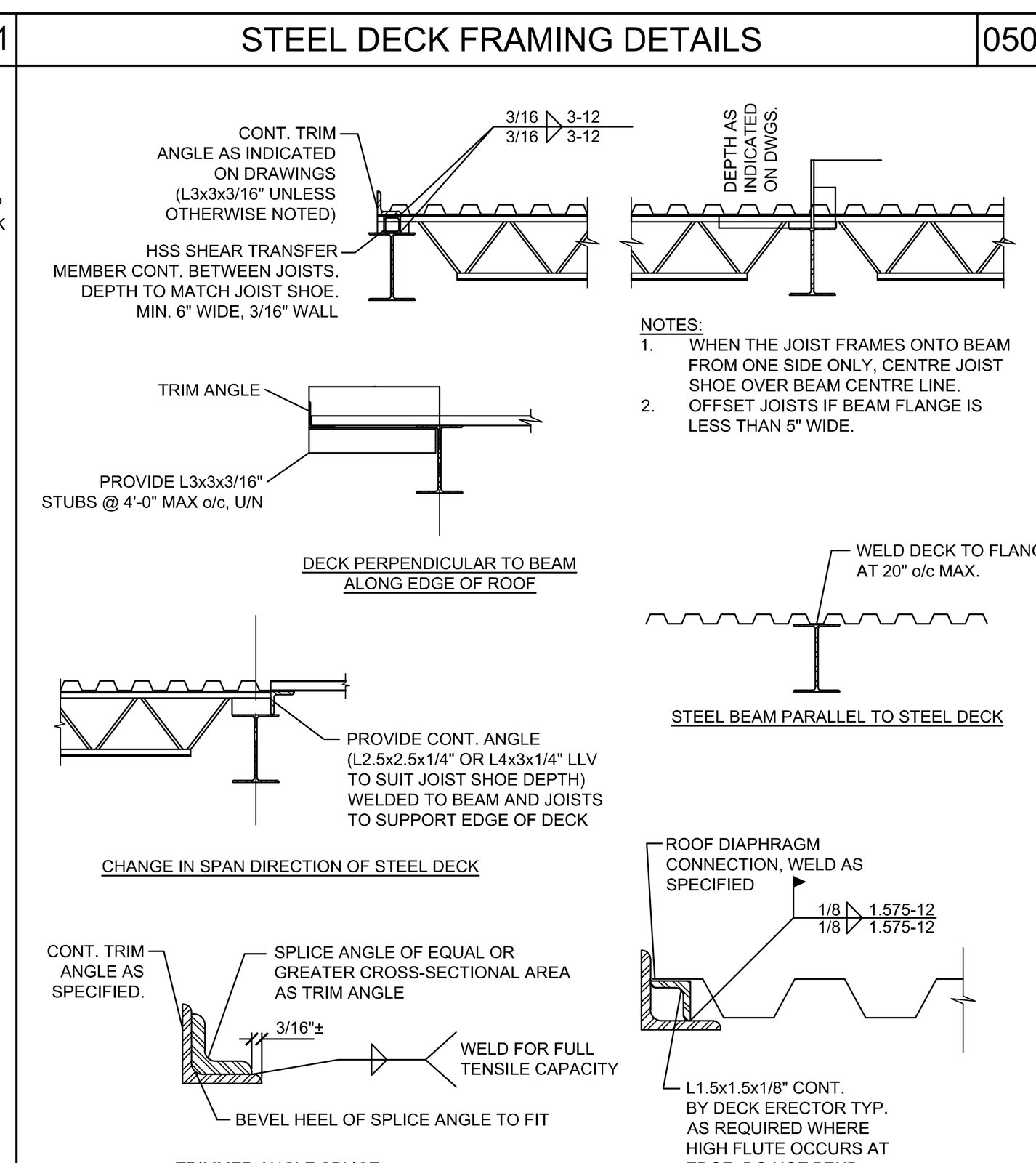
NOTES:
1. WHEN THE JOIST FRAME ONTO BEAM FROM ONE SIDE ONLY, CENTRE JOIST SHOE OVER BEAM CENTRE LINE.
2. OFFSET JOISTS IF BEAM FLANGE IS LESS THAN 5" WIDE.



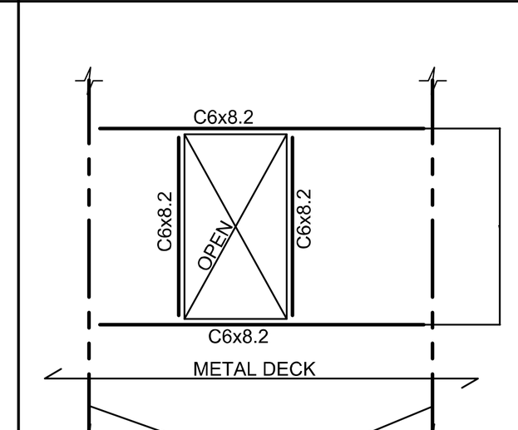
DETAIL AT FRAMED OPENINGS

NOTES:
1. PROVIDE CONTINUOUS PERMANENT BRIDGING FOR TOP AND BOTTOM JOIST CHORDS IN ACCORDANCE WITH ANSI/AISC 360-16.
2. TAKE ALL NECESSARY PRECAUTIONS WHEN POURING CONCRETE SLAB OVER STEEL DECK TO PREVENT LARGE CONCENTRATIONS OF CONCRETE WHICH MIGHT LOCALLY OVERSTRESS THE DECK.
3. IF NECESSARY, OFFSET JOIST BEARING AND/OR ANY CONNECTION PLATE FROM MASONRY CONTROL JOISTS.
4. PROVIDE A MINIMUM DEPTH OF 8" OF 100% SOLID MASONRY UNITS FOR STEEL JOISTS BEARING ON MASONRY.

STEEL DECK FRAMING DETAILS 0503

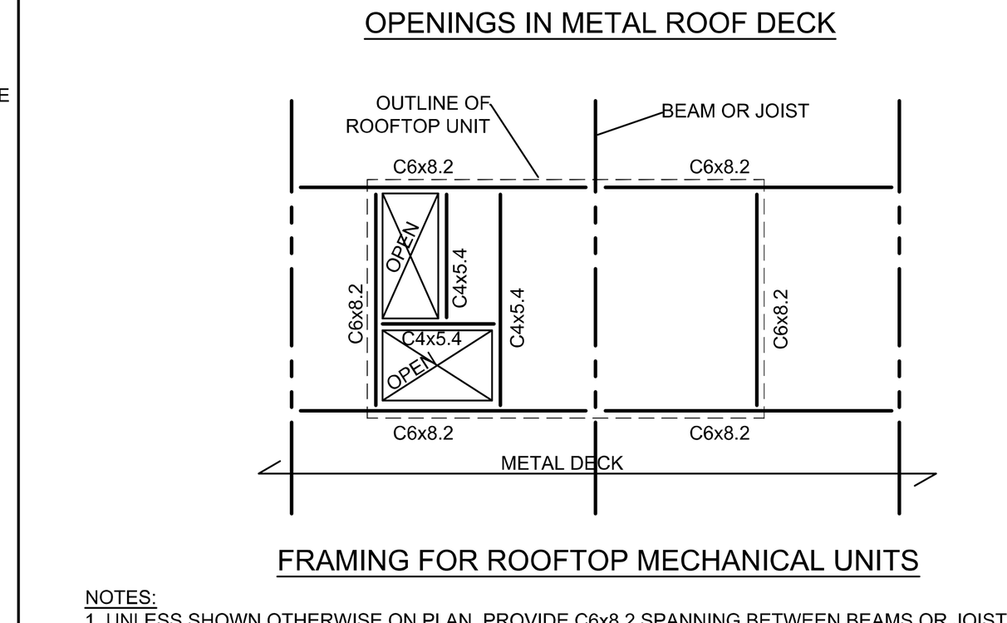


FRAMING FOR ROOFTOP UNITS AND OPENINGS IN DECK 0504



NOTES:
1. OPENINGS UP TO 6" SQUARE OR IN DIAMETER AND SPACED A MINIMUM 12" CLEAR FROM ADJACENT OPENINGS DO NOT REQUIRE REINFORCING.
2. OPENINGS LARGER THAN 6", BUT NOT EXCEEDING 18", ARE TO BE REINFORCED BY THE METAL DECK SUPPLER AS REQUIRED.
3. OPENINGS LARGER THAN 18" UP TO MAXIMUM 4'-0" SQUARE ARE TO BE REINFORCED AS SHOWN, UNLESS INDICATED OTHERWISE ON PLAN.
4. CONNECT FRAMING FOR A MINIMUM END REACTION OF 2.25 KIPS.

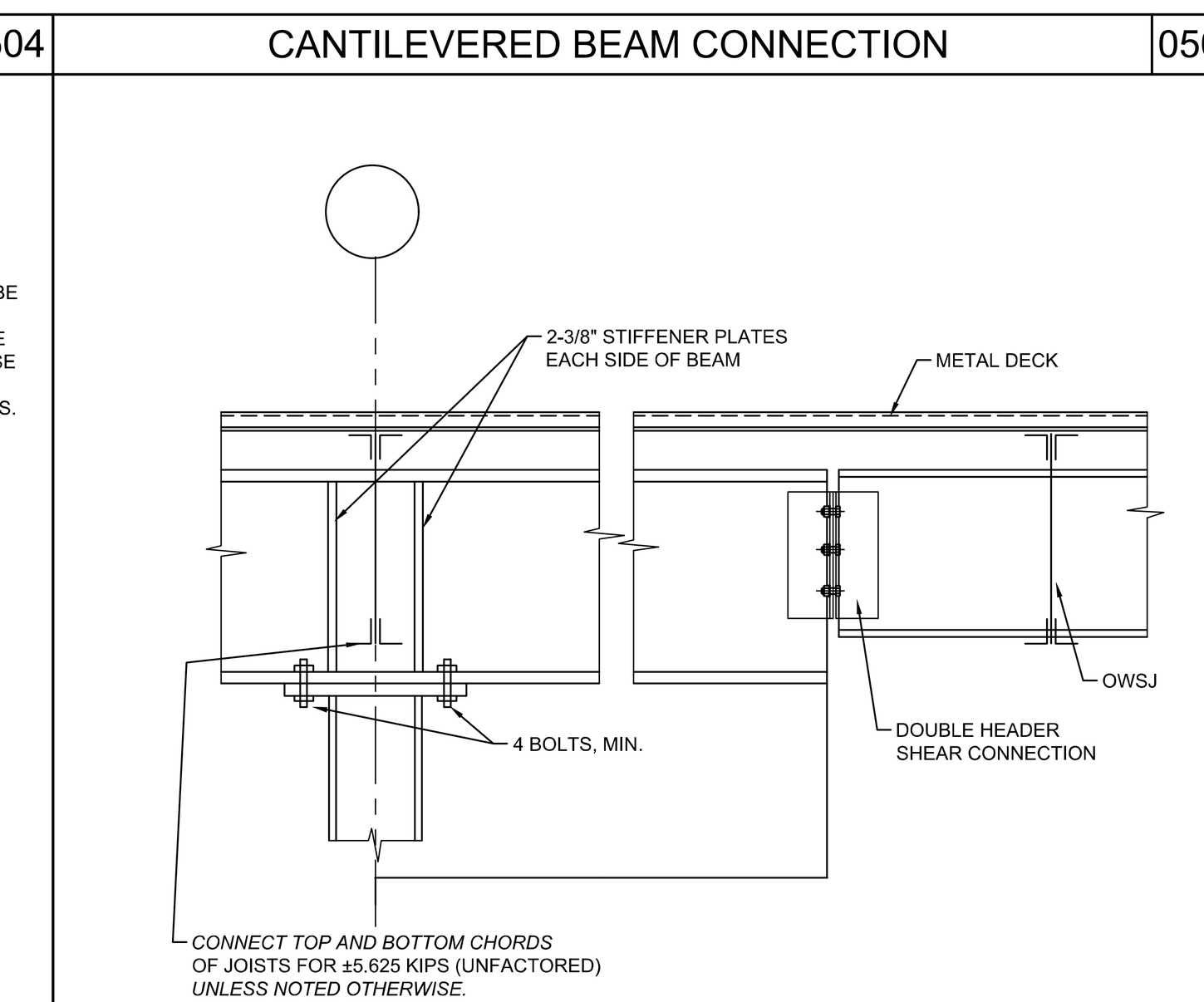
OPENINGS IN METAL ROOF DECK



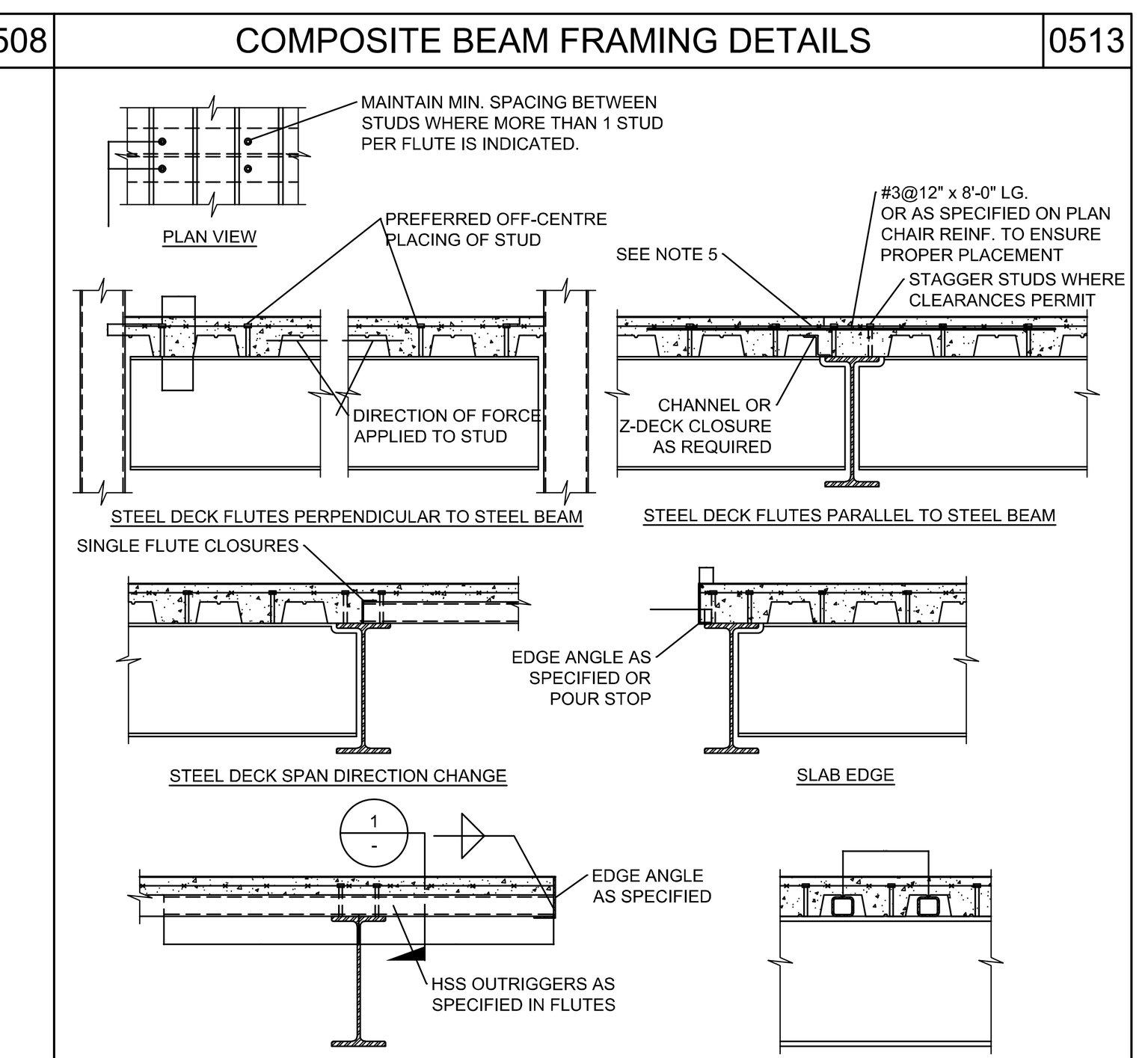
FRAMING FOR ROOFTOP MECHANICAL UNITS

NOTES:
1. UNLESS SHOWN OTHERWISE ON PLAN, PROVIDE C6x8.2 SPANNING BETWEEN BEAMS OR JOISTS EACH SIDE OF ROOFTOP UNIT, AS WELL AS AT EACH END, IN THE MANNER INDICATED ABOVE. CHANNELS TO BE LOCATED DIRECTLY UNDER CURBS SUPPORTING UNIT. OMIT CHANNEL IF CURB IS DIRECTLY OVER A BEAM OR JOIST.
2. PROVIDE C4x5.4 TO SUPPORT EDGES OF DECK AT OPENINGS IN THE MANNER INDICATED ABOVE. IF THE DECK UNDER THE ENTIRE UNIT IS TO BE OMITTED, THE C4x5.4 FRAMING CAN BE DELETED.
3. CONNECT C6x8.2 TO SUPPORTING BEAMS OR JOISTS FOR A MINIMUM WORKING LOAD OF 2 KIPS OR ¼ OF THE WEIGHT OF THE UNIT PLUS 1.125 KIPS, WHICHEVER IS MORE. CONNECT WITHOUT DAMAGING THE SUPPORTING STEEL.
4. CONNECT C4x5.4 TO SUPPORTS FOR A MINIMUM WORKING LOAD OF 1.125 KIPS.
5. COORDINATE LOCATION AND SIZE OF UNIT AND OPENINGS, AND LOCATION OF STEEL FRAMING WITH THE MECHANICAL CONSULTANT AND/OR MECHANICAL CONTRACTOR AND THE STRUCTURAL CONSULTANT.
6. THIS DETAIL APPLIES TO NEW CONSTRUCTION ONLY.

CANTILEVERED BEAM CONNECTION 0508



COMPOSITE BEAM FRAMING DETAILS 0513



NOTES:
1. REFER TO THE BEAM SCHEDULE FOR NUMBER OF STUDS REQUIRED ON EACH BEAM.
2. THE STUDS ARE TO BE INSTALLED EQUALLY, BEGINNING FROM EACH END OF THE BEAM. THE MAXIMUM SPACING OF STUDS IS 32"; ADD ADDITIONAL STUDS IF NECESSARY TO MAINTAIN THIS MAXIMUM SPACING.
3. ALL STUDS ON BEAMS ARE TO BE FIELD WELDED UNLESS NOTED OTHERWISE.
4. STUDS SHALL EXTEND A MINIMUM OF 2 DIAMETERS ABOVE THE TOP OF THE STEEL DECK.
5. MESH SHALL BE CHAIRED ABOVE DECK TO ENSURE PROPER PLACEMENT.

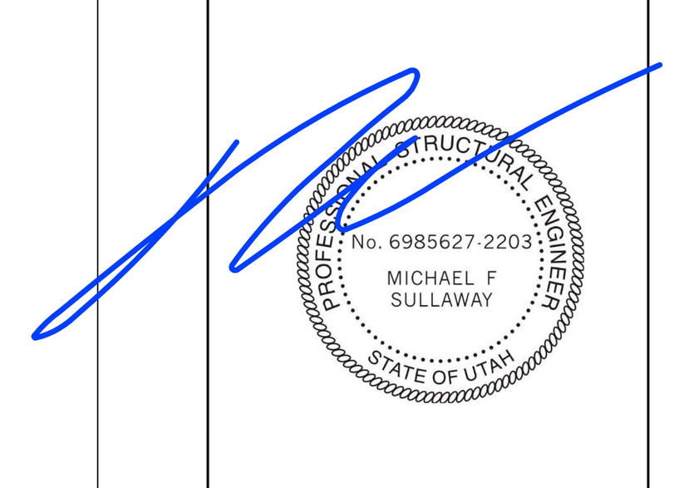
PLAN REVIEW ACCEPTANCE
FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW.
☒ BUILDING ☒ STRUCTURAL
☒ MECHANICAL ☒ PLUMBING
☒ ELECTRICAL ☒ ENERGY
☐ ACCESSIBILITY ☐ FIRE
PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS.
BY: MEM DATE: 08/22/18
WEST COAST CODE CONSULTANTS, INC.

PLOT DATE: July 27, 2018
FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg

Lot 14 Mountain House
Summit Powder Mountain
Eden, Utah

MacKay-Lyons Sweetapple
Architects Limited
2188 Gottingen St.
Halifax, Nova Scotia
Canada B3K 3B4
ph: (902) 429.1867
fax: (902) 429.6276

Blackwell
Toronto | Waterloo | Victoria | Halifax
416.593.5300 | blackwell.ca



No.	Description	Date
05	Issued for Building Permit	2018.08.15
04	Issued for SSI No 01	2018.07.27
03	Issued for Building Permit	2018.07.13
02	Issued for Review	2018.06.13
01	Issued for Costing	2018.05.30

Revision:

NOTES:

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The use of this drawing shall be governed by standard copyright law as generally accepted in architectural practice.

ARCHITECT'S REQUIREMENTS AND APPROVALS:
It is the Builder's responsibility to notify MacKay-Lyons Sweetapple Architects Ltd. and to seek prior written approval for materials and workmanship which deviates from instructions provided by the Architect.

ENGINEER'S REQUIREMENTS AND APPROVALS:
It is the Builder's responsibility to notify MacKay-Lyons Sweetapple Architects Ltd. and to seek prior written approval for materials and workmanship which deviates from instructions provided by the Engineer.

AUTHORITIES' REQUIREMENTS AND APPROVALS:
All materials and workmanship must comply with the requirements of all authorities having jurisdiction over the work. It is the Builder's responsibility to gain necessary approval from all relevant Authorities.

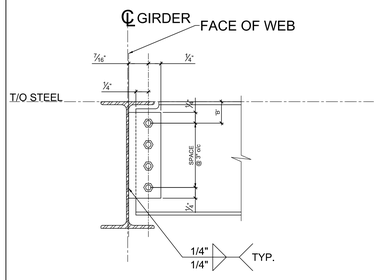
DIMENSIONS:
All dimensions must be verified on site. Do not scale off drawings. Plans take precedent over elevations. In the absence of dimensions, or if discrepancies exist, consult Architect. All minimum dimensions are to comply with the National Building Code of Canada.

SHOP DRAWINGS:
Submit shop drawings to the Architect and Engineer for approval prior to manufacture of prefabricated elements of the building.

Typical Details1

scale: NOT TO SCALE
date: 2018-05-20
drawn: DP
chk'd: TJ

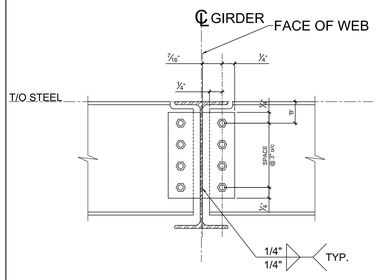
S002



REFER TO DETAIL 5 FOR SHEAR PL AND BOLTS

ONE-SIDED CONNECTION

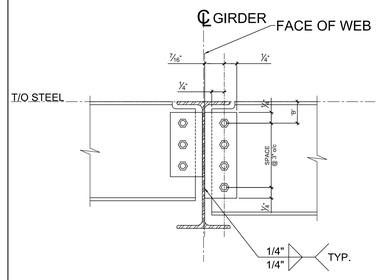
BEAM TO GIRDER	3/4" = 1'-0"	1



REFER TO DETAIL 5 FOR SHEAR PL AND BOLTS

TWO-SIDED CONNECTION

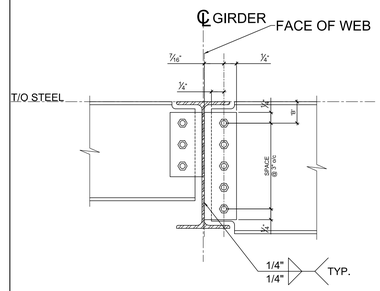
BEAM TO GIRDER	3/4" = 1'-0"	2



REFER TO DETAIL 5 FOR SHEAR PL AND BOLTS

TWO-SIDED CONNECTION

BEAM TO GIRDER	3/4" = 1'-0"	3



REFER TO DETAIL 5 FOR SHEAR PL AND BOLTS

TWO-SIDED CONNECTION

BEAM TO GIRDER	3/4" = 1'-0"	4

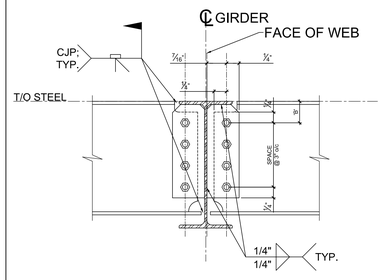
CONNECTION SCHEDULE

BEAM SIZE	PLATE THICKNESS	BOLTS DIA.	BOLTS QTY.	DIM. 'B'	Vr [kip] (5) (MAX)
W8	5/16	3/4	2	2½	24
W10	5/16	3/4	2	2½	24
W12	5/16	3/4	3	2½	41
W14	5/16	3/4	3	2½	43
W16	5/16	3/4	4	2½	62
W18	5/16	3/4	5	2½	81

CONNECTION SCHEDULE NOTES

1. ALL HOLE SIZES ARE 13/16"Ø UNLESS NOTED
2. BOLTS TO BE ASTM A325N OR A325X
3. ALL WELDS TO BE E70XX
4. SHEAR PLATES SHALL BE MINIMUM GRADE A36
5. REFER TO BEAM SCHEDULE FOR FACTORED DESIGN REACTIONS. THE REACTION FORCES NOTED IN THE RESPECTIVE BEAM SCHEDULES SHALL NOT EXCEED THE CONNECTION CAPACITY NOTED IN THIS TABLE.

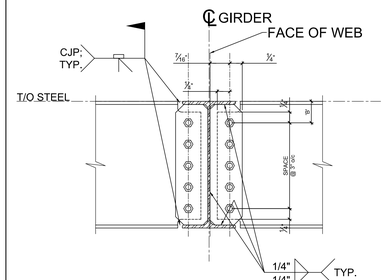
CONNECTION SCHEDULE	NTS	5



REFER TO DETAIL 5 FOR SHEAR PL AND BOLTS

UNEQUAL BEAM AND GIRDER DEPTH

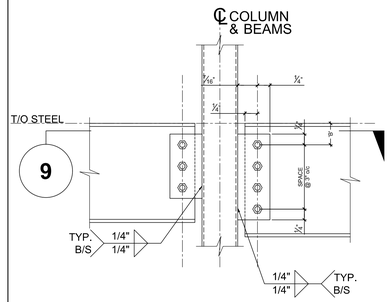
MOMENT CONNECTION	3/4" = 1'-0"	6



REFER TO DETAIL 5 FOR SHEAR PL AND BOLTS

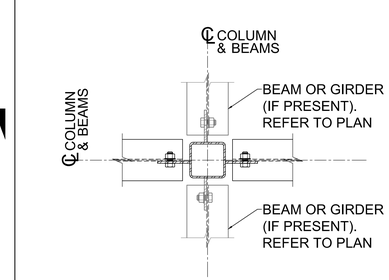
EQUAL BEAM AND GIRDER DEPTH

MOMENT CONNECTION	3/4" = 1'-0"	7



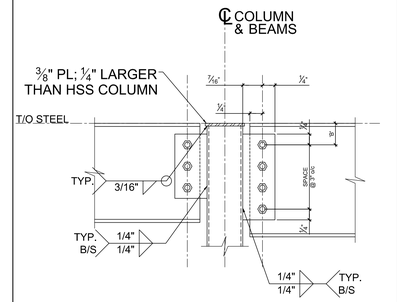
REFER TO DETAIL 5 FOR SHEAR PL AND BOLTS

GIRDER TO COLUMN	3/4" = 1'-0"	8



REFER TO DETAIL 5 FOR SHEAR PL AND BOLTS

GIRDER TO COLUMN	3/4" = 1'-0"	9



REFER TO DETAIL 5 FOR SHEAR PL AND BOLTS

TOP OF HSS COLUMN	3/4" = 1'-0"	10

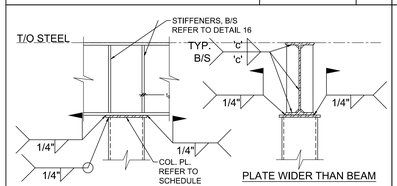


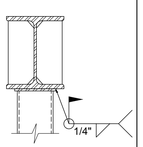
PLATE WIDER THAN BEAM

COL. PLATE SCHEDULE

COLUMN SIZE	PLATE THICKNESS	PLATE SIZE (SQ. U/N)	Cr (4) [kip]
3"x3"	5/16"	3¾"x3¾"	48
4"x4"	5/16"	4¾"x4¾"	85
5"x5"	5/16"	5¾"x5¾"	120
6"x6"	5/16"	6¾"x6¾"	155

COL. PLATE SCHEDULE NOTES

1. ALL WELDS TO BE E70XX
2. PLATES SHALL BE GRADE A36
3. MIN. COLUMN HEIGHT: 8'-0"
4. MAXIMUM COLUMN LOAD. REFER TO PLAN FOR DESIGN FORCES
5. REFER TO DETAIL 16 FOR WELD SIZE 'c' FOR STIFFENERS, TYP.



BEAM WIDER THAN PLATE

NOTE THIS DETAIL IS ONLY APPLICABLE IF A BEAM THAT IS MIN. ½ OF THE DEPTH OF THE CANT. BEAM FRAMES INTO THE CANT. BEAM OVER THE COLUMN

BEAM OVER COL. - WELD	3/4" = 1'-0"	12

COLUMN PLATE SCHEDULE

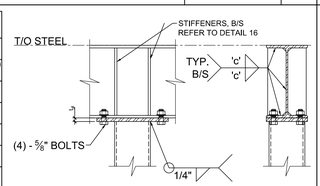
COLUMN SIZE	BEAM FLANGE WIDTH	PLATES t_{pl}	PLATES L_{pl}	PLATES W_{pl}	PLATES s (7)
3"x3"	< 4½"	1/2"	9"	4"	2¼"
	4½" - 6"	1/2"	9"	5"	2¾"
	6"-7"	1/2"	9"	6"	3½"
	7"-8¼"	1/2"	9"	7"	4"
	> 7¾"	1/2"	9"	8"	5½"
4"x4"	< 4½"	1/2"	10"	4"	2¼"
	4½" - 6"	1/2"	10"	5"	2¾"
	6"-7"	1/2"	10"	6"	3½"
	7"-8¼"	1/2"	10"	7"	4"
	> 7¾"	1/2"	10"	8"	5½"

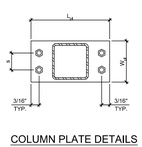
COLUMN SIZE	BEAM FLANGE WIDTH	PLATES t_{pl}	PLATES L_{pl}	PLATES W_{pl}	PLATES s (7)
5"x5"	< 4½"	1/2"	11"	4"	2¼"
	4½" - 6"	1/2"	11"	5"	2¾"
	6"-7"	1/2"	11"	6"	3½"
	7"-8¼"	1/2"	11"	7"	4"
	> 7¾"	1/2"	11"	8"	5½"
6"x6"	< 4½"	1/2"	12"	4"	2¼"
	4½" - 6"	1/2"	12"	5"	2¾"
	6"-7"	1/2"	12"	6"	3½"
	7"-8¼"	1/2"	12"	7"	4"
	> 7¾"	1/2"	12"	8"	5½"

COLUMN PLATE SCHEDULE NOTES

1. ALL HOLE SIZES ARE 13/16"Ø UNLESS NOTED
2. BOLTS TO BE ASTM A325N OR A325X
3. ALL WELDS TO BE E70XX
4. PLATES SHALL BE GRADE A36
5. MINIMUM COLUMN HEIGHT: 8'-0"
6. MAXIMUM FACTORED COLUMN LOADS. REFER TO PLAN:
   HSS3"x3" = 48 kip HSS5"x5" = 120 kip
   HSS4"x4" = 85 kip HSS6"x6" = 155 kip
7. THE GAUGE 's' FOR ALL W16 AND W18 SECTIONS SHALL BE 3½"
8. REFER TO DETAIL 16 FOR WELD SIZE 'c' FOR STIFFENERS, TYP.

BEAM OVER COLUMN - BOLTS	3/4" = 1'-0"	13





COLUMN PLATE DETAILS

NOTE THIS DETAIL IS ONLY APPLICABLE IF A BEAM THAT IS MIN. ½ OF THE DEPTH OF THE CANT. BEAM FRAMES INTO THE CANT. BEAM OVER THE COLUMN

GIRDER TO COLUMN	NTS	16

STIFFENER SCHEDULE

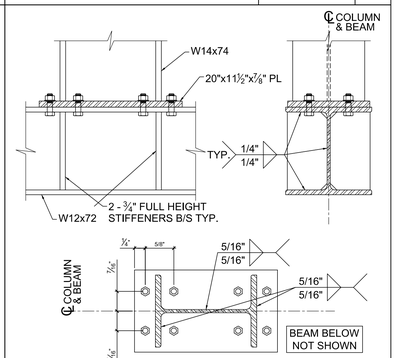
COLUMN SIZE	BEAM SIZE	PLATES (2) t_S	PLATES (2) c
3"x3"	W8	5/16"	¼"
	W10	5/16"	¼"
	W12	3/8"	¼"
	W14	3/8"	¼"
	W16	3/8"	¼"
	W18	3/8"	¼"
4"x4"	W8	5/16"	¼"
	W10	3/8"	¼"
	W12	½"	¼"
	W14	½"	¼"
	W16	½"	¼"
	W18	½"	¼"

COLUMN SIZE	BEAM SIZE	PLATES (2) t_S	PLATES (2) c
5"x5"	W8	5/16"	¼"
	W10	3/8"	¼"
	W12	½"	¼"
	W14	½"	¼"
	W16	½"	¼"
	W18	½"	¼"
6"x6"	W8	5/16"	¼"
	W10	3/8"	¼"
	W12	½"	¼"
	W14	½"	¼"
	W16	½"	¼"
	W18	¾"	¼"

STIFFENER SCHEDULE NOTES

1. SECTIONS HEAVIER THAN WHAT IS NOTED IN THE SCHEDULE DO NOT REQUIRE STIFFENERS
2. PLATE WIDTH (REFERENCE DIMENSION A SHALL BE TAKEN AS b_f/2 - 2" WHERE b_f IS THE WIDTH OF THE BEAM FLANGE
3. ALL WELDS TO BE E70XX
4. PLATES SHALL BE GRADE A36

STIFFENER SCHEDULE	NTS	16



NOTES

1. ALL HOLE SIZES ARE 13/16"Ø UNLESS NOTED
2. BOLTS TO BE ASTM A325N OR A325X
3. ALL WELDS TO BE E70XX
4. PLATES SHALL BE GRADE MIN. GRADE A36
5. REFER TO 2/S202

F.5 MOMENT FRAME	3/4" = 1'-0"	18

PLAN REVIEW ACCEPTANCE
FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW:
☒BUILDING ☒STRUCTURAL
☒MECHANICAL ☒PLUMBING
☒ELECTRICAL ☒ENERGY
☐ACCESSIBILITY ☐FIRE
PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS.
BY: MEM DATE: 09/22/18
WEST COAST CODE CONSULTANTS, INC.

PLOT DATE: July 27, 2018
FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg

Lot 14 Mountain House
Summit Powder Mountain
Eden, Utah

MacKay-Lyons Sweetapple
Architects Limited
2188 Gottingen St.
Halifax, Nova Scotia
Canada B3K 3B4
ph: (902) 429.1867
fax: (902) 429.6276

Blackwell
Toronto | Waterloo | Victoria | Halifax
416.593.5300 | blackwell.ca

No.	Description	Date
05	Issued for Building Permit	2018.08.15
04	Issued for SSI No 01	2018.07.27
03	Issued for Building Permit	2018.07.13
02	Issued for Review	2018.06.13
01	Issued for Costing	2018.05.30
Revision:		

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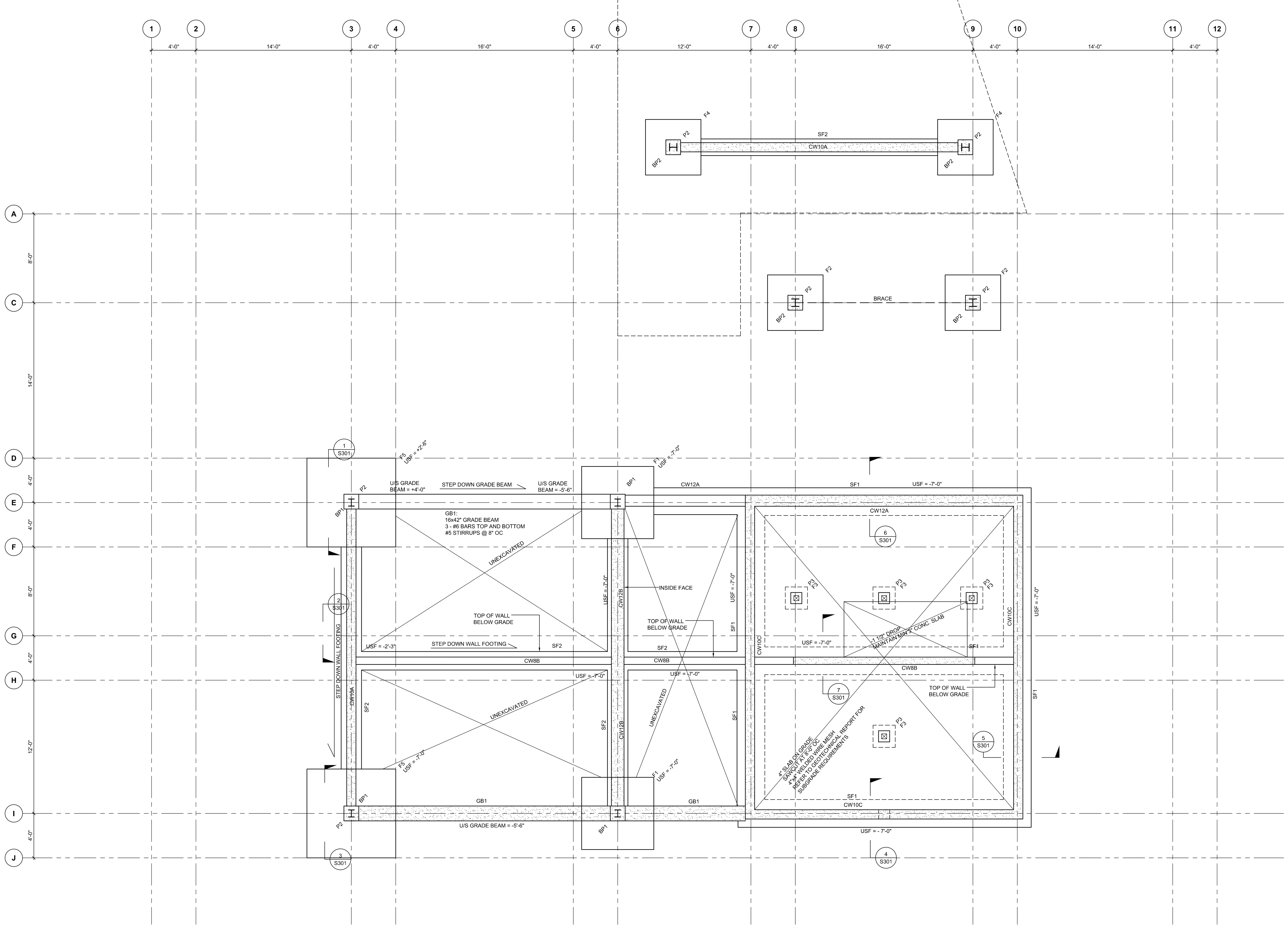
SHOP DRAWINGS:
Submit shop drawings to the Architect and Engineer for approval prior to manufacture of prefabricated elements of the building.

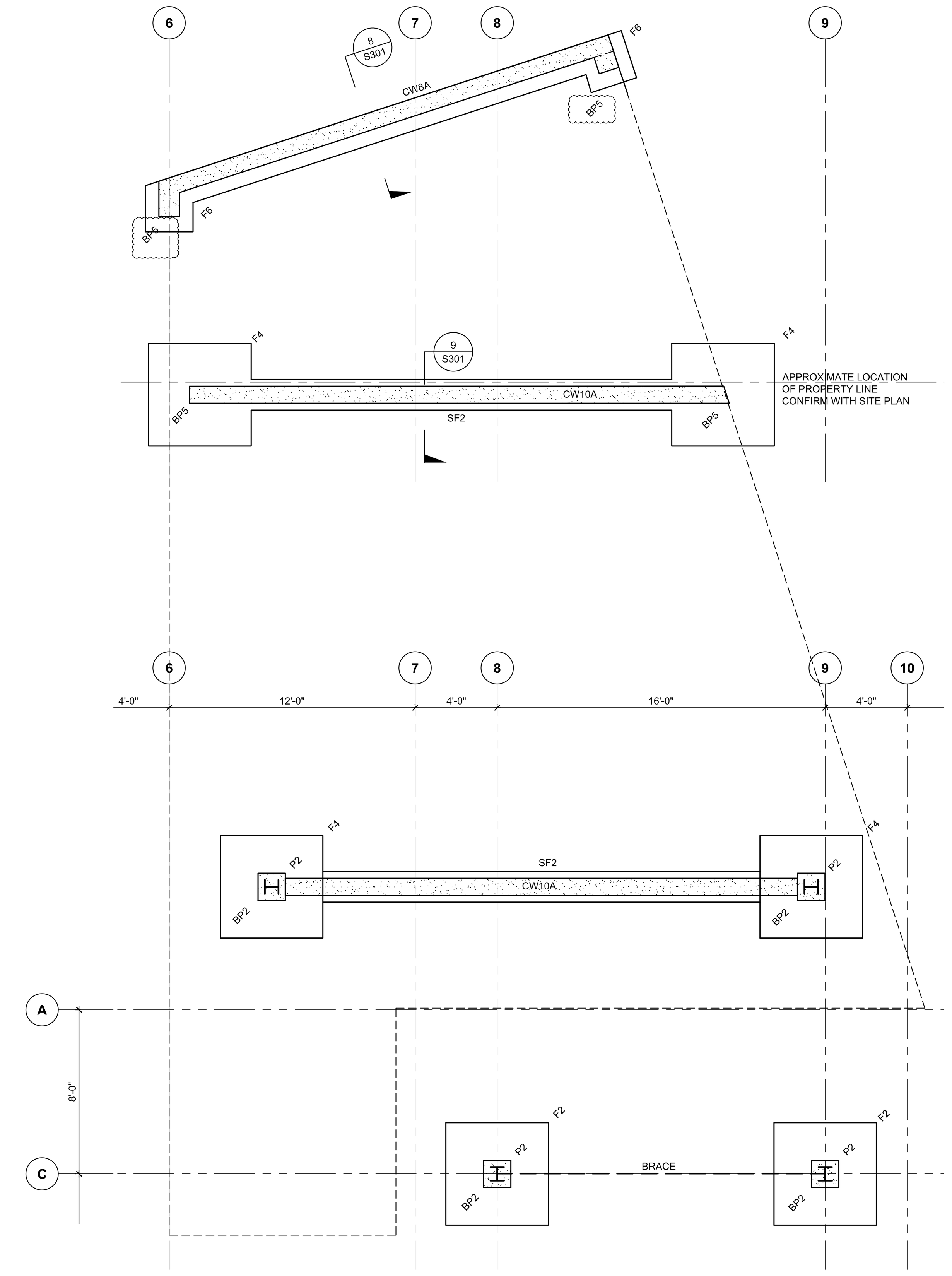
Typical Steel Details

scale: NOT TO SCALE
date: 2018-05-20
drawn: DP
chk'd: TJ

S003



1 S100 FOUNDATION PLAN
1/4"=1'-0"



2 S103 PART PLAN - BRIDGE FOUNDATIONS
1/4" = 1'-0"

FOUNDATION PLAN NOTES

1. BASEMENT FLOOR DATUM ELEVATION, -20'-0" BELOW MAIN FLOOR DATUM.
2. WHERE CROSSED AND NOTED THE LOCAL DATUM FOR RAISED OR LOWERED AREAS ARE GIVEN RELATIVE TO THE LOWER FLOOR DATUM.
3. EXCEPT AS CROSSED AND NOTED TOP OF FINISHED FLOOR IS 0" BELOW THE LOWER FLOOR DATUM.
4. WHERE CROSSED AND NOTED, SLAB DEPRESSIONS OR LOCALLY RAISED AREAS ARE GIVEN RELATIVE TO THE BASEMENT FLOOR DATUM.
5. FOUND FOOTINGS AT THE ELEVATIONS RELATIVE TO THE LOWEST FLOOR DATUM SHOWN IN PLAN
6. REFER TO THE ARCHITECTURAL DRAWINGS FOR THE GEODETIC ELEVATION OF THE GROUND FLOOR DATUM

FOOTING SCHEDULE

F1:
6'-6"x6'-6"x1'-0" PAD FOOTING
8 - #5 BARS BOTTOM EACH WAY

F2:
6'-0"x6'-0"x1'-0" PAD FOOTING
8 - #6 BARS BOTTOM EACH WAY
FOUND UNDERSIDE OF FOOTINGS
MINIMUM 3'-6" BELOW PROPOSED
FINISHED GRADE - REFER TO
GRADING PLAN

F3:
2'-0"x2'-0"x10" PAD FOOTING
PLAIN

F4:
5'-0"x5'-0"x0'-10" PAD FOOTING
6 - #6 BARS BOTTOM EACH WAY
FOUND UNDERSIDE OF FOOTINGS
MINIMUM 3'-6" BELOW PROPOSED
FINISHED GRADE - REFER TO
GRADING PLAN

F5:
8'-0"x8'-0"x1'-2" PAD FOOTING
10 - #6 BARS BOTTOM EACH WAY

F6:
2'-6"x2'-6"x10" PAD FOOTING
3 - #5 BARS BOTTOM EACH WAY

STRIP FOOTING SCHEDULE

SF1:
2'-6"x10" STRIP FOOTING
4 - #6 CONTINUOUS LONG DIRECTION

SF2:
1'-8"x10" STRIP FOOTING
2 - #6 CONTINUOUS LONG DIRECTION

CONCRETE WALL SCHEDULE

CW8A:
8" CONCRETE WALL
#5 BARS @ 16" HORIZONTAL AND VERTICAL EACH FACE
SEE SECTION 1/S301

CW8B:
8" CONCRETE WALL
#5 @ BARS @ 16" HORIZONTAL AND VERTICAL

CW10A:
10" CONCRETE WALL
#5 BARS @ 16" HORIZONTAL AND VERTICAL EACH FACE

CW10B:
10" CONCRETE WALL
#5 BARS @ 16" HORIZONTAL AND VERTICAL EACH FACE

CW10C:
10" CONCRETE WALL
#5 BARS @ 16" HORIZONTAL AND VERTICAL EACH FACE

CW12A:
12" CONCRETE WALL
#8 BARS @ 8" HORIZONTAL AND VERTICAL INSIDE FACE
#5 BARS @ 16" HORIZONTAL AND VERTICAL OUTSIDE FACE

CW12B:
12" CONCRETE WALL
#5 BARS @ 8" HORIZONTAL INSIDE FACE
#5 BARS @ 16" VERTICAL INSIDE FACE
#5 BARS @ 16" HOR AND VERT OUTSIDE FACE

CONCRETE PIER SCHEDULE

P1:
NOT USED

P2:
16x16"
4 - No. 6 BARS VERT
No. 5 TIES @ 12"

P3:
12x12"
4 - No. 5 BARS VERT
No. 5 TIES @ 12"

BASEPLATE SCHEDULE

BP1:
12x12x3/4" CAST-IN TO TOP OF PIER
4 - #7 REBAR STUBS, 36" LONG
WELD BARS TO U/S OF BASEPLATE
REFER TO 1/S301 FOR ADDN'L INFORMATION

BP2:
12x12x3/4" CAST-IN TO TOP OF PIER
4 - #7 REBAR STUBS, 36" LONG
WELD BARS TO U/S OF BASEPLATE

BP3:
16x10x1" CAST-IN TO TOP OF CONCRETE WALL
4 - #8 REBAR STUBS, 48" LONG
WELD BARS TO U/S OF BASEPLATE
ADD 4 - #8 BARS IN WALL BELOW TO LAP WITH BASEPLATE BARS

BP4:
20x12x1" CAST-IN TO TOP OF CONCRETE WALL
6 - #8 REBAR STUBS, 48" LONG
WELD BARS TO U/S OF BASEPLATE
ADD 6 - #8 BARS IN WALL BELOW TO LAP WITH BASEPLATE BARS

BP5:
8x12x3/4" CAST-IN TO TOP OF CONCRETE WALL
4 - #5 REBAR STUBS, 24" LONG
WELD BARS TO U/S OF BASEPLATE

PLAN REVIEW ACCEPTANCE
FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW.
[X] BUILDING [X] STRUCTURAL
[X] MECHANICAL [X] PLUMBING
[X] ELECTRICAL [X] ENERGY
[ ] ACCESSIBILITY [ ] FIRE
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BY: MEM DATE: 08/22/18
WEST COAST CODE CONSULTANTS, INC.

Lot 14 Mountain House
Summit Powder Mountain
Eden, Utah

MacKay-Lyons Sweetapple
Architects Limited
2188 Gottingen St.
Halifax, Nova Scotia
Canada B3K 3B4
ph: (902) 429.1867
fax: (902) 429.6276

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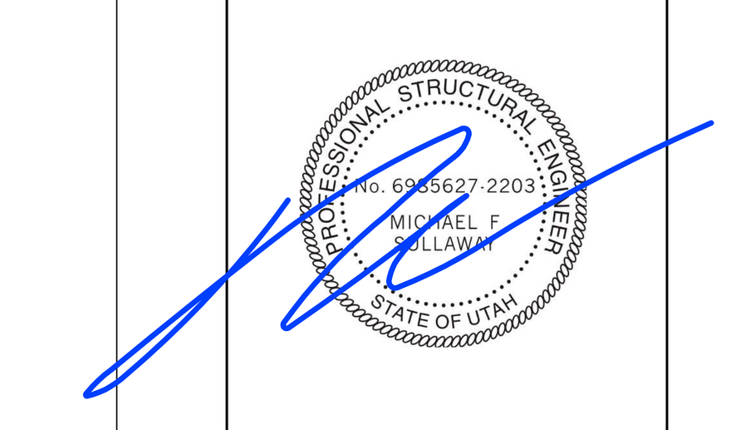

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Revision:

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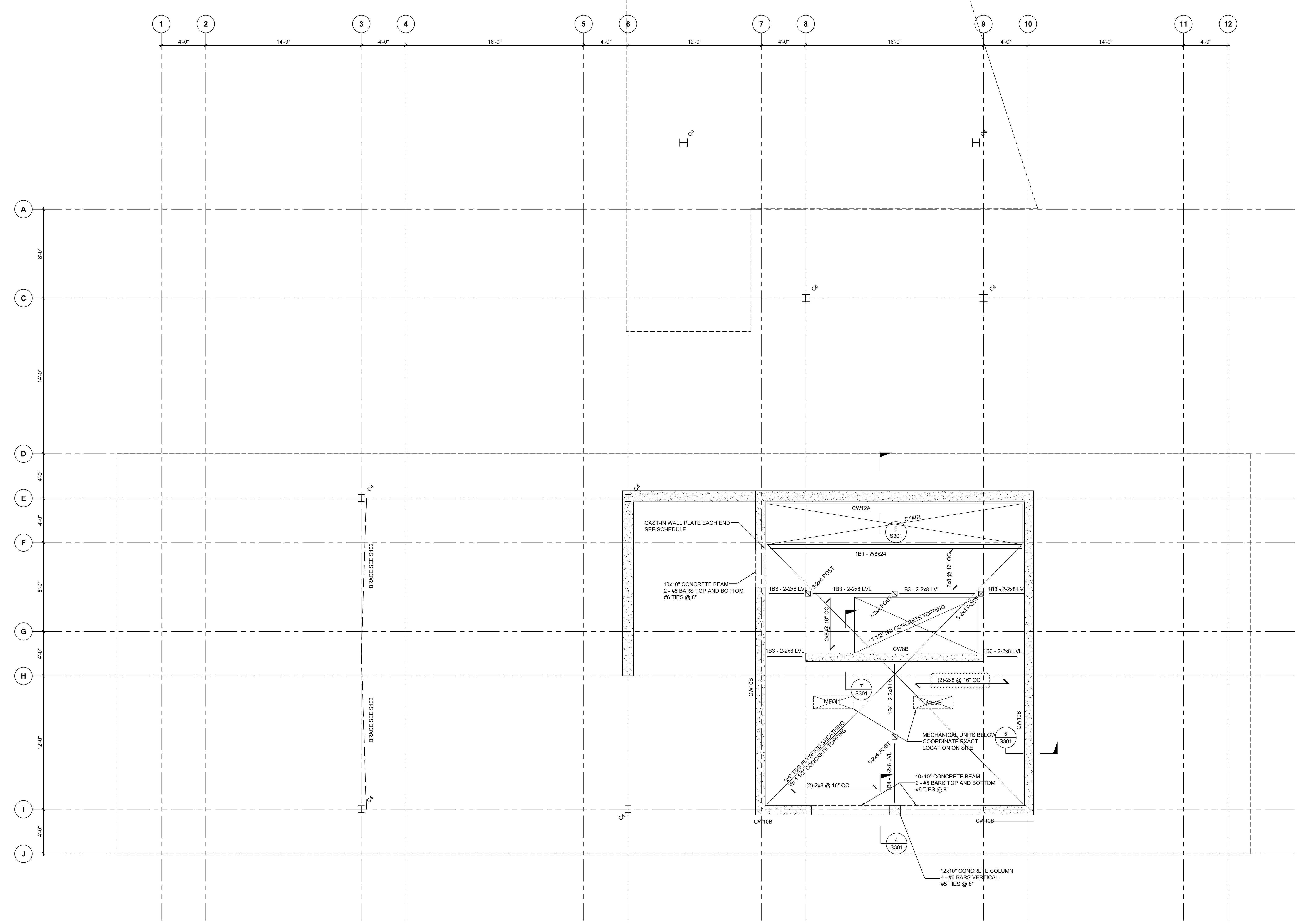
Foundation Plan

scale: 1/4" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

S100



1 / S101 LOWER LEVEL FRAMING PLAN
1/4"=1'-0"

LOWER FLOOR FRAMING PLAN NOTES

1. THE LOWER LEVEL FLOOR DATUM IS AT -11'-0" BELOW MAIN FLOOR DATUM.
2. THE TOP OF THE FINISHED FLOOR IS 0" BELOW THE DATUM.
3. THE TOP OF THE FLOOR SHEATHING IS 1 1/2" mm BELOW THE DATUM.
4. TOP OF WOOD FRAMING IS 2 1/4" BELOW THE DATUM UNLESS NOTED OTHERWISE IN PLAN.
5. LOADS USED IN THE DESIGN:

LIVE LOAD:	40psf
DEAD LOADS:	
PARTITIONS	10psf
FLOOR FINISH	5psf
CONC TOPPING	20psf - 1 1/2" MAX
SHEATHING	2psf
JOISTS	2psf kPa
SUSPENDED	6psf kPa
TOTAL	45psf

6. SEE ROOF PLAN FOR COLUMN SCHEDULE
7. PROVIDE CONTINUOUS SOLID WOOD BLOCKING @ 4' c/c MAX BETWEEN WOOD JOISTS

BEAM SCHEDULE

BEAM MARK	BEAM SECTION	REACTIONS				CAMBER	REMARKS
		LEFT END		RIGHT END			
		LIVE	DEAD	LIVE	DEAD		
1B-1	W8x24	1.8	2.4	1.8	2.4		10x10x1/2" WALL PLATE EA. END 4 - 1/2" NELSON STUDS EACH
1B-3	2-2x8 LVL						
1B-4	2-2x8 LVL						BEAM HANGER OFF CONCRETE
1B-5	4-2x10 LVL						

BEAM SCHEDULE NOTES

1. LEFT AND RIGHT ENDS OF BEAMS ARE DEFINED BY THE ORIENTATION OF THE BEAM MARK ON PLAN.
2. WHERE A BEAM MARK IS INDICATED WITH THE SUFFIX "R" ON PLAN THE REACTIONS ARE TO APPLY AT THE OPPOSITE ENDS.
3. REACTIONS GIVEN ARE SERVICE LOADS IN kips.
4. BEARING PLATE DIMENSION GIVEN FIRST IS PARALLEL TO THE BEAM WEB.
5. CENTRE BEARING PLATES UNDER BEAMS UNLESS NOTED OTHERWISE. PROVIDE 2-3/4" DIA. ANCHOR BOLTS, 16" LG. WITH 3" HOOK AND 4" THREADS, FOR BEARING PLATES BEARING ON CONCRETE OR MASONRY, UNLESS NOTED OTHERWISE. ALTERNATIVELY ANCHOR RODS MAY BE WELDED TO UNDERSIDE OF BEARING PLATE. GROUT MASONRY AS INDICATED IN THE GENERAL NOTES, UNLESS NOTED.
6. DESIGN CONNECTIONS FOR AXIAL FORCE (Pf), END MOMENT (Mf), TORSION (Tf) OR OUT OF PLANE HORIZONTAL FORCE (Hf) SHOWN IN THE REMARKS COLUMN, IN ADDITION TO THE VERTICAL SHEAR PROVIDED IN THE REACTION COLUMN.
7. CAMBERS ARE IN INCHES.

PLAN REVIEW ACCEPTANCE
FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW.
[X] BUILDING [X] STRUCTURAL
[X] MECHANICAL [X] PLUMBING
[X] ELECTRICAL [X] ENERGY
[ ] ACCESSIBILITY [ ] FIRE
PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS.
BY: MEM DATE: 08/22/18
WEST COAST CODE CONSULTANTS, INC.

Lot 14 Mountain House
Summit Powder Mountain
Eden, Utah

MacKay-Lyons Sweetapple
Architects Limited
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ph: (902) 429.1867
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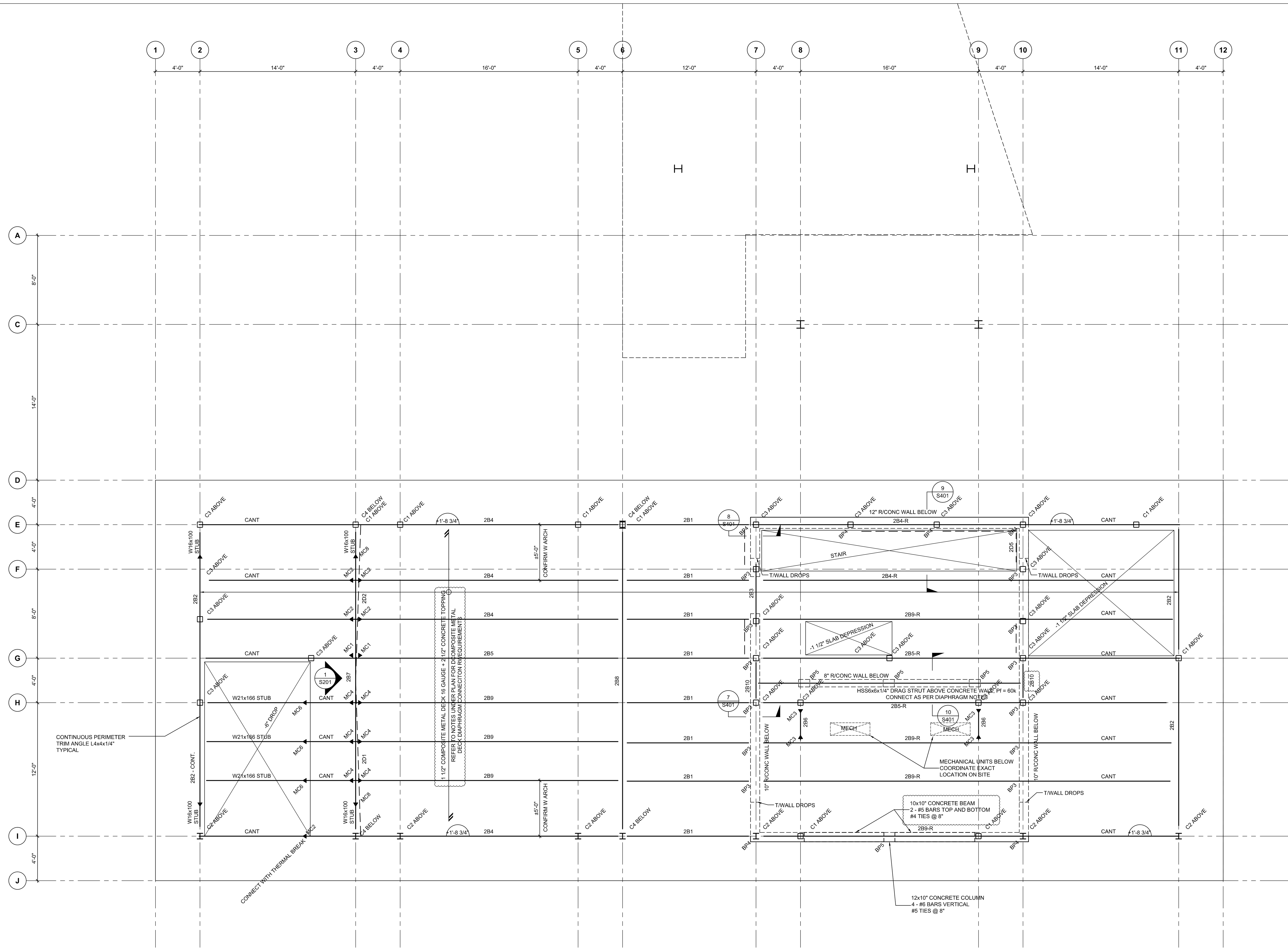
SHOP DRAWINGS:
Submit shop drawings to the Architect and Engineer for approval prior to manufacture of prefabricated elements of the building.

Lower Floor Framing Plan

scale: 1/4" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

S101

PLOT DATE: July 27, 2018
FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg



1 / S102 MAIN FLOOR FRAMING PLAN
1/4"=1'-0"

MAIN FLOOR FRAMING PLAN NOTES

1. THE MAIN FLOOR DATUM IS AT +0'-0"
2. TOP OF STEEL FRAMING IS 4" BELOW THE DATUM UNLESS NOTED OTHERWISE IN PLAN.
3. WHERE NOTED IN PLAN TOP OF STEEL IS GIVEN RELATIVE TO THE MAIN FLOOR DATUM
4. LOADS USED IN THE DESIGN:

LIVE LOAD:	40psf
DEAD LOADS:	
PARTITIONS	10psf
FLOOR FINISH	5psf
4" COMPOSITE SLAB	40psf
STEEL BEAMS	8psf
SUSPENDED	12psf
TOTAL	85psf

5. SEE ROOF PLAN FOR COLUMN SCHEDULE

COMPOSITE METAL DECK DIAPHRAGM CONNECTION REQUIREMENTS

- FASTEN METAL DECK DOWN TO ALL INTERIOR SUPPORTS WITH HILTI X-ENP-19 POWDER ACTUATED FASTENERS AT 6" OC
- FASTEN SIDE LAPS WITH #10 SCREWS AT 4" OC
- FASTEN AT ALL PERIMETER SUPPORTS WITH HILTI X-ENP-19 POWDER ACTUATED FASTENERS AT 6" OC

BEAM SCHEDULE

BEAM MARK	BEAM SECTION	REACTIONS				CAMBER	REMARKS
		LEFT END		RIGHT END			
		LIVE	DEAD	LIVE	DEAD		
2B-1	W12x26	1.0	2.2	1.0	2.2		
2B-2	W27x129	CONNECT FOR Vf = 50kips AT EA. FLOOR BEAM					
2B-3	W27x84	64.8	22.4	14.6	4.7		
2B-4	W27x129	64.8	22.4	14.6	4.7		MC2 = 650 ft-kips
2B-5	W27x129	82.8	24.9	±24.6	1.9		MC1 = 1060 ft-kips
2B-6	W12x26	1.0	1.0	1.0	1.0		AXIAL Peq = 200k MC3 = 25 ft-kips
2B-7	W27x146	41.4	43.3	41.4	43.3	CAMBER FOR DEAD LOAD	PROVIDE 3/4" SHEAR STUD CONNECTORS INTO COMP. DECK @ 16" OC
2B-8	W16x67	10.3	22.8	10.3	22.8		
2B-9	W27x129	64.8	22.4	14.6	4.7		CAMBER FOR DEAD LOAD MC4 = 145 ft-kips
2B-10	HSS6x6x1/4						SUPPORT FOR DRAG STRUT Vf = Hf = 15k
2D1	1 1/2"Ø 460 MPA TENSION ROD						OCBF, SEE ELEVATIONS S201-S204
2D2	1 1/2"Ø 460 MPA TENSION ROD						OCBF, SEE ELEVATIONS S201-S204

STEEL BEAM SCHEDULE NOTES

1. LEFT AND RIGHT ENDS OF BEAMS ARE DEFINED BY THE ORIENTATION OF THE BEAM MARK ON PLAN.
2. WHERE A BEAM MARK IS INDICATED WITH THE SUFFIX "R" ON PLAN THE REACTIONS ARE TO APPLY AT THE OPPOSITE ENDS.
3. REACTIONS GIVEN ARE SERVICE LOADS IN kips.
4. BEARING PLATE DIMENSION GIVEN FIRST IS PARALLEL TO THE BEAM WEB.
5. CENTRE BEARING PLATES UNDER BEAMS UNLESS NOTED OTHERWISE. PROVIDE 2-3/4" DIA. ANCHOR BOLTS, 16" LG. WITH 3" HOOK AND 4" THREADS, FOR BEARING PLATES BEARING ON CONCRETE OR MASONRY, UNLESS NOTED OTHERWISE. ALTERNATIVELY ANCHOR RODS MAY BE WELDED TO UNDERSIDE OF BEARING PLATE. GROUT MASONRY AS INDICATED IN THE GENERAL NOTES, UNLESS NOTED.
6. DESIGN CONNECTIONS FOR AXIAL FORCE (Pf), END MOMENT (Mf), TORSION (Tf) OR OUT OF PLANE HORIZONTAL FORCE (Hf) SHOWN IN THE REMARKS COLUMN, IN ADDITION TO THE VERTICAL SHEAR PROVIDED IN THE REACTION COLUMN.
7. CAMBERS ARE IN INCHES.

PLAN REVIEW ACCEPTANCE
FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW.
[X] BUILDING [X] STRUCTURAL
[X] MECHANICAL [X] PLUMBING
[X] ELECTRICAL [X] ENERGY
[ ] ACCESSIBILITY [ ] FIRE
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BY: MEM DATE: 08/22/18
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STATE OF UTAH

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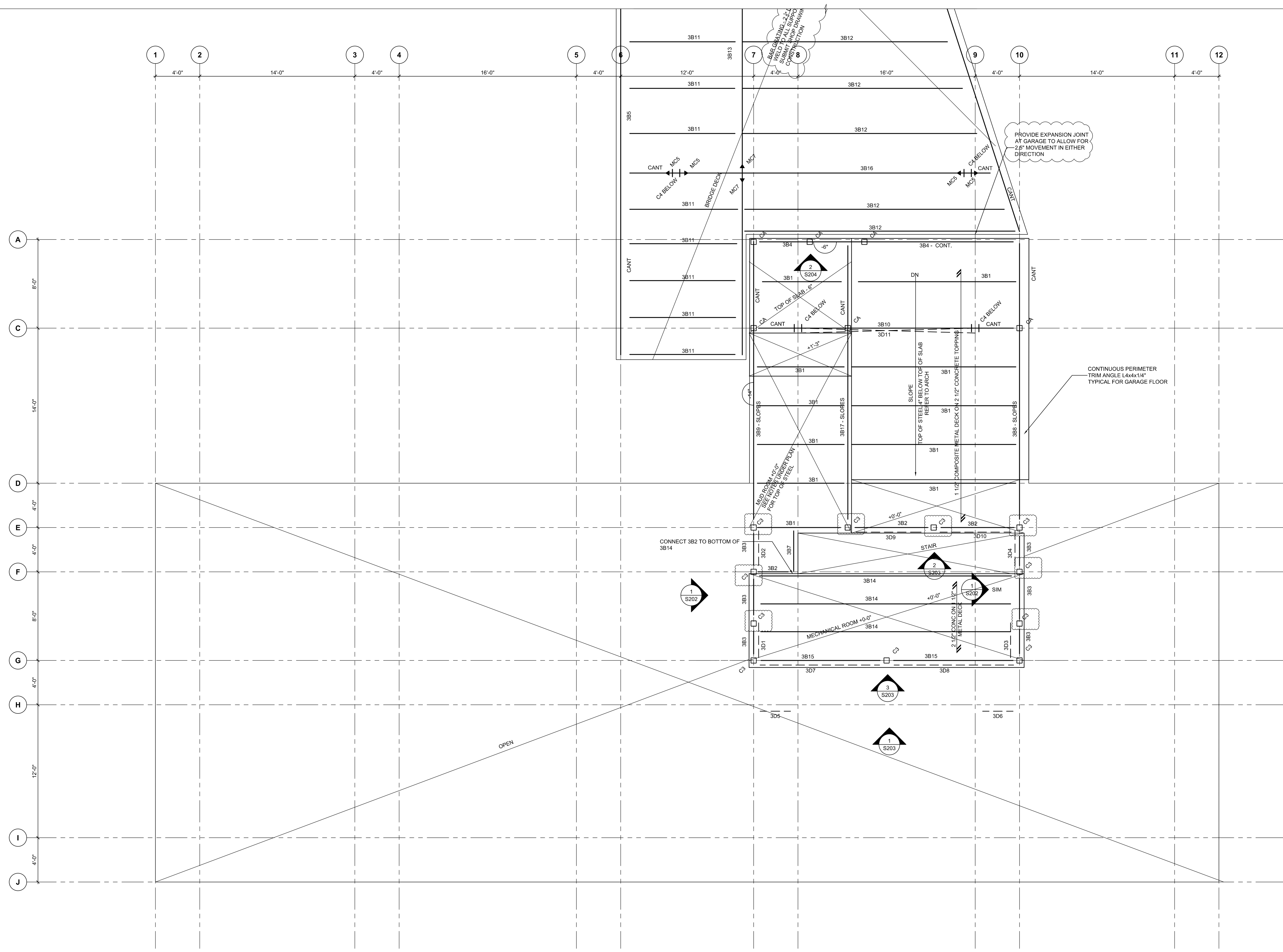
Main Floor Framing Plan

scale: 1/4" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

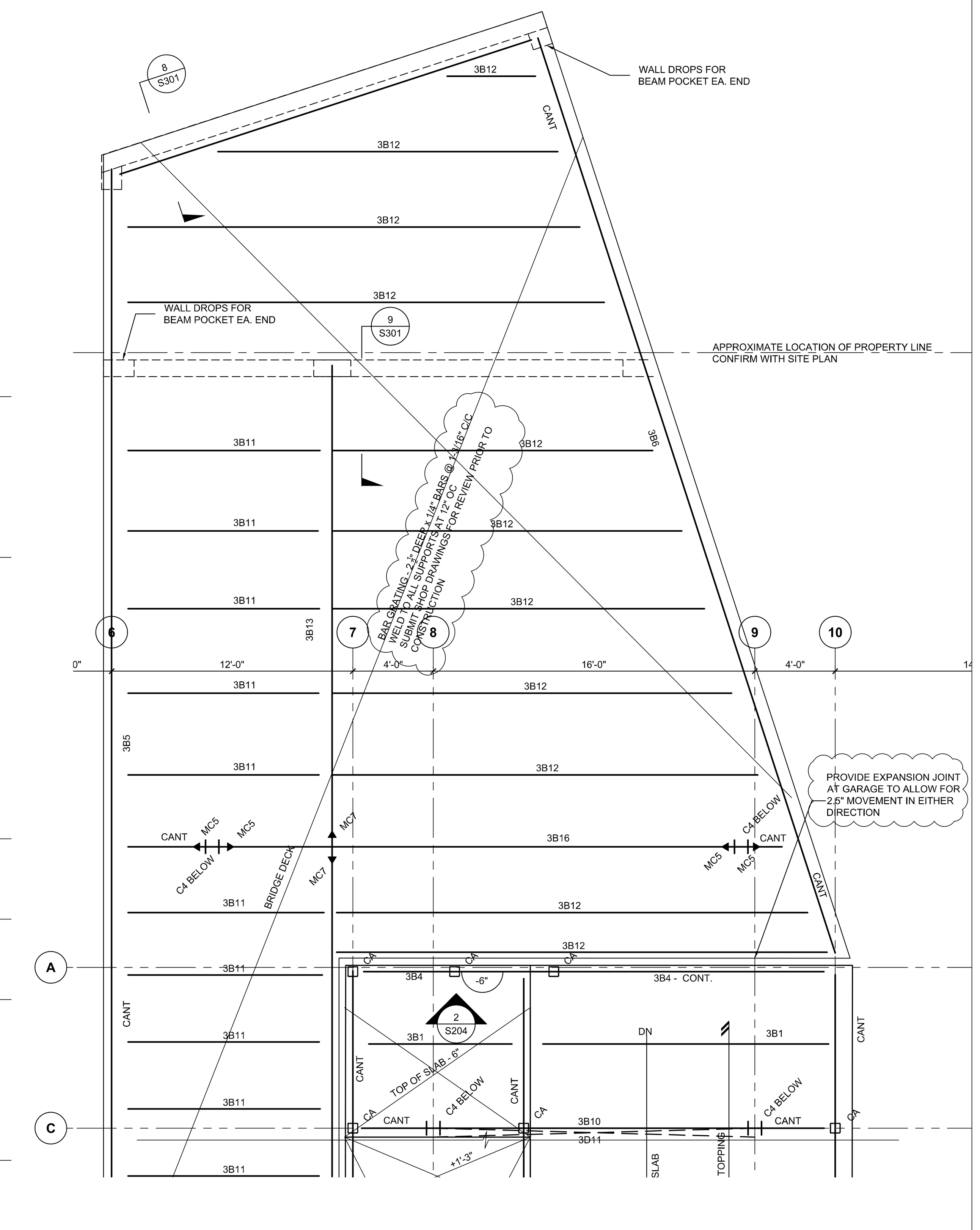
S102



1/S103 GARAGE FLOOR FRAMING PLAN
1/4"=1'-0"

GARAGE FLOOR FRAMING PLAN NOTES

1. THE GARAGE FLOOR DATUM IS AT + 9'-0" ABOVE THE 2ND FLOOR DATUM
2. TOP OF STEEL FRAMING IS 4" BELOW THE DATUM UNLESS NOTED OTHERWISE IN PLAN.
3. LOADS USED IN THE DESIGN:
   - LIVE LOAD: 40psf
   - DEAD LOADS:
     - PARTITIONS 10psf
     - FLOOR FINISH 5psf
     - 4" COMPOSITE SLAB 40psf
     - STEEL BEAMS 8psf
     - SUSPENDED 12psf
     - TOTAL 85psf
4. SEE ROOF PLAN FOR COLUMN SCHEDULE



2/S103 PART PLAN - BRIDGE FRAMING
1/4" = 1'-0"

Lot 14 Mountain House

Summit Powder Mountain
Eden, Utah

MacKay-Lyons Sweetapple **Architects Limited**

2188 Gottingen St.
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Canada B3K 3B4

ph: (902) 429.1867
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PROFESSIONAL STRUCTURAL ENGINEER
No. 6985627-2203
MICHAEL F SULLAWAY
STATE OF UTAH

No.	Description	Date
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FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW.
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[X] MECHANICAL [X] PLUMBING
[X] ELECTRICAL [X] ENERGY
[ ] ACCESSIBILITY [ ] FIRE
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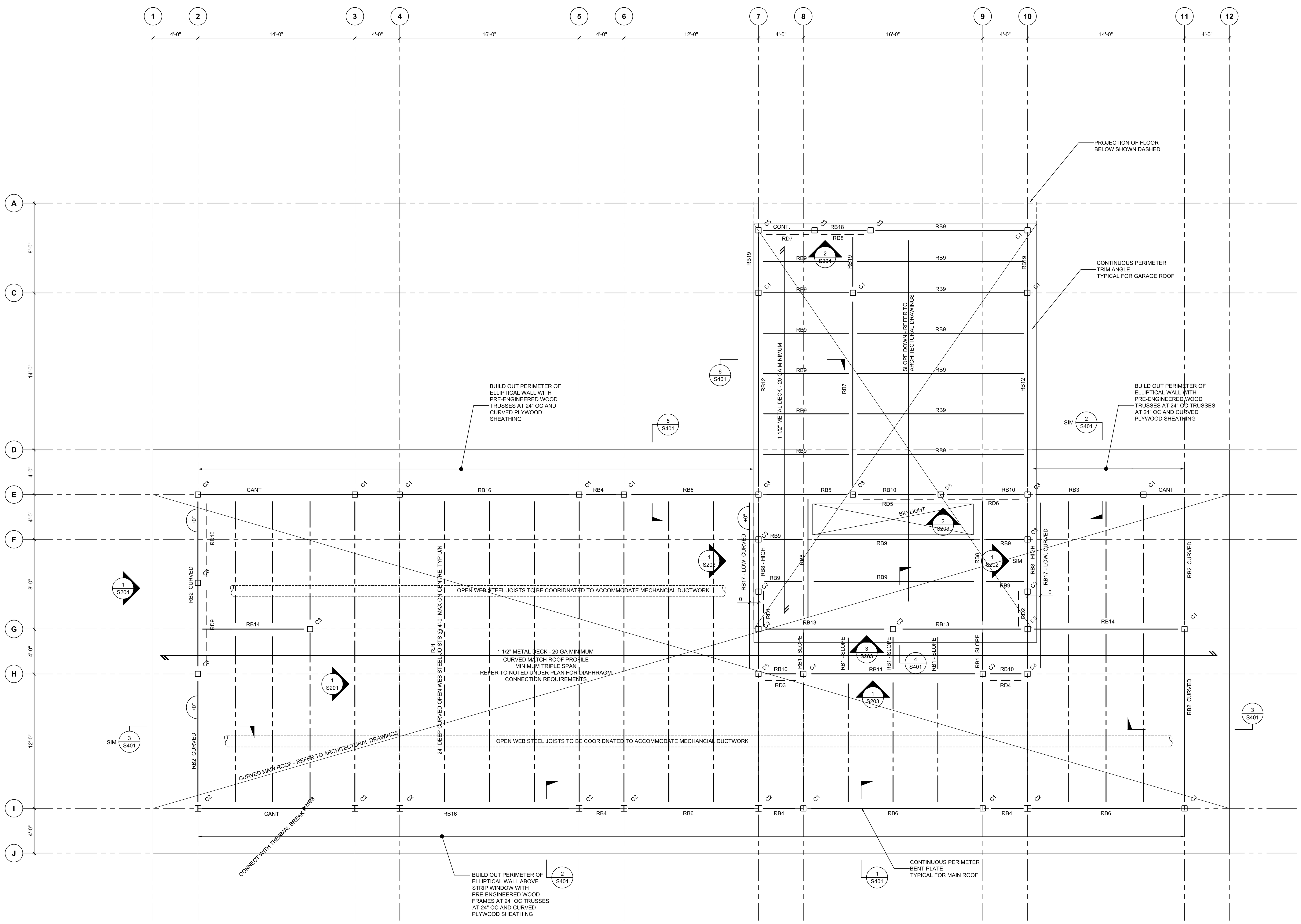
BY: MEM DATE: 08/22/18
WEST COAST CODE CONSULTANTS, INC.

Garage Floor Framing Plan

scale: 1/4" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

S103

PLOT DATE: July 27, 2018
FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_S-TB-E.dwg



1 S104 ROOF FRAMING PLAN
1/4"=1'-0"

ROOF FRAMING NOTES

1. ROOF IS CURVED - REFER TO ARCHITECTURAL DRAWINGS FOR UNDERSIDE OF DECK ELEVATIONS
2. TOP OF STEEL BEAMS ARE 4" BELOW THE UNDERSIDE OF THE METAL DECK UNLESS NOTED OTHERWISE IN PLAN.
3. WHERE NOTED IN PLAN THE TOP OF STEEL ELEVATIONS ARE GIVEN RELATIVE TO UNDERSIDE OF DECK
4. TOP OF STEEL AT UNDERSIDE OF DECK FOR GARAGE ROOF BEAMS
5. SUPERIMPOSED LOADS USED IN THE DESIGN:
   SNOW + RAIN - 192psf PLUS SNOW ACCUMULATION SHOWN ON PLAN
   DEAD:
   ROOFING AND INSULATION 10psf
   SUSPENDED 5psf
   MECHANICAL EQUIPMENT 5psf
6. SELF WEIGHT OF STRUCTURE USED IN THE DESIGN:
   METAL DECK 2psf
   STRUCTURAL STEEL 8psf (INLUDES OPEN WEB STEEL JOISTS)

METAL DECK DIAPHRAGM CONNECTION REQUIREMENTS

- FASTEN METAL DECK DOWN TO ALL SUPPORTS WITH HILTI X-ENP19 POWDER ACTUATED FASTENERS AT 6" OC
- FASTEN SIDE LAPS WITH #10 SCREWS @ 12" c/c
- FASTEN AT ALL PERIMETER SUPPORTS WITH HILTI X-ENP19 POWDER ACTUATED FASTENERS AT 6" OC

COLUMN SCHEDULE

MEMBER MARK	MEMBER DESCRIPTION	REMARKS
C1	HSS6x6x1/4	MAX LOAD, Pf = 75 kips
C2	W6x25	MAX LOAD, Pf = 75 kips
C3	HSS6x6x5/8	MAX LOAD, Pf = 75kips SCBF, ALL WELDS AT BASEPLATE TO BE DEMAND CRITICAL WELDS
C4	W10x49	MAX LOAD, Pf = 330 kips

STEEL COLUMN SCHEDULE NOTES

1. CENTRE COLUMNS, CAPS AND FOOTINGS ON GRIDS UNLESS NOTED OTHERWISE.
2. COLUMN LOADS INDICATED ARE FACTORED COMPRESSION.
3. REFER TO STANDARD DETAIL 0303 TYPICAL FOOTING AND COLUMN BASE DETAILS UNLESS NOTED OTHERWISE.
4. PROVIDE 4-3/4" DIAM. ANCHOR BOLTS AS PER STANDARD DETAIL 0303 UNLESS NOTED OTHERWISE.
5. FOR DEMAND CRITICAL WELD REQUIREMENTS SEE GENERAL NOTES

ROOF BEAM SCHEDULE

BEAM MARK	BEAM SECTION	REACTIONS (kips)				CAMBER	REMARKS
		LEFT END		RIGHT END			
		SNOW	DEAD	SNOW	DEAD		
RB1	W10x22	2.4	0.5	2.4	0.5		AXIAL FORCE Peq, SEE ELEVATIONS CURVE TO MATCH ROOF
RB2	W16x31	10.0	1.5	10.0	1.5		CURVE TO MATCH ROOF
RB3	W16x31	4.2	0.5	19.8	3.7		
RB4	W16x31	0.5	0.5	0.5	0.5		
RB5	W10x22	22.2	3.1	22.2	3.1		
RB6	W16x26	12.6	1.9	12.6	1.9		
RB7	W10x54	21.1	3.2	21.1	3.2		
RB8	W10x22	11.9	1.7	11.9	1.7		
RB9	C10x15.3	6.1	1.0	6.1	1.0		
RB10	W16x31	5.3	0.8	5.3	0.8		AXIAL FORCE Peq, SEE ELEVATIONS
RB11	W16x31	19.1	2.7	23.8	3.4		
RB12	W10x39	12.7	2.0	12.7	2.0		
RB13	W16x31	15.2	2.2	15.2	2.2		AXIAL FORCE Peq, SEE ELEVATIONS
RB14	W16x26	14.0	2.0	14.0	2.0		
RB15	W16x26	11.0	1.7	11.0	1.7		
RB16	W24x117	14.9 91.0	±4.6	13.5 2.9	18.4		MC8 = 255 ft-kips
RB17	W12x35	0.5	0.5	0.5	0.5		
RB18	W16x31	6.1	1.0	6.1	1.0		AXIAL FORCE Peq, SEE ELEVATIONS CONTINUOUS OVER COLUMN
RB19	W10x22	6.8	1.0	6.8	1.0		
RD1	HSS 2x2x1/4"						SCBF, SEE ELEVATIONS S201-S204
RD2	HSS 2x2x1/4"						SCBF, SEE ELEVATIONS S201-S204
RD3	HSS 2x2x1/4"						SCBF, SEE ELEVATIONS S201-S204
RD4	HSS 2x2x1/4"						SCBF, SEE ELEVATIONS S201-S204
RD5	HSS 3x3x1/4"						SCBF, SEE ELEVATIONS S201-S204
RD6	HSS 3x3x1/4"						SCBF, SEE ELEVATIONS S201-S204
RD7	HSS 2 1/2x2 1/2x3/16"						SCBF, SEE ELEVATIONS S201-S204
RD8	HSS 2 1/2x2 1/2x3/16"						SCBF, SEE ELEVATIONS S201-S204
RD9	HSS 3 1/2x3 1/2x1/4"						SCBF, SEE ELEVATIONS S201-S204
RD10	HSS 3 1/2x3 1/2x1/4"						SCBF, SEE ELEVATIONS S201-S204

STEEL BEAM SCHEDULE NOTES

1. LEFT AND RIGHT ENDS OF BEAMS ARE DEFINED BY THE ORIENTATION OF THE BEAM MARK ON PLAN.
2. WHERE A BEAM MARK IS INDICATED WITH THE SUFFIX "R" ON PLAN THE REACTIONS ARE TO APPLY AT THE OPPOSITE ENDS.
3. REACTIONS GIVEN ARE SERVICE LOADS IN kips.
4. BEARING PLATE DIMENSION GIVEN FIRST IS PARALLEL TO THE BEAM WEB.
5. CENTRE BEARING PLATES UNDER BEAMS UNLESS NOTED OTHERWISE. PROVIDE 2 - 3/4" DIA. ANCHOR BOLTS, 16" LG. WITH 3" HOOK AND 4" THREADS, FOR BEARING PLATES BEARING ON CONCRETE OR MASONRY, UNLESS NOTED OTHERWISE. ALTERNATIVELY ANCHOR RODS MAY BE WELDED TO UNDERSIDE OF BEARING PLATE. GROUT MASONRY AS INDICATED IN THE GENERAL NOTES, UNLESS NOTED.
6. DESIGN CONNECTIONS FOR AXIAL FORCE (Pf), END MOMENT (Mf), TORSION (Tf) OR OUT OF PLANE HORIZONTAL FORCE (Hf) SHOWN IN THE REMARKS COLUMN, IN ADDITION TO THE VERTICAL SHEAR PROVIDED IN THE REACTION COLUMN.
7. CAMBERS ARE IN INCHES.

PLAN REVIEW ACCEPTANCE
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☒ BUILDING ☒ STRUCTURAL
☒ MECHANICAL ☒ PLUMBING
☒ ELECTRICAL ☒ ENERGY
☐ ACCESSIBILITY ☐ FIRE
PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS.
BY: MEM DATE: 08/22/18
WEST COAST CODE CONSULTANTS, INC.

Lot 14 Mountain House
Summit Powder Mountain
Eden, Utah

MacKay-Lyons Sweetapple
Architects Limited
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Halifax, Nova Scotia
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fax: (902) 429.6276

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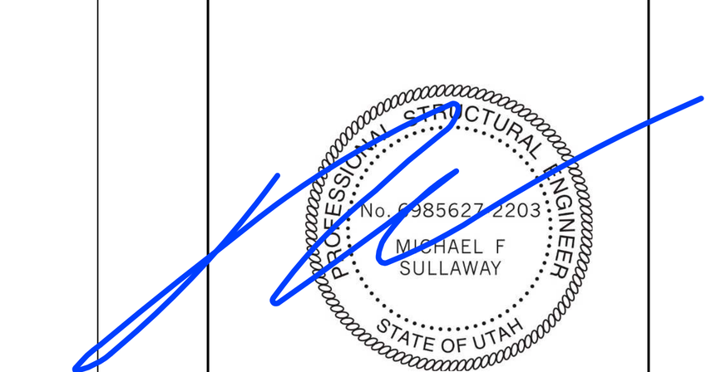

No.	Description	Date
05	Issued for Building Permit	2018.08.15
04	Issued for SSI No 01	2018.07.27
03	Issued for Building Permit	2018.07.13
02	Issued for Review	2018.06.13
01	Issued for Costing	2018.05.30

Revision:

NOTES:

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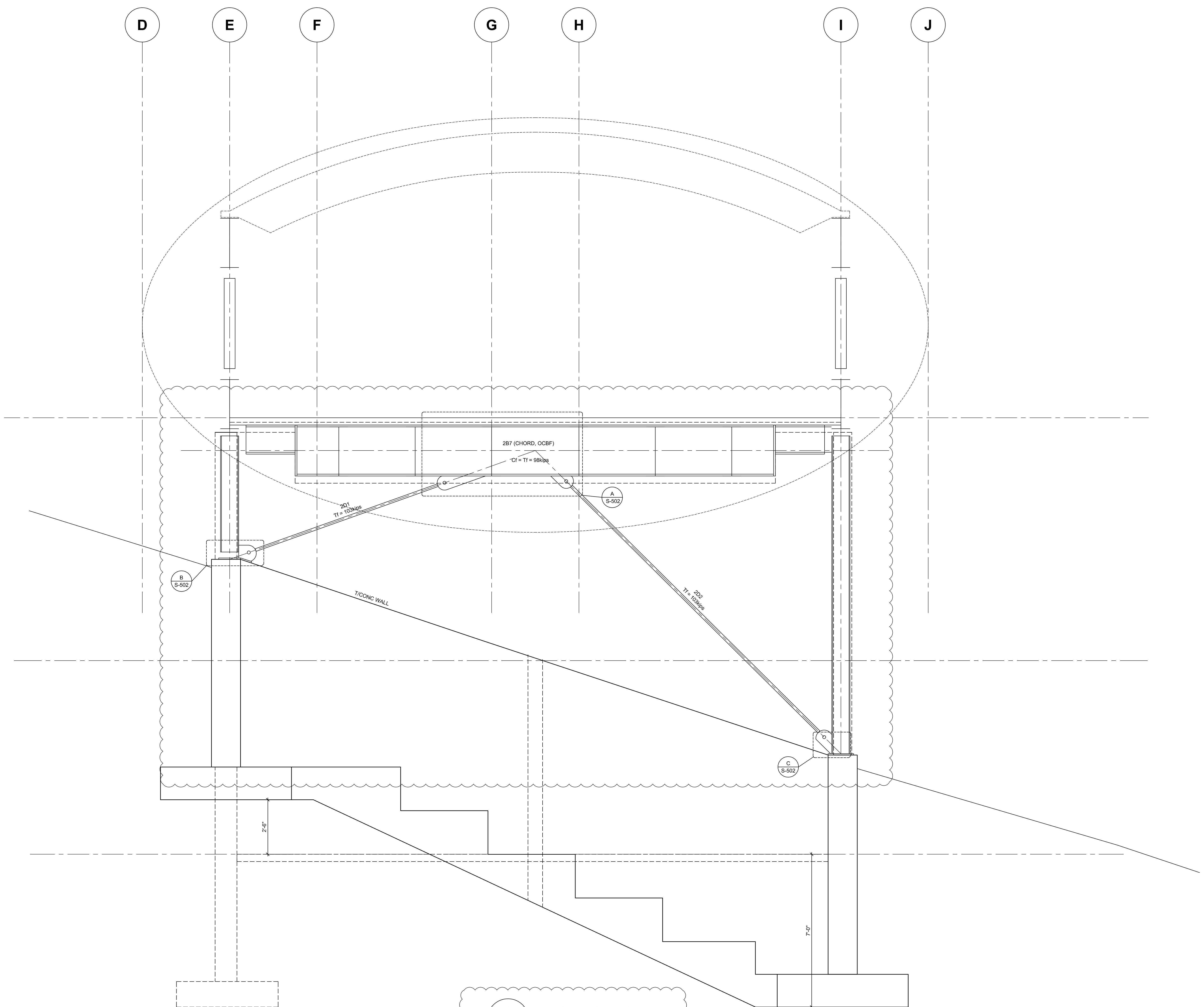
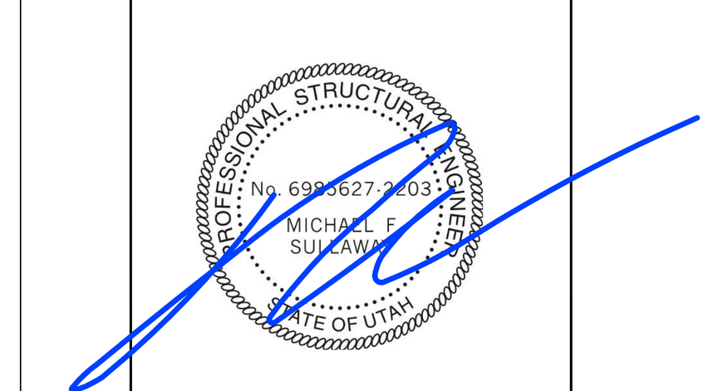
Roof Framing Plan

scale: 1/4" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

S104



1
S201
BRACING GRID 3
1/2"=1'-0"



PLAN REVIEW ACCEPTANCE

FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW.

- [X] BUILDING
- [X] STRUCTURAL
- [X] MECHANICAL
- [X] PLUMBING
- [X] ELECTRICAL
- [X] ENERGY
- [ ] ACCESSIBILITY
- [ ] FIRE

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BY: MEM DATE: 08/22/18

WEST COAST CODE CONSULTANTS, INC.

PLOT DATE: July 27, 2018

FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg

Lot 14 Mountain House

Summit Powder Mountain
Eden, Utah

MacKay-Lyons
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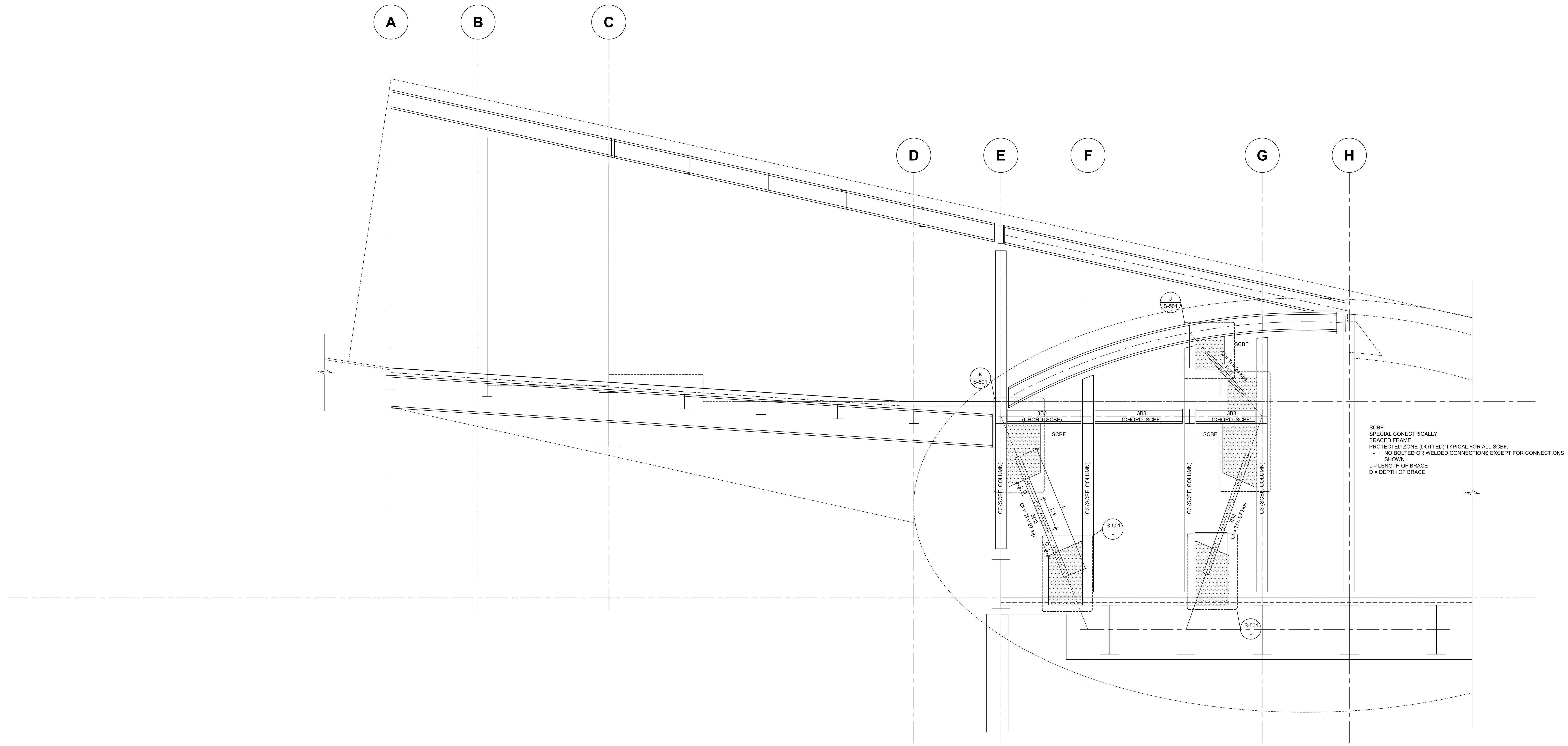
Framing
Elevations

scale: 1/2" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

S201



1
S202
BRACING GRID 7
1/2"=1'-0"

PLOT DATE: July 27, 2018

FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg

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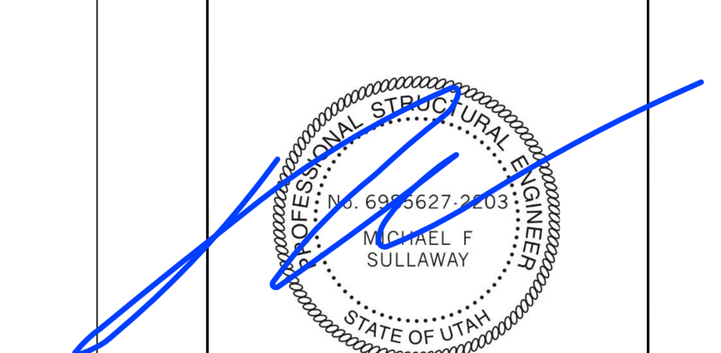

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- [X] STRUCTURAL
- [X] MECHANICAL
- [X] PLUMBING
- [X] ELECTRICAL
- [X] ENERGY
- [ ] ACCESSIBILITY
- [ ] FIRE

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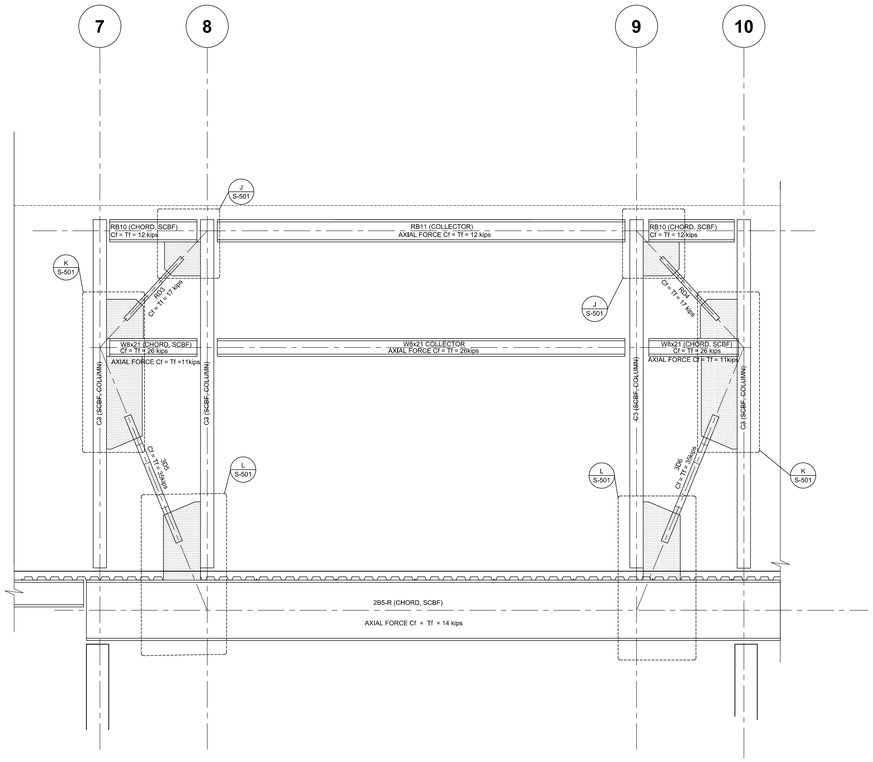
BY: MEM DATE: 08/22/18
WEST COAST CODE CONSULTANTS, INC.

Framing Elevations

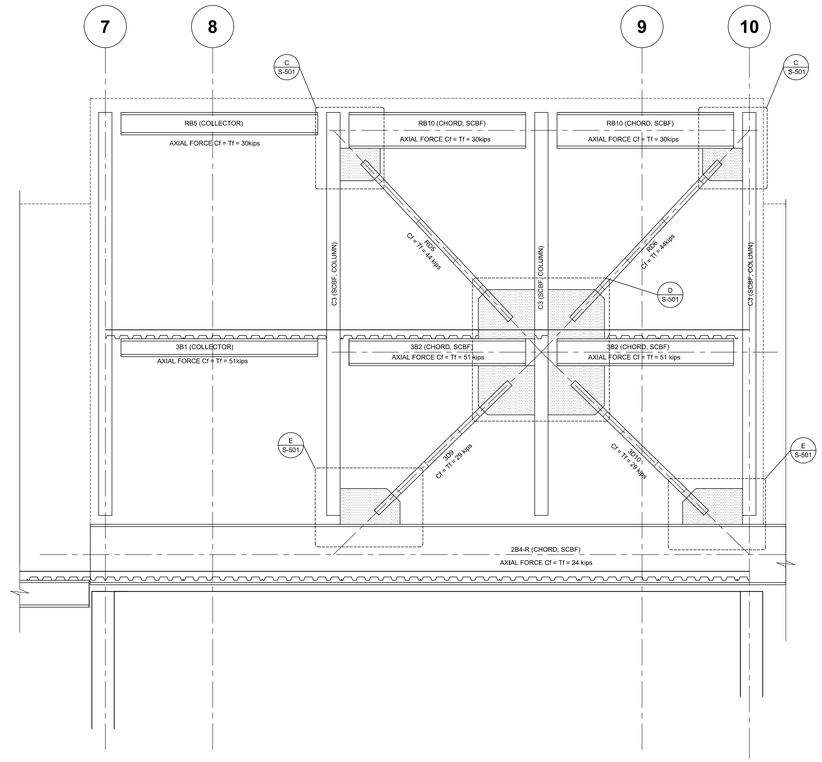
scale: 1/2" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

S202



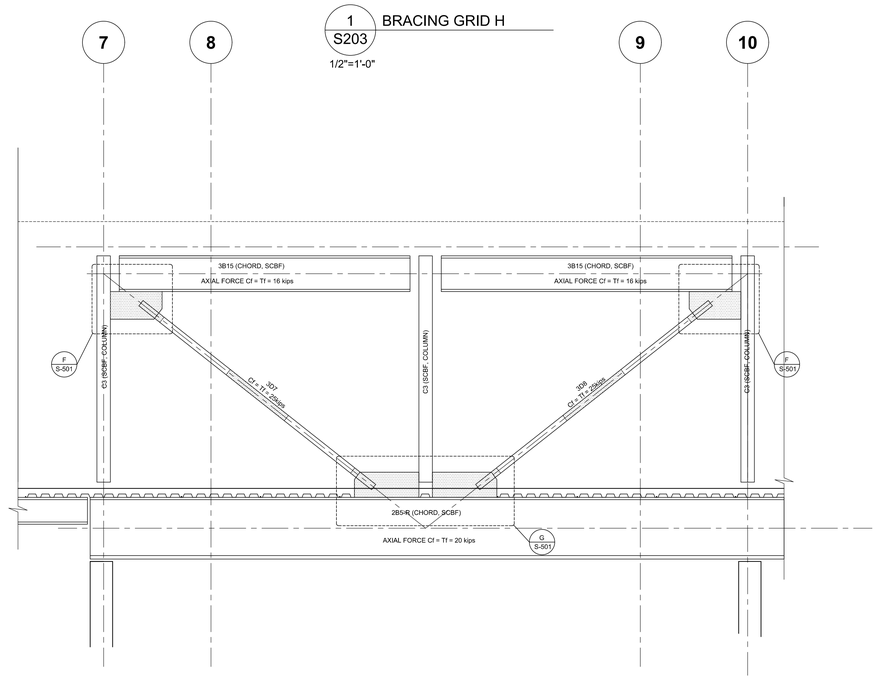
1 S203 BRACING GRID H

1/2"=1'-0"



2 S203 BRACING GRID E

1/2"=1'-0"



3 S203 BRACING GRID G

1/2"=1'-0"

PLOT DATE: July 27, 2018

FILE: J:\Job-2017 - 170000 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg

Lot 14 Mountain House

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fax: (902) 429.6276

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PROFESSIONAL STRUCTURAL ENGINEER
MICHAEL F SULLAWAY
STATE OF UTAH

No.	Description	Date
05	Issued for Building Permit	2018.08.15
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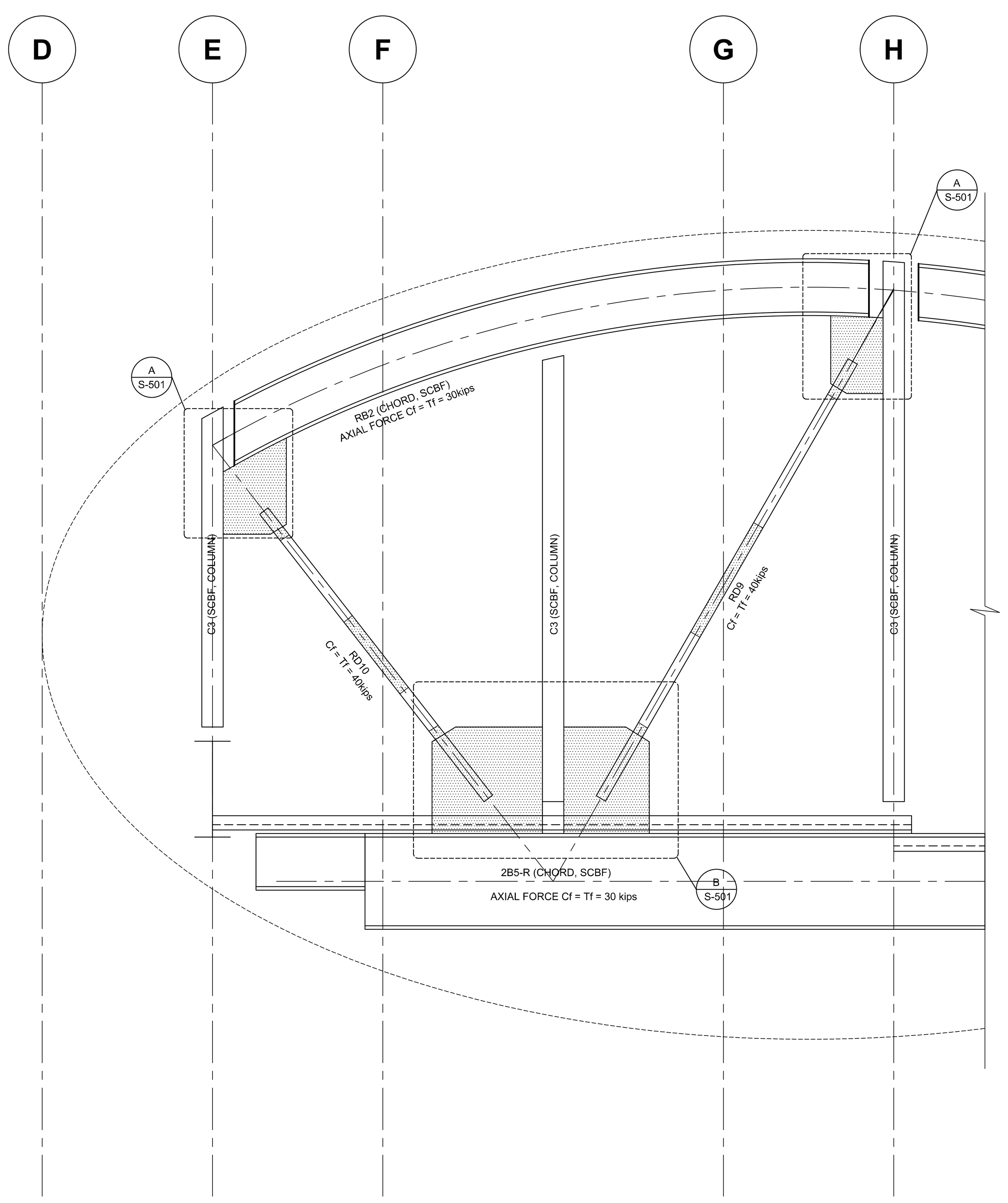
BY: MEM DATE: 08/22/18

WEST COAST CODE CONSULTANTS, INC.

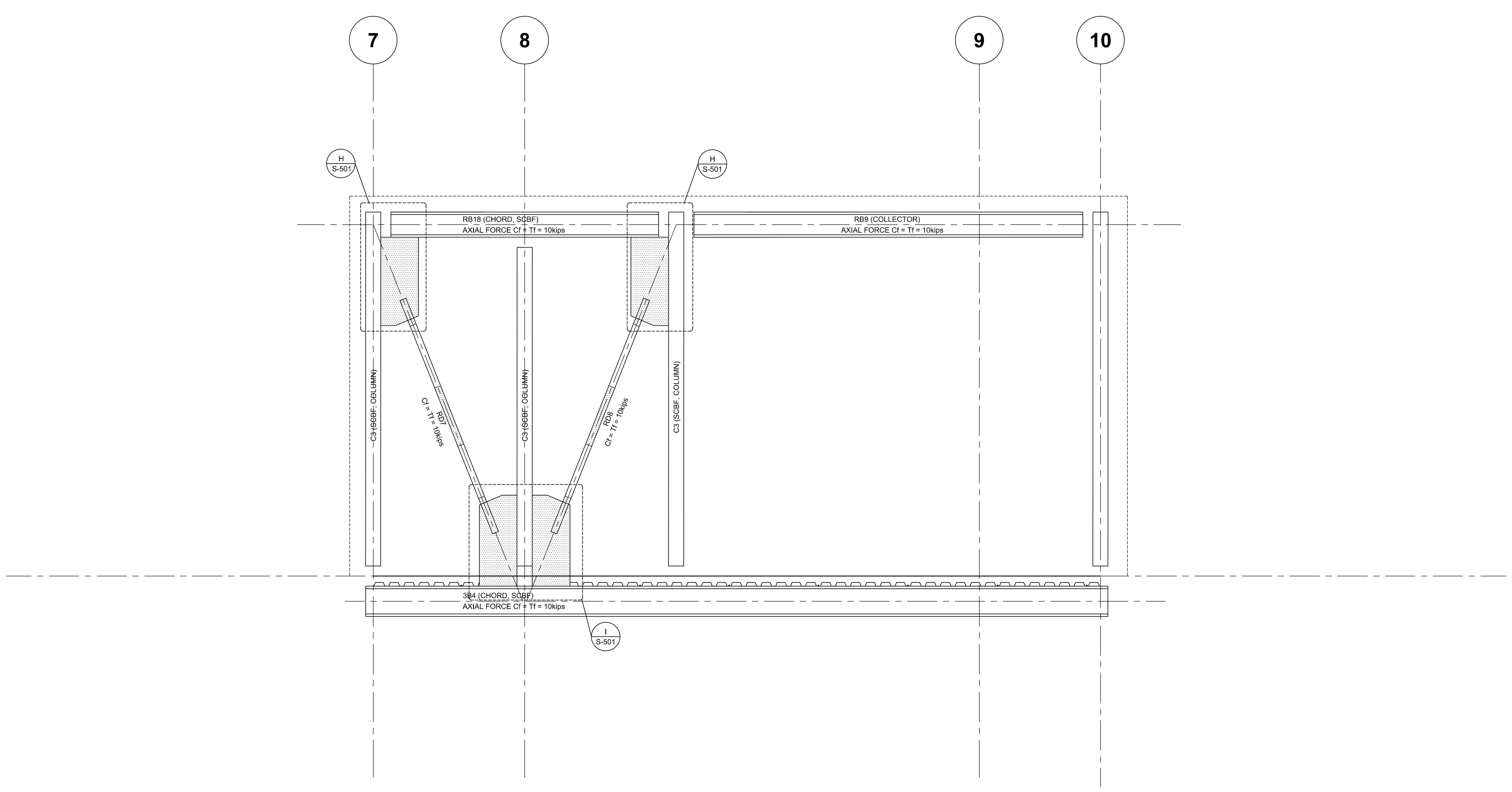
Framing Elevations

scale: 1/2" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

S203



1
S204
BRACING GRID 2
1/2"=1'-0"



2
S204
BRACING GRID B
1/2"=1'-0"

PLAN REVIEW ACCEPTANCE

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- [X] BUILDING
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PLOT DATE: July 27, 2018

FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg

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PROFESSIONAL STRUCTURAL ENGINEER
No. 6985627-2203
MICHAEL F SULLAWAY
STATE OF UTAH

No.	Description	Date
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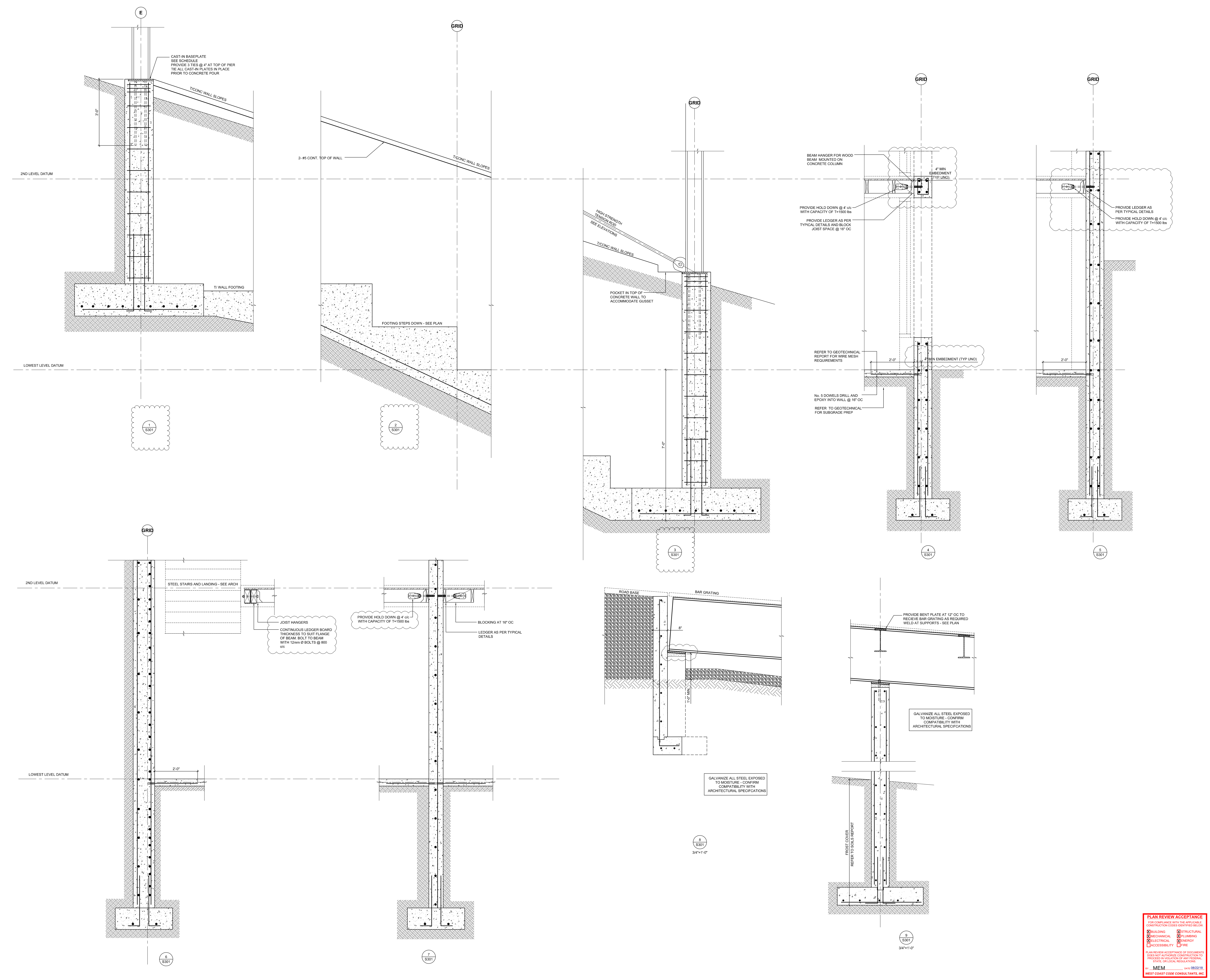
scale: 1/2" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

S204



PLOT DATE: July 27, 2018
FILE: J:\Job\2017 - 170000 to 170999\170000\170650 Summit Lot 14\R\Drawings Structural\170650_S-7B-E.dwg

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MacKay-Lyons
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MICHAEL F
SULLAWAY
STATE OF UTAH

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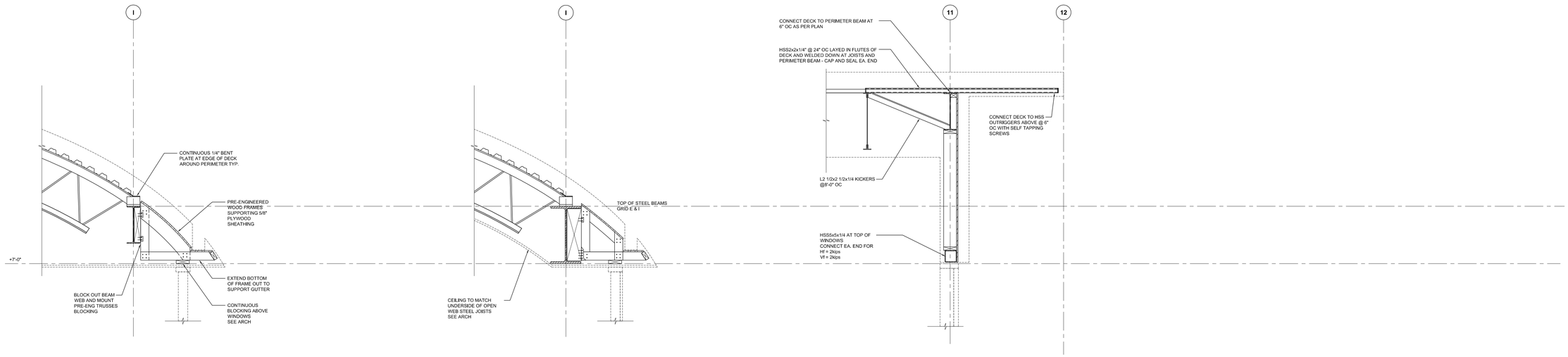
BY: MEM DATE: 08/22/18
WEST COAST CODE CONSULTANTS, INC.

Foundation Sections

scale: 3/4" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

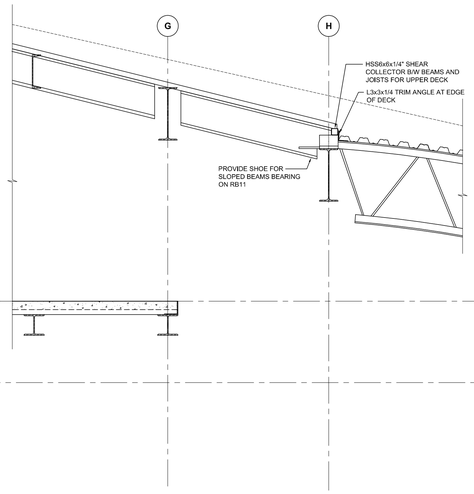
S301

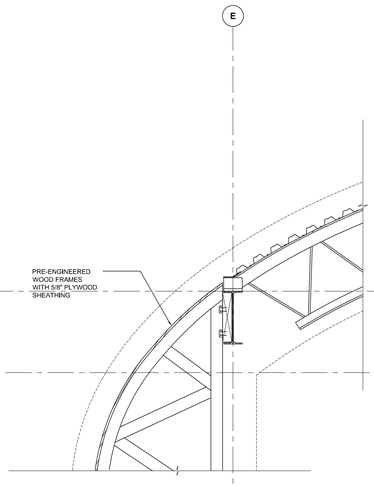


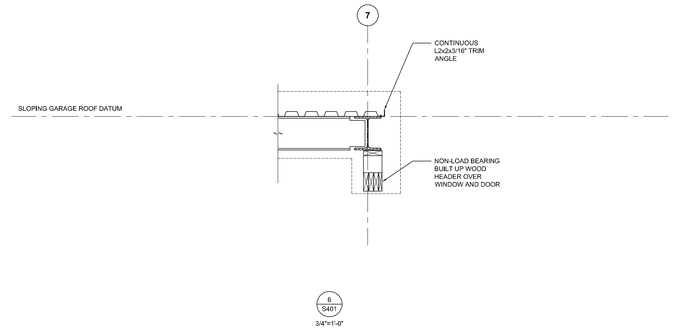
1 S401
3/4"=1'-0"

2 S401
3/4"=1'-0"

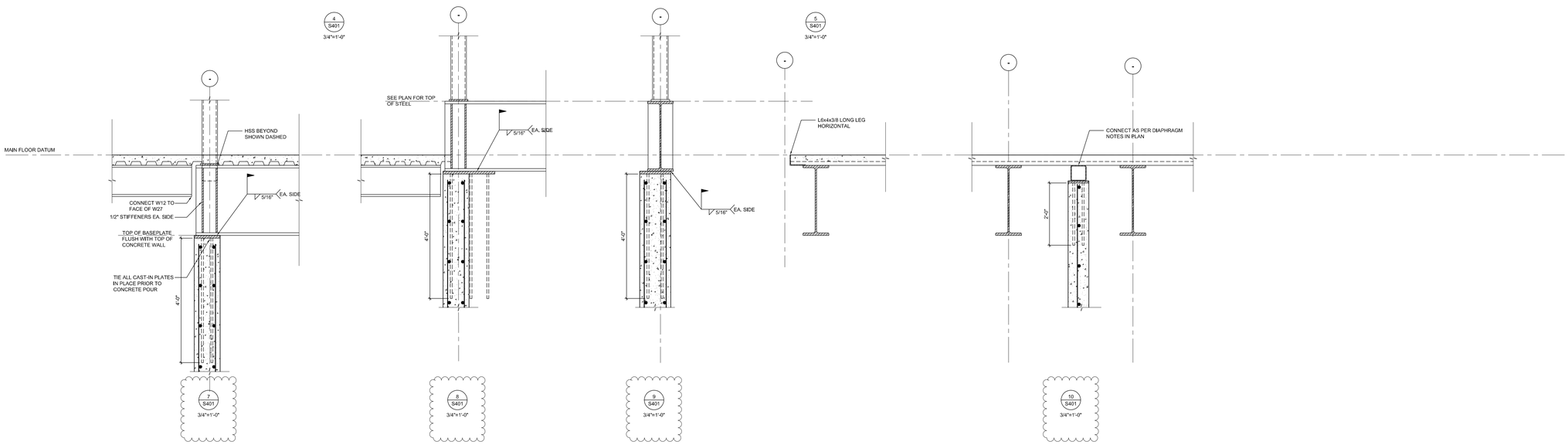
3 S401
3/4"=1'-0"







6 S401
3/4"=1'-0"



7 S401
3/4"=1'-0"

8 S401
3/4"=1'-0"

9 S401
3/4"=1'-0"

10 S401
3/4"=1'-0"

PLAN REVIEW ACCEPTANCE

FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW

- [X] BUILDING
- [X] MECHANICAL
- [X] ELECTRICAL
- [ ] ACCESSIBILITY
- [X] STRUCTURAL
- [X] PLUMBING
- [X] ENERGY
- [ ] FIRE

PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS.

BY: MEM DATE: 08/22/18

WEST COAST CODE CONSULTANTS, INC.

PLOT DATE: July 27, 2018

FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg

Lot 14 Mountain House

Summit Powder Mountain
Eden, Utah

MacKay-Lyons Sweetapple
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ph: (902) 429.1867
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Blackwell

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416.593.5300 | blackwell.ca

PROFESSIONAL STRUCTURAL ENGINEER
LICENSE NO. 6985627-2203
MICHAEL F SULLAWAY
STATE OF UTAH

No.	Description	Date
05	Issued for Building Permit	2018.08.15
04	Issued for SD No 01	2018.07.27
03	Issued for Building Permit	2018.07.13
02	Issued for Review	2018.06.13
01	Issued for Costing	2018.05.30

Revision:

NOTES:

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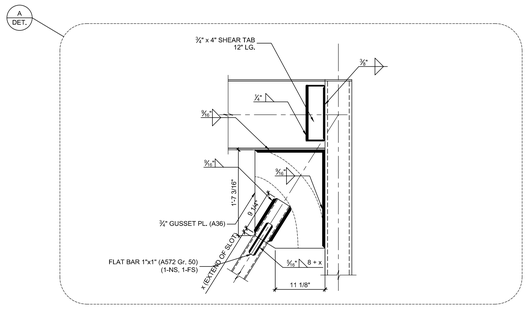
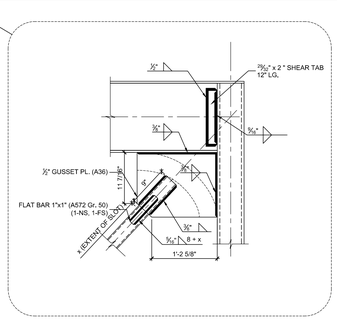
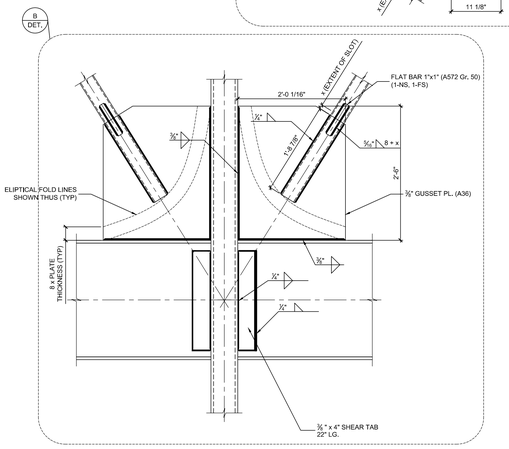
Framing Sections

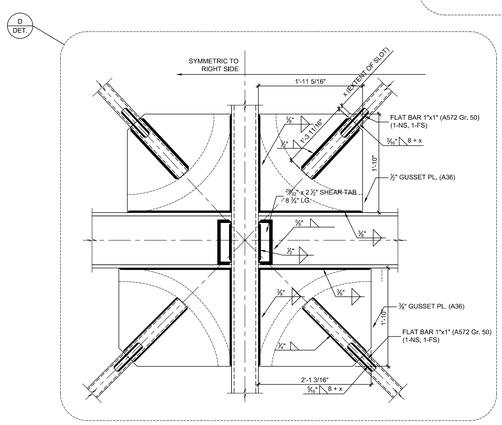
scale: 3/4" = 1'-0"
date: 2018-05-29
drawn: DP
chk'd: TJ

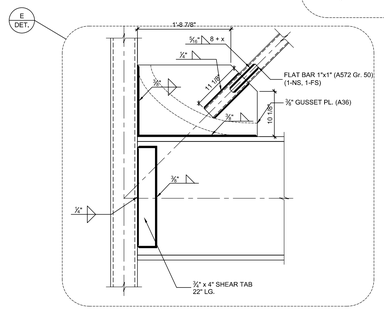
S401

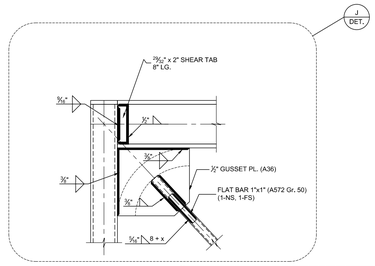


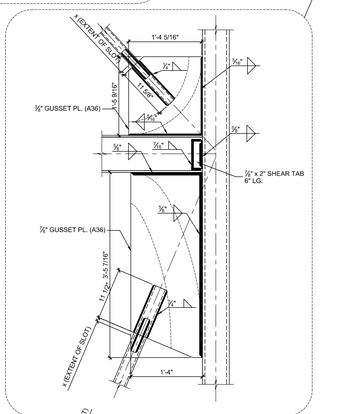


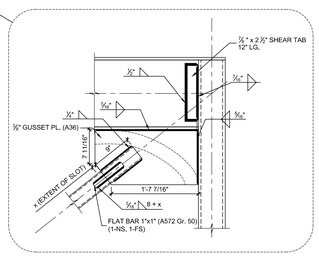


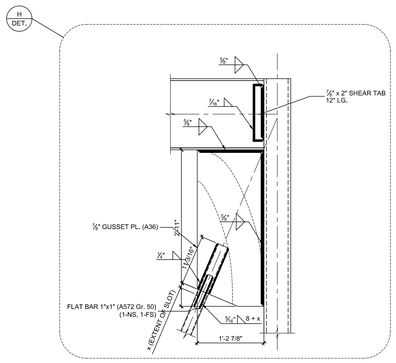


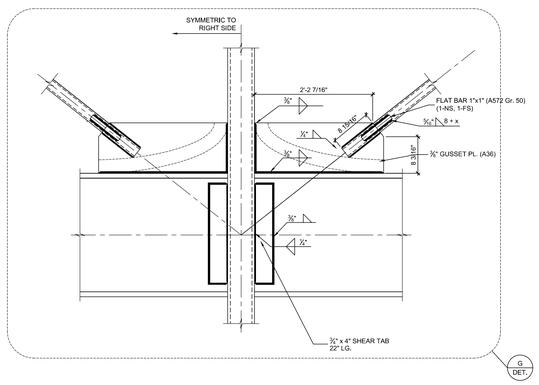


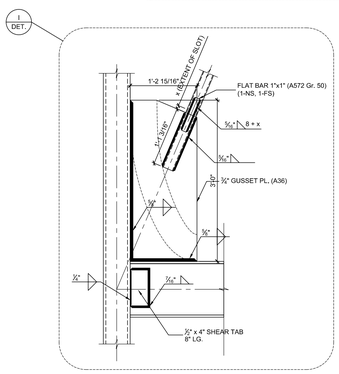


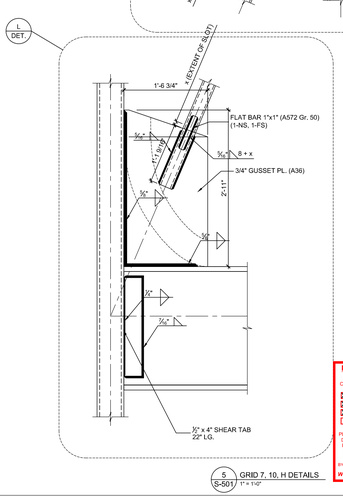












1 S-501 GRID 2 DETAILS 1" = 1'-0"

2 S-501 GRID E DETAILS 1" = 1'-0"

3 S-501 GRID G DETAILS 1" = 1'-0"

4 S-501 GRID B DETAILS 1" = 1'-0"

5 S-501 GRID 7, 10, H DETAILS 1" = 1'-0"

PLOT DATE: July 27, 2018

FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg

Lot 14 Mountain House

Summit Powder Mountain
Eden, Utah

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No.	Description	Date
	Revision:	
05	Issued for Building Permit	2018.08.15
04	Issued for SSI No 01	2018.07.27
03	Issued for Building Permit	2018.07.13
02	Issued for Review	2018.06.13
01	Issued for Costing	2018.05.30

NOTES:

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PLAN REVIEW ACCEPTANCE
FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW

☒ BUILDING	☒ STRUCTURAL
☒ MECHANICAL	☒ PLUMBING
☒ ELECTRICAL	☒ ENERGY
☐ ACCESSIBILITY	☐ FIRE

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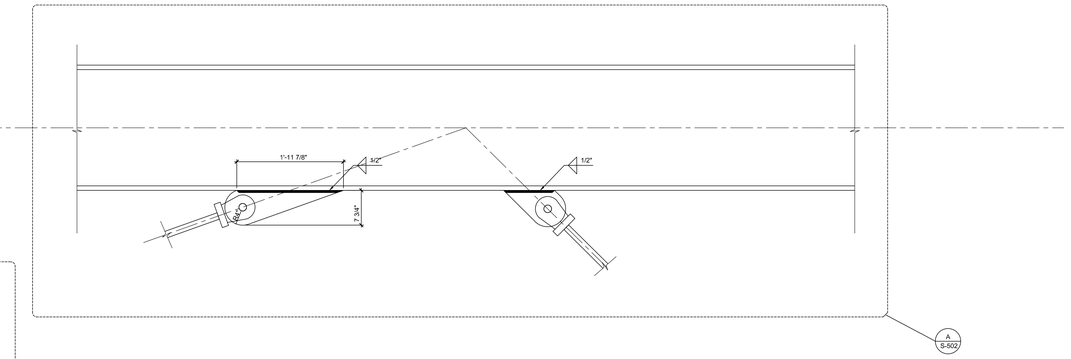
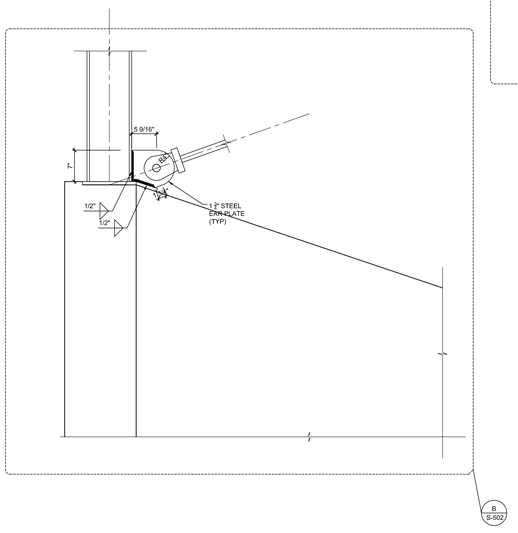
BY MEM DATE 08/22/18
WEST COAST CODE CONSULTANTS, INC.

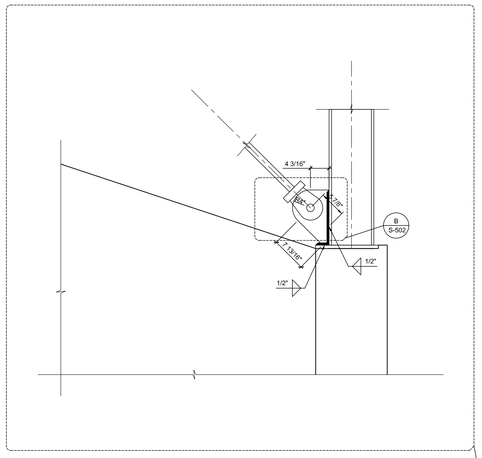
Connection Details

scale: 1" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

S501







1 S-501 GRID 3 DETAILS
1" = 1'-0"

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PLOT DATE: July 27, 2018
FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_XTB-E.dwg

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PROFESSIONAL STRUCTURAL ENGINEER
LICENSED TO PRACTICE
MICHAEL F SULLAWAY
STATE OF UTAH

No.	Description	Date
	Revision:	
05	Issued for Building Permit	2018.08.15
04	Issued for SSI No 01	2018.07.27
03	Issued for Building Permit	2018.07.13
02	Issued for Review	2018.06.13
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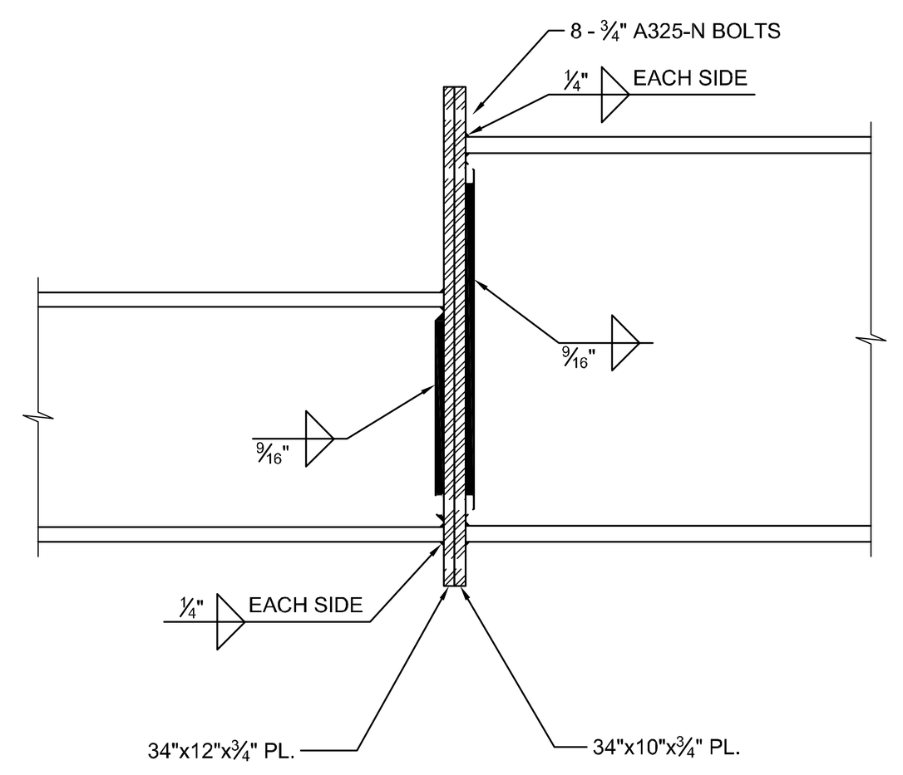
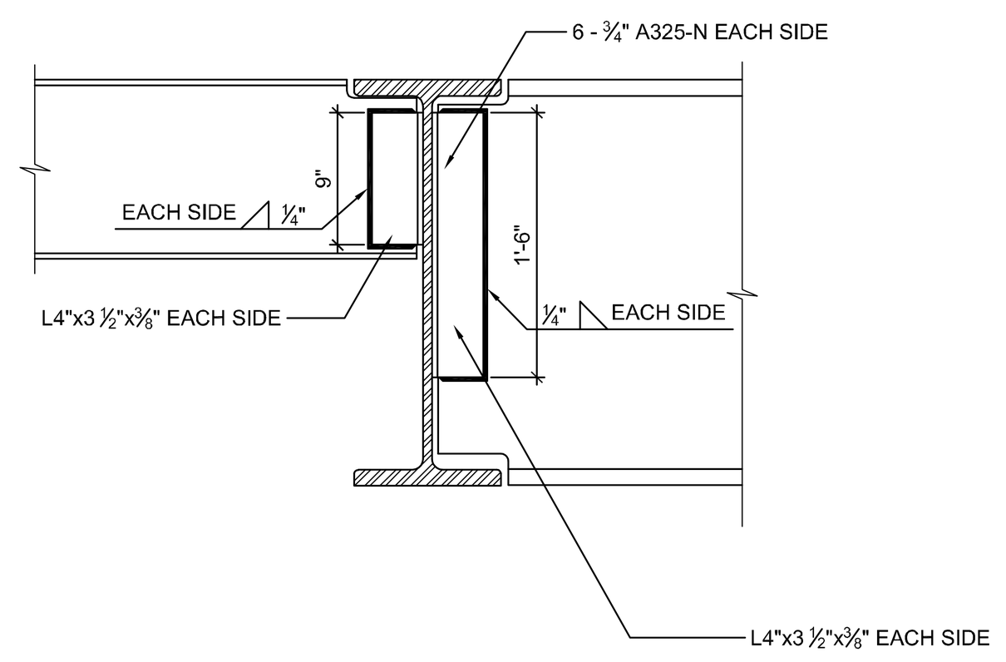
Connection Details

scale: 1" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ

S502



1 S-502 2B-2 STUB CONNECTION
1" = 1'-0"



2 S-502 2B-3 SHEAR CONNECTION
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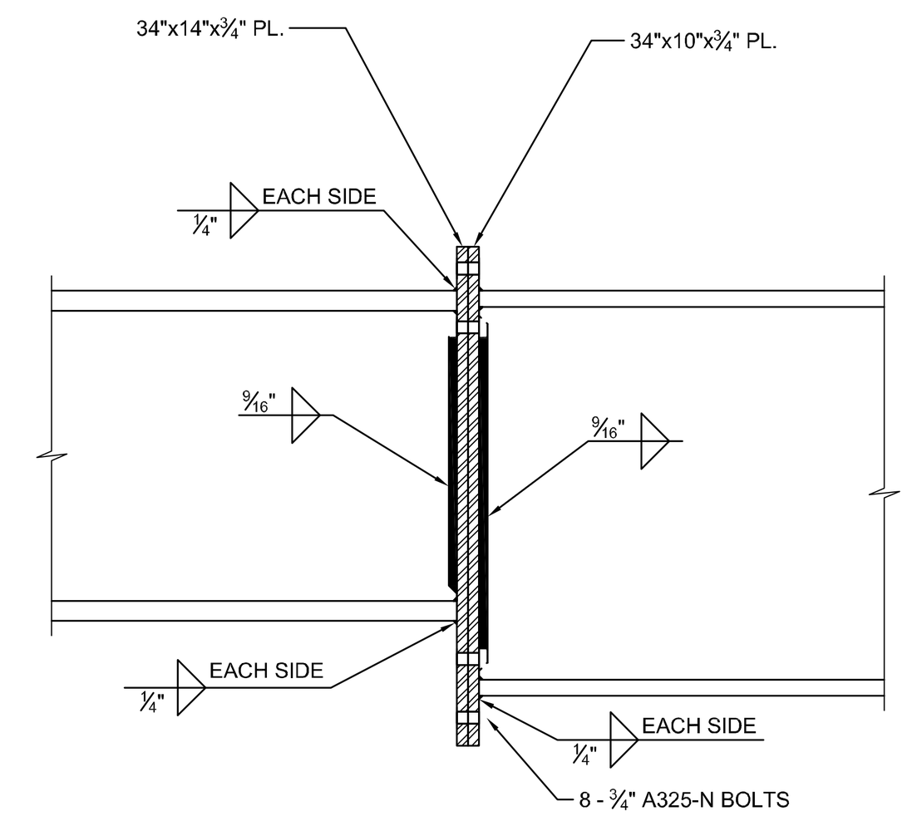
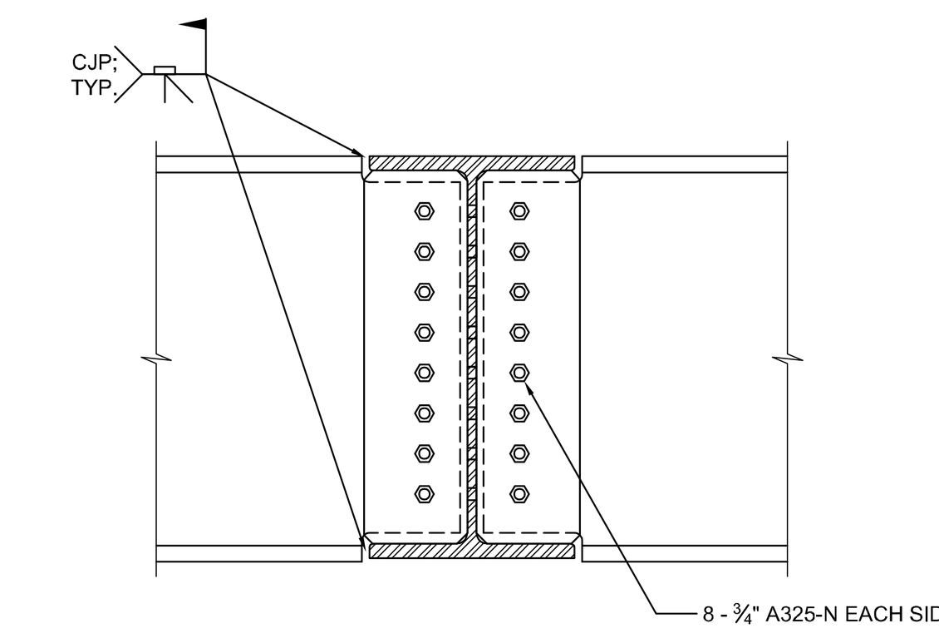
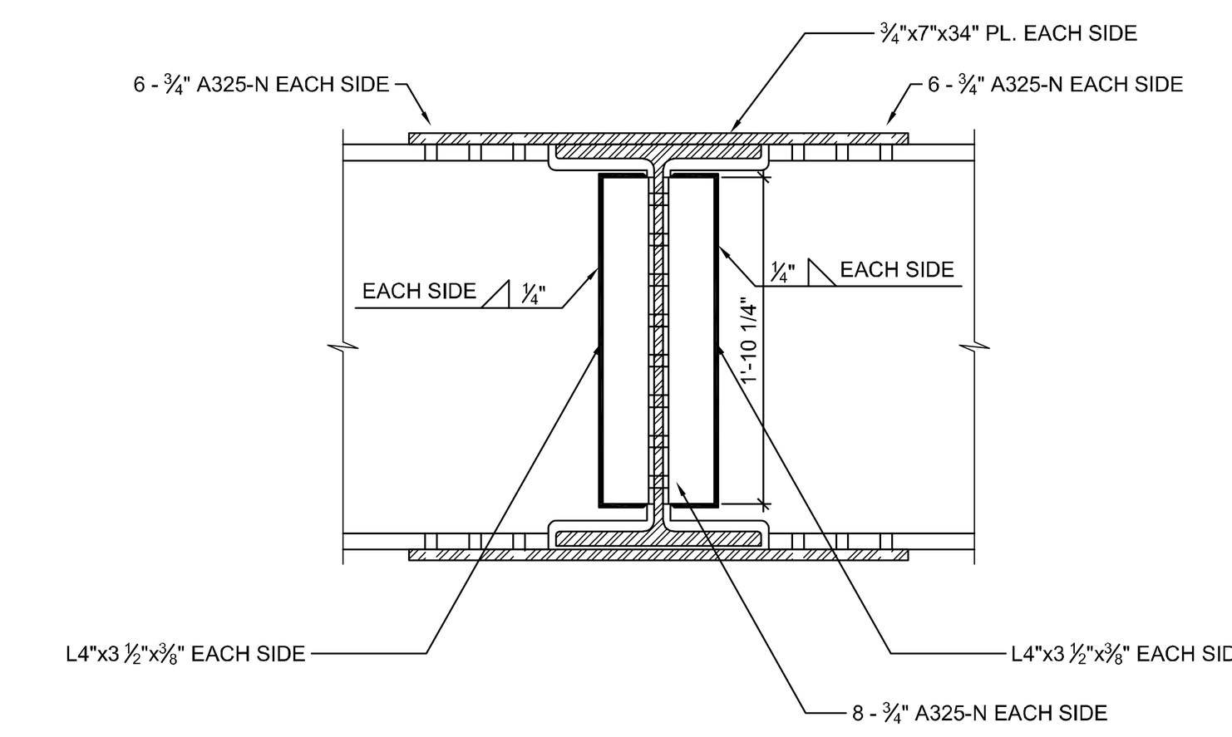


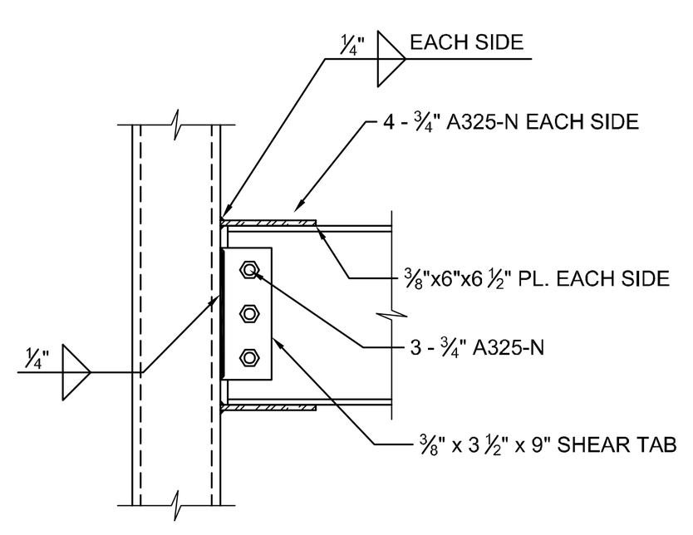
3 S-502 2B-9 STUB CONNECTION
1" = 1'-0"



4 S-502 2B-5 MOMENT CONNECTION
1" = 1'-0"



5 S-502 2B-9 MOMENT CONNECTION
1" = 1'-0"



6 S-502 2B-6 MOMENT CONNECTION
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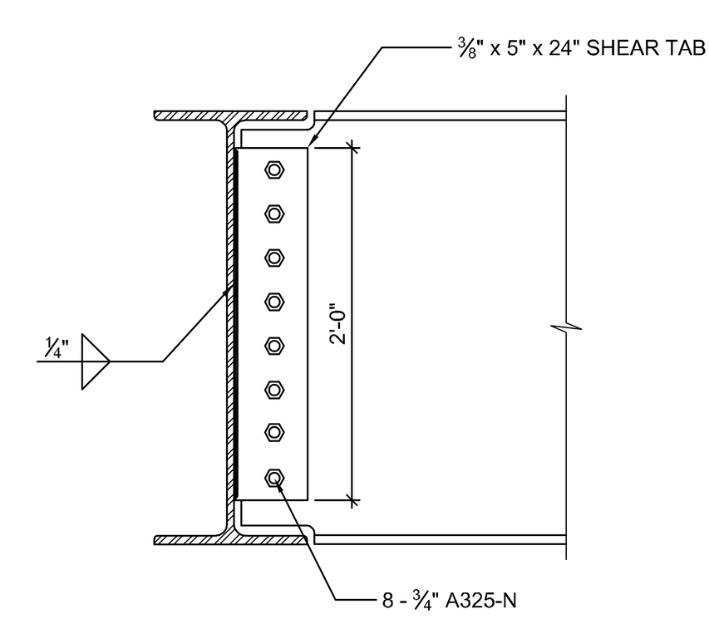
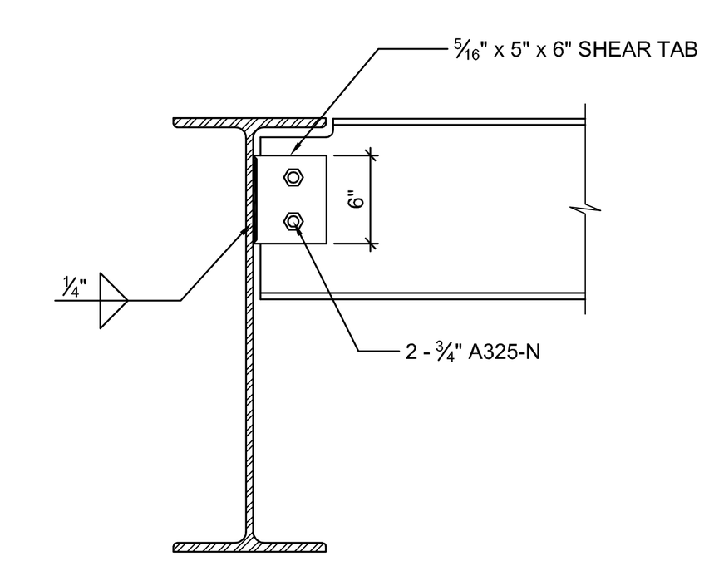
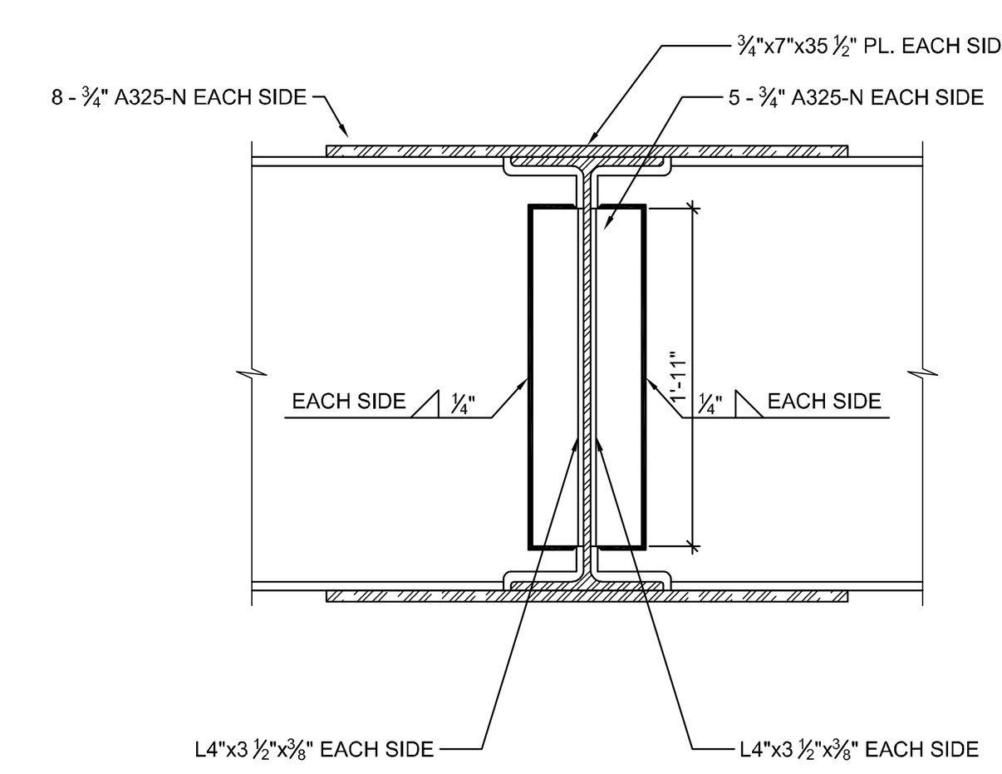


7 S-502 3B-5/6 SHEAR CONNECTION to 3B-16
1" = 1'-0"



8 S-502 3B-11/12 TO 3B-5/6 SHEAR CONNECTION
1" = 1'-0"



9 S-502 3B-13 MOMENT CONNECTION
1" = 1'-0"

PLOT DATE: July 27, 2018
FILE: J:\Job-2017 - 170900 to 170999\170950 Summit Lot 14R\Drawings Structural\170950_X-TB-E.dwg

PLAN REVIEW ACCEPTANCE
FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW.
- [X] BUILDING
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BY: MEM DATE: 08/22/18
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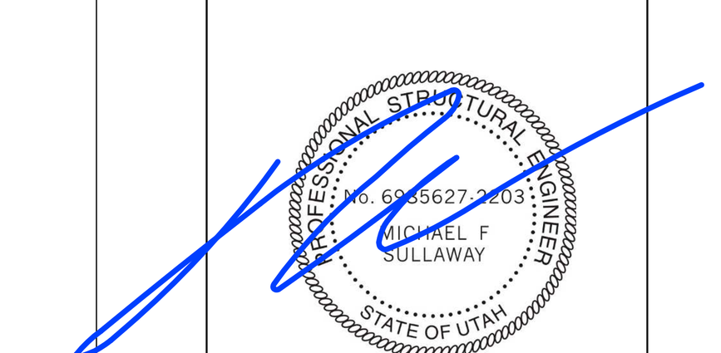

No.	Description	Date
05	Issued for Building Permit	2018.08.15
04	Issued for SSI No 01	2018.07.27
03	Issued for Building Permit	2018.07.13
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01	Issued for Costing	2018.05.30

Revision:

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Connection Details

scale: 1" = 1'-0"
date: 2018-05-20
drawn: DP
chk'd: TJ