

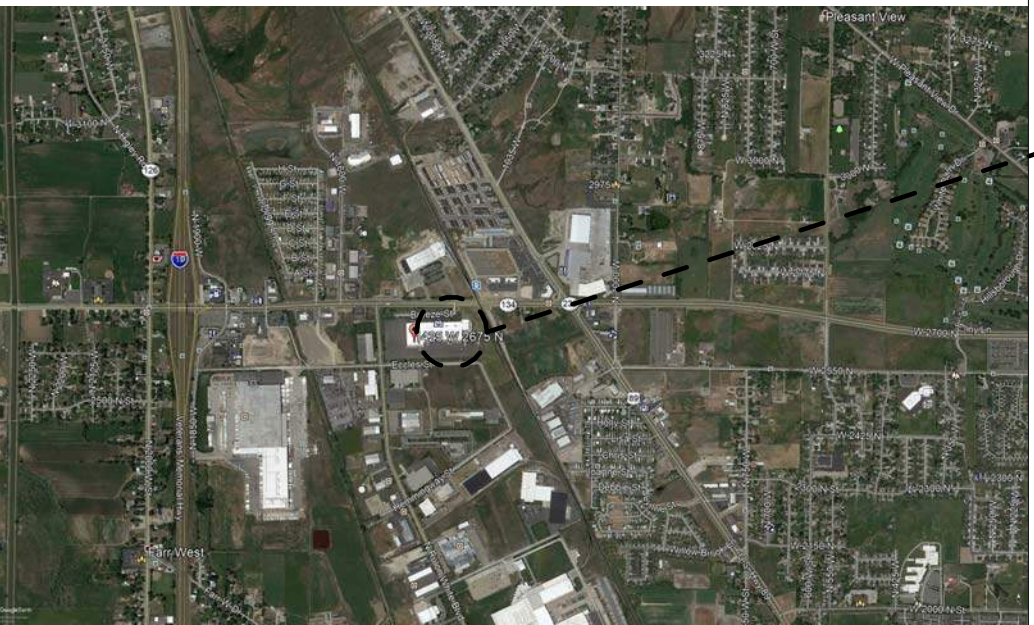
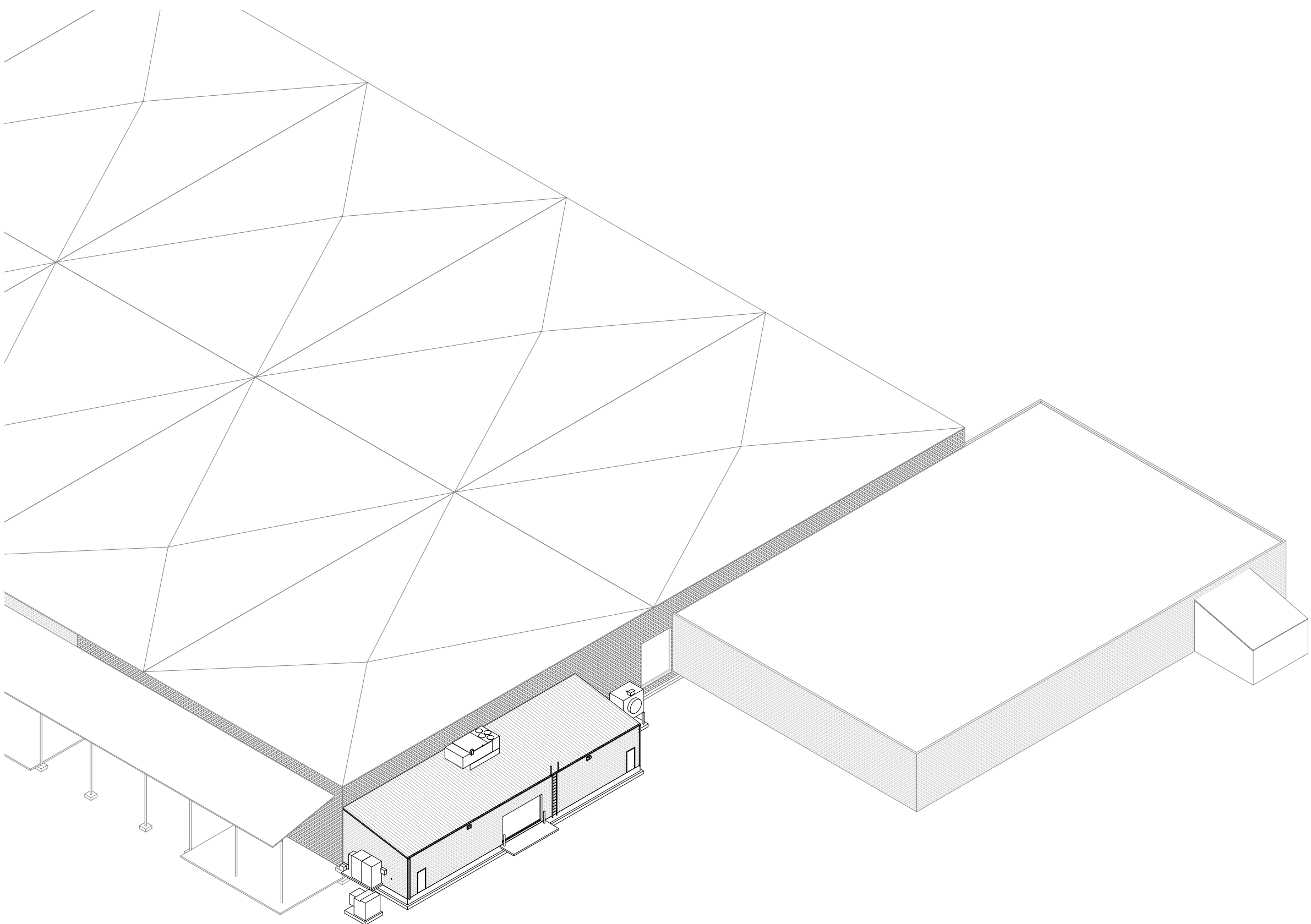


PARKER/HANNFIN
CORP.
ADDITION TO THE
CONTROL SYSTEMS
DIVISION PLANT

Case, Lowe and Hart, Inc.
2484 Washington Blvd. Ste 510
Ogden, Utah 84401

ARW Engineers
1594 West Park Circle
Ogden, Utah 84404

Reeve & Associates
5160 1500 West
Riverdale, Utah 84405



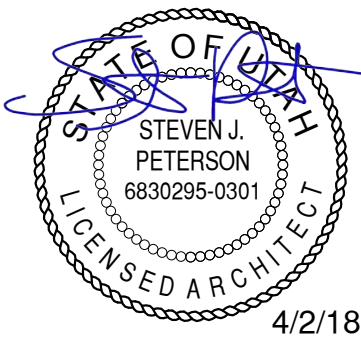
DRAWING INDEX	
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C1	COVER/INDEX
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C4	DEMOLITION PLAN
C5	PROPOSED SITE PLAN
C6	STORM WATER POLLUTION PREVENTION PLAN EXHIBIT
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M001	MECHANICAL SCHEDULES
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E001	ELECTRICAL LEGEND, SCHEDULES & DETAILS
E100	OVERALL ELECTRICAL POWER PLAN
E101	ENLARGED ELECTRICAL PLANS
E801	ELECTRICAL DETAILS
E802	ELECTRICAL DETAILS
E701	ELECTRICAL PANEL SCHEDULES



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PLANT
PARKER/HANNIFIN CORP.
1425 WEST 2675 NORTH OGDEN, UTAH

MARK	DATE	DESCRIPTION
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ISSUE DATE: 27 MARCH, 2018
PROJECT NO: 18110
CAD DWG FILE:
DRAWN BY: Author
CHK'D BY: Checker

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27 MARCH, 2018

SHEET TITLE

TITLE SHEET

SHEET NO:

G001

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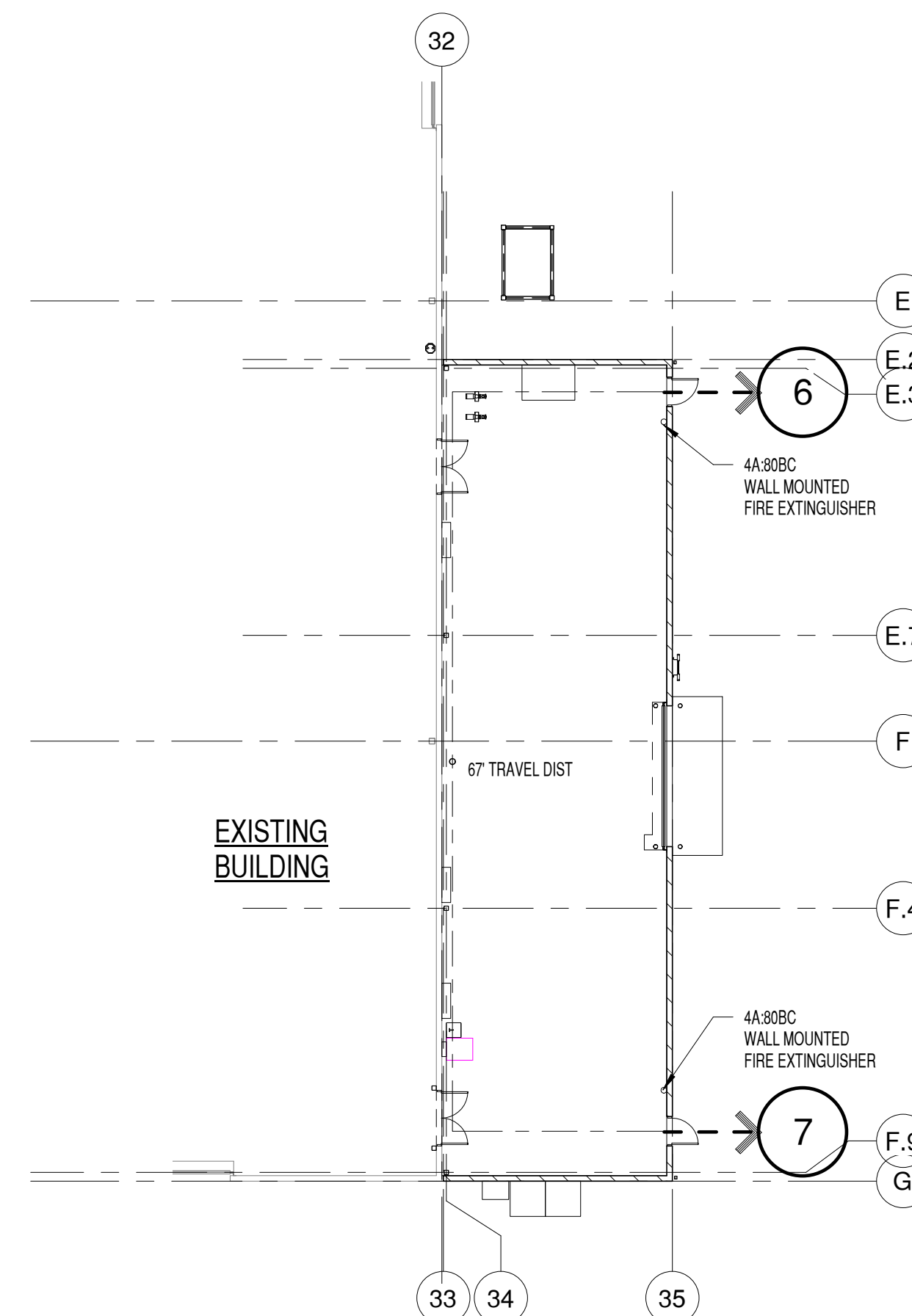
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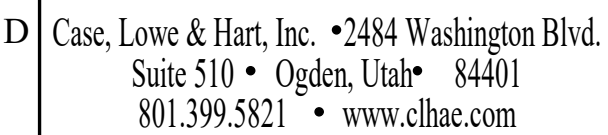
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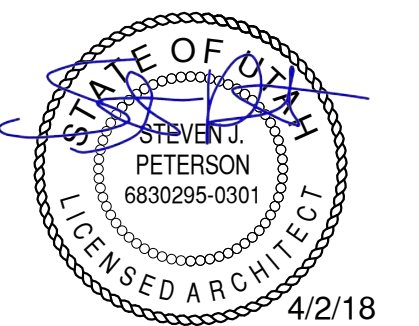
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PLANT
PARKER/HANNIFIN CORP.
1425 WEST 2675 NORTH OGDEN, UTAH

MARK	DATE	DESCRIPTION
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ISSUE DATE:	27 MARCH, 2018
PROJECT NO:	18110
CAD DWG FILE:	
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CHK'D BY:	Checker

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SHEET TITLE

CODE REVIEW &
LIFE SAFETY

SHEET NO:

G002

Project Narrative/Notes/Revisions

1. 03/02/2018 JM – COMPLETED DESIGN FOR CLIENT & CITY REVIEW.

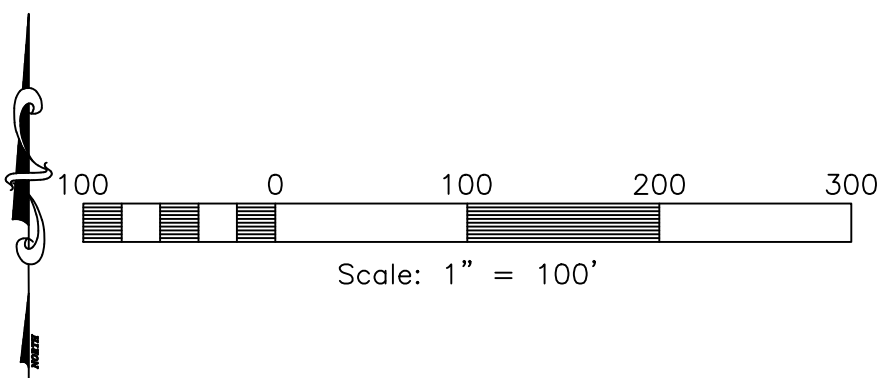
Parker/Hannifin Corp.
1425 West 2675 North
Improvement Plans

CITY OF OGDEN, WEBER COUNTY, UTAH
MARCH 2018



Vicinity Map
NOT TO SCALE

- Sheet Index**
Sheet 1 - Cover/Index Sheet
Sheet 2 - Notes/Legend
Sheet 3 - Existing Conditions Plan
Sheet 4 - Demolition Plan
Sheet 5 - Proposed Site Plan
Sheet 6 - Storm Water Pollution Prevention Plan Exhibit
Sheet 7 - Storm Water Pollution Prevention Plan Details



Engineer's Notice To Contractors

THE EXISTENCE AND LOCATION OF ANY UNDERGROUND UTILITY PIPES OR STRUCTURES SHOWN ON THESE PLANS WERE OBTAINED FROM AVAILABLE INFORMATION PROVIDED BY OTHERS. THE LOCATIONS SHOWN ARE APPROXIMATE AND SHALL BE CONFIRMED IN THE FIELD BY THE CONTRACTOR, SO THAT ANY NECESSARY ADJUSTMENT CAN BE MADE IN ALIGNMENT AND/OR GRADE OF THE PROPOSED IMPROVEMENT. THE CONTRACTOR IS REQUIRED TO CONTACT THE UTILITY COMPANIES AND TAKE DUE PRECAUTIONARY MEASURE TO PROTECT ANY UTILITY LINES SHOWN, AND ANY OTHER LINES OBTAINED BY THE CONTRACTOR'S RESEARCH, AND OTHERS NOT OF RECORD OR NOT SHOWN ON THESE PLANS.

Developer Contact:

Blue Stakes Location Center

Call: Toll Free
1-800-662-4111

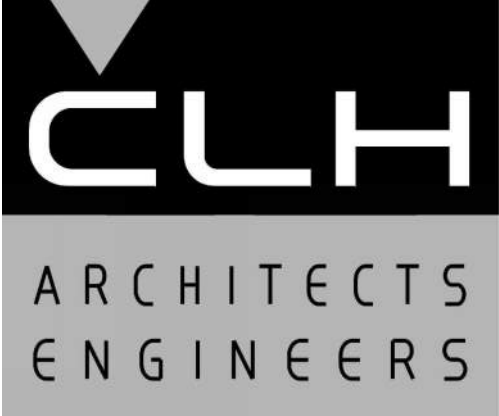
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PROJECT NUMBER: 5799-494
DRAWN BY: TRP
ENGINEER: JNR



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PROJECT NUMBER: 6528-28

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CONTROL SYSTEMS DIV.
PLANT
PARKER/HANNIFIN CORP.
1425 WEST 2675 NORTH OGDEN, UTAH

MARK	DATE	DESCRIPTION
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PROJECT NO:	18110
CAD DWG FILE:	
DRAWN BY:	DRW
CHK'D BY:	CHK

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02 March, 2018

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Cover/Index

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- UTILITY NOTES:

CONTRACTOR SHALL COORDINATE LOCATION OF NEW "DRY UTILITIES" WITH THE APPROPRIATE UTILITY COMPANY, INCLUDING BUT NOT LIMITED TO: TELEPHONE SERVICE, GAS SERVICE, CABLE, POWER, INTERNET.

EXISTING UTILITIES HAVE BEEN SHOWN ON THE PLANS USING A COMBINATION OF ON-SITE SURVEYS (BY OTHERS), PRIOR TO COMMENCING ANY WORK, IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO HAVE EACH UTILITY COMPANY LOCATE IN THE FIELD, THEIR MAIN AND SERVICE LINES 48 HOURS IN ADVANCE OF PERFORMING ANY EXCAVATION WORK. THE CONTRACTOR SHALL RECORD THE BLUE STAKES ORDER NUMBER AND FURNISH ORDER NUMBER TO OWNER AND ENGINEER PRIOR TO ANY EXCAVATION. IT WILL BE THE CONTRACTOR'S SOLE RESPONSIBILITY TO DIRECTLY CONTACT ANY OTHER UTILITY COMPANIES THAT ARE NOT MEMBERS OF BLUE STAKES. IT SHALL BE THE CONTRACTOR'S SOLE RESPONSIBILITY TO PROTECT ALL EXISTING UTILITIES SO THAT NO DAMAGE RESULTS TO THEM DURING THE PERFORMANCE OF THIS CONTRACT. ANY REPAIRS NECESSARY TO DAMAGED UTILITIES SHALL BE PAID FOR BY THE CONTRACTOR. THE CONTRACTOR SHALL BE REQUIRED TO COOPERATE WITH OTHER CONTRACTORS AND UTILITY COMPANIES INSTALLING NEW STRUCTURES, UTILITIES AND SERVICE TO THE PROJECT.

CONTRACTOR SHALL POT HOLE ALL UTILITIES TO DETERMINE IF CONFLICTS EXIST PRIOR TO BEGINNING ANY EXCAVATION. NOTIFY ENGINEER OF ANY CONFLICTS. CONTRACTOR SHALL VERIFY LOCATION AND INVERTS OF EXISTING UTILITIES TO WHICH NEW UTILITIES WILL BE CONNECTED. PRIOR TO COMMENCING ANY EXCAVATION WORK THE CONTRACTOR SHALL NOTIFY ALL UTILITY COMPANIES IN ACCORDANCE WITH THE REQUIRED PROCEDURES.

CARE SHOULD BE TAKEN IN ALL EXCAVATIONS DUE TO POSSIBLE EXISTENCE OF UNRECORDED UTILITY LINES. EXCAVATION REQUIRED WITHIN PROXIMITY OF EXISTING UTILITY LINES SHALL BE DONE BY HAND. CONTRACTOR SHALL REPAIR ANY DAMAGE TO EXISTING UTILITY LINES OR STRUCTURES INCURRED DURING CONSTRUCTION OPERATIONS AT HIS EXPENSE.

ALL VALVES AND MANHOLE COVERS SHALL BE RAISED OR LOWERED TO MEET FINISHED GRADE.

CONTRACTOR SHALL CUT PIPES OFF FLUSH WITH THE INSIDE WALL OF THE BOX OR MANHOLE.

CONTRACTOR SHALL GROUT AT CONNECTION OF PIPE TO BOX WITH NON-SHRINKING GROUT, INCLUDING PIPE VOIDS LEFT BY CUTTING PROCESS, TO A SMOOTH FINISH.

CONTRACTOR SHALL GROUT WITH NON-SHRINK GROUT BETWEEN GRADE RINGS AND BETWEEN BOTTOM OF INLET LID FRAME AND TOP OF CONCRETE BOX.

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CONTRACTOR SHALL CLEAN ASPHALT, TAR OR OTHER ADHESIVES OFF OF ALL MANHOLE LIDS AND INLET GRATES TO ALLOW ACCESS.

EACH TRENCH SHALL BE EXCAVATED SO THAT THE PIPE CAN BE LAID TO THE ALIGNMENT AND GRADE AS REQUIRED. THE TRENCH WALL SHALL BE SO BRACED THAT THE WORKMEN MAY WORK SAFELY AND EFFICIENTLY. ALL TRENCHES SHALL BE DRAINED SO THE PIPE LAYING MAY TAKE PLACE IN DE-WATERED CONDITIONS.

CONTRACTOR SHALL PROVIDE AND MAINTAIN AT ALL TIMES AMPLE MEANS AND DEVICES WITH WHICH TO REMOVE PROMPTLY AND TO PROPERLY DISPOSE OF ALL WATER ENTERING THE TRENCH EXCAVATION.

MAINTAIN A MINIMUM 18" VERTICAL SEPARATION DISTANCE BETWEEN ALL UTILITY CROSSINGS.

CONTRACTOR SHALL START INSTALLATION AT LOW POINT OF ALL NEW GRAVITY UTILITY LINES.

ALL BOLTED FITTINGS MUST BE GREASED AND WRAPPED.

UNLESS SPECIFICALLY NOTED OTHERWISE, MAINTAIN AT LEAST 2 FEET OF COVER OVER ALL STORM DRAIN LINES AT ALL TIMES (INCLUDING DURING CONSTRUCTION).

ALL WATER LINES SHALL BE INSTALLED A MINIMUM OF 60" BELOW FINISHED GRADE.

ALL SEWER LINES AND SEWER SERVICES SHALL HAVE A MINIMUM SEPARATION OF 10 FEET, PIPE EDGE TO PIPE EDGE, FROM THE WATER LINES. IF A 10 FOOT SEPARATION CAN NOT BE MAINTAINED, THE SEWER LINE AND WATER LINE SHALL BE LAID IN SEPARATE TRENCHES AND THE BOTTOM OF THE WATER LINE SHALL BE AT LEAST 18" ABOVE THE TOP OF THE SEWER LINE.

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- MAINTENANCE:

ALL BEST MANAGEMENT PRACTICES (BMP'S) SHOWN ON THIS PLAN MUST BE MAINTAINED AT ALL TIMES UNTIL PROJECT CLOSE-OUT.

THE CONTRACTOR'S RESPONSIBILITY SHALL INCLUDE MAKING BI-WEEKLY CHECKS ON ALL EROSION CONTROL MEASURES TO DETERMINE IF REPAIR OR SEDIMENT REMOVAL IS NECESSARY. CHECKS SHALL BE DOCUMENTED AND COPIES OF THE INSPECTIONS KEPT ON SITE.

SEDIMENT DEPOSITS SHOULD BE REMOVED AFTER EACH RAINFALL. THEY MUST BE REMOVED WHEN THE LEVEL OF DEPOSITION REACHES APPROXIMATELY ONE-HALF THE HEIGHT OF BARRIER.

SEDIMENT TRACKED ONTO PAVED ROADS MUST BE CLEANED UP AS SOON AS PRACTICAL, BUT IN NO CASE LATER THAN THE END OF THE NORMAL WORK DAY. THE CLEAN UP WILL INCLUDE SWEEPING OF THE TRACKED MATERIAL, PICKING IT UP, AND DEPOSITING IT TO A CONTAINED AREA.

EXPOSED SLOPES:

ANY EXPOSED SLOPE THAT WILL REMAIN UNTOUCHED FOR LONGER THAN 14 DAYS MUST BE STABILIZED BY ONE OR MORE OF THE FOLLOWING METHODS:

A) Spraying DISTURBED AREAS WITH A TACKIFIER VIA HYDROSEED

B) TRACKING STRAW PERPENDICULAR TO SLOPES

C) INSTALLING A LIGHT-WEIGHT, TEMPORARY EROSION CONTROL BLANKET
- LEGEND

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= PROPOSED CULINARY WATER LINE

= EXISTING CULINARY WATER LINE

= PROPOSED SANITARY SEWER LINE

= EXISTING SANITARY SEWER LINE

= PROPOSED STORM DRAIN LINE

= EXISTING STORM DRAIN LINE

= FENCE LINE

= PROPOSED FIRE HYDRANT

= EXISTING FIRE HYDRANT

= PROPOSED MANHOLE

= EXISTING MANHOLE

= PROPOSED SEWER CLEAN-OUT

= PROPOSED GATE VALVE

= EXISTING GATE VALVE

= PROPOSED WATER METER

= EXISTING WATER METER

= PROPOSED CATCH BASIN

= EXISTING CATCH BASIN

= PLUG W/ 2" BLOW-OFF

= STREET LIGHT

= SIGN

= BUILDING

= CURB & GUTTER

= CATCH BASIN

= CUBIC FEET

= CUBIC FEET PER SECOND

= FENCE CORNER

= FINISH FLOOR

= FINISH FLOOR ELEVATION

= FINISHED GRADE

= FIRE HYDRANT

= FLOW LINE

= GRADE BREAK

= INVERT

= LINEAR FEET

= NATURAL GRADE

= POWER/UTILITY POLE

= PUBLIC UTILITY EASEMENT

= REINFORCED CONCRETE PIPE

= RIM OF MANHOLE

= RIGHT-OF-WAY

= STORM DRAIN

= SANITARY SEWER

= TOP BACK OF CURB

= TOP OF ASPHALT

= TOP OF CONCRETE

= TOP OF FINISHED FLOOR

= TOP OF PUMP ISLAND

= TOP OF SIDEWALK

= CULINARY WATER

= WATER METER

= EXISTING ASPHALT ROADWAY

=EX. ASPHALT TAXIWAY

= PROPOSED PAVEMENT

= PROPOSED CONCRETE
- CLH

ARCHITECTS

ENGINEERS
- Case, Lowe & Hart, Inc.

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- CONSULTANTS

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PROJECT NUMBER: 6528-28

STAMP

REGISTERED PROFESSIONAL ENGINEER

375528

J. NATE REEVE

03/07/2018

STATE OF UTAH
- Parker
- ADDITION TO THE

CONTROL SYSTEMS DIV.

PLANT

PARKER/HANNIFIN CORP.

1425 WEST 2675 NORTH OGDEN, UTAH
- MARK

DATE

DESCRIPTION
- ISSUE DATE: 22 February, 2018

PROJECT NO: 18110

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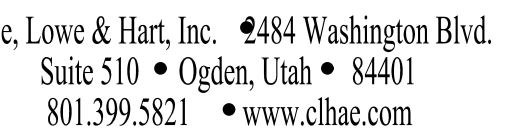
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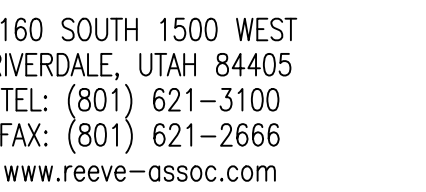
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PROJECT NUMBER: 6528-28

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ADDITION TO THE
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PLANT
PARKER/HANNIFIN CORP.
425 WEST 2675 NORTH OGDEN, UTAH

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SUE DATE:	22 February, 2018
PROJECT NO:	18110
AD DWG FILE:	
DRAWN BY:	DRW
CHECK'D BY:	CHK

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2 March, 2018

SHEET TITLE

Existing Condition Plan

SHEET NO:

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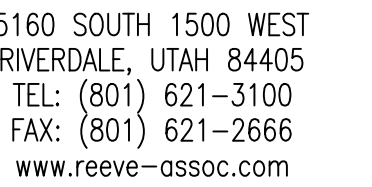
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PROJECT NO:	18110
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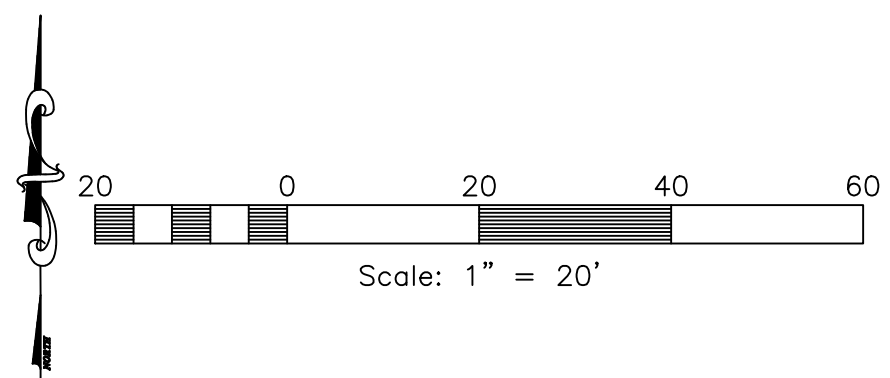
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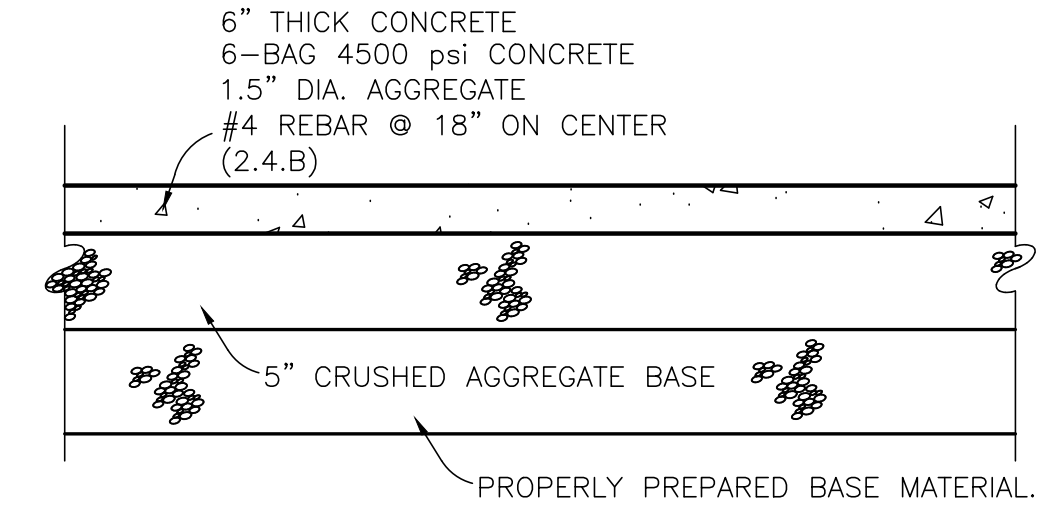
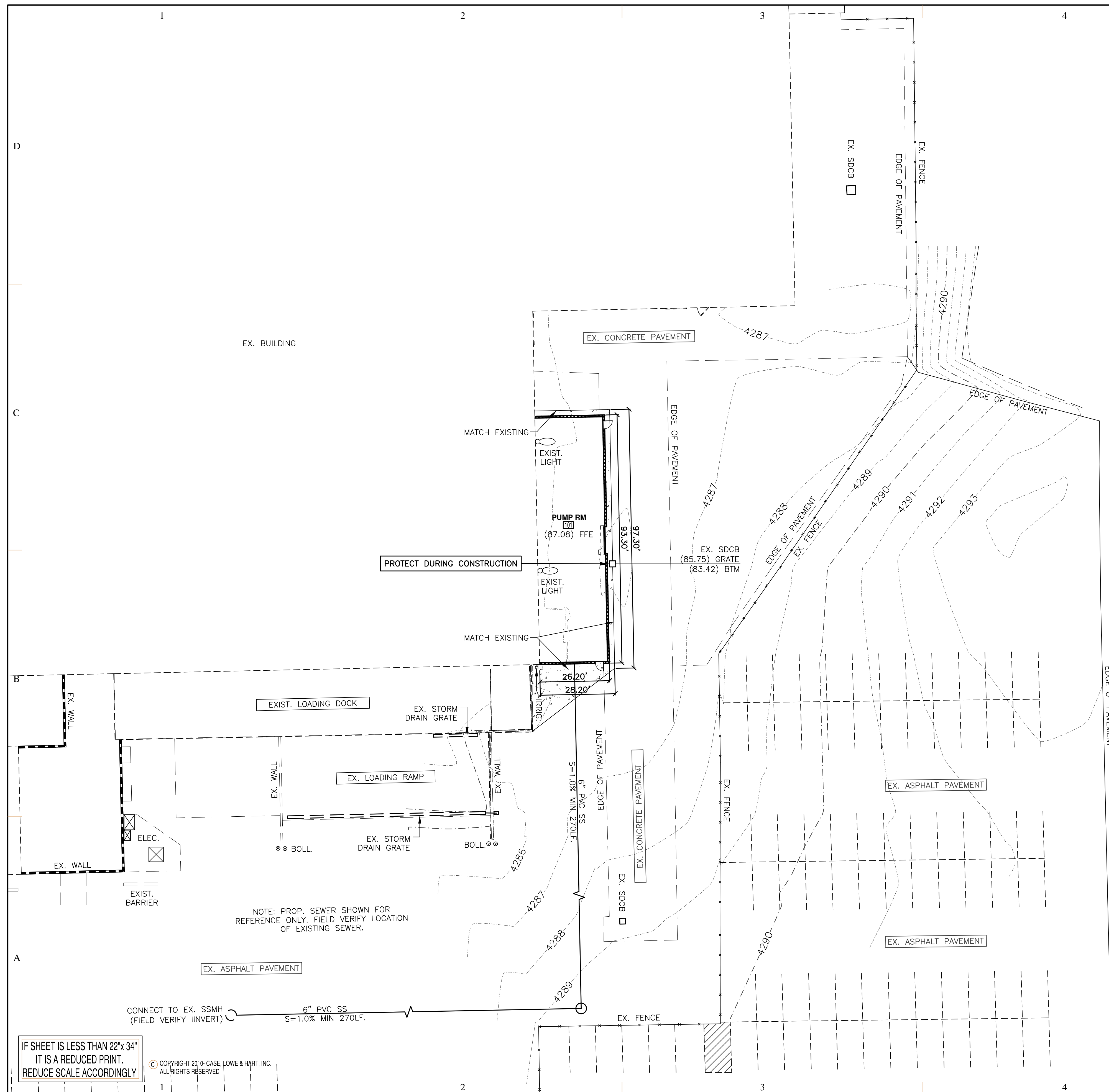
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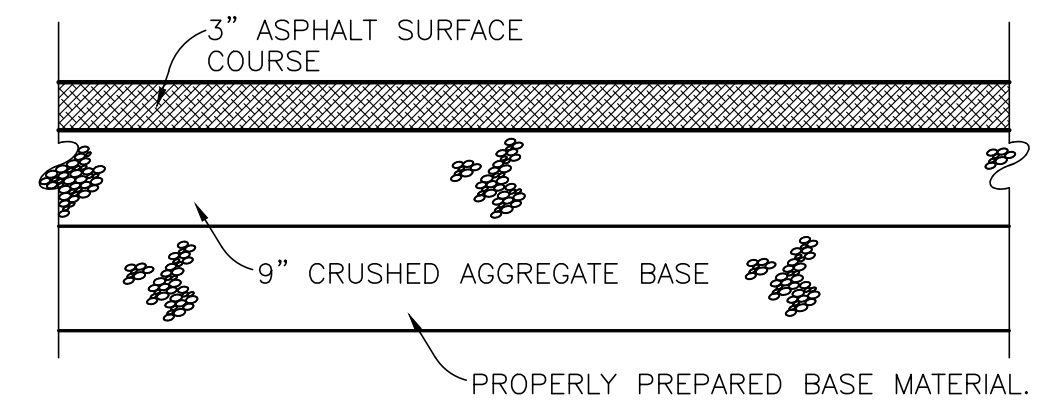
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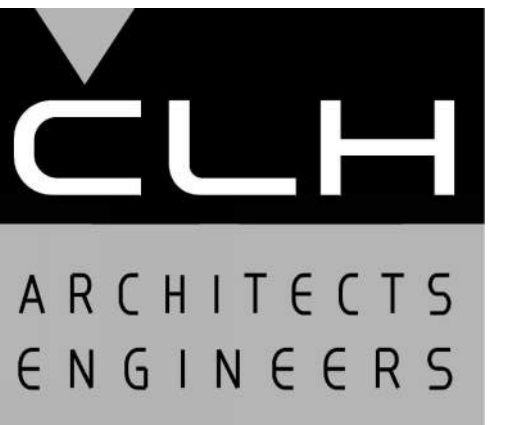
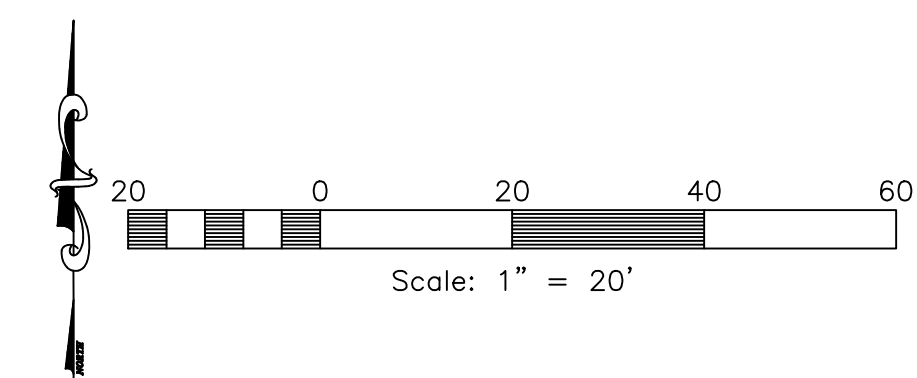
(REFER TO THE SITE SPECIFIC GEOTECHNICAL REPORT;
GEOTECHNICAL REPORT TO GOVERN & CONTROL.)

1 Typical On-Site Concrete Paving



(REFER TO THE SITE SPECIFIC GEOTECHNICAL REPORT;
GEOTECHNICAL REPORT TO GOVERN & CONTROL.)

2 Typical On-Site Asphalt Paving - Alternate Bid



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STRUCTURAL ENGINEERS * LANDSCAPE ARCHITECTS

PROJECT NUMBER:	6528-28
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STAMP



ADDITION TO THE
CONTROL SYSTEMS DIV.
PLANT
PARKER/HANNIFIN CORP.
1425 WEST 2675 NORTH OGDEN, UTAH

MARK	DATE	DESCRIPTION
ISSUE DATE:		22 February, 2018
PROJECT NO:		18110
CAD DWG FILE:		
DRAWN BY:		DRW
CHK'D BY:		CHK

30% SUBMITTAL SET

02 March, 2018

SHEET TITLE

Proposed Site Plan

SHEET NO:

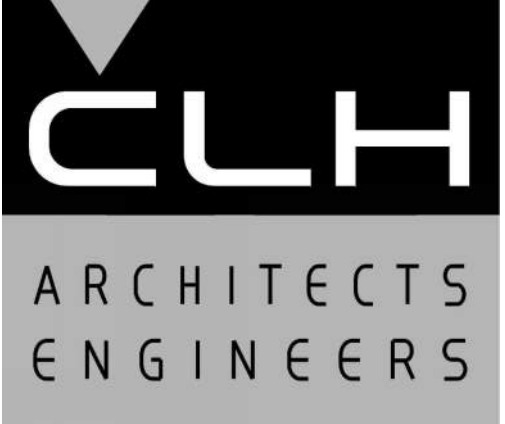
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Parker/Hannifin Corp.
1425 West 2675 North
Storm Water Pollution Prevention Plan Exhibit

CITY OF OGDEN, WEBER COUNTY, UTAH
MARCH 2018



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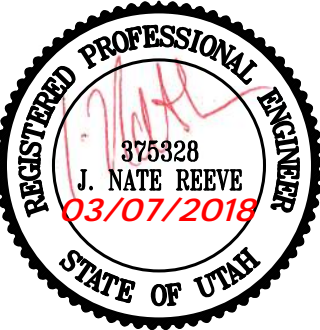


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PROJECT NUMBER: 6528-28

STAMP



ADDITION TO THE
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PLANT
PARKER/HANNIFIN CORP.
1425 WEST 2675 NORTH OGDEN, UTAH

MARK DATE DESCRIPTION

ISSUE DATE: 22 February, 2018
PROJECT NO: 18110
CAD DWG FILE:
DRAWN BY: DRW
CHK'D BY: CHK

30% SUBMITTAL SET
02 March, 2018

SHEET TITLE

Storm Water
Pollution Prevention
Plan Exhibit

SHEET NO:

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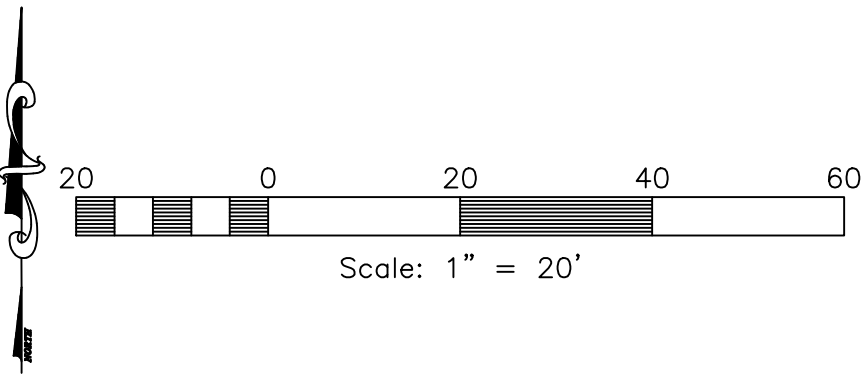
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Construction Activity Schedule

- PROJECT LOCATION.....OGDEN CITY, WEBER COUNTY, UT
- PROJECT BEGINNING DATE.....MARCH 2018
- BMP'S DEPLOYMENT DATE.....MARCH 2018
- STORM WATER MANAGEMENT CONTACT / INSPECTOR.....CONTACT NAME (000) 000-0000
- SPECIFIC CONSTRUCTION SCHEDULE INCLUDING BMP CONSTRUCTION SCHEDULE TO BE INCLUDED WITH SWPPP BY OWNER/DEVELOPER



STREETS TO BE SWEEPED WITHIN
1000 FEET OF CONSTRUCTION
ENTRANCE DAILY IF NECESSARY

INLET PROTECTION
TYP. (SEE DETAIL)

INLET PROTECTION
TYP. (SEE DETAIL)

INLET PROTECTION
TYP. (SEE DETAIL)

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STRUCTURAL NOTES :

A. GENERAL

- THE STRUCTURAL NOTES ARE INTENDED TO COMPLEMENT THE PROJECT SPECIFICATIONS WHICH ARE PART OF THE CONSTRUCTION DOCUMENTS. SPECIFIC NOTES AND DETAILS ON THE DRAWINGS SHALL GOVERN OVER THE STRUCTURAL NOTES AND TYPICAL DETAILS.
- THESE DRAWINGS (AND, WHERE APPLICABLE, ACCOMPANYING WRITTEN SPECIFICATIONS) ARE THE ONLY CONTRACT DOCUMENTS PROVIDED BY ARW ENGINEERS FOR THE PROJECT REPRESENTED HEREIN. NOTHING IN ANY DIGITAL MODEL OR DIGITAL FILE RELATED TO THIS PROJECT SHALL BE TAKEN TO SUPERSEDE ANY INFORMATION SHOWN IN THESE DRAWINGS (INCLUDING, BUT NOT LIMITED TO, DIMENSIONS, SIZES, ETC.)
- THE ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS. THE STRUCTURAL DRAWINGS ARE SUPPLEMENTARY TO AND MUST BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS AND OTHER CONSULTANTS DRAWINGS. ALL OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY WORK INVOLVED. IN CASE OF CONFLICT, FOLLOW THE MOST STRINGENT REQUIREMENT AS DIRECTED BY THE ARCHITECT AT NO ADDITIONAL COST TO THE OWNER.
- SEE SPECIFICATIONS FOR REQUIRED SUBMITTALS. SUBMITTALS SHALL BE MADE IN A TIMELY MANNER AS INDICATED IN SPECIFICATIONS. REVIEW OF SUBMITTALS BY ARW ENGINEERS IS FOR GENERAL COMPLIANCE ONLY AND IS NOT INTENDED AS APPROVAL. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL SIZES, DIMENSIONS, AND ELEVATIONS ON SUBMITTALS AS RELATED TO DESIGN DOCUMENTS. PREPARATION OF SHOP DRAWINGS FOR STRUCTURAL ELEMENTS WILL REQUIRE INFORMATION (I.E. DIMENSIONS, ETC.) FOUND IN THE ARCHITECTURAL, STRUCTURAL, AND OTHER CONSULTANTS DRAWINGS.
- THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE. IF ACTUAL CONDITIONS DIFFER FROM THOSE SHOWN ON CONTRACT DOCUMENTS, CONTRACTOR SHALL NOTIFY ARCHITECT PRIOR TO FABRICATION OR CONSTRUCTION OF ANY AFFECTED ELEMENTS.
- THE CONTRACTOR SHALL COORDINATE AND VERIFY ALL LOCATIONS AND SIZES OF MECHANICAL EQUIPMENT OR OTHER EQUIPMENT BEFORE FABRICATING AND ERECTING STRUCTURAL ELEMENTS. SIZES AND LOCATIONS THAT DIFFER FROM THOSE SHOWN ON THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT.
- THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST TO THE ARCHITECT FOR ARCHITECT AND/OR ENGINEER APPROVAL BEFORE PROCEEDING WITH ANY CHANGES, MODIFICATIONS, OR SUBSTITUTIONS.
- OBSERVATION VISITS TO THE SITE BY ARW ENGINEERS FIELD REPRESENTATIVES SHALL NEITHER BE CONSTRUED AS INSPECTION NOR APPROVAL OF CONSTRUCTION.
- DURING AND AFTER CONSTRUCTION, BUILDER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS AS NOTED IN THESE DOCUMENTS.
- TYPICAL OR SIMILAR DETAILS AND SECTIONS SHALL APPLY WHERE SPECIFIC DETAILS ARE NOT SHOWN. TYPICAL OR SIMILAR DETAILS REFER TO THE CONDITION ADDRESSED AND ARE NOT NECESSARILY DETAILS LABELED "TYPICAL" OR "SIMILAR" IN THE PLANS AND DOCUMENTS.
- DRAWINGS AND DETAILS HAVE BEEN PREPARED WITH THE INTENT TO VISUALLY REPRESENT INFORMATION PROVIDED IN SCALED FORM; HOWEVER CONTRACTOR/SUPPLIERS SHOULD NOT SCALE PLANS OR DETAILS FOR DIMENSIONAL INFORMATION.
- THE CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY SHORING AND BRACING FOR ALL STRUCTURAL ELEMENTS UNTIL THE ENTIRE STRUCTURAL SYSTEM IS COMPLETED. DESIGN OF ALL SHORING AND BRACING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- ENGINEER SHALL NOT BE RESPONSIBLE FOR ACTIVITIES UNDER CONTROL OF THE CONTRACTOR SUCH AS CONSTRUCTION SITE SAFETY, MEANS, METHODS AND SEQUENCING OF CONSTRUCTION. ENGINEER SHALL NOT BE RESPONSIBLE FOR FABRICATION, ERECTION AND CONSTRUCTION REQUIREMENTS AS PRESCRIBED BY OSHA OR OTHER REGULATORY AGENCIES REGARDLESS OF INDICATIONS IN THESE DOCUMENTS.
- NOTICE OF COPYRIGHT: THESE STRUCTURAL DRAWINGS ARE HEREBY COPYRIGHTED BY ARW ENGINEERS, INC. ALL RIGHTS ARE RESERVED. THESE DOCUMENTS DEFINE A STRUCTURE AND ARE INSTRUMENTS OF SERVICE, FOR ONE USE ONLY. REPRODUCTION AND DISTRIBUTION OF THESE DRAWINGS IS ONLY ALLOWED AS REQUIRED FOR REGULATORY AGENCIES AND FOR CONVEYANCE OF INFORMATION TO PARTIES INVOLVED IN THE CONSTRUCTION OF THIS PROJECT. THESE DOCUMENTS SHALL NOT BE REPRODUCED OR COPIED, IN PART OR WHOLE BY ANY PARTY FOR USE IN PREPARATION OF SHOP DRAWINGS OR OTHER SUBMITTALS.

B. STATEMENT OF SPECIAL INSPECTIONS AND SPECIAL INSPECTIONS

- THE DESIGNATED SEISMIC/WIND SYSTEMS AND SEISMIC/WIND-FORCE-RESISTING SYSTEMS THAT ARE SUBJECT TO SPECIAL INSPECTIONS IN ACCORDANCE WITH IBC SECTION 1705.11 AND 1705.12 ARE IDENTIFIED IN THE SPECIAL INSPECTION SCHEDULE ON SHEET S004 & S006.
- SPECIAL INSPECTIONS AND TESTING ARE TO BE PROVIDED AS REQUIRED BY IBC SECTIONS 1704 THROUGH 1705 AND OTHER APPLICABLE SECTIONS OF THE IBC. THE TYPE AND FREQUENCY OF TESTING AND SPECIAL INSPECTIONS SHALL BE AS NOTED IN THE SPECIAL INSPECTION SCHEDULE, JOB SPECIFICATIONS, AND ACCORDANCE WITH IBC SECTION 110 AND CHAPTER 17. CONTRACTOR SHALL COORDINATE AND PARTICIPATE IN ALL REQUIRED INSPECTIONS.
- ALL TESTING AND SPECIAL INSPECTION SHALL BE PROVIDED BY A QUALIFIED INDEPENDENT SPECIAL INSPECTION AGENCY IN ACCORDANCE WITH IBC 1704 AND AS OUTLINED IN THE JOB SPECIFICATIONS. REPORTS OF FINDINGS OR DISCREPANCIES SHALL BE NOTED AND FORWARDED TO THE CONTRACTOR, ARCHITECT, ENGINEERS, AND BUILDING OFFICIAL IN A TIMELY MANNER.
- STRUCTURAL OBSERVATION VISITS SHALL BE PERFORMED BY A REPRESENTATIVE FROM ARW ENGINEERS IN ACCORDANCE WITH THE CONTRACT AS NEEDED TO OBSERVE THE CONSTRUCTION OF CRITICAL BUILDING ELEMENTS (I.E. FOOTINGS, BRACED FRAMES, MOMENT FRAMES, DRAG STRUTS AND THEIR CONNECTIONS, COLLECTORS, AND ROOF AND FLOOR DIAPHRAGMS). STRUCTURAL OBSERVATION REPORTS FOR EACH VISIT SHALL BE SENT DIRECTLY TO THE ARCHITECT FOR DISTRIBUTION TO THE CONTRACTOR AND BUILDING OFFICIAL. STRUCTURAL OBSERVATION VISITS SHALL NEITHER BE CONSTRUED AS SPECIAL INSPECTION NOR APPROVAL OF COMPLETED CONSTRUCTION.
- IN ACCORDANCE WITH IBC 1704.4, THE CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER.

C. BASIS OF DESIGN

- GOVERNING BUILDING CODE : INTERNATIONAL BUILDING CODE (IBC) 2015
RISK CATEGORY : II
- ROOF LOADS
a. FLAT-ROOF SNOW LOAD, P_f: 30 PSF
1. GROUND SNOW LOAD, P_g: 43 PSF
2. SNOW EXPOSURE FACTOR, C_e: 1.0
3. SNOW LOAD IMPORTANCE FACTOR, I_s: 1.0
4. THERMAL FACTOR, C_t: 1.0
b. LIVE LOAD = 20 PSF
c. DEAD LOAD = 15 PSF
- WIND DESIGN
a. BASIC WIND SPEED (3 SECOND GUST) : 115 MPH
b. WIND EXPOSURE : C
c. COMPONENT AND CLADDING DESIGN WIND PRESSURE SHALL BE AS REQUIRED PER ASCE 7-10.
- SEISMIC DESIGN :
a. SEISMIC IMPORTANCE FACTOR, I_e: 1.0
b. SITE CLASS : D
c. MAPPED SPECTRAL RESPONSE ACCELERATIONS : S_s = 1.522 , S₁ = 0.529
d. SPECTRAL RESPONSE COEFFICIENTS : S_{DS} = 1.014 , S_{DI} = .0529
e. SEISMIC DESIGN CATEGORY : D
f. BASIC SEISMIC-FORCE-RESISTING SYSTEM : SPECIAL REINFORCED MASONRY SHEAR WALLS
g. SEISMIC RESPONSE COEFFICIENT, C_s: 0.2
h. RESPONSE MODIFICATION FACTOR, R : 5.0
i. ANALYSIS PROCEDURE : EQUIVALENT LATERAL FORCE PROCEDURE

D. FOUNDATION

- DESIGN SOIL PRESSURE : 3000 PSF
- SOILS REPORT BY : ACEC
REPORT # : 25292
- ALL FOOTINGS SHALL BE PLACED ON MECHANICALLY COMPACTED FILL COMPACTED TO NOT LESS THAN 95% OF MODIFIED PROCTOR DENSITY (ASTM D-1557).
- SOIL PREPARATION UNDER FOOTINGS AND SLABS-ON-GRADE SHALL BE IN ACCORDANCE WITH THE SOILS REPORT.
- UNLESS NOTED OTHERWISE, ALL CONCRETE SLABS ON EARTH SHALL BEAR ON STRUCTURAL FILL COMPACTED TO 90% OF MODIFIED PROCTOR DENSITY (ASTM D-1557).
- TOP OF FOOTING ELEVATIONS SHOWN ON THE FOOTING AND FOUNDATION PLAN ARE BASED ON PRELIMINARY GRADING INFORMATION AND MUST BE VERIFIED PRIOR TO CONSTRUCTION. STEPS WHERE SHOWN ARE AT APPROXIMATE LOCATIONS. ALL EXTERIOR FOOTINGS MUST BEAR A MINIMUM OF 30 INCHES BELOW LOWEST ADJACENT FINAL GRADE.
- ALL WALLS (EXCEPT CANTILEVERED RETAINING WALLS) SHALL BE ADEQUATELY BRACED AGAINST LATERAL MOVEMENT PRIOR TO BACKFILLING. DESIGN AND ERECTION OF BRACING/SHORING IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. BRACING SHALL REMAIN IN PLACE UNTIL SUPPORTING STRUCTURAL ELEMENTS ARE IN PLACE AND HAVE ATTAINED FULL STRENGTH.
- UNLESS NOTED OTHERWISE, ALL FOOTINGS AT COLUMNS TO BE CENTERED BELOW COLUMNS.
- UNLESS NOTED OTHERWISE, ALL FOOTINGS SHALL HAVE VERTICAL FACES FORMED WITH STANDARD FORMING MATERIALS (WOOD, METAL, ETC.). WITH PRIOR APPROVAL OF ARCHITECT AND ENGINEER, CONCRETE FOR FOOTINGS CAN BE PLACED IN EXCAVATED "SOIL" FORMS PROVIDED THAT THE DIMENSIONS ARE INCREASED 3" ON EACH SIDE.

E. CONCRETE

- ALL CONCRETE MIX DESIGNS SHALL COMPLY WITH THE PROJECT SPECIFICATIONS AND THE REQUIREMENTS LISTED BELOW :
a. FOOTINGS, GRADE BEAMS, FOUNDATION WALLS :
1. WHERE THE TOP OF THE ELEMENT IS EXPOSED OR LOCATED WITHIN 30" OF THE LOWEST ADJACENT GRADE (EXPOSURE CATEGORY F1) :
a. 28 DAY COMPRESSIVE STRENGTH : 4500 PSI
b. MAXIMUM W/C RATIO : 0.45
c. MAXIMUM AGGREGATE SIZE : 1"
d. AIR CONTENT : 4.5% +/- 1.5%
2. WHERE THE TOP OF THE ELEMENT IS NOT EXPOSED OR LOCATED WITHIN 30" OF THE LOWEST ADJACENT GRADE (EXPOSURE CATEGORY F0) :
b. 28 DAY COMPRESSIVE STRENGTH : 3000 PSI
INTERIOR SLABS ON GRADE (EXPOSURE CATEGORY F0) :
28 DAY COMPRESSIVE STRENGTH : 3000 PSI
c. MAXIMUM W/C RATIO : 0.45
2. WATER USED MIXING CONCRETE SHALL CONFORM TO ASTM C1602.
- NO PIPES, DUCTS, SLEEVES, ETC. SHALL BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER. NO ALUMINUM PRODUCTS SHALL BE EMBEDDED IN CONCRETE. PENETRATIONS THRU STRUCTURAL CONCRETE ELEMENTS MUST BE APPROVED BY THE ENGINEER AND SHALL BE BUILT INTO THE ELEMENT PRIOR TO CONCRETE PLACEMENT.
- REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, ETC. TO BE CAST IN TO CONCRETE. AIR FOR EXHAUST AND LOCATION OF DEPRESSIONS, CURBS, RAK HTH HY.
- UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL CONCRETE FOUNDATION WALLS SHALL BE AS FOLLOWS:
TOP &
THICKNESS BOTTOM BARS VERTICAL HORIZONTAL
8" (2) #5 #4 AT 18" O.C. #4 AT 12" O.C.
UNLESS NOTED OTHERWISE, CONCRETE SLABS ON EARTH SHALL BE REINFORCED AS FOLLOWS:
6" THICK - #4 AT 18" O.C. EACH WAY.
REINFORCING SHALL BE CONTINUOUSLY SUPPORTED AT 36" O.C. MAXIMUM SPACING.
- UNLESS NOTED OTHERWISE, FOR NON-DETAILED OPENINGS IN CONCRETE WALLS LARGER THAN 12" AND SMALLER THAN 24" IN ANY DIRECTION ADD (2) #5 BARS ON ALL SIDES IN ADDITION TO REGULAR WALL REINFORCING AND EXTEND 24" EACH WAY BEYOND OPENING. IF 24" IS NOT AVAILABLE ON EVERY SIDE, NOTIFY STRUCTURAL ENGINEER FOR FURTHER DIRECTION. OPENINGS SHALL HAVE A MINIMUM OF 12" OF CONCRETE ABOVE THE OPENING, TYP.
- CONSTRUCTION JOINTS NOT SHOWN ON THE PLANS SHALL BE MADE AND LOCATED SO AS TO NOT IMPAIR THE STRENGTH OF THE STRUCTURE AND AS APPROVED BY THE STRUCTURAL ENGINEER. PROVIDE 2 X 4 (SHAPED) KEYWAY IN ALL VERTICAL AND HORIZONTAL JOINTS UNLESS NOTED OR DETAILED OTHERWISE. ALL STEEL REINFORCING SHALL BE CONTINUOUS THROUGH COLD JOINTS UNLESS NOTED OTHERWISE. SEE TYPICAL DETAILS FOR COLD/CONSTRUCTION JOINTS FOR SLABS ON GRADE.

F. ANCHOR BOLTS/EMBEDDED BOLTS

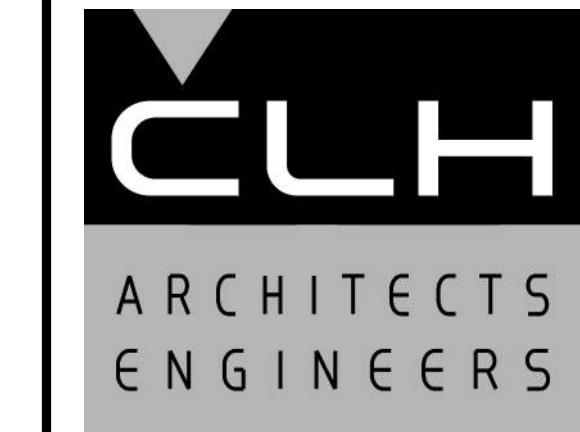
- ALL ANCHOR BOLTS SHALL HAVE ASTM A-563 HEAVY HEX NUT AND ASTM F-436 WASHERS AT STANDARD OR OVERSIZED HOLES PER AISC SPECIFICATION TABLE J3.3. WHERE HOLE SIZES DO NOT COMPLY WITH THE LIMITATIONS FOR OVERSIZED HOLES THE STRUCTURAL ENGINEER SHALL BE NOTIFIED TO DETERMINE STEEL PLATE WASHER REQUIREMENTS. ANCHOR BOLTS SHALL COMPLY WITH THE FOLLOWING :
a. AT ALL OTHER ANCHOR BOLTS (UNLESS NOTED OTHERWISE) - ASTM F1554 GRADE 36 HEADED BOLTS. (ASTM A36 THREADED ROD MAY BE USED WITH DOUBLE NUT AND WASHER.)
- EMBEDDED BOLTS IN MASONRY SHALL BE (UNLESS NOTED OTHERWISE) ASTM A-307 GRADE HEADED BOLTS.
- SEE TYPICAL ANCHOR BOLT DETAIL FOR DEFINITIONS OF EMBEDMENT LENGTH, ETC.
- FURNISH TEMPLATES AND OTHER DEVICES AS NECESSARY FOR PRESETTING ALL BOLTS PRIOR TO PLACING CONCRETE AND/OR GROUT.
- IF THREADED RODS ARE USED AS PERMITTED ABOVE, THEY SHALL BE CLEAR OF SOIL AND DIRT.
- WHERE REQUIRED FOR ERECTION, HOLES LARGER THAN OVERSIZED MAY BE PERMITTED WITH THE USE OF STEEL PLATE WASHERS AT THE DISCRETION OF THE STRUCTURAL ENGINEER.

G. ADHESIVE/MECHANICAL ANCHORS

- ALL ADHESIVE/MECHANICAL ANCHORS SHALL BE INSTALLED, INCLUDING HOLE DRILLING AND PREPARATION, IN ACCORDANCE WITH AN APPROVED INDEPENDENT EVALUATION REPORT (ICC-ES, IAPMO, OR APPROVED EQUAL), AS INDICATED BELOW, AND IN ACCORDANCE WITH ALL MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPI).
- ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION. ADHESIVE ANCHORS SHALL NOT BE FULLY LOADED UNTIL CONCRETE HAS REACHED DESIGN STRENGTH.
- UNLESS NOTED OTHERWISE, THE ENGINEER OF RECORD, CONCRETE AND DRILLED ANCHOR HOLES SHALL BE DRY AND FREE OF WATER FOR 24 HOURS PRIOR TO ADHESIVE INSTALLATION. CONTACT THE ENGINEER OF RECORD FOR GUIDANCE IF THE CONTRACTOR CHOOSES TO INSTALL IN WET OR DAMP HOLES.
- CONCRETE TEMPERATURE AT THE TIME OF INSTALLATION SHALL BE MONITORED BY THE CONTRACTOR. CONTRACTOR SHALL COMPLY WITH ALL MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPI) RELATIVE TO SUBSTRATE TEMPERATURE.
- INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT IN ACCORDANCE WITH ACI 318-11 D.9.2.2. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO INSTALLATION. CONTINUOUS SPECIAL INSPECTION SHALL BE PROVIDED FOR THESE ANCHORS.
- UNLESS NOTED OTHERWISE, ALL ADHESIVE ANCHORS INTO CONCRETE SHALL BE:
a. HILTI HIT-RE 500/3 (ESR-3814), OR HILTI HIT-HY 200 (ESR-3187).
b. SIMPSON SET-XP (ESR-2508), OR AT-XP (ER-0283).
c. DEWALT PURE 100+ (ESR-2322), OR AC100+ GOLD (ESR-2592-COLD WEATHER).
- UNLESS NOTED OTHERWISE, ALL ADHESIVE ANCHORS INTO MASONRY SHALL BE:
a. HILTI HIT-HY-70 (ESR-2682).
b. SIMPSON SET-XP (ER-0265), OR AT-XP (ER-0281).
c. DEWALT AC100+ GOLD (ESR-3200).
- UNLESS NOTED OTHERWISE, ALL MECHANICAL ANCHORS INTO CONCRETE SHALL BE:
a. HILTI KWIK BOLT TZ (ESR-1917).
b. DEWALT POWER STUD+ SD2 (ESR-2502).
c. SIMPSON STRONG-BOLT 2 (ESR-3037).
- UNLESS NOTED OTHERWISE, ALL MECHANICAL ANCHORS INTO MASONRY SHALL BE:
a. HILTI KWIK HUS-EZ (ESR-3056).
b. SIMPSON STRONG BOLT 2 WEDGE ANCHOR (ER-0240).
c. DEWALT POWER STUD+ SD1 (ESR-2998), DEWALT SCREWBOLT+ (ESR-1678).
- UNLESS NOTED OTHERWISE, ALL SCREW ANCHORS INTO CONCRETE SHALL BE:
a. SIMPSON TITEN HD (ESR-2713).
b. DEWALT SCREWBOLT+ (ESR-2526).
c. HILTI KWIK HUS-EZ (ESR-3027).
- UNLESS NOTED OTHERWISE, ALL SCREW ANCHORS INTO MASONRY SHALL BE:
a. SIMPSON TITEN HD (ESR-1056).
b. DEWALT SCREWBOLT+ (ESR-1678).
- ALL MASONRY CELLS WITHIN 8" OF THE ANCHOR SHALL BE SOLID GROUTED.
- THE TESTING LABORATORY WILL PERFORM VISUAL INSPECTION OF ANCHORS AND DOWELS AS SPECIFIED IN THE SPECIAL INSPECTION SCHEDULE AND THE APPROVED INDEPENDENT EVALUATION REPORT. TENSION TESTING CAN BE REQUIRED AT THE DIRECTION OF THE STRUCTURAL ENGINEER OF RECORD OR THE SPECIAL INSPECTOR.
- IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON THAT HOLE AND SHIFT THE ANCHOR LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM SPACE OF (2) ANCHOR HOLE DIAMETERS OR 1 INCH, WHICHEVER IS LARGER, OF SOUND CONCRETE/MASONRY BETWEEN THE ANCHOR AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT. AT CONTRACTOR'S OPTION, LOCATE EXISTING REINFORCEMENT PRIOR TO DRILLING/CORING. IF THE ANCHOR OR DOWEL CANNOT BE SHIFTED AS NOTED ABOVE, THE ENGINEER WILL DETERMINE A NEW LOCATION.
- LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH MECHANICAL ANCHORS.
- SUBSTITUTION OF EQUIPMENT OR PRODUCTS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. SUBSTITUTION REQUESTS SHALL INCLUDE AN ICC ESR OR IAPMO REPORT AND SUPPORTING CALCULATIONS INDICATING COMPLIANCE WITH DESIGN INTENT.

H. REINFORCING STEEL

- REINFORCING BAR STRENGTH REQUIREMENTS:
a. ALL REINFORCING BARS EXCEPT AS INDICATED IN NOTE b, SHALL CONFORM TO ASTM STANDARD A-615 GRADE 60 AND ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM STANDARD A-1064 AND SHALL BE SUPPLIED IN FLAT SHEETS. ADEQUATELY TIE AND SUPPORT ALL REINFORCING STEEL AS SPECIFIED BY ACI 117, TO MAINTAIN EXACT REQUIRED POSITION.
- HEADED SHEAR STUD ASSEMBLIES SHALL CONFORM TO ASTM A1044.
- STEEL DISCONTINUOUS FIBER REINFORCEMENT SHALL BE DEFORMED AND CONFORM TO ASTM A820 AND SHALL HAVE A LENGTH TO DIAMETER RATIO NOT SMALLER THAN 40 AND NOT GREATER THAN 100.
- DEFORMED BARS SHALL CONFORM TO ASTM A603. CONSTRUCTION OF INTERSECTIONS OF THE BAR DEFORMATIONS, IF ANY, SHALL NOT EXTEND MORE THAN 2 BAR DIAMETERS FROM THE BEARING FACE OF THE HEAD.
- ALL FIELD BENT DOWELS SHALL BE GRADE 40 WITH SPACING INDICATED REDUCED BY 1/3.
- UNLESS NOTED OTHERWISE, REINFORCEMENT SHALL HAVE THE FOLLOWING CONCRETE COVERAGE :
a. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
b. EXPOSED TO EARTH OR WEATHER :
1. #6 & LARGER 2"
2. #5 & SMALLER 1-1/2"
c. NOT EXPOSED TO WEATHER OR EARTH :
1. SLABS, WALLS, JOISTS, #11 & SMALLER 3/4"
2. BEAMS, COLUMNS: MAIN REINFORCING OR TIES 1-1/2"
d. SLAB ON GRADE :
1. PLACE REINFORCING AT CENTER OF SLAB UNLESS INDICATED OTHERWISE.
- EXCEPT WHERE NOTED ON PLANS OR DETAILS CONTINUOUS REINFORCEMENT SHALL BE SPLICED AT POINTS OF MINIMUM STRESS BY LAPPING PER THE REBAR LAP SCHEDULE.
- REINFORCING STEEL MAY BE SPLICED WITH MECHANICAL COUPLERS THAT HAVE A TENSION CAPACITY OF AT LEAST 125% OF THE STRENGTH OF THE BAR. MECHANICAL COUPLERS SHALL BE A POSITIVE CONNECTING TYPE COUPLER, AND SHALL BE INSTALLED IN ACCORDANCE WITH AN APPROVED ICC RESEARCH REPORT. WHERE THESE ARE USED, SPLICES ON ADJACENT BARS SHALL BE STAGGERED AT LEAST 24 INCHES ALONG THE LENGTH OF THE BARS.
- ALL VERTICAL REINFORCING IN STRUCTURAL ELEMENTS ABOVE SHALL BE SPLICED WITH MATCHING DOWELS EMBEDDED WITHIN THE FOOTINGS OR STRUCTURE BELOW. SPLICE LENGTHS SHALL COMPLY WITH REBAR LAP SCHEDULE. DOWELS INTO FOOTINGS SHALL TERMINATE WITH A STANDARD HOOK, AND SHALL EXTEND TO WITHIN 4" OF THE BOTTOM OF THE FOOTING, BUT NEED NOT EXTEND MORE THAN 20" INTO FOOTING. FOR MASONRY CONSTRUCTION SEE STRUCTURAL NOTE K.6.A.
- DO NOT WELD REINFORCING EXCEPT AS NOTED ON PLANS, WHERE REINFORCING IS WELDED, USE ASTM A-706 REINFORCING.
- REINFORCING BARS, TIES, AND TENDONS SHALL BE SUPPORTED BY NYLON CONES, PLASTIC-COATED TIE-WIRES, OR PLASTIC-COATED CHAIRS. REINFORCING IN FOOTINGS IS PERMITTED TO BE SUPPORTED ON CONCRETE DOBIES.
- UNLESS NOTED OTHERWISE, HOOKS, STIRRUPS, TIES, AND OTHER BENDS IN REINFORCING STEEL SHALL MEET THE STANDARDS SET FORTH IN ACI 318/318R-14. UNLESS OTHERWISE PERMITTED BY THE ENGINEER, ALL REINFORCEMENT SHALL BE BENT COLD. REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT, EXCEPT AS SHOWN ON THESE DRAWINGS OR OTHERWISE PERMITTED BY THE ENGINEER.
- UNLESS SPECIFICALLY NOTED AND/OR DETAILED IN THE STRUCTURAL DRAWINGS CONDUIT SHALL NOT BE IN CONTACT WITH REINFORCING STEEL.



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PLANT

PARKER/HANNIFIN CORP.

1425 WEST 2675 NORTH OGDEN, UTAH

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MARK	DATE	DESCRIPTION
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ISSUE DATE: 26 March, 2018
PROJECT NO: 18110
ARW PROJECT NO: 18039
CAD DWG FILE:
DRAWN BY: D.Bartelson
CHK'D BY: S.Ericksen

PERMIT SET

26 Mar, 2018

SHEET TITLE

STRUCTURAL
NOTES

SHEET NO:

S001

IF SHEET IS LESS THAN 22"x 34"
IT IS A REDUCED PRINT.
REDUCE SCALE ACCORDINGLY

Structural Sheet Index	
SHEET NUMBER	SHEET NAME
S001	STRUCTURAL NOTES
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S003	SCHEDULES
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S101	FOOTING AND FOUNDATION PLAN
S102	ROOF FRAMING PLAN
S201	DETAILS
S202	DETAILS
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S401	SCHEMATIC REFERENCE

3

5

7. STRUCTURAL NOTES CONTINUED :

I. STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE FOLLOWING:
 - a. ANSI/AISC 360-10 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", WITH "COMMENTARY" AND "SUPPLEMENTS" AS REQUIRED BY BUILDING CODE.
 - b. AISI 303-10 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" EXCLUDING THE FOLLOWING SECTIONS: 4.4, 4.4.1, AND 4.4.2.
 - c. AISI "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
 - d. AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".
 - e. AWS D1.1 AND 1.3, "STRUCTURAL WELDING CODE" (EXCEPT SPECIFIC ITEMS DO NOT APPLY IF THEY CONFLICT WITH AISC).
 - f. ANSI/AISC 341-10 "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS".
2. STRUCTURAL STEEL SHALL COMPLY WITH THE FOLLOWING:
 - a. WIDE FLANGE SHAPES AND WT SHAPES - ASTM A992
 - b. OTHER SHAPES AND PLATES - ASTM A-36 (UNO)
 - c. TUBES (TS) AND HOLLOW STRUCTURAL SECTIONS (HSS) - ASTM A-500, GRADE B (SQUARE AND RECTANGULAR SHAPES $F_y = 46$ KSI) AND ROUND SHAPES $F_y = 42$ KSI)
 - d. PIPE COLUMNS - ASTM A-53, GRADE B TYPE E OR S
 - e. STAINLESS STEEL SHAPES, PLATES, AND FASTENERS - ASTM 304
 - f. DEFORMED BAR ANCHORS (DBA) - ASTM A-496, WELDED IN ACCORDANCE WITH AWS D1.1
 - g. HEADED STUD ANCHORS (HSA) - ASTM A-108, GRADE 1015 STEEL AND WELDED IN ACCORDANCE WITH AWS D1.1 FOR TYPE "B". USE 3/4" DIAMETER STUDS, UNLESS NOTED OTHERWISE.
 - h. THREADED ROD - ASTM A-449.
 - i. NON-SHRINK GROUT - ASTM C110. NON-SHRINK GROUT SHALL BE PRE-PACKAGED, NON-METALLIC, WITH A 28-DAY COMPRESSIVE STRENGTH OF 6,000 PSI.
3. CONNECTIONS SHALL COMPLY WITH THE STRUCTURAL DRAWINGS UNLESS WRITTEN APPROVAL TO CHANGE IS GIVEN BY THE STRUCTURAL ENGINEER.
4. ALL SHOP FABRICATIONS SHALL BE PERFORMED BY AN APPROVED FABRICATOR IN ACCORDANCE WITH SECTIONS 1702 AND 1704 OF THE IBC OR WITH SHOP INSPECTION BY AN INDEPENDENT AGENCY IN ACCORDANCE WITH SECTION 1704.2.5 OF THE IBC.
5. WELDING
 - a. ALL WELDING AND CUTTING SHALL BE PERFORMED BY AWS QUALIFIED WELDERS IN ACCORDANCE WITH ANSI/AWS D1.1 (LATEST EDITION).
 - b. E-70XX ELECTRODES UNLESS NOTED OTHERWISE. E-60XX MAY BE USED FOR WELDING STEEL DECK.
 - c. ALL INTERSECTING STEEL SHAPES WHICH ARE NOT CONNECTED WITH BOLTS SHALL BE WELDED TOGETHER WITH A FILLET WELD ALL AROUND UNLESS NOTED OTHERWISE. WHERE WELD SIZES ARE NOT SHOWN USE THE FOLLOWING:
 1. WHERE ALL CONNECTED PARTS ARE THICKER THAN 1/4", WELD IS 1/16" LESS THAN THE THICKNESS OF THE THINNEST PART.
 2. WHERE ANY OF THE CONNECTED PARTS IS LESS THAN 1/4" THICK, WELD IS SAME AS THICKNESS OF THE THINNEST PART.
 - d. WELDING OF ISA'S AND DBA'S SHALL CONFORM TO THE MANUFACTURER'S SPECIFICATIONS.
 - e. WHEREVER POSSIBLE, WELDS SHALL BE SHOP WELDS. SPECIAL CONSIDERATIONS, SUCH AS ITEMS WHICH MAY NEED ADJUSTMENT AT THE SITE, REQUIRE THAT SOME WELDS BE FIELD WELDS. WHERE QUESTIONS OR DISCREPANCIES OCCUR THE CONTRACTOR SHALL COORDINATE THE WORK BETWEEN THE SHOP FABRICATOR AND THE STEEL ERECTOR.
6. BOLTING
 - a. UNLESS NOTED OTHERWISE, ALL STRUCTURAL STEEL TO STEEL CONNECTIONS SHALL USE HIGH STRENGTH BOLTS CONFORMING TO ASTM A-325.
 - b. UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE CLASSIFIED AS NON-SLIP CRITICAL BEARING TYPE CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLANE. TIGHTEN BOLTS TO A SNUG TIGHT CONDITION, WITH ALL PLIES OF THE JOINT IN FIRM CONTACT.
 - c. WHERE OVERSIZED OR SLOTTED HOLES OCCUR IN THE OUTER PLY, AN ASTM F436 WASHER OR 5/16" THICK COMMON PLATE WASHER SHALL BE USED AS REQUIRED TO COMPLETELY COVER THE HOLE.
 - d. BOLTS SHALL BE CENTERED IN SLOTTED HOLES, UNLESS NOTED OTHERWISE.
 - e. WHERE A STEEL BEAM TO BEAM CONNECTION IS NOT SHOWN, PROVIDE AN AISC STANDARD FRAMED CONNECTION SIZED FOR 1/2 OF THE TOTAL LOAD CAPACITY OF THE BEAM FOR THE SPAN AND STEEL SPECIFIED.
7. METAL DECKING
 - a. UNLESS NOTED OTHERWISE, METAL ROOF DECK SHALL BE 22 GAUGE TYPE B GALVANIZED STEEL DECK. SEE ROOF DECK SCHEDULE FOR ATTACHMENTS.
 - b. ALL DECK SHALL BE CONTINUOUS OVER 3-SPANS. WHERE NOT POSSIBLE, THE DECK SUPPLIER/CONTRACTOR SHALL PROVIDE HEAVIER GAUGE DECK AS NEEDED TO PROVIDE THE EQUIVALENT PERFORMANCE OF THE SPECIFIED DECK WITH 3-SPAN CONTINUITY.
 - c. SEE TYPICAL DETAILS FOR SUPPORT OF DECK AT OPENINGS.
 - d. PROVIDE L2"x2"x3/16" FOR DECK SUPPORT AT LOCATIONS WHERE COLUMNS EXTEND THROUGH DECK.
 - e. PAINTED STEEL DECK SHALL CONFORM TO ASTM A1008 AND GALVANIZED STEEL DECK SHALL CONFORM TO A653 GRADE G90.
 - f. BUILDING ELEMENTS MAY BE SUPPORTED BY HANGING DIRECTLY FROM METAL DECKING, PROVIDED THAT THE TOTAL WEIGHT PER CONNECTION IS LESS THAN 50 LBS AND THAT THE ATTACHMENT TO THE DECKING IS DISTRIBUTED ACROSS AT LEAST TWO RIBS AND SPACED AT LEAST 6 FEET APART IN ANY DIRECTION.
8. PROVIDE FULL DEPTH WEB STIFFENER PLATES AT EACH SIDE OF STEEL BEAMS AT ALL BEARING (EXCEPT SECONDARY FRAMING) POINTS. STIFFENER PLATES SHALL BE THICKNESS SHOWN UNLESS NOTED OTHERWISE AND SHALL BE WELDED BOTH SIDES WITH FILLET WELDS ALL AROUND.

FLANGE WIDTH	STIFFENER THICKNESS	WELD THICKNESS
< 8 1/4"	1/4"	3/16"
8 1/4" < BF < 12 1/2"	3/8"	1/4"
12 1/2" < BF < 18"	1/2"	5/16"
9. FABRICATORS AND SUPPLIERS SHALL COORDINATE PAINT/FINISHES WITH REQUIREMENTS FOR DIRECT APPLIED INSULATION, FIREPROOFING, ETC. AS NOTED IN THE PROJECT SPECIFICATIONS.
10. WHEN DETERMINING THE FIRE RESISTANCE OF ASSEMBLIES, USE THE FOLLOWING: STEEL ROOF MEMBERS ARE CONSIDERED UN-RESTRAINED AND STEEL FLOOR FRAMING MEMBERS ARE CONSIDERED RESTRAINED.
11. UNLESS NOTED OTHERWISE, ALL HORIZONTAL FRAMING MEMBERS SHALL BE ERECTED WITH THE NATURAL GRAIN UP.
12. UNLESS OTHERWISE SHOWN OR DETAILED IN THE PLANS, ALL STEEL COLUMNS, BEAMS, BRACES, STRUTS, ETC. SHALL BE CONTINUOUS BETWEEN CONNECTIONS OR SUPPORTS. SPLICES IN MEMBERS SHALL NOT BE PERMITTED WITHOUT WRITTEN APPROVAL BY THE ENGINEER OF RECORD.

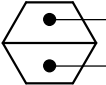
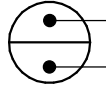





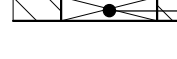




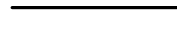
K. MASONRY

1. ALL HOLLOW MASONRY UNITS SHALL CONFORM TO ASTM C-90.
1"m (MINIMUM, FACTORED) 2,000 PSI
MINIMUM UNIT STRENGTH 1,900 PSI (TESTED IN ACCORDANCE WITH ASTM C-140)
ACCEPTABLE RANGE OF UNIT WEIGHT : 105 PCF TO 125 PCF
2. ALL GROUT (SITE MIXED OR PRE-MIXED) SHALL CONFORM TO ASTM C-476 OR SECTION 2.2A OF TMS 602-13/ACI 530.1-13/ASCE 6-13. GROUT SHALL BE PLACED WITH SUFFICIENT WATER FOR POURING WITHOUT SEGREGATION. DO NOT USE MORTAR FOR GROUT. MECHANICALLY VIBRATE ALL GROUT.
3. GROUT STOPS SHALL BE AN APPROVED PRODUCT DESIGNED AND MANUFACTURED FOR USE AS A GROUT STOP. GROUT STOP SUBMITTALS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER FOR REVIEW. OTHER GROUT STOP MATERIALS SUCH AS ASPHALT IMPREGNATED MATERIALS ARE NOT PERMITTED.
4. MORTAR SHALL BE TYPE S AND SHALL CONFORM TO ASTM C 270.
5. ALL MASONRY WORK SHALL CONFORM TO CHAPTER 21 OF THE IBC.
6. UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL 8" MASONRY WALLS SHALL BE AS FOLLOWS:
 - a. VERTICAL : #5 BARS IN CELLS ADJACENT TO ALL OPENINGS, AT CORNERS AND AT A MAXIMUM SPACING OF 32" THROUGHOUT THE WALL. ALL VERTICAL REINFORCEMENT INCLUDING, BUT NOT LIMITED TO JAMBS, COLUMNS, AND WALL REINFORCING SHALL BE DOWELED INTO AND THROUGH THE FOUNDATION WALL AND INTO THE FOOTING BELOW UNLESS SPECIFICALLY DETAILED OTHERWISE.
 - b. HORIZONTAL : (2) #4 BARS IN 8" DEEP "H" BLOCK BOND BEAM UNITS AT 48" O.C. AND AT FLOORS, ROOF AND TOP OF WALL. BOND BEAMS AT ROOF WILL SLOPE TO MATCH SLOPING ROOF.
7. ALL BLOCK CELLS CONTAINING REINFORCING, BOLTS, OR ANCHORS SHALL BE GROUTED SOLID.
8. PROVIDE (1) #5 (MINIMUM), IN GROUTED SPACE, ON ALL SIDES AND ADJACENT TO EVERY OPENING WHICH EXCEEDS 24" IN EITHER DIRECTION. HORIZONTAL BARS SHALL EXTEND 24" BEYOND THE CORNERS OF THE OPENING AND VERTICAL BARS SHALL EXTEND TO TOP OF WALL. VERTICAL REINFORCING SHALL BE PROVIDED AT ENDS, CORNERS AND EACH SIDE OF CONTROL JOINTS. SEE TYPICAL DETAILS FOR CONTROL JOINTS.
9. SOLID GROUTING OF MASONRY IS UNACCEPTABLE EXCEPT AS SPECIFICALLY NOTED ON PLANS AND SCHEDULES.
10. WHERE WALLS ARE NOT GROUTED SOLID, EACH GROUT POUR SHALL TERMINATE FLUSH WITH THE TOP OF THE UPPERMOST UNIT EXCEPT AT CELLS WITH VERTICAL REINFORCING WHERE GROUT SHALL BE 1-1/2" BELOW TOP OF UNIT TO PROVIDE CONSTRUCTION KEY. WHERE WALLS ARE GROUTED SOLID, EACH GROUT POUR SHALL TERMINATE 1-1/2" BELOW TOP OF UNIT.
11. GROUTING PROCEDURES SHALL BE AS FOLLOWS:
 - a. THE USE OF HIGH LIFT GROUTING PROCEDURES ARE REQUIRED.
 - b. THE USE OF HIGH LIFT GROUTING PROCEDURES REQUIRE THE APPROVAL OF THE ARCHITECT AND ENGINEER AND SHALL NOT EXCEED THE MAXIMUM HEIGHTS GIVEN IN TABLE 3.2.1 OF TMS 602-13/ACI 530-13/ASCE 5-13. GROUT DEMONSTRATION PANELS, AS PRESCRIBED BY THE ARCHITECT AND ENGINEER, SHALL BE REQUIRED WHERE REQUESTED GROUTING PROCEDURES DO NOT MEET THE LIMITS OF TABLE 3.2.1. ADDITIONALLY, ALL HIGH LIFT GROUTING SHALL REQUIRE SPECIAL INSPECTION PROCEDURES NEEDED TO VERIFY GROUT PLACEMENT DURING CONSTRUCTION. DURING THE CONSTRUCTION OF THE WALL, THE CONTRACTOR SHALL NOT BE ALLOWED TO MAKE ANY CHANGES IN WALL ELEVATION AND MASONRY THICKNESS. CONTROL JOINTS SHALL NOT BE LOCATED DIRECTLY OVER OR CLOSER THAN 24" TO WALL OPENINGS (DOORS, WINDOWS, MECHANICAL OPENINGS, ETC.), OR WITHIN MASONRY JAMBS.
12. REINFORCED MASONRY : 40 FT
SUSPENDED STRUCTURAL ELEMENTS.
13. HORIZONTAL REINFORCEMENT SHALL TERMINATE AT EACH SIDE OF CONTROL JOINTS EXCEPT AT FLOOR AND ROOF LEVEL BOND BEAMS AND AT TOP OF PARAPET.
14. CONTROL JOINTS SHALL BE PROVIDED AT THE MASONRY SIDE OF EMBEDDED STEEL COLUMNS TO CONTROL CRACKING OF FACE SHELLS.
15. SUPPORT NON-BEARING, NON-STRUCTURAL WALLS AT TOP OF MASONRY AS PER TYPICAL DETAILS AT LOCATIONS WHERE INTERSECTING OR PERPENDICULAR WALLS ARE 12'-0" OR MORE APART OR WHERE END OF WALL OCCURS 6'-0" OR MORE FROM INTERSECTING WALL.
16. EMBED CHANNELS AND PLATES TO BE PLACED SO AS TO CREATE FLUSH SURFACE WITH FACE OF MASONRY. FLANGES OR CHANNEL EMBEDS SHALL BE HORIZONTAL.
17. ALL VERTICAL REINFORCING SHALL BE SECURED IN THE MASONRY TO GROUTING USING WIRE POSITIONERS OR OTHER ACCEPTABLE DEVICES. REINFORCING SHALL BE SECURED AT BAR-SPICE LOCATIONS AND AT A SPACING NOT MORE THAN 120 BAR DIAMETERS.
20. UNLESS NOTED OTHERWISE, MASONRY WALLS SHALL BE CONSTRUCTED UTILIZING COMMON RUNNING-BOND WITH FULLY MORTARED BED JOINTS AROUND GROUTED CELLS.
21. MASONRY VENEER SHALL BE ANCHORED USING THE HOHMANN AND BARNARD VENEER ANCHOR ASSEMBLY SYSTEM, OR AN APPROVED EQUAL. REGARDLESS OF BACK-UP SYSTEM, PROVIDE A CONTINUOUS HORIZONTAL 9 GAUGE WIRE AT 16'-0" C. IN VENEER MORTAR JOINTS FOR ANCHOR ATTACHMENT. POSITIVE ANCHORAGE TO THE WIRE USING THE SEISMICLIP INTERLOCK SYSTEM SHALL BE PROVIDED TO SUPPORT NOT MORE THAN 2 SQUARE FEET OF WALL, WITH A HORIZONTAL SPACING NOT EXCEEDING 16".
 - a. WOOD AND METAL STUDS; USE HOHMANN AND BARNARD HB-213 S.I.S. (SEISMICLIP INTERLOCK SYSTEM) HEAVY DUTY ANCHORS OR AN APPROVED EQUAL. THE HB-213 ASSEMBLY SHALL BE ATTACHED TO WOOD STUDS USING A # 12 X 2' WOOD SCREWS OR TO METAL STUDS USING #10 STEEL BOLTS.
 - b. BRICK AND BLOCK WALLS; USE HOHMANN AND BARNARD 270-ML S.I.S. (SEISMICLIP INTERLOCK SYSTEM) MIGHTY-LOK SEISMIC ANCHORS OR AN APPROVED EQUAL, AT SPACINGS NOTED ABOVE. INSTALL A 2 WIRE 9 GAUGE LADDER TYPE JOINT REINFORCEMENT AT 16'-0" C. IN THE BACK-UP WALL FOR ANCHORAGE ATTACHMENT.
 - c. CONCRETE WALLS; USE HOHMANN AND BARNARD HB 303SV SEISMIC NOTCH DOVE TAIL ANCHOR SYSTEM OR AN APPROVED EQUAL, AT SPACINGS NOTED ABOVE.
22. ELECTRICAL CONDUIT SHALL BE PLACED IN CELLS THAT CONTAIN REBAR. CONDUIT IS ALLOWED TO PASS THROUGH REINFORCED CELLS WHEN IT OCCURS PERPENDICULAR TO THE REBAR. CONDUIT SHALL NOT CONTACT REBAR AS IT PASSES, THERE SHALL BE 1" CLEAR BETWEEN CONDUIT AND REBAR.

L. EXISTING BUILDING NOTES

1. ARW ENGINEERS EXPRESSLY DISCLAIMS RESPONSIBILITY FOR ANY PORTION OF THE EXISTING BUILDING NOT SPECIFICALLY ADDRESSED IN THESE DRAWINGS.
2. DRAWINGS AND DETAILS HAVE BEEN PREPARED TO REFLECT THE EXISTING CONDITIONS AND CONFIGURATIONS OF THE EXISTING BUILDING. THE CONTRACTOR IS ULTIMATELY RESPONSIBLE FOR VERIFYING ALL EXISTING CONDITIONS AND ALERTING THE ENGINEER OF ANY DISCREPANCIES FOUND PRIOR TO FABRICATING OR INSTALLING STRUCTURAL ELEMENTS.
3. THE CONTRACTOR IS RESPONSIBLE FOR MAKING SURE THAT THE BUILDING AND ELEMENTS WITHIN THE BUILDING REMAIN STABLE UNTIL CONSTRUCTION IS COMPLETE. AT NO ADDITIONAL COST TO THE CLIENT, THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING AND TEMPORARY SUPPORT OF STRUCTURAL MEMBERS UNTIL THE FINAL CONFIGURATION HAS BEEN COMPLETED.

LEGEND OF SYMBOLS AND ABBREVIATIONS

AB	=	ANCHOR BOLT		FOOTING MARK
ABV	=	ABOVE		
ARCH	=	ARCHITECT		
BLW	=	BELOW		
CL	=	CENTERLINE		
CMU	=	CONCRETE MASONRY UNIT		SECTION MARK
COL	=	COLUMN		SHEET NUMBER
CONC	=	CONCRETE		
CP	=	CONCRETE PIER		
DIA / Ø	=	DIAMETER		
DBA	=	DEFORMED BAR ANCHOR		
DBE	=	DECK BEARING ELEVATION		
ELEV	=	ELEVATION		FOOTING STEP
EOD	=	EDGE OF DECK		
FDN	=	FOUNDATION		MASONRY WALL
FTG	=	FOOTING		MASONRY WALL W/ CONCRETE FOUNDATION BELOW
FFE	=	FINISHED FLOOR ELEVATION		
GB	=	CONCRETE GRADE BEAM		DEPRESS FDN. WALL AND POUR FLOOR SLAB OVER AT CONCRETE FOUNDATION WALL
HSA	=	HEADED STUD ANCHOR		
KB	=	KICKER BRACE		MASONRY BEAM
MAX	=	MAXIMUM		
MB	=	MASONRY BEAM		
MC	=	MASONRY COLUMN		
MECH	=	MECHANICAL		
MEZZ	=	MEZZANINE		
MIN	=	MINIMUM		
MJ	=	MASONRY JAMB		ELEVATION
MW	=	MASONRY WALL		FRAMING ANGLE SEE TYPICAL DETAIL
NS, FS	=	NEAR SIDE, FAR SIDE		
OAE	=	OR APPROVED EQUAL		FRAMING CHANNEL SEE TYPICAL DETAIL
OPP	=	OPPOSITE		
PAF	=	POWDER ACTUATED FASTENER		
PL	=	PLATE		
REINF	=	REINFORCING		
REQ'D	=	REQUIRED		
SIM	=	SIMILAR		ITEMS, DETAILS, & SYSTEMS WHICH ARE PART OF THE LATERAL FORCE RESISTING SYSTEM.
TOC	=	TOP OF CONCRETE SLAB		
TOF	=	TOP OF FOOTING		
TOM	=	TOP OF MASONRY		
TYP	=	TYPICAL		
UNO	=	UNLESS NOTED OTHERWISE		KICKER BRACE



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ISSUE DATE: 26 March, 2018

PROJECT NO:	18110
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ARW PROJECT NO: 18039

CAD DWG FILE:

DRAWN BY: Author

CHK'D BY: S. Ericksen

PERMIT SET

26 Mar, 2018

SHEET TITLE

STRUCTURAL NOTES

SHEET NO:

S002

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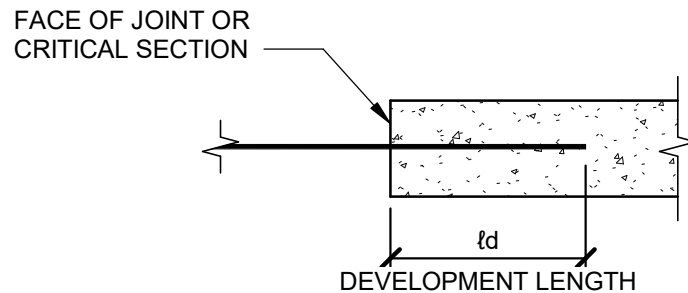
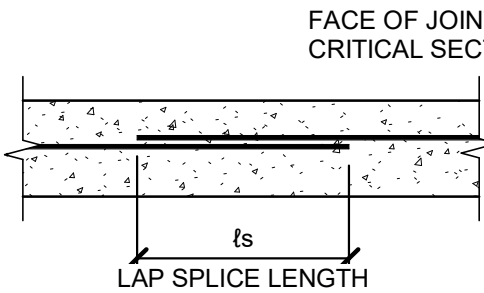
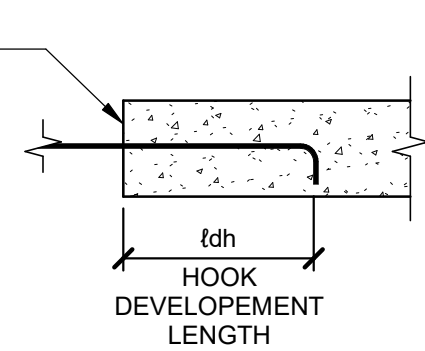
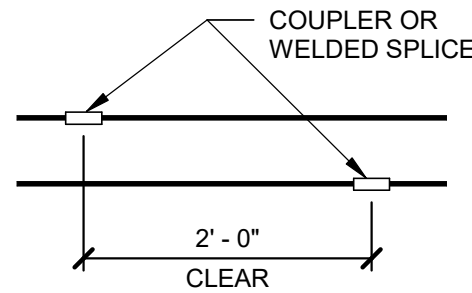
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ROOF DECK SCHEDULE												
AREA	DECK			ATTACHMENT					MIN. SHEAR CAPACITY		MAX. FLEXIBILITY FACTOR	
	DEPTH	TYPE	GA.	SUPPORTS DIA. WELD	SUPPORTS PATTERN	SIDE SEAMS #12 TEK SCREWS	SIDE SEAMS TOP SEAM WELD	SIDE SEAMS PUNCH LOCK	SUPPORTS PARALLEL TO FLUTES Ø WELD	SUPPORTS PARALLEL TO FLUTES SPA		
A	1 1/2	B	22	3/4	36/7	@24	---	---	3/4	6	329 PLF	14.1
FASTENING PATTERNS												
 WELD PATTERN : 36/7												
NOTES : 1. TOP SEAM WELDS SHALL BE 1-1/2" LONG AND SHALL BE ACCORDING TO SDI STANDARDS. 2. USE NESTABLE (OVERLAPPING) SIDE SEAMS AT SCREW ATTACHMENTS AND INTERLOCKING SIDE SEAMS AT WELDS. 3. IF N DECK IS NOT NESTABLE, N DECK END BUTT JOINTS OVER STEEL JOISTS SHALL USE 16 GA. x 6" CONTINUOUS SHEET BETWEEN DECK AND JOIST TOP CHORD ANGLES. DECK WELDS TO PENETRATE SHEET AND ENGAGE JOIST CHORD. 4. ALL DECK WITH A PROFILE DEPTH OF 2" OR LESS SHALL HAVE NESTED OR TELESOPED END LAPS. 5. SUBMIT CURRENT ICC APPROVAL FOR ALL DECKS. 6. ALTERNATE SYSTEMS SHALL MEET OR EXCEED THE MINIMUM SHEAR CAPACITY AND SHALL PROVIDE LESS THAN OR EQUAL TO THE MAXIMUM FLEXIBILITY FACTOR LISTED IN THE SCHEDULE. 7. ALL ALTERNATE SYSTEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL.												
 KEY PLAN												

2015 IBC CONC. REBAR LAP SPLICE SCHEDULE																															
FOR CONCRETE APPLICATIONS (ACI 318 - 14)																															
<div><div></div><div></div><div></div><div></div></div>																															
BAR LOCATION	CONCRETE REINFORCING & SPLICE LENGTHS (IN)																														
	CONCRETE		BAR SIZE																												
	TYPE	STRENGTH	#3			#4			#5			#6			#7			#8			#9			#10			#11			COMMENTS	
		td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh			
VERT. WALL BARS, FILL ON METAL DECK	NWC	3000 PSI	17	22	8	22	29	8	28	36	10	33	43	12	48	62	13	55	72	15	62	17	69	19	76	30					
HORIZ. WALL BARS, FOOTING TOP BARS	NWC	3000 PSI	17	22	8	22	29	8	28	36	10	33	43	12	48	62	13	55	72	15	62	17	69	19	76	30					
BEAM BOTTOM BARS, COLUMN BARS	NWC	3000 PSI	17	22	8	22	29	11	28	36	14	33	43	16	48	62	19	55	72	22	62	25	69	27	76	30					
FOOTING BOTTOM BARS	NWC	3000 PSI	12	16	8	14	18	8	17	22	10	20	26	12	29	38	13	33	43	15	37	17	42	19	46	30					
BEAM TOP BARS	NWC	3000 PSI	22	29	8	29	38	11	36	47	14	43	56	16	63	82	19	72	94	22	81	25	90	27	98	30					
SLAB ON GRADE	NWC	3000 PSI	12	16	8	14	18	8	17	22	10	20	26	12	32	42	13	42	55	15	53	17	69	19	76	30					
BAR LOCATION	CONCRETE REINFORCING & SPLICE LENGTHS (IN)																														
	CONCRETE		BAR SIZE																												
	TYPE	STRENGTH	#3			#4			#5			#6			#7			#8			#9			#10			#11			COMMENTS	
		td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh			
VERT. WALL BARS, FILL ON METAL DECK	NWC	4000 PSI	15	20	7	19	25	7	24	31	8	29	38	10	42	55	12	48	62	13	54	15	60	17	66	26					
HORIZ. WALL BARS, FOOTING TOP BARS	NWC	4000 PSI	15	20	7	19	25	7	24	31	8	29	38	10	42	55	12	48	62	13	54	15	60	17	66	26					
BEAM BOTTOM BARS, COLUMN BARS	NWC	4000 PSI	15	20	7	19	25	9	24	31	12	29	38	14	42	55	17	48	62	19	54	21	60	24	66	26					
FOOTING BOTTOM BARS	NWC	4000 PSI	12	16	7	12	16	7	15	20	8	18	23	10	25	33	12	29	38	13	33	15	36	17	40	26					
BEAM TOP BARS	NWC	4000 PSI	19	25	7	25	33	9	31	40	12	37	48	14	54	70	17	62	81	19	70	21	78	24	85	26					
SLAB ON GRADE	NWC	4000 PSI	12	16	7	12	16	7	15	20	8	18	23	10	28	36	12	36	47	13	46	15	60	17	66	26					
BAR LOCATION	CONCRETE REINFORCING & SPLICE LENGTHS (IN)																														
	CONCRETE		BAR SIZE																												
	TYPE	STRENGTH	#3			#4			#5			#6			#7			#8			#9			#10			#11			COMMENTS	
		td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh	td	ts	tdh			
VERT. WALL BARS, FILL ON METAL DECK	NWC	4500 PSI	14	18	7	18	23	6	23	30	8	27	35	9	40	52	11	45	59	13	51	14	56	16	62	25					
HORIZ. WALL BARS, FOOTING TOP BARS	NWC	4500 PSI	14	18	7	18	23	6	23	30	8	27	35	9	40	52	11	45	59	13	51	14	56	16	62	25					
BEAM BOTTOM BARS, COLUMN BARS	NWC	4500 PSI	14	18	7	18	23	9	23	30	11	27	35	13	40	52	16	45	59	18	51	20	56	22	62	25					
FOOTING BOTTOM BARS	NWC	4500 PSI	12	16	7	12	16	6	14	18	8	17	22	9	24	31	11	27	35	13	31	14	34	16	37	25					
BEAM TOP BARS	NWC	4500 PSI	18	23	7	24	31	9	30	39	11	35	46	13	51	66	16	59	77	18	66	20	73	22	80	25					
SLAB ON GRADE	NWC	4500 PSI	12	16	7	12	16	6	14	18	8	17	22	9	27	35	11	34	44	13	44	14	56	16	62	25					
<div>NOTES :</div> <div><div>1. MECHANICAL COUPLERS MAY BE USED IN LIEU OF LAP SPLICES SHOWN. SEE STRUCTURAL NOTES FOR MINIMUM COUPLER CAPACITY. WHERE MECHANICAL COUPLERS ARE USED, STAGGER ADJACENT SPLICES A MINIMUM OF 24" AS INDICATED ABOVE.</div><div>2. DEVELOPMENT LENGTHS SHALL BE INCREASED BY 50% FOR STRAIGHT BAR DEVELOPMENT AND 20% FOR HOOKED BARS WHERE EPOXY COATING IS USED.</div><div>3. WHEN SPLICING BARS OF DIFFERENT SIZES, USE LAP SPLICE LENGTH OF LARGER BARS UNO.</div><div>4. SPLICE BARS LARGER THAN #11 USING MECHANICAL COUPLERS.</div></div>																															

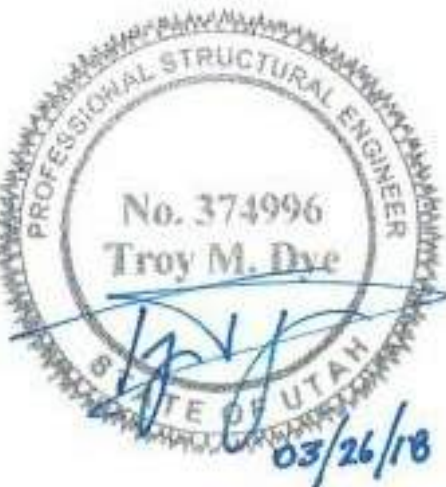
IF SHEET IS LESS THAN 22"x 34"
IT IS A REDUCED PRINT.
REDUCE SCALE ACCORDINGLY



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ADDITION TO THE
CONTROL SYSTEMS DIV.
PLANT
PARKER/HANNIFIN CORP.
1425 WEST 2675 NORTH OGDEN, UTAH

MARK DATE DESCRIPTION

ISSUE DATE: 26 March, 2018
PROJECT NO: 18110
ARW PROJECT NO: 18039
CAD DWG FILE:
DRAWN BY: D.Bartelson
CHK'D BY: S. Ericksen

PERMIT SET

26 Mar, 2018


SHEET TITLE

SCHEDULES

SHEET NO:

S003


1	2	3	4	5
STRUCTURAL STEEL SPECIAL INSPECTION SCHEDULE				
ESTABLISHED PER 2015 IBC SECTION 1705.2.1				
INSPECTION TASKS PRIOR TO WELDING (TABLE N5.4-1)		FABRICATOR QUALITY CONTROL		SPECIAL INSPECTOR QUALITY ASSURANCE
		CONTINUOUS	PERIODIC	CONTINUOUS PERIODIC
WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE		●		●
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE		●		●
MATERIAL IDENTIFICATION (TYPE / GRADE)			●	●
WELDER IDENTIFICATION SYSTEM ¹			●	●
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)				
* JOINT PREPARATION				
* DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)				
* CLEANLINESS (CONDITION OF STEEL SURFACES)			●	●
* TACKING (TACK WELD QUALITY AND LOCATION)				
* BACKING TYPE AND FIT (IF APPLICABLE)				
CONFIGURATION AND FINISH OF ACCESS HOLES			●	●
FIT-UP OF FILLET WELDS				
* DIMENSIONS (ALIGNMENT, GAPS AT ROOT)			●	●
* CLEANLINESS (CONDITION OF STEEL SURFACES)				
* TACKING (TACK WELD QUALITY AND LOCATION)				
CHECK WELDING EQUIPMENT			●	
¹ THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.				
INSPECTION TASKS DURING WELDING (TABLE N5.4-2)		CONTINUOUS	PERIODIC	CONTINUOUS PERIODIC
USE OF QUALIFIED WELDERS			●	●
CONTROL AND HANDLING OF WELDING CONSUMABLES				
* PACKAGING			●	●
* EXPOSURE CONTROL				
NO WELDING OVER CRACKED TACK WELDS			●	●
ENVIRONMENTAL CONDITIONS				
* WIND SPEED WITHIN LIMITS			●	●
* PRECIPITATION AND TEMPERATURE				
WPS FOLLOWED				
* SETTINGS ON WELDING EQUIPMENT				
* TRAVEL SPEED				
* SELECTED WELDING MATERIALS			●	●
* SHIELDING GAS TYPE / FLOW RATE				
* PREHEAT APPLIED				
* INTERPASS TEMPERATURE MAINTAINED (MIN. / MAX)				
* PROPER POSITION (F, V, H, OH)				
WELDING TECHNIQUES				
* INTERPASS AND FINAL CLEANING				
* EACH PASS WITHIN PROFILE LIMITATIONS			●	●
* EACH PASS MEETS QUALITY REQUIREMENTS				
INSPECTION TASKS AFTER WELDING (TABLE N5.4-3)		CONTINUOUS	PERIODIC	CONTINUOUS PERIODIC
WELDS CLEANED			●	●
SIZE, LENGTH AND LOCATION OF WELDS		●		●
WELDS MEET VISUAL ACCEPTANCE CRITERIA				
* CRACK PROHIBITION				
* WELD / BASE-METAL FUSION		●		●
* CRATER CROSS SECTION				
* WELD PROFILES				
* WELD SIZE				
* UNDERCUT				
* POROSITY				
ARC STRIKES		●		●
K-AREA ¹		●		●
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)		●		●
REPAIR ACTIVITIES		●		●
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER		●		●
¹ WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. (75mm) OF THE WELD				
GENERAL STEEL SPECIAL INSPECTION NOTES :				
1. QUALITY ASSURANCE (QA) INSPECTION OF FABRICATED ITEMS SHALL BE MADE AT THE FABRICATOR'S PLANT. THE QUALITY ASSURANCE INSPECTOR (QAI) SHALL SCHEDULE THIS WORK TO MINIMIZE INTERRUPTION TO THE WORK OF THE FABRICATOR.				
2. QA INSPECTION OF THE ERECTED STEEL SYSTEM SHALL BE MADE AT THE PROJECT SITE. THE QAI SHALL SCHEDULE THIS WORK TO MINIMIZE INTERRUPTION TO THE WORK OF THE ERECTOR.				
3. WHERE A TASK IS NOTED TO BE PERFORMED BY BOTH QC AND QA, IT IS PERMITTED TO COORDINATE THE INSPECTION FUNCTION BETWEEN THE QCI AND QAI SO THAT THE INSPECTION FUNCTIONS ARE PERFORMED BY ONLY ONE PARTY. WHERE QA RELIES UPON INSPECTION FUNCTIONS PERFORMED BY QC, THE APPROVAL OF THE ENGINEER OF RECORD AND THE AUTHORITY HAVING JURISDICTION IS REQUIRED.				
4. THE FABRICATOR'S QCI SHALL INSPECT THE FABRICATED STEEL TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE SHOP DRAWINGS, SUCH AS PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. THE ERECTOR'S QCI SHALL INSPECT THE ERECTED STEEL FRAME TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE ERECTION DRAWINGS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.				
5. THE QAI SHALL BE ON THE PREMISES FOR INSPECTION DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. AS A MINIMUM, THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE VERIFIED PRIOR TO PLACEMENT OF THE CONCRETE.				
6. THE QAI SHALL INSPECT THE FABRICATED STEEL OR ERECTED STEEL FRAME, AS APPROPRIATE, TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.				
7. QUALITY ASSURANCE (QA) INSPECTIONS, EXCEPT NONDESTRUCTIVE TESTING (NDT), MAY BE WAIVED WHEN THE WORK IS PERFORMED IN A FABRICATING SHOP OR BY AN ERECTOR APPROVED BY THE AUTHORITY HAVING JURISDICTION (AHJ) TO PERFORM THE WORK WITHOUT QA. NDT OF WELDS COMPLETED IN AN APPROVED FABRICATOR'S SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE AHJ. WHEN THE FABRICATOR PERFORMS THE NDT, THE QA AGENCY SHALL REVIEW THE FABRICATOR'S NDT REPORTS.				
8. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE FABRICATOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.				
9. IDENTIFICATION AND REJECTION OF MATERIAL OR WORKMANSHIP THAT IS NOT IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS, SHALL BE PERMITTED AT ANY TIME DURING THE PROGRESS OF THE WORK. HOWEVER, THIS PROVISION SHALL NOT RELIEVE THE OWNER OR THE INSPECTOR OF THE OBLIGATION FOR TIMELY, IN-SEQUENCE INSPECTIONS. NONCONFORMING MATERIAL AND WORKMANSHIP SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE FABRICATOR OR ERECTOR, AS APPLICABLE.				
10. NONCONFORMING MATERIAL OR WORKMANSHIP SHALL BE BROUGHT INTO CONFORMANCE, OR MADE SUITABLE FOR ITS INTENDED PURPOSE AS DETERMINED BY THE ENGINEER OF RECORD.				
11. CONCURRENT WITH THE SUBMITTAL OF SUCH REPORTS TO THE AHJ, EOR OR OWNER, THE QA AGENCY SHALL SUBMIT TO THE FABRICATOR AND ERECTOR:				
(1) NONCONFORMANCE REPORTS				
(2) REPORTS OF REPAIR, REPLACEMENT OR ACCEPTANCE OF NONCONFORMING ITEMS.				
INSPECTION TASKS PRIOR TO BOLTING (TABLE N5.6-1)		CONTINUOUS	PERIODIC	CONTINUOUS PERIODIC
MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS			●	●
FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS			●	●
PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)			●	●
PROPER BOLTING PROCEDURES SELECTED FOR JOINT DETAIL			●	●
CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS			●	●
PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED		●		●
PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS			●	●
INSPECTION TASKS DURING BOLTING (TABLE N5.6-2)		CONTINUOUS	PERIODIC	CONTINUOUS PERIODIC
FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED			●	●
JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION			●	●
FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING			●	●
FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES			●	●
INSPECTION TASKS AFTER BOLTING (TABLE N5.6-3)		CONTINUOUS	PERIODIC	CONTINUOUS PERIODIC
DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS		●		●
INSPECTION OF STEEL ELEMENTS OF COMPOSITE CONSTRUCTION PRIOR TO CONCRETE PLACEMENT (TABLE N6.1)		CONTINUOUS	PERIODIC	CONTINUOUS PERIODIC
PLACEMENT AND INSTALLATION OF STEEL DECK		●		●
PLACEMENT AND INSTALLATION OF STEEL STUD ANCHORS		●		●
DOCUMENT ACCEPTANCE OR REJECTION OF STEEL ELEMENTS		●		●
NOTES				
1. PERIODIC - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.				
2. CONTINUOUS - PERFORM THESE TASKS FOR EACH BOLTED CONNECTION.				
3. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR.				
4. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), APPLICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE, EXCEPT AS PERMITTED IN ACCORDANCE WITH SECTION N7.				
5. FOR SNUG-TIGHT JOINTS, PRE-INSTALLATION VERIFICATION TESTING AS SPECIFIED IN TABLE N5.6-1 AND MONITORING OF THE INSTALLATION PROCEDURES AS SPECIFIED IN TABLE N5.6-2 ARE NOT APPLICABLE. THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS IN SNUG-TIGHT JOINTS.				
6. FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE TURN-OF-NUT METHOD WITH MATCHMARKING TECHNIQUES, THE DIRECT-TENSION-INDICATOR METHOD, OR THE TWIST-OFF-TYPE TENSION CONTROL BOLT METHOD, MONITORING OF BOLT PRETENSIONING PROCEDURES SHALL BE AS SPECIFIED IN TABLE N5.6-2. THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS WHEN THESE METHODS ARE USED BY THE INSTALLER.				
7. FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE CALIBRATED WRENCH METHOD OR THE TURN-OF-NUT METHOD WITHOUT MATCHMARKING, MONITORING OF BOLT PRETENSIONING PROCEDURES SHALL BE AS SPECIFIED IN TABLE N5.6-2. THE QCI AND QAI SHALL BE ENGAGED IN THEIR ASSIGNED INSPECTION DUTIES DURING INSTALLATION OF FASTENERS WHEN THESE METHODS ARE USED BY THE INSTALLER.				
8. OBSERVATION OF BOLTING OPERATIONS SHALL BE THE PRIMARY METHOD USED TO CONFIRM THAT THE MATERIALS, PROCEDURES AND WORKMANSHIP INCORPORATED IN CONSTRUCTION ARE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS AND THE PROVISIONS OF THE RCSC SPECIFICATION.				
NOTES				
1. O - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.				
2. P - PERFORM THESE TASKS FOR EACH BOLTED CONNECTION.				
3. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR.				
4. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), APPLICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE, EXCEPT AS PERMITTED IN ACCORDANCE WITH SECTION N7.				
5. FOR THOSE ITEMS FOR QUALITY CONTROL (QC) THAT CONTAIN AN OBSERVE DESIGNATION, THE QC INSPECTION SHALL BE PERFORMED BY THE ERECTOR'S QUALITY CONTROL INSPECTOR (QCI).				
6. FOR WELDING OF STEEL HEADED STUD ANCHORS, THE PROVISIONS OF AWS D1.1 / D1.1M, APPLY.				
7. FOR WELDING OF STEEL DECK, OBSERVATION OF WELDING OPERATIONS AND VISUAL INSPECTION OF IN-PROCESS AND COMPLETED WELDS SHALL BE THE PRIMARY METHOD TO CONFIRM THAT THE MATERIALS, PROCEDURES AND WORKMANSHIP ARE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS. ALL APPLICABLE PROVISIONS OF AWS D1.3 / D1.3M, STRUCTURAL WELDING CODE - SHEET STEEL, SHALL APPLY. DECK WELDING INSPECTION SHALL INCLUDE VERIFICATION OF THE WELDING CONSUMABLES, WELDING PROCEDURE SPECIFICATIONS AND QUALIFICATIONS OF WELDING PERSONNEL PRIOR TO THE START OF THE WORK, OBSERVATIONS OF THE WORK IN PROGRESS, AND A VISUAL INSPECTION OF ALL COMPLETED WELDS. FOR STEEL DECK ATTACHED BY FASTENING SYSTEMS OTHER THAN WELDING, INSPECTION SHALL INCLUDE VERIFICATION OF THE FASTENERS TO BE USED PRIOR TO THE START OF THE WORK, OBSERVATIONS OF THE WORK IN PROGRESS TO CONFIRM INSTALLATION IN CONFORMANCE WITH THE MANUFACTURER'S RECOMMENDATIONS, AND A VISUAL INSPECTION OF THE COMPLETED INSTALLATION.				




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ADDITION TO THE
CONTROL SYSTEMS DIV.
PLANT
PARKER/HANNIFIN CORP.
1425 WEST 2675 NORTH OGDEN, UTAH

MARK	DATE	DESCRIPTION

ISSUE DATE: 26 March, 2018
PROJECT NO: 18110
ARW PROJECT NO: 18039
CAD DWG FILE:
DRAWN BY: D.Bartelson
CHK'D BY: S.Ericksen

PERMIT SET
26 Mar, 2018

SHEET TITLE
A
SCHEDULES

SHEET NO:
S004

D

C

B

A

BEAM CONNECTION SCHEDULE							
BEAM DEPTH	SHEAR PLATE INFORMATION			BOLTS W/ STANDARD WASHERS OVER SLOTS		WELD 'A'	COMMENTS
	PL. DIMENSIONS W/ SHORT-SLOTTED HOLES	Lev	Leh	No.	SIZE		
W8 x, W10 x	PL. 1/4" x 4"	1 1/2"	2"	2	3/4" Ø	3/16"	
W12 x	PL. 5/16" x 4"	1 1/2"	2"	3	3/4" Ø	1/4"	
W14 x 90 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	3	3/4" Ø	1/4"	
W16 x 77 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	4	3/4" Ø	1/4"	
W18 x 65 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	5	3/4" Ø	1/4"	
W21 x 73 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	6	3/4" Ø	1/4"	
W24 x 94 & LIGHTER	PL. 3/8" x 4"	1 1/2"	2"	7	7/8" Ø	1/4"	
W27 x 114 & LIGHTER	PL. 3/8" x 4"	1 1/2"	2"	7	7/8" Ø	1/4"	
W30 x 124 & LIGHTER	PL. 1/2" x 4"	1 3/4"	2"	8	1" Ø	5/16"	
W33 x 130 & LIGHTER	PL. 1/2" x 4"	1 3/4"	2"	9	1" Ø	5/16"	
W36 x 160 & LIGHTER	PL. 1/2" x 4 1/2"	2"	2 1/4"	10	1-1/8" Ø	5/16"	

FACE OF COLUMN / BEAM WEB

1 1/2"

1/2"

Leh

Lev

3" TYP.

Lev

SHEAR PL. - SEE SCHEDULE

SHORT-SLOTTED HOLES IN SHEAR PLATE W/ WASHER PER THE STRUCTURAL NOTES

W BEAM - SEE PLAN

'A'

SEE SCHED.

FOOTING SCHEDULE								
MARK	WIDTH	LENGTH	THICK	LENGTHWISE REINF.		CROSSWISE REINF.		REMARKS
				NO.	SIZE	NO.	SIZE	
FC2	2'-0"	CONT.	12"	(2)	#5	--	--	

3" CLEAR

EQ.

EQ.

EQ.

3" CLEAR

3" CLEAR

TYP. FOOTING REINFORCING

TYP. FOOTING SECTION

2015 IBC MASONRY REBAR LAP SPLICE SCHEDULE										
FOR MASONRY APPLICATIONS (ACI 530 - 13)										
<div><p>CASE #1 = SINGLE BAR, CENTERED IN CELL</p><p>CASE #2 = WHEN REINFORCING BAR IS PLACED ADJACENT TO FACE SHELL</p></div>										
BAR LOCATION	MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 1500psi)									
	BAR SIZE									
	#3		#4		#5		#6		#7	
	CASE #		CASE #		CASE #		CASE #		CASE #	
BEAM / WALL HORIZONTAL	19"		26"		32"		38"		45"	
WALL VERTICAL COLUMN AND JAMB	12"	16"	14"	29"	22"	45"	43"	54"	59"	63"
BAR LOCATION	MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 2000psi)									
	BAR SIZE									
	#3		#4		#5		#6		#7	
	CASE #		CASE #		CASE #		CASE #		CASE #	
BEAM / WALL HORIZONTAL	19"		26"		32"		38"		45"	
WALL VERTICAL COLUMN AND JAMB	12"	14"	12"	25"	19"	40"	37"	54"	51"	63"
BAR LOCATION	MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 2500psi)									
	BAR SIZE									
	#3		#4		#5		#6		#7	
	CASE #		CASE #		CASE #		CASE #		CASE #	
BEAM / WALL HORIZONTAL	19"		26"		32"		38"		45"	
WALL VERTICAL COLUMN AND JAMB	12"		12"	22"	17"	36"	33"	54"	46"	63"
NOTES : 1. MECHANICAL COUPLERS MAY BE USED IN LIEU OF LAP SPLICES SHOWN. SEE STRUCTURAL NOTES FOR MINIMUM COUPLER CAPACITY. WHERE MECHANICAL COUPLERS ARE USED, STAGGER ADJACENT SPLICES A MINIMUM OF 24" AS INDICATED ABOVE. 2. DEVELOPMENT LENGTHS SHALL BE INCREASED BY 50% WHERE EPOXY COATED REBAR IS USED. 3. WHEN SPLICING BARS OF DIFFERENT SIZES, USE LAP SPLICE LENGTH OF LARGER BARS UNO. 4. ALL REBAR #8 AND LARGER IN MASONRY SHALL BE SPLICED USING MECHANICAL SPLICES. SEE STRUCTURAL NOTES FOR MINIMUM COUPLER CAPACITY.										

BEAM INTO MASONRY (CHANNEL) SCHEDULE						
<div><p>TYP. BOTH ANGLES SEE SCHED</p><p>WELD @ BOTTOM, TOE, AND 1/2" RETURN ON TOP</p><p>A325 BOLTS AS PER SCHEDULE W/ WASHERS THAT COMPLETELY COVER THE SLOTS PER THE STRUCTURAL NOTES (DO NOT USE TC BOLTS)</p><p>W BEAM - SEE PLAN</p><p>(2) L 3 x 5 W/ 1 1/2" SLOTTED HOLES SEE SCHEDULE</p><p>CHANNEL EMBED W/ 3/4"DIA x 5" H.S.A.'s SEE SCHED.</p><p>2 1/4"</p><p>1/2"</p><p>MASONRY WALL</p><p>2" TYP. EQUAL SPACING</p><p>EMBED CHANNEL</p><p>TYP. H.S.A. PATTERN LAYOUT</p></div>						
BEAM SIZE	ANGLE SIZE (EA. SIDE)	WELD 'A'	BOLTS	EMBED CHANNEL	# OF H.S.A.	H.S.A. PATTERN
W6, W8, C8	3 x 5 x 1/4" x 0'-4"	3/16"	(2) 3/4"Ø	C8 x 13.75 x 0'-8"	4	::
W10, C10	3 x 5 x 5/16" x 0'-7"	1/4"	(2) 3/4"Ø	C12 x 20.7 x 1'-4"	6	:::
W12, C12	3 x 5 x 5/16" x 0'-9"	1/4"	(3) 3/4"Ø	C12 x 20.7 x 1'-4"	6	:::
W14	3 x 5 x 5/16" x 0'-10"	1/4"	(3) 3/4"Ø	C15 x 33.9 x 1'-4"	9	:::
W16	3 x 5 x 5/16" x 1'-0"	1/4"	(4) 3/4"Ø	C15 x 33.9 x 2'-0"	12	:::
W18	3 x 5 x 5/16" x 1'-3"	1/4"	(5) 3/4"Ø	(2) C12 x 20.7 x 2'-0"	12	:::
1. AT THE CONTRACTORS OPTION, (2) C8 x 13.7 MAY BE SUBSTITUTED FOR THE C15 x 33.9. HSA PATTERN SHALL BE RECONFIGURED & EQUALLY DISTRIBUTED BETWEEN THE TWO CHANNELS. 2. DOUBLE CHANNELS SHALL BE WELDED TOGETHER WITH A 2" STITCH WELD EVERY 4" AT THE FLANGE TOE.						

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ADDITION TO THE
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PLANT
PARKER/HANNIFIN CORP.
1425 WEST 2675 NORTH OGDEN, UTAH

MARK	DATE	DESCRIPTION
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ISSUE DATE: 26 March, 2018
PROJECT NO: 18110
ARW PROJECT NO: 18039
CAD DWG FILE:
DRAWN BY: D.Bartelson
CHK'D BY: S. Ericksen

PERMIT SET

26 Mar, 2018

SHEET TITLE

SCHEDULES

SHEET NO:

S005

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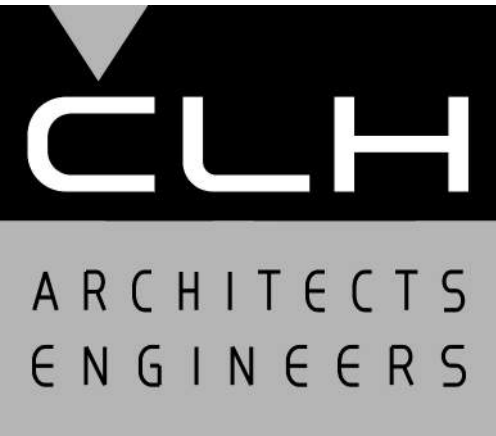
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SHEET TITLE

SCHEDULES

SHEET NO:

S006

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ARW PROJECT NO:	18039
CAD DWG FILE:	
DRAWN BY:	Author
CHK'D BY:	S. Ericksen

PERMIT SET

26 Mar, 2018

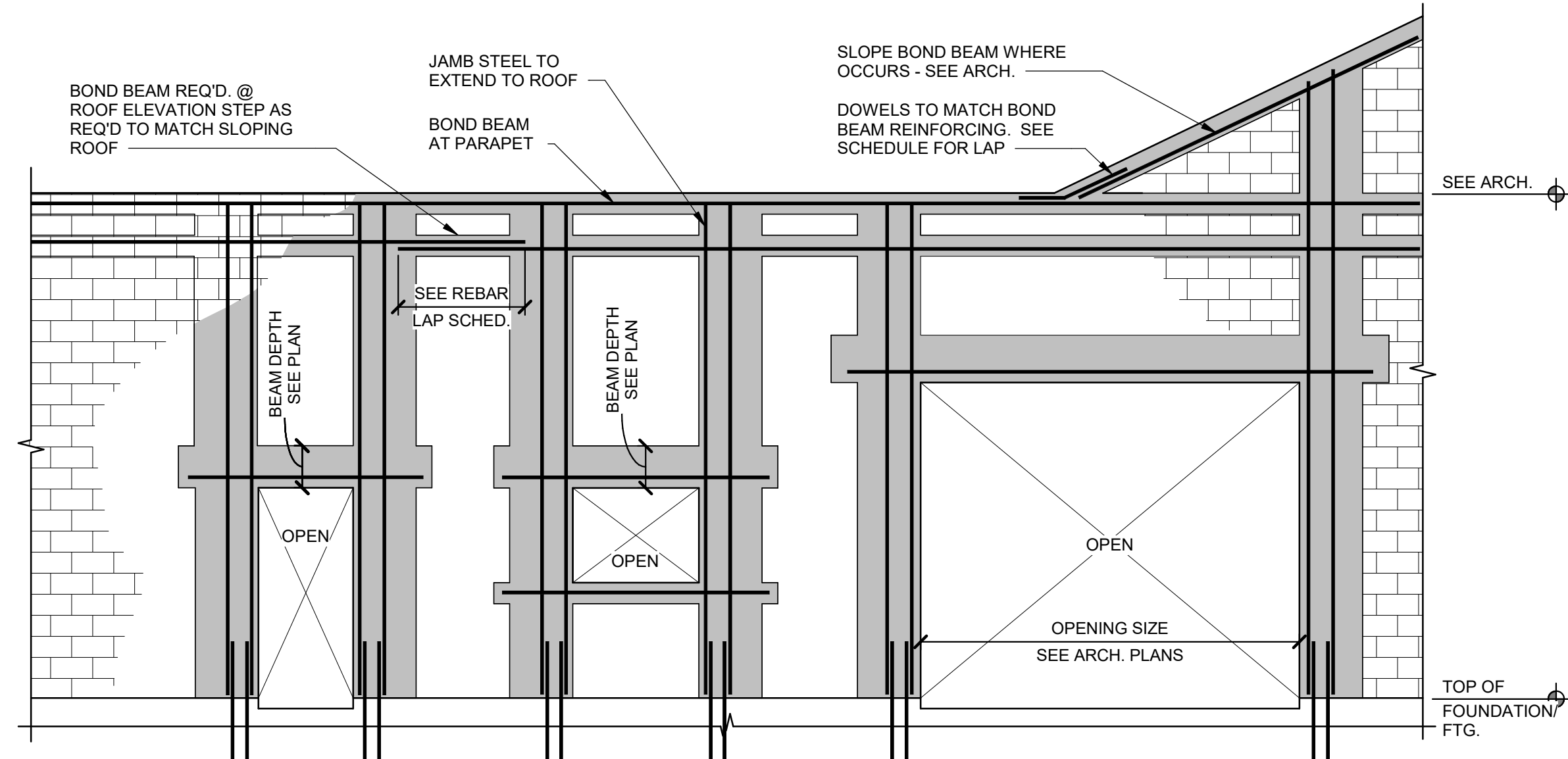
SHEET TITLE

SCHEDULES

SHEET NO:

S007

TYPICAL MASONRY / JAMB REINFORCING SCHEDULE



NOTES:

1. USE OPEN-END UNITS AT INTERSECTIONS OF BEAMS AND JAMBS.
2. TYPICAL HORIZONTAL BOND BEAMS MAY BE ADJUSTED UP OR DOWN BY ONE COURSE PROVIDED THE OVERALL NUMBER OF REQUIRED BOND BEAMS ARE INSTALLED.
3. TYPICAL HORIZONTAL AND VERTICAL WALL REINFORCING NOT SHOWN FOR CLARITY, SEE PLAN AND SCHEDULE FOR TYPICAL WALL REINFORCING.
4. JAMB REINFORCING SHOWN IS SCHEMATIC. SEE SCHEDULE & DETAILS FOR ACTUAL JAMB REINF.

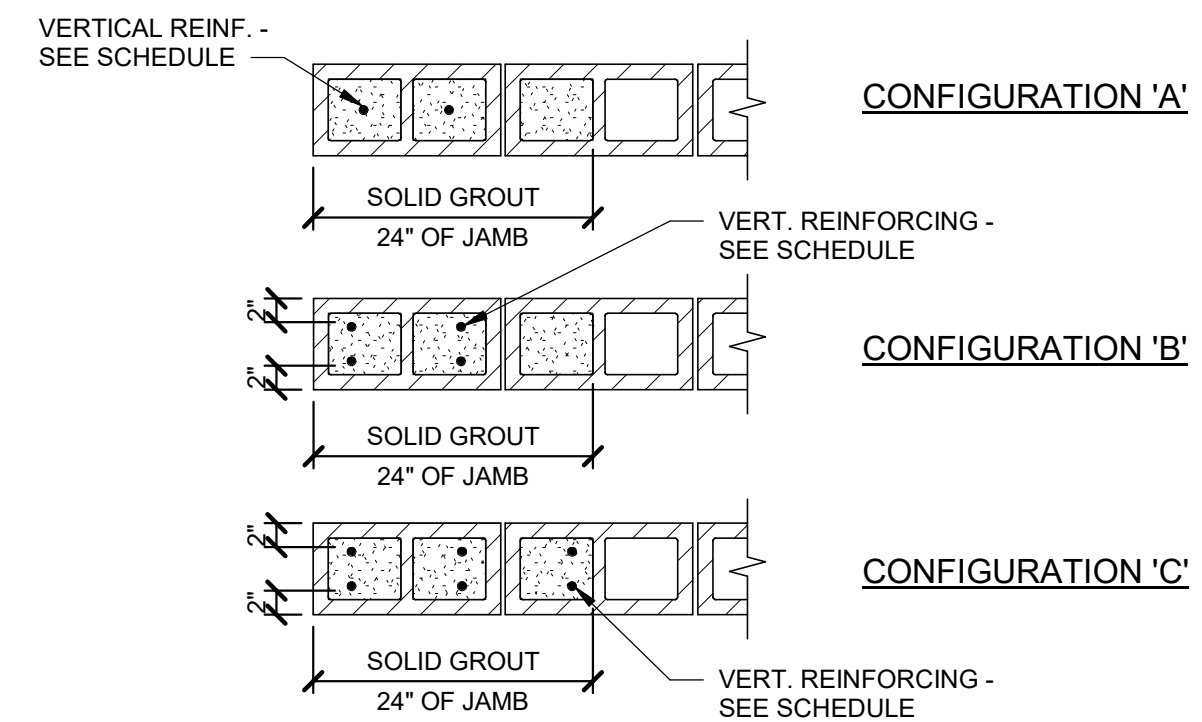
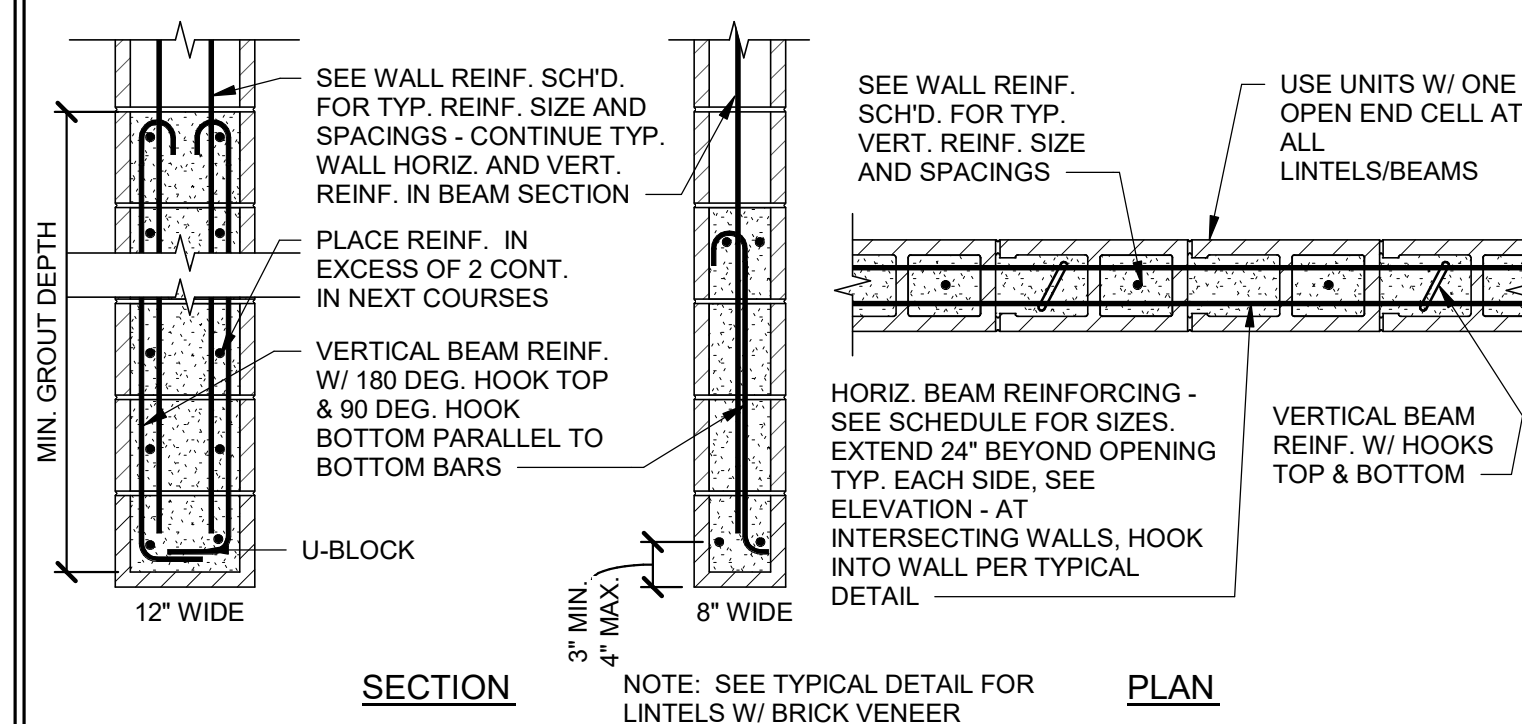
MASONRY BEAM SCHEDULE								MASONRY JAMB SCHEDULE						
MARK	NOMINAL THICKNESS	BOTTOM REINF.	TOP REINF.	VERTICAL REINF.	MIN. GROUT DEPTH	OPENING ① SIZE	COMMENTS	MARK	NOMINAL THICKNESS	VERTICAL REINF.	TIES	CONFIG.	OPENING ① SIZE	COMMENTS
MB-1	8"	(2) #5	(2) #5	#4 @ 32"	24"	2'-8" TO 6'-0"		MJ-1	8"	(2) #5	---	A	2'-8" TO 6'-0"	
MB-2	8"	(2) #5	(2) #5	#4 @ 32"	32"	6'-1" TO 10'-0"		MJ-2	8"	(4) #5	---	B	6'-1" TO 8'-0"	
								MJ-3	8"	(6) #5	---	C	8'-1" TO 10'-0"	

NOTES:

1. WHERE SPECIFIC BEAMS ARE NOT NOTED ON THE PLANS - REFER TO OPENING SIZE FOR REQUIRED BEAM DEPTH AND REINFORCING.
2. FIRST VERTICAL BAR TO BE WITHIN 8" OF END OF BEAM.
3. SEE TYPICAL ELEVATION - VIEW OF BEAM.
4. VERTICAL REINFORCING SHALL HAVE HOOKS TOP AND BOTTOM.

NOTES:

1. WHERE SPECIFIC JAMBS ARE NOT NOTED ON THE PLANS - REFER TO OPENING SIZE FOR REQUIRED REINFORCING AND CONFIGURATION.
2. ALL VERT. REINFORCING SHALL HAVE MATCHING DOWELS CAST INTO FOUNDATIONS.
3. HORIZONTAL REINFORCING NOT SHOWN.
4. JAMBS TO BE GROUTED SOLID.



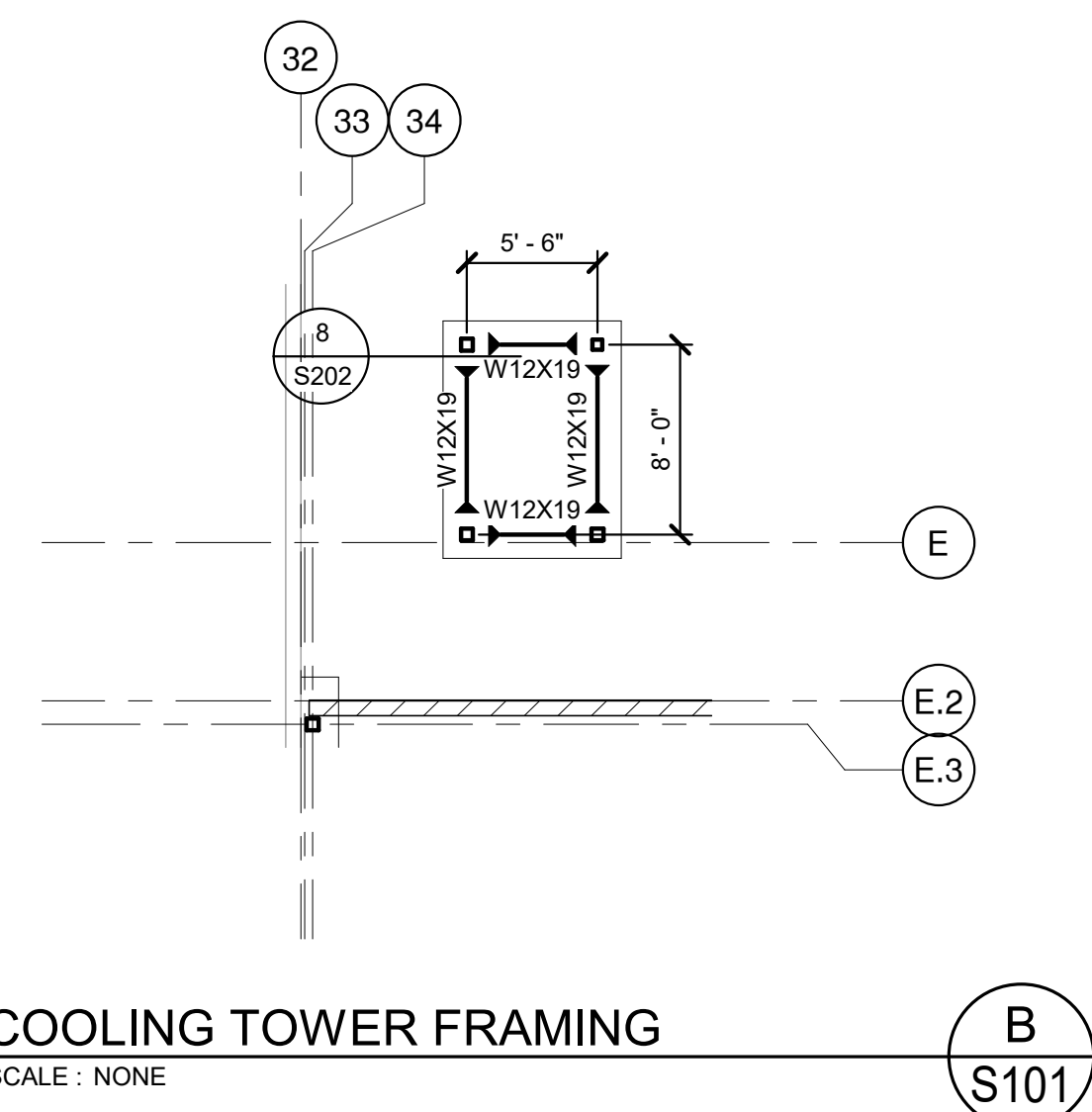
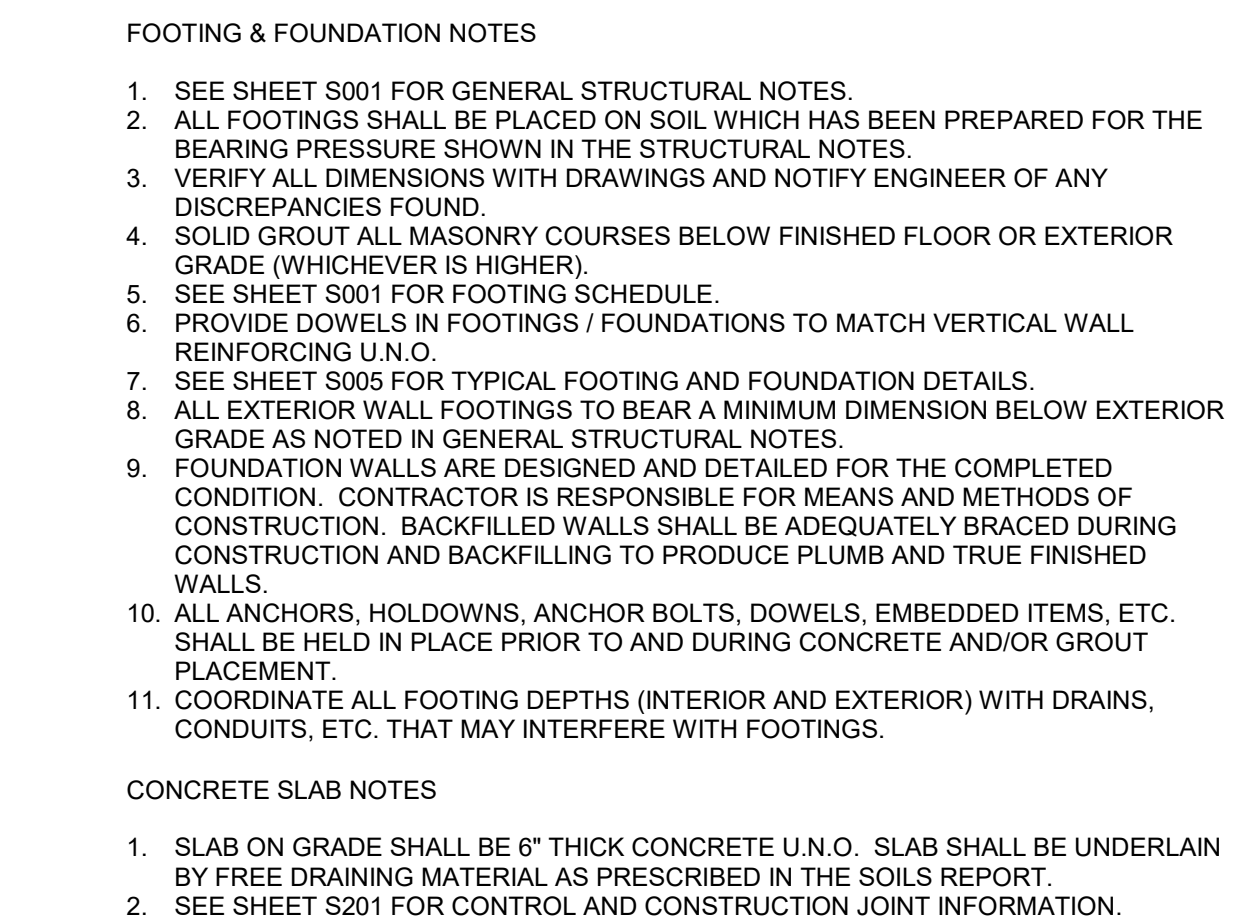
MASONRY WALL SCHEDULE

MARK	THICK.	VERT. REINF.		HORIZ. BOND BEAM REINF.					COMMENTS
		SIZE	SPACE	NO.	SIZE	SPACE	@ ROOF	@ ELEVATED FLOOR	
MW-1	8	#5	32	(2)	#4	48	(2) #5	--	

NOTES:

1. FOR ANY CMU WALLS NOT SPECIFICALLY CALLED OUT IN PLANS, USE MW1
2. VERT. REINFORCING TO BE @ CL. OF WALL UNLESS OTHERWISE NOTED.
3. SOLID GROUTING OF WALLS IS UNACCEPTABLE EXCEPT WHERE SPECIFICALLY NOTED.
4. SEE STRUCTURAL NOTES FOR ADDITIONAL INFORMATION.
5. A BOND BEAM SHALL BE LOCATED IN THE FIRST COURSE ABOVE THE FOUNDATION IF VERTICAL DOWELS HAVE BEEN BENT TO ALIGN WITH VERTICAL CELLS. WHETHER OR NOT MASONRY WEBS HAVE BEEN CUT

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ISSUE DATE: 26 March, 2018		
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ARW PROJECT NO:		18039
CAD DWG FILE:		
DRAWN BY:		D.Bartelson
CHK'D BY:		S. Ericksen

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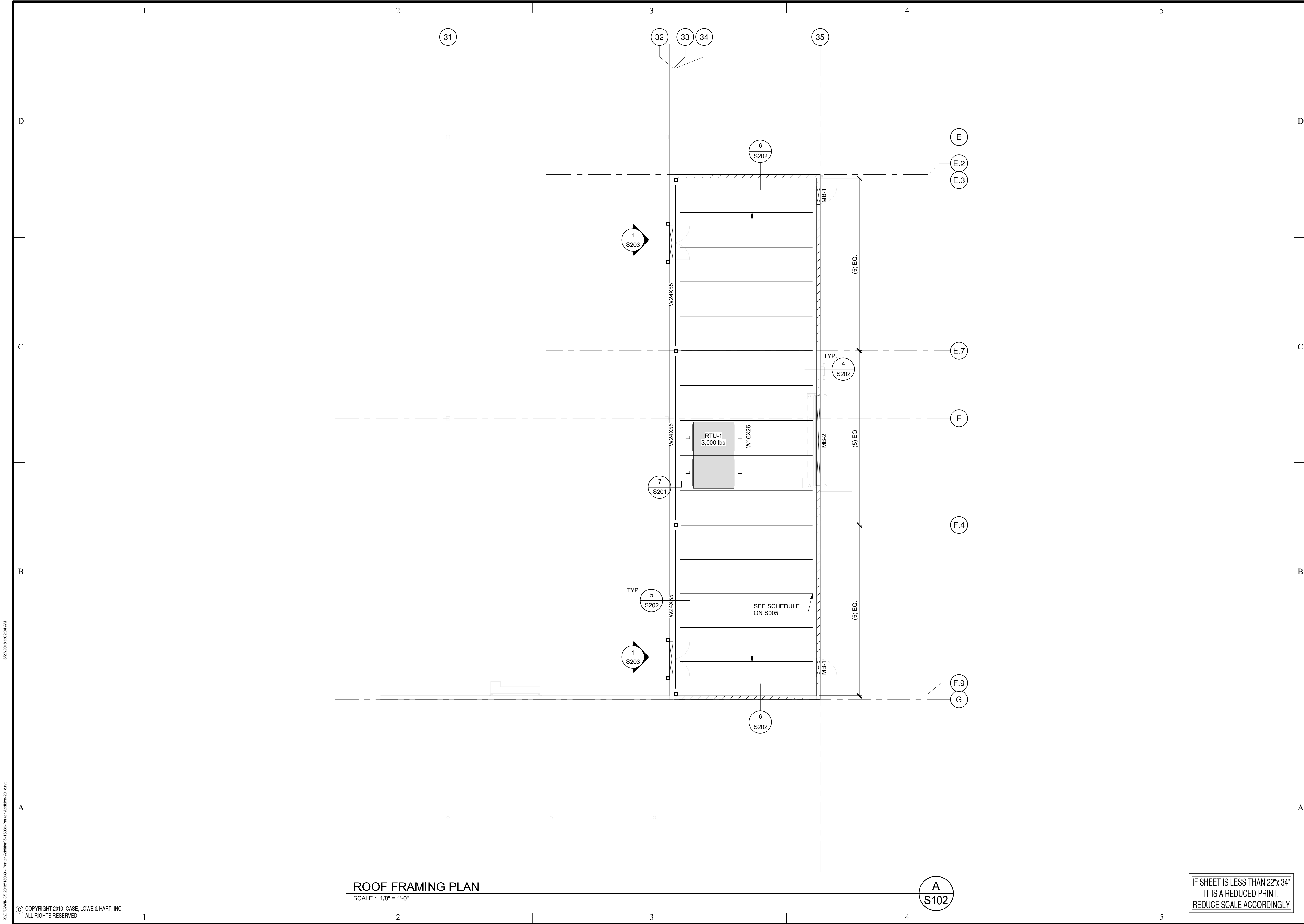
FOOTING AND FOUNDATION PLAN

SHEET NO:

S101

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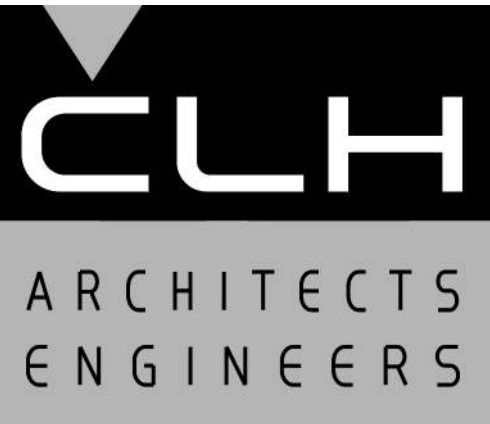
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ROOF FRAMING PLAN
SCALE : 1/8" = 1'-0"

A
S102

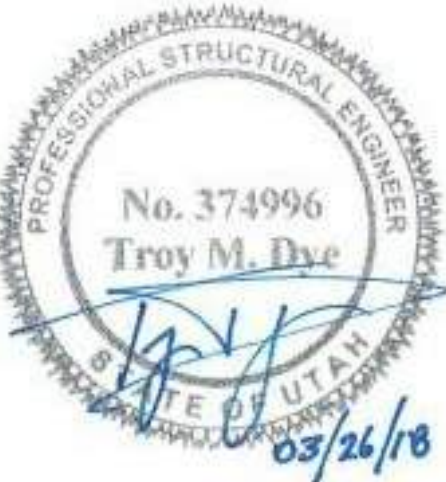
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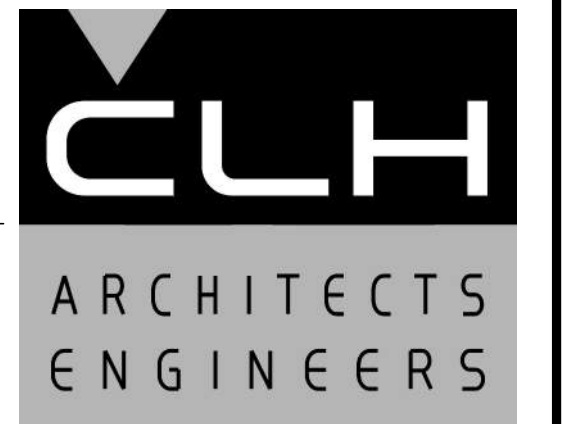
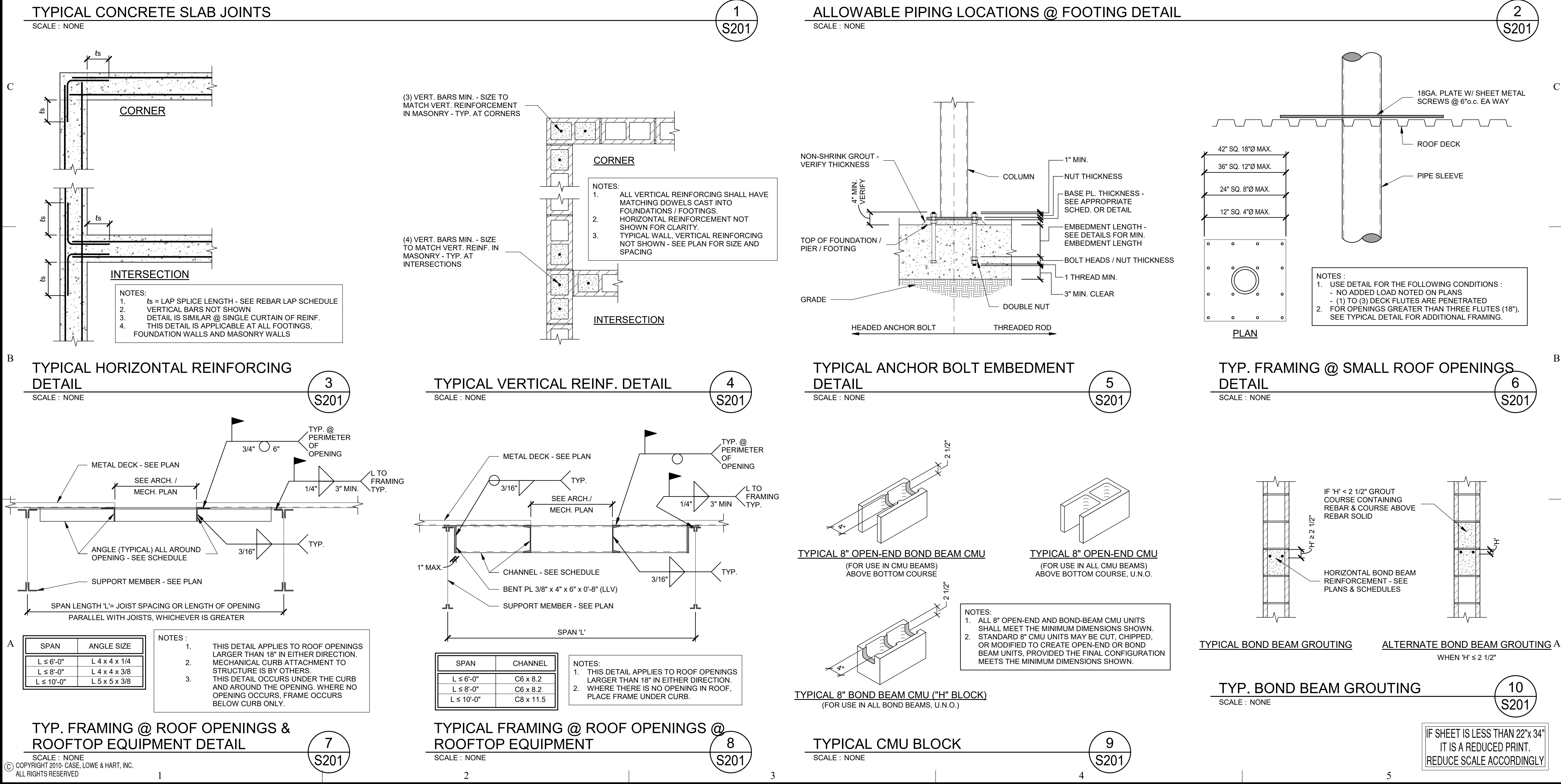
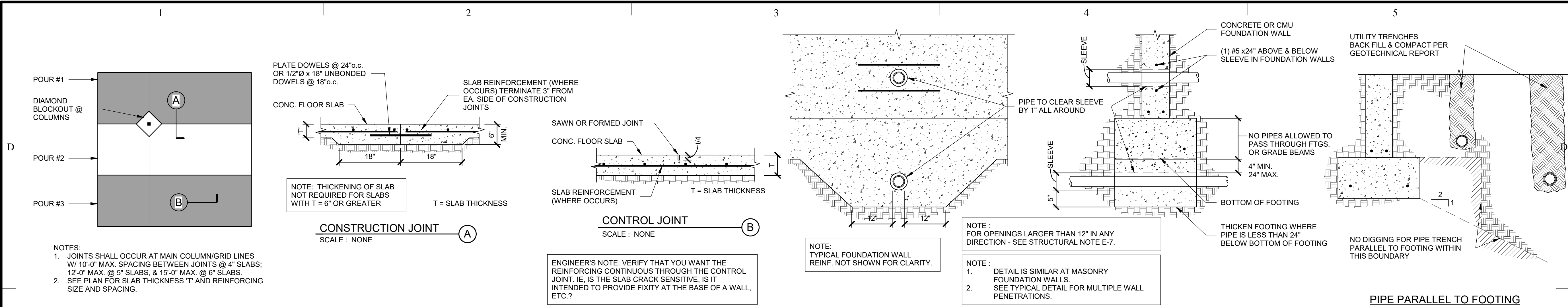
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ROOF FRAMING
PLAN

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S102



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DRAWN BY: D.Bartelson
CHK'D BY: S.Ericksen

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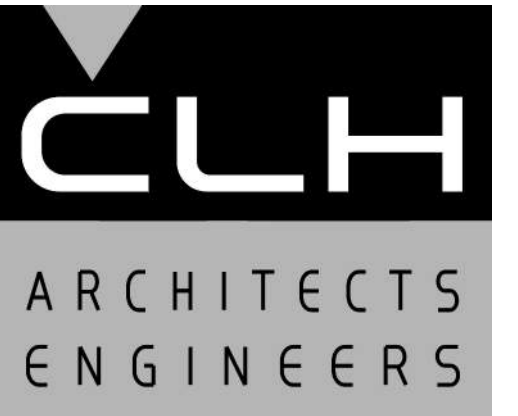
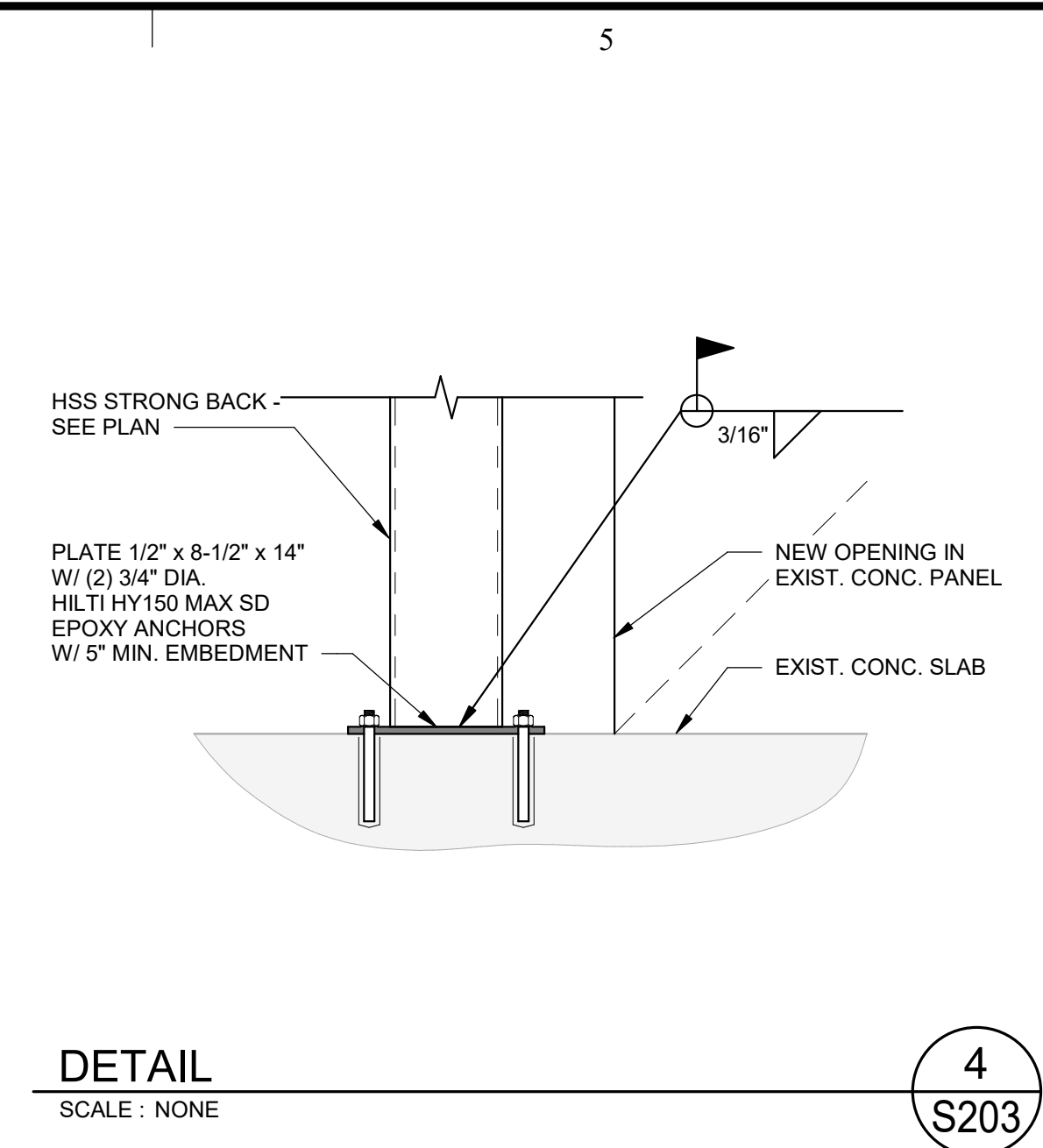
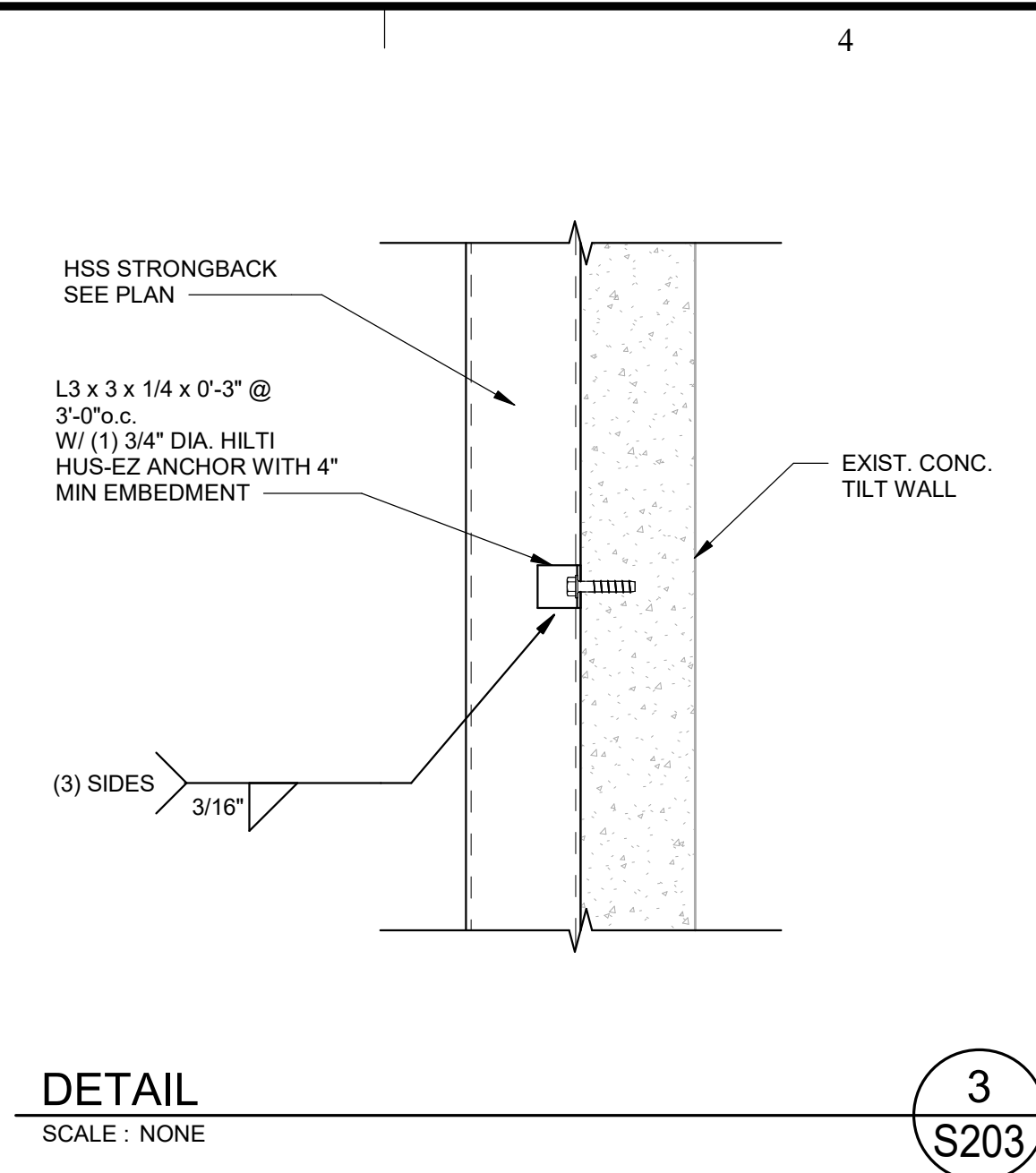
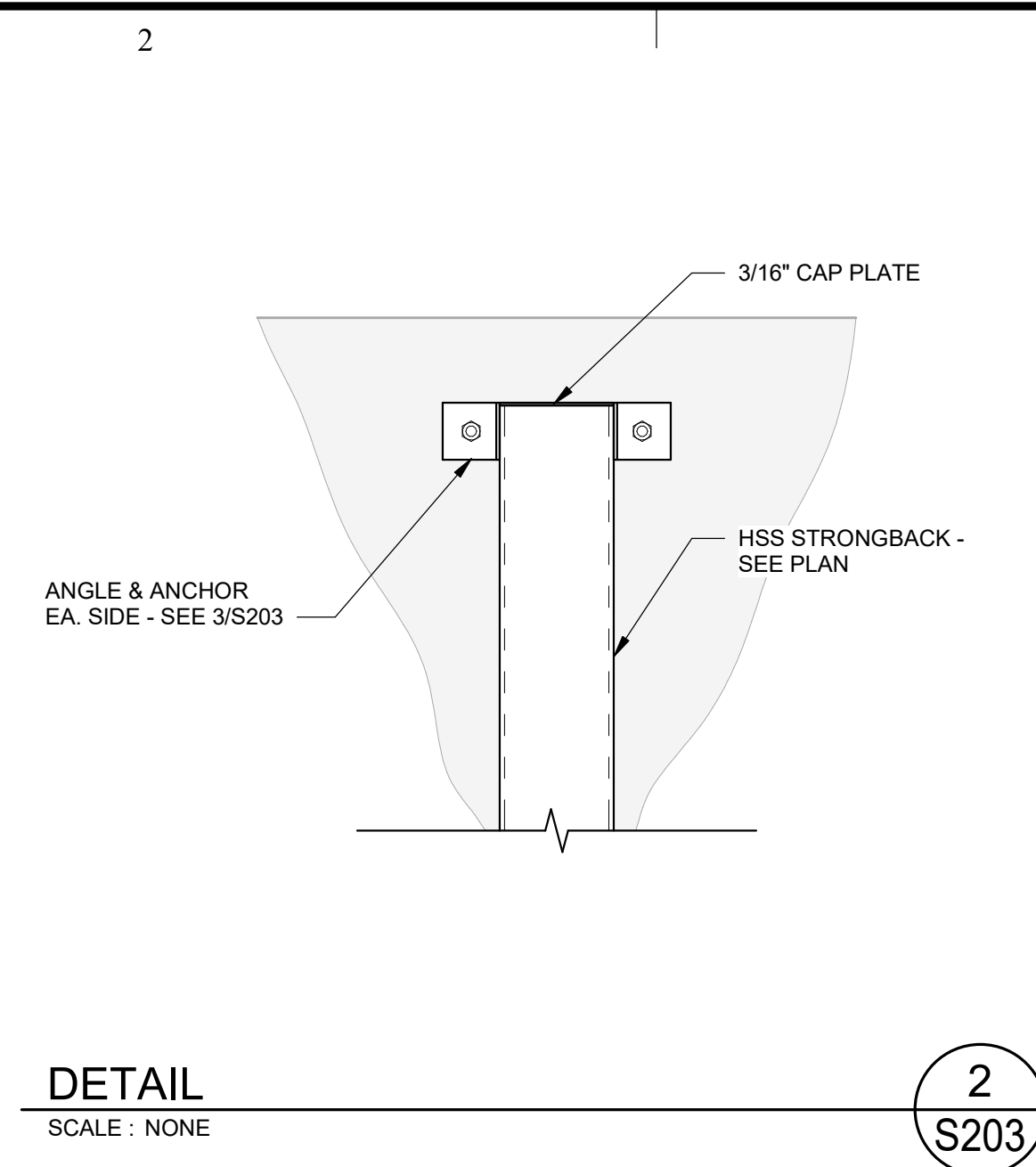
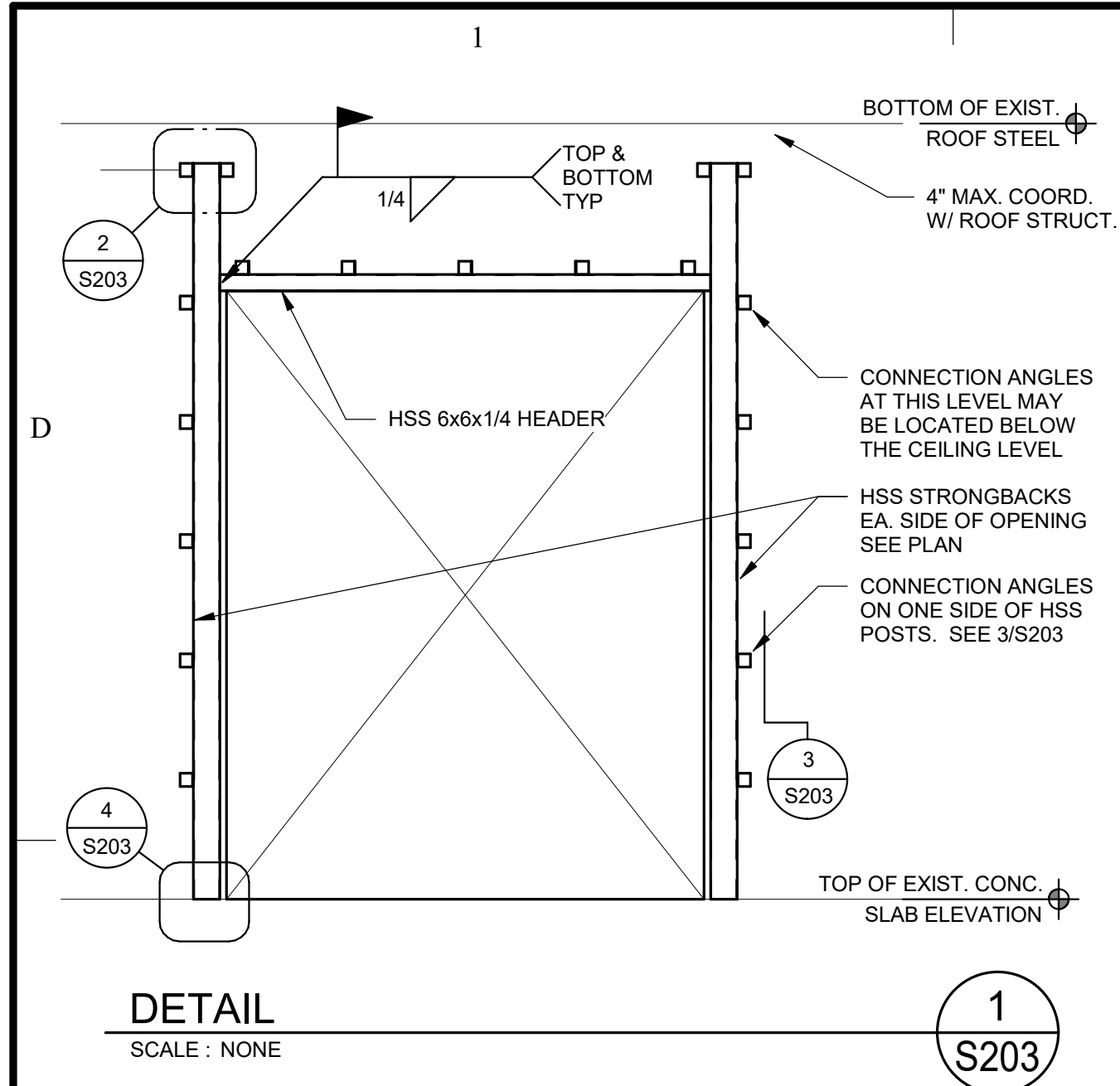
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DETAILS

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S201



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DRAWN BY:	Author	
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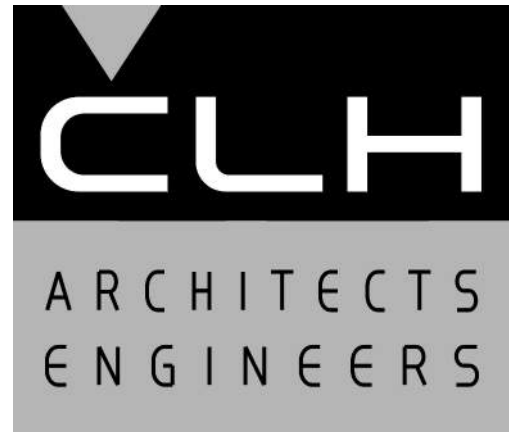
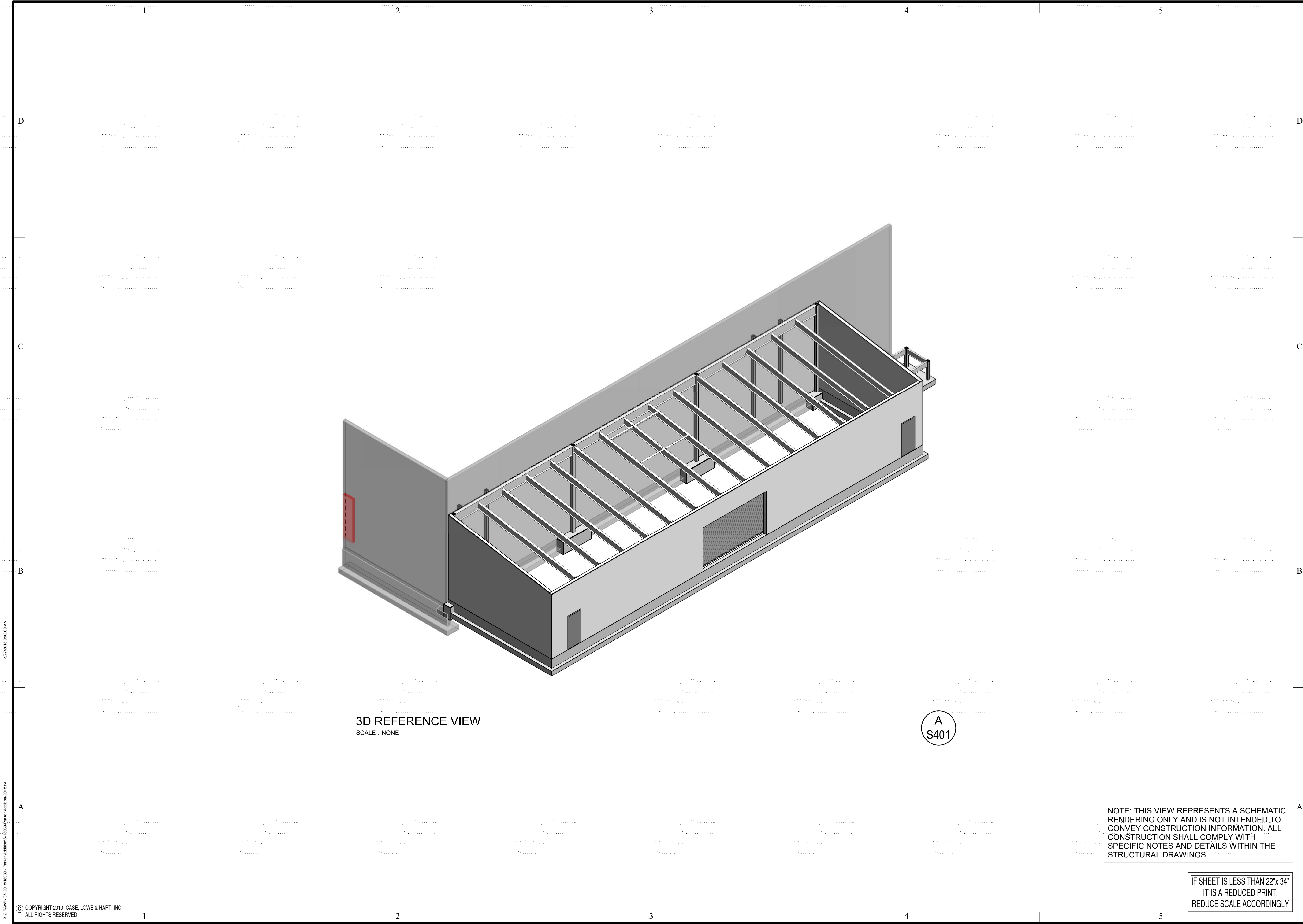
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DETAILS

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SHEET TITLE

SCHEMATIC
REFERENCE

SHEET NO:

S401

NOTE: THIS VIEW REPRESENTS A SCHEMATIC
RENDERING ONLY AND IS NOT INTENDED TO
CONVEY CONSTRUCTION INFORMATION. ALL
CONSTRUCTION SHALL COMPLY WITH
SPECIFIC NOTES AND DETAILS WITHIN THE
STRUCTURAL DRAWINGS.

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ARCHITECTURAL NOTES

1.

THE ARCHITECTURAL DRAWINGS ARE THE PRIMARY CONTRACT DOCUMENTS. ANY CONFLICTS BETWEEN ARCHITECTURAL DRAWINGS AND EXISTING CONDITIONS AND/OR DRAWINGS OF OTHER DISCIPLINES SHALL BE IMMEDIATELY REPORTED TO THE ARCHITECT.
2.

THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS PRIOR TO AY WORK. ITEMS AND DIMENSIONS BETWEEN EXISTING AND NEW PORTIONS OF THE PROJECT SHALL BE VERIFIED TO ENSURE COORDINATION.
3.

THE CONTRACTOR SHALL SUBMIT ANY PROPOSED CHANGES OR MODIFICATIONS OF THE CONTRACT DOCUMENTS, IN WRITING, TO THE ARCHITECT BEFORE PROCEEDING WITH ANY ACTION.
4.

WHERE SPECIFIC DETAILS ARE NOT PROVIDED, TYPICAL OR SIMILAR INDUSTRY STANDARD DETAILS SHALL APPLY. IF FURTHER DETAIL IS REQUIRED CONTACT ARCHITECT.
5.

DETAILS ARE PROVIDED FOR VISUAL REPRESENTATION OF DESIGN INTENT. OFTEN THE DETAILS ARE BASED ON A BASIS-OF-DESIGN PRODUCT AND/OR MATERIAL AND MAY BE DIAGRAMMATIC IN NATURE.
6.

IF A DIFFERENT PRODUCT OR MATERIAL FROM THAT INDICATED ON THE DRAWINGS OR SPECIFICATIONS IS SUBSTITUTED, IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALTERNATE DETAILS AS REQUIRED FOR THE ARCHITECT TO REVIEW.
7.

GENERALLY, DIMENSIONS SHOWN OF ARCHITECTURAL DRAWINGS ARE TAKEN FROM THE CORE STRUCTURE FACE (IE. CONCRETE WALL=FACE OF WALL; STUD WALL=FACE OF STUD).
8.

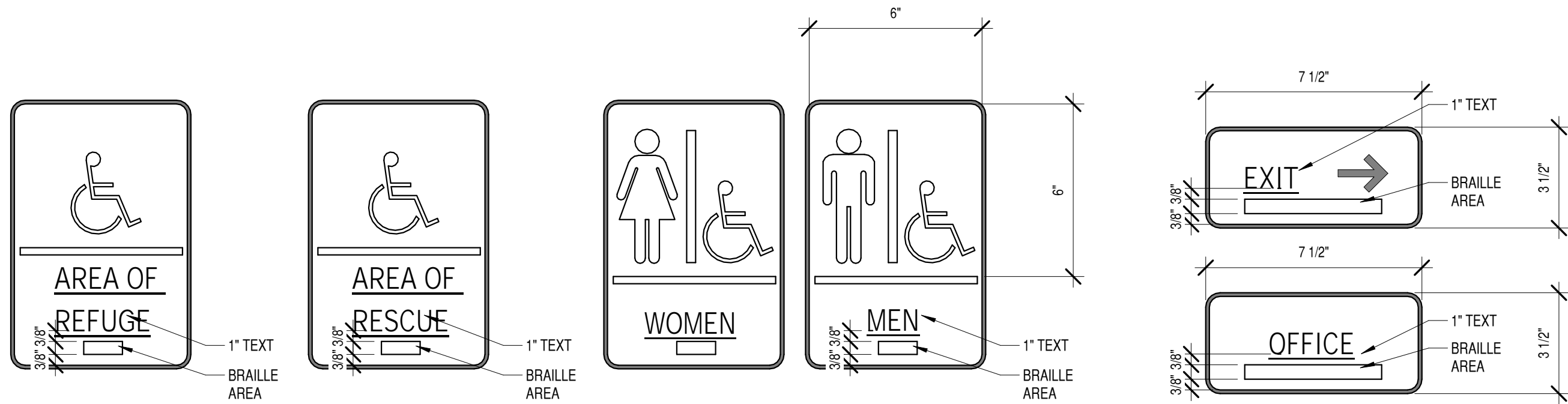
ANY ADDITIONAL BLOCKING, BRACING, TRIM, FLASHING, SEALANTS, ETC. REQUIRED FOR INSTALLATION OF COMPLETE SYSTEMS_ PERTAINING TO DOORS, WINDOWS, OPENINGS, PENETRATIONS, ETC. ARE EXPECTED TO BE PROVIDED AND INSTALLED BY THE CONTRACTOR.
9.

ASSUME ALL GYP. BD. WALLS TO HAVE TOPSET RUBBER BASE INSTALLED UNLESS NOTED OTHERWISE.
10.

PROVIDE SEALANT OR TRIM AS APPROPRIATE WHERE DISSIMILAR MATERIALS COME IN CONTACT.
11.

PROVIDE FLOORING TRANSITION WHERE DISSIMILAR FLOORING MATERIALS OCCUR.
12.

PAINT ALL MISCELLANEOUS SURFACES, SUPPORTS, METALS, ETC. IF PERMANENTLY ATTACHED TO PAINTED SURFACE OR EXPOSED TO THE ELEMENTS.



A1 TYP. SIGNS
3" = 1'-0"

SYMBOLS

1	View Name 1/8" = 1'-0"	VIEW TITLE
		GRAPHIC SCALE
		NORTH ARROW w/ TRUE NORTH
		GRID INDICATOR
		SECTION CALLOUT
		DETAIL CALLOUT
		DETAIL CALLOUT
		ELEVATION CALLOUT
		LEVEL / ELEVATION CALLOUT
		SPOT ELEVATION CALLOUT
		ROOF SLOPE INDICATOR
		ROOM TAG
		DOOR TAG
		WALL TAG
		WINDOW TAG
		DEMOLITION KEYNOTE
		FIRE RISER

ABBREVIATIONS

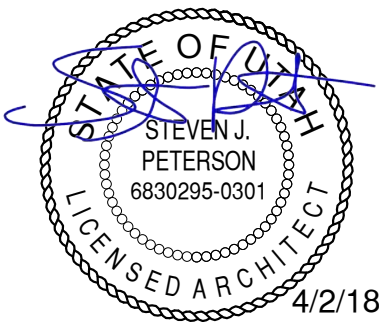
& L @ #	AND ANGLE AT FOUND OR NUMBER	JAN JST JT	JANITOR JOIST JOINT
AC A.F.F. ALUM APPROX ARCH ASPH	ACOUSTICAL ABOVE FINISH FLOOR ALUMINUM APPROXIMATE ARCHITECTURAL ASPHALT	K.O. LAM LAV	KNOCK OUT LAMINATE LAVATORY
BD BITUM BLDG BLKG BRG BTM	BOARD BITUMINOUS BUILDING BLOCKING BEARING BOTTOM	MAX MAS MECH MEMB MTL MFTR MH MIN MISC M.O. MTD	MAXIMUM MASONRY MECHANICAL MEMBRANE METAL MANUFACTURER MANHOLE MINIMUM MISCELLANEOUS MASONRY OPENING MOUNTED
C C.I. C.J. C.L. CLG CLR C.M.U. C.O. C.O.T.G. COL CONC CONN CONSTR CONT C.T. CTR	TOP OF FINISH CONCRETE CAST IRON CONTROL JOINT CENTER LINE CEILING CLEAR CONCRETE MASONRY UNIT CLEAN OUT CLEAN OUT AT GRADE COLUMN CONCRETE CONNECTION CONSTRUCTION CONTINUOUS CERAMIC TILE CENTER	N N.I.C. NO or # NOM N.T.S.	NORTH NOT IN CONTRACT NUMBER NOMINAL NOT TO SCALE
D.C.W. D.H.W. D.F. DTL DIA DIM DISP DN DRN DS DWG	DOMESTIC COLD WATER DOMESTIC HOT WATER DRINKING FOUNTAIN DETAIL DIAMETER DIMENSION DISPENSER DOWN DRAIN DOWNSPOUT DRAWING	O.C. O.D. OFF OH OPNG OPP	ON CENTER OUTSIDE DIAMETER (DIM) OFFICE OVERHEAD OPENING OPPOSITE
E EA E.I.F.S. E.J. EL ELEC ENGR EQ EQUIP (E) EXP EXT	EAST EACH EXTERIOR INSULATION FINISH SYSTEM EXPANSION JOINT ELEVATION ELECTRICAL ENGINEER EQUAL EQUIPMENT EXISTING EXPANSION EXTERIOR	PL PLAM PLYWD P.O.C. PNL PR PT	PLATE PLASTIC LAMINATE PLYWOOD POINT OF CONNECTION PANEL PAIR POINT
F.A. F.D. FDN F.E. F.E.C. FIN FLR FLASH FLUOR F.O. F.R. FT FTG FUT	FIRE ALARM FLOOR DRAIN FOUNDATION FIRE EXTINGUISHER FIRE EXTINGUISHER CABINET FINISH FLOOR FLASHING FLUORESCENT FACE OF FIRE RATED FOOR OR FEET FOOTING FUTURE	Q.T. RAD R.D. REF REINF REQD RESIL RFG RM RS R.O.	QUARRY TILE RADIUS ROOF DRAIN REFERENCE REINFORCED REQUIRED RESILIENT ROOFING ROOM RESINOUS FLOORING ROUGH OPENING
GA GALV GND GR G.W.B. GYP	GAUGE GALVANIZED GROUND GRADE GYPSUM WALL BOARD GYPSUM	S SCH SECT SHT SIM SPECS SQ S.S. S.ST STD STL STOR STR SUSP SYM SYS	SOUTH SCHEDULE SECTION SHEET SIMILAR SPECIFICATION SQUARE SANITARY SEWER STAINLESS STEEL STANDARD STEEL STORAGE STRUCTURAL SUSPENDED SYMMETRICAL SYSTEM
H.B. HC H.M. HORIZ HGT	HOSE BIBB HANDICAP HOLLOW METAL HORIZONTAL HEIGHT	TLT TRTD T & B T.O. TRANS TYP	TOILET (ROOM) TREATED (PRESERVATIVE) TOP & BOTTOM TOP OF TRANSFORMER TYPICAL
I.D. IN INSUL INT	INSIDE DIAMETER (DIM) INCH, INCHES INSULATION INTERIOR	U.N.O. UT VERT VEST	UNLESS NOTED OTHERWISE URINAL VERTICAL VESTIBULE
		W W/ WC WD W/O WP	WEST WITH WATER CLOSET WOOD WITHOUT WATERPROOF



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1425 WEST 2675 NORTH OGDEN, UTAH

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PROJECT NO: 18110
CAD DWG FILE:
DRAWN BY: Author
CHK'D BY: Checker

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27 MARCH, 2018

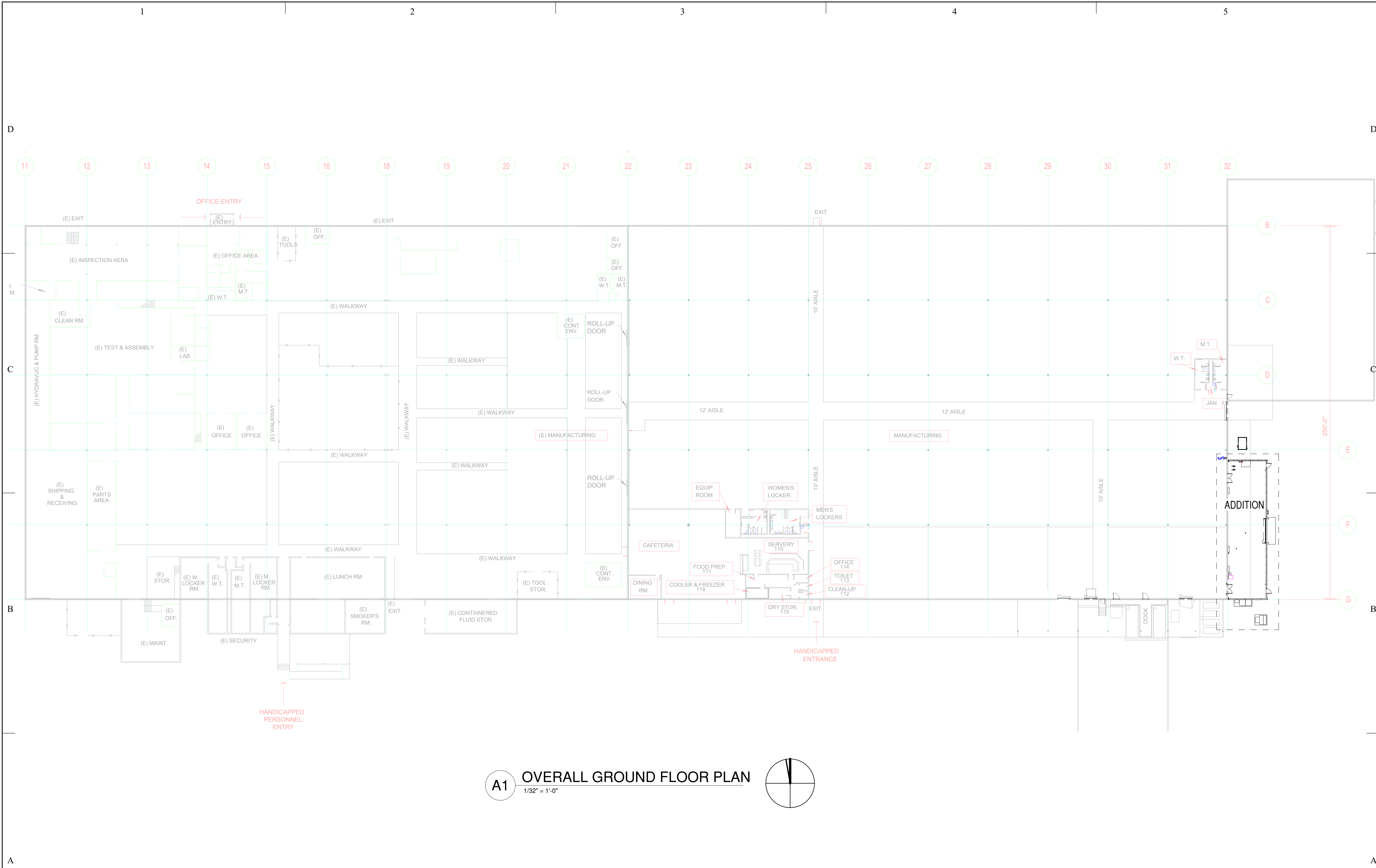
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ARCHITECTURAL
NOTES

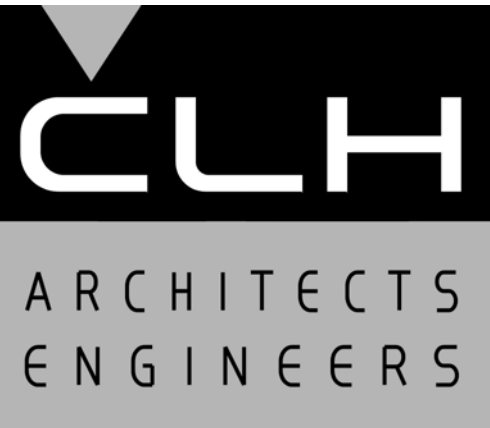
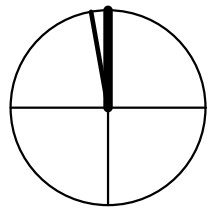
SHEET NO:

A001

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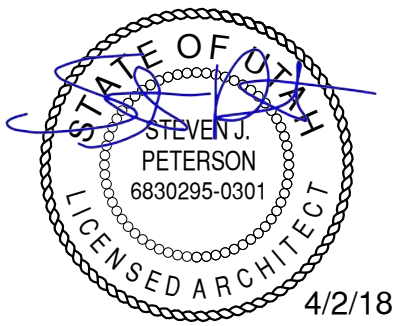
A1 OVERALL GROUND FLOOR PLAN
1/32" = 1'-0"



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PLANT
PARKER/HANNIFIN CORP.
1425 WEST 2675 NORTH OGDEN, UTAH

MARK	DATE	DESCRIPTION
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ISSUE DATE:	27 MARCH, 2018
PROJECT NO:	18110
CAD DWG FILE:	
DRAWN BY:	Author
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PERMIT SET

27 MARCH, 2018

SHEET TITLE

OVERALL
FLOOR PLAN

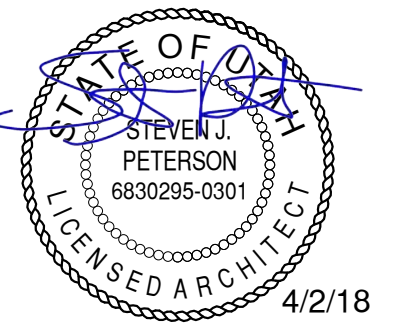
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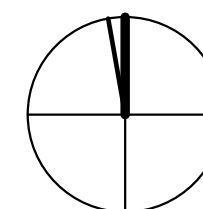
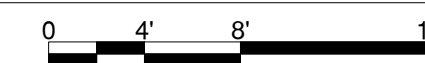
27 MARCH, 2018

SHEET TITLE

FLOOR PLAN

SHEET NO:

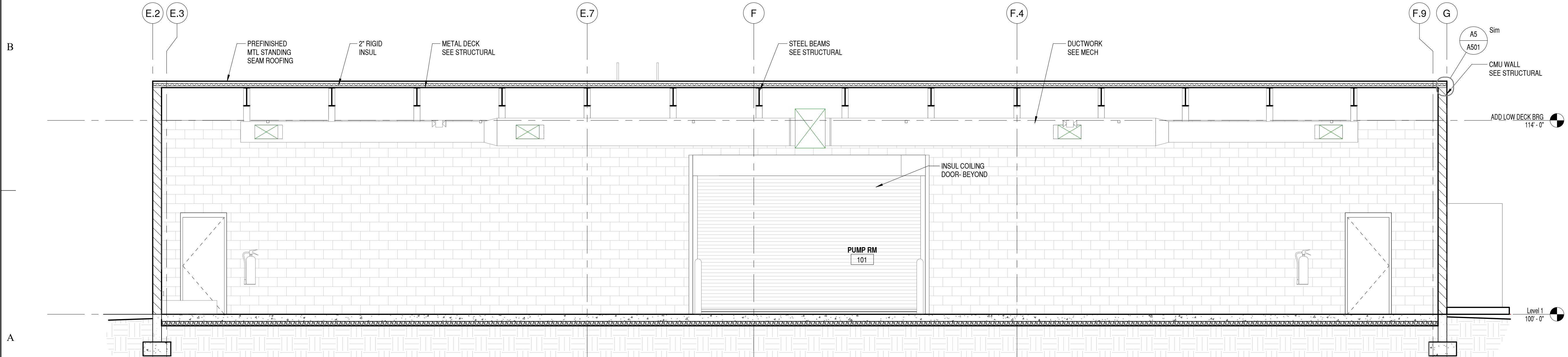
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REDUCE SCALE ACCORDINGLY

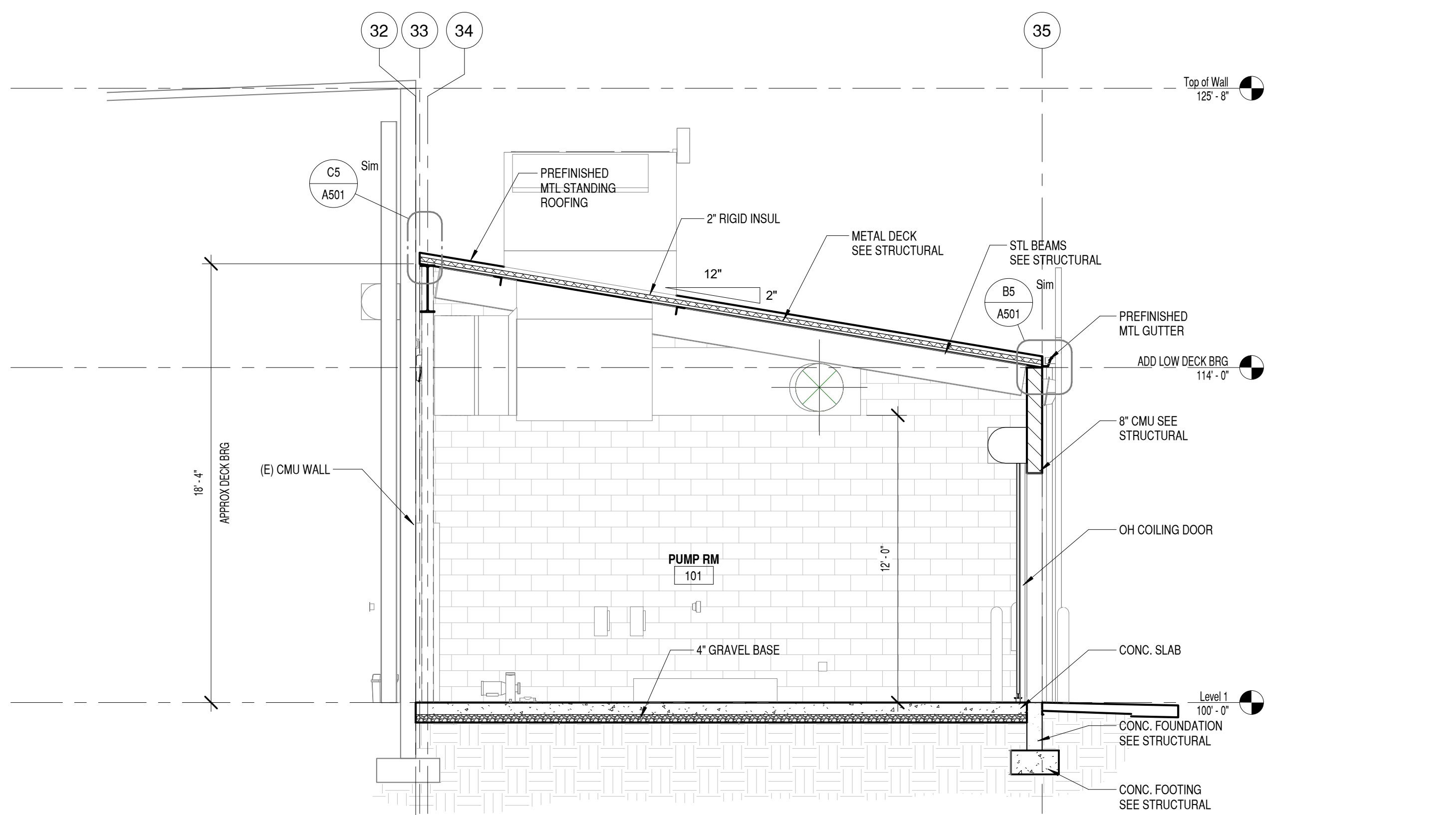
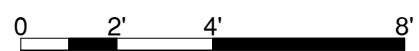
4/2/2018 8:27:51 AM

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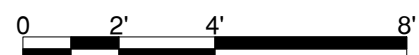
A1 BUILDING SECTION

1/4" = 1'-0"



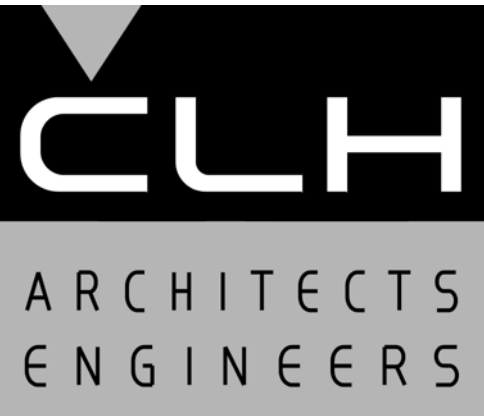
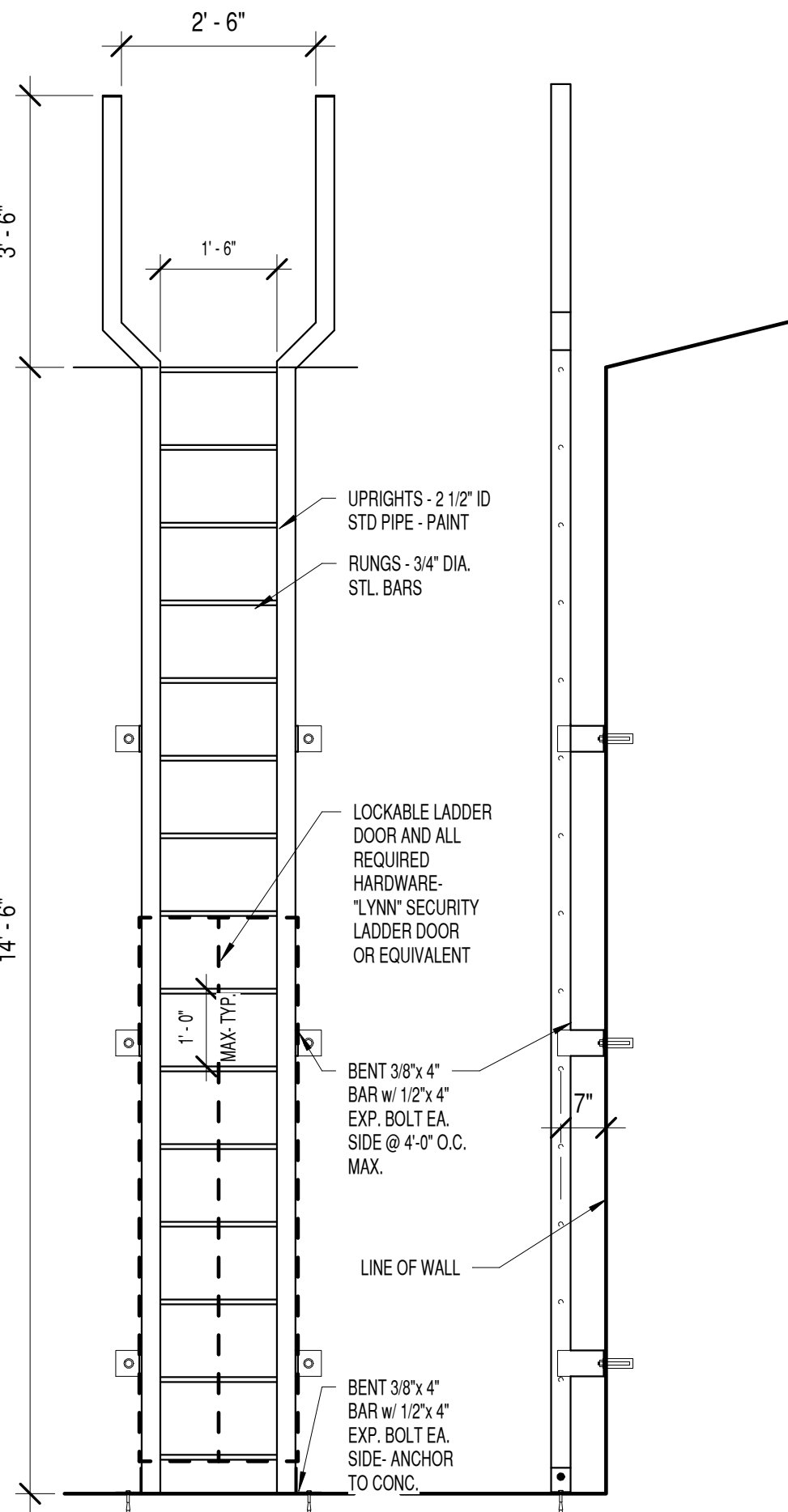
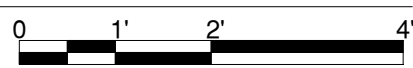
C3 BUILDING SECTION

1/4" = 1'-0"



C1 PIPE LADDER DETAIL

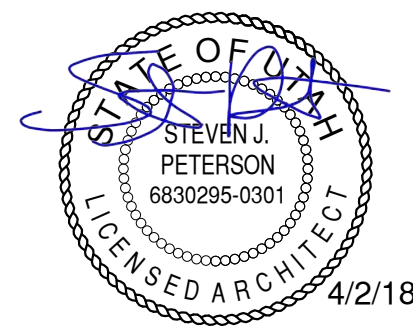
1/2" = 1'-0"



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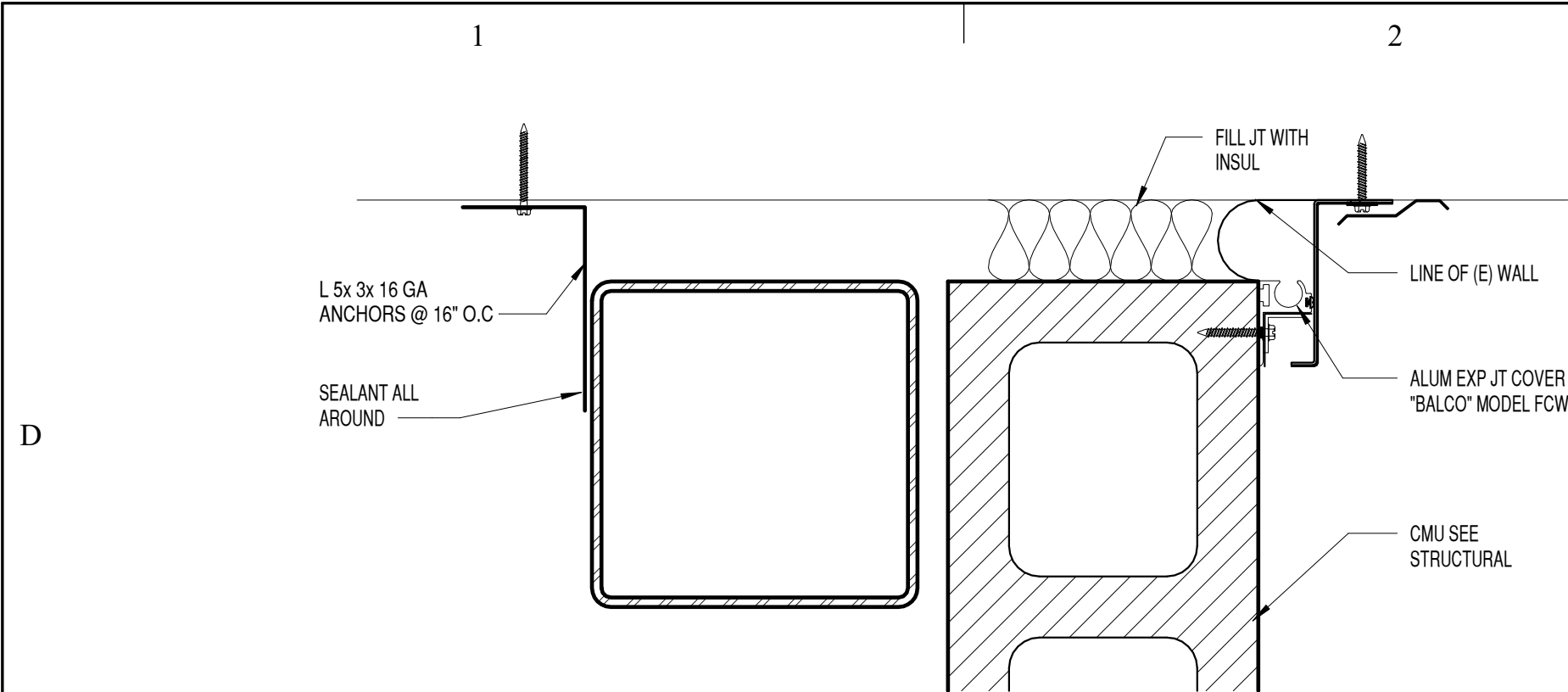
SHEET TITLE

**BUILDING
SECTION**

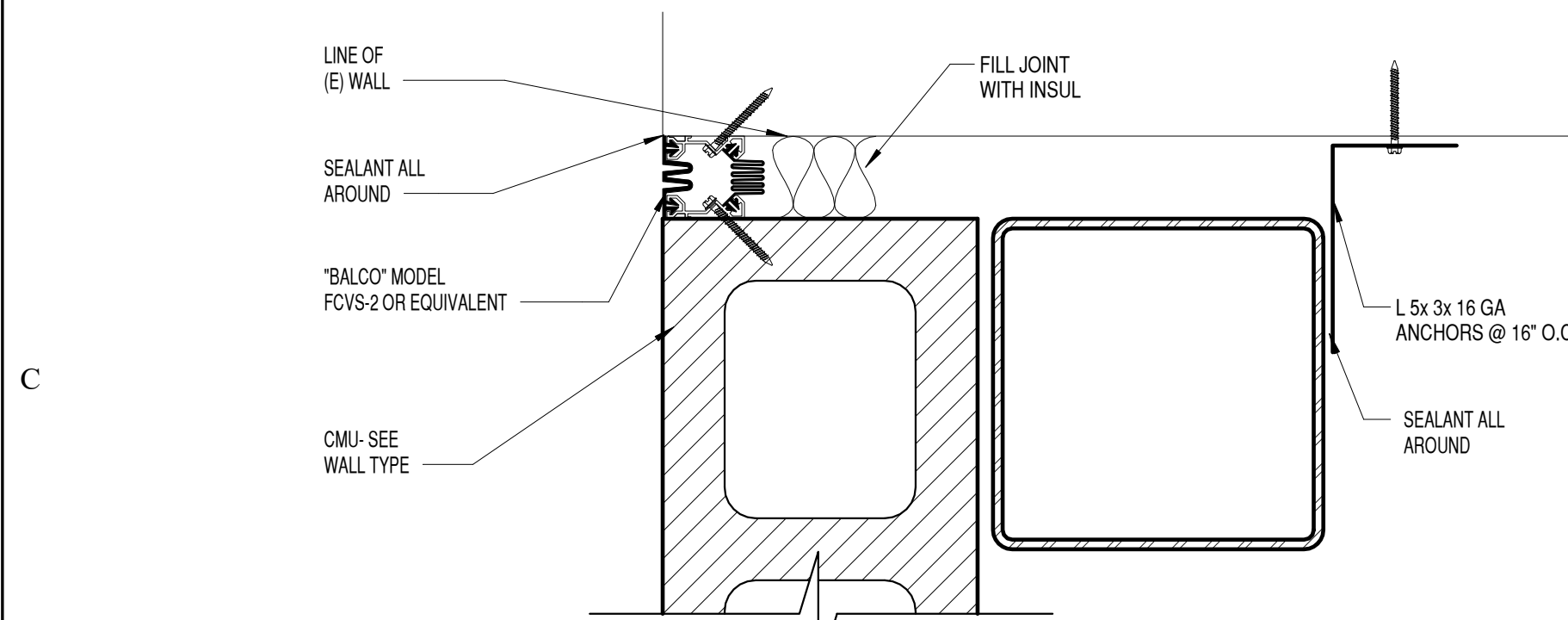
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A301

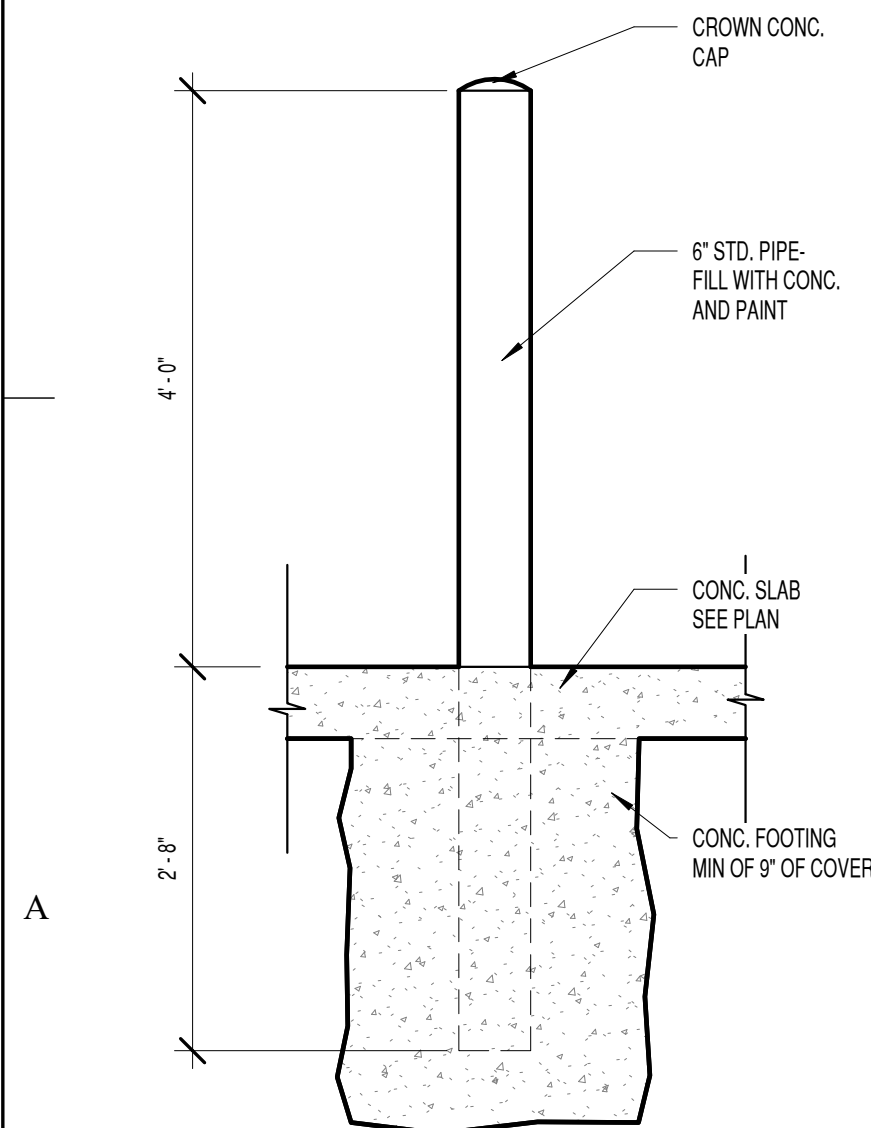
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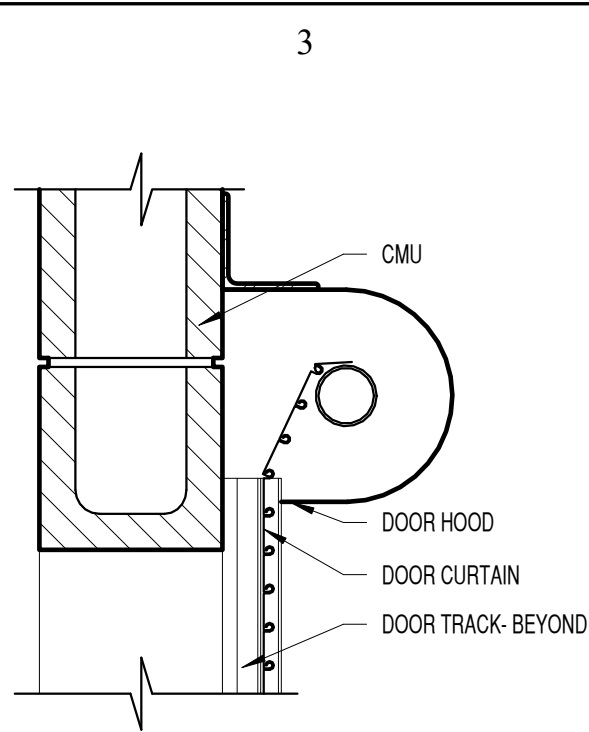
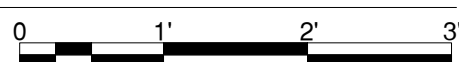
D1 CMU TO (E) CMU
3" = 1'-0"



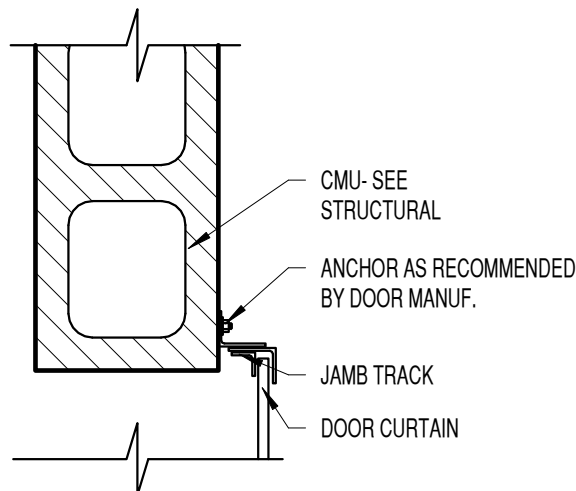
C1 CMU TO (E) WALL AT CORNER
3" = 1'-0"



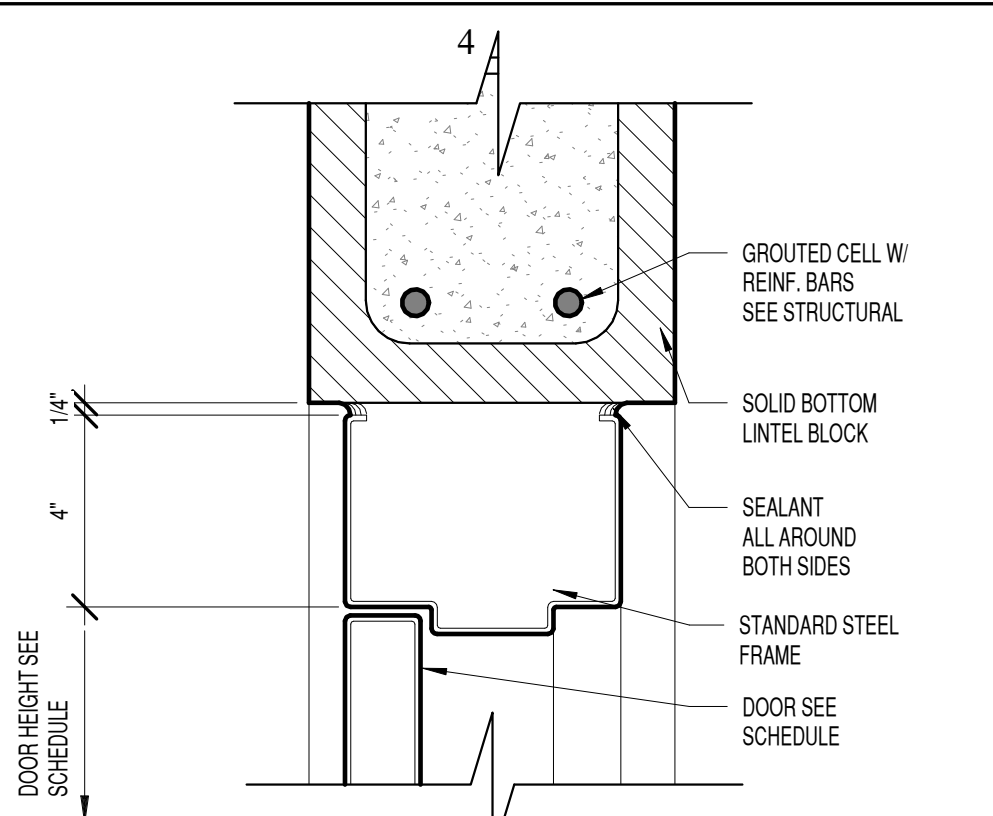
A1 STEEL PIPE BOLLARD
3/4" = 1'-0"



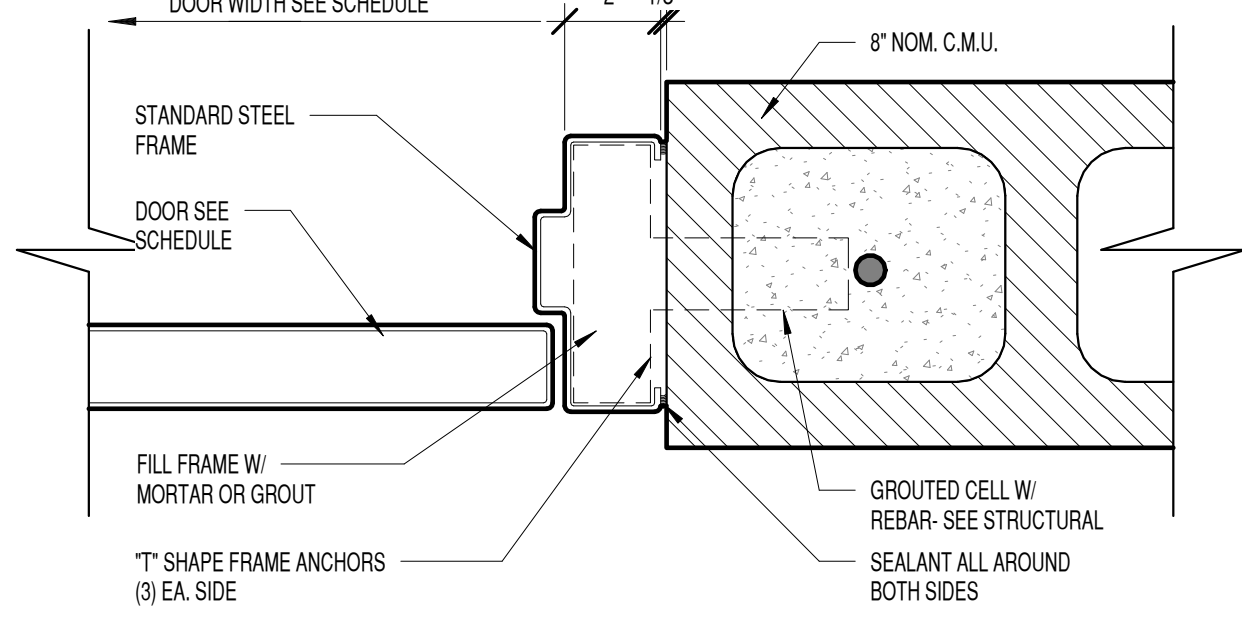
D3 OH DOOR HEAD- CMU
1 1/2" = 1'-0"



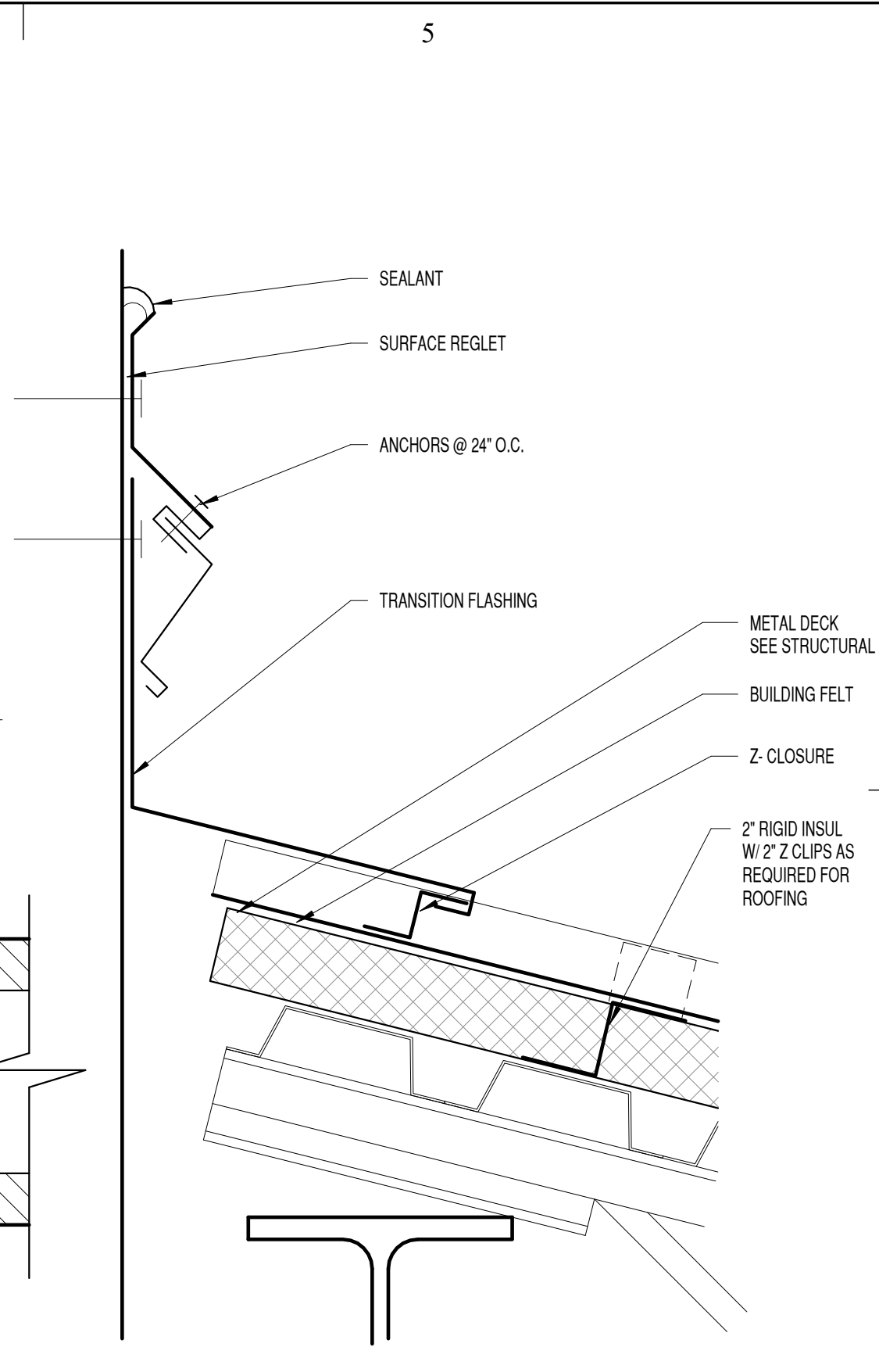
C3 OH DOOR JAMB- CMU
1 1/2" = 1'-0"



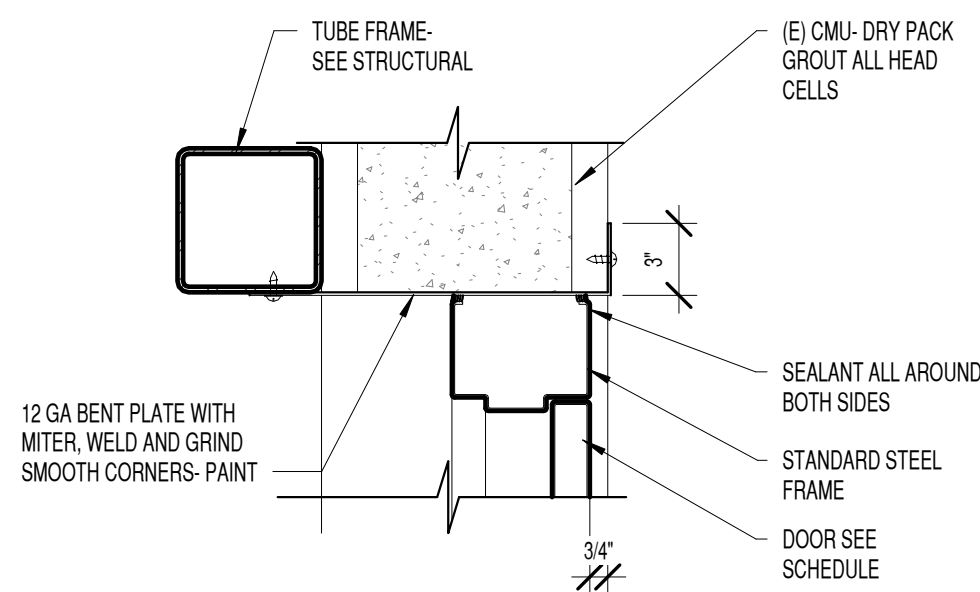
D4 DOOR HEAD- CMU PARTITION
3" = 1'-0"



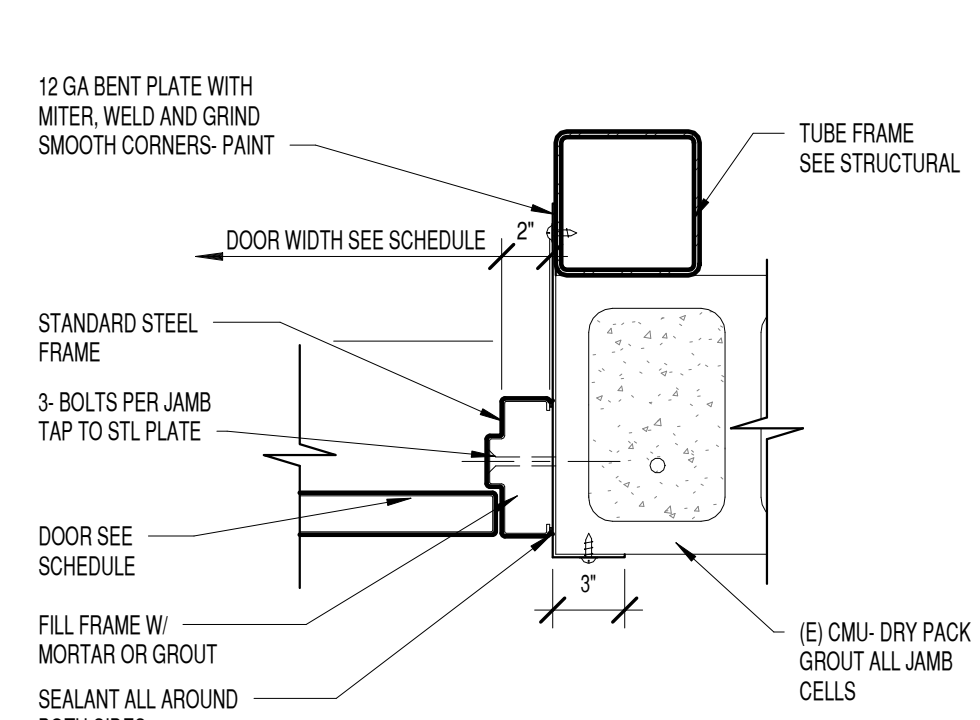
C4 DOOR JAMB- CMU PARTITION
3" = 1'-0"



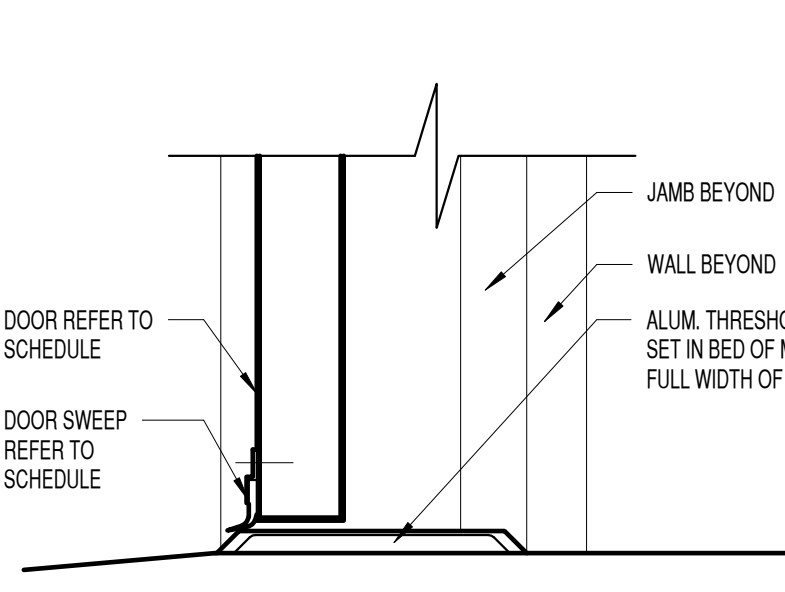
C5 WALL REGLET
3" = 1'-0"



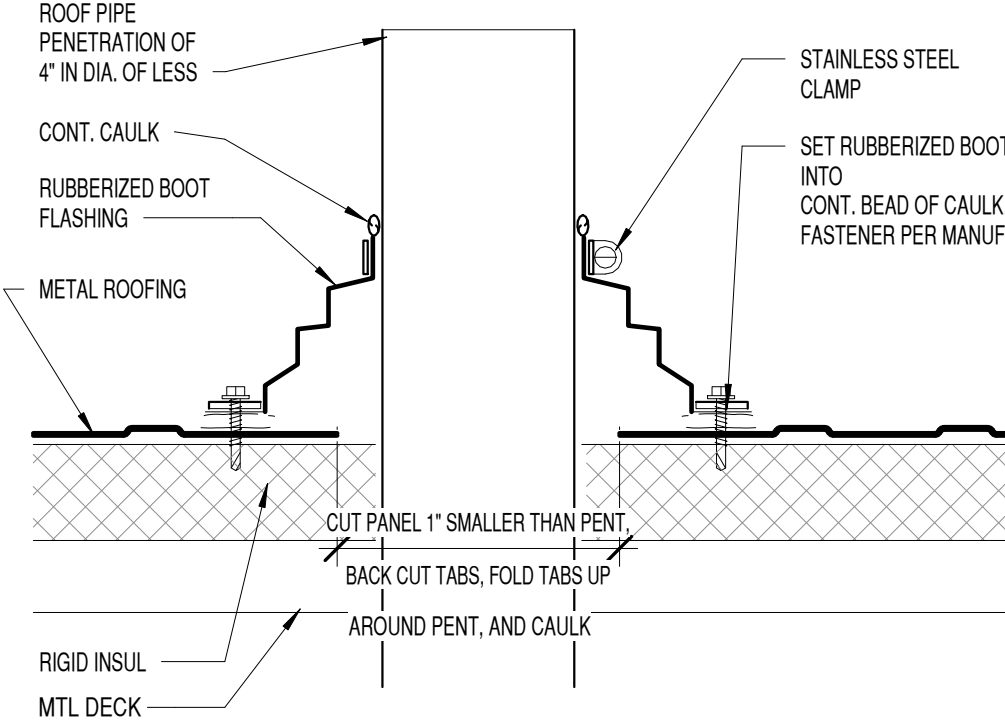
B3 DOOR HM (E) CMU HEAD
1 1/2" = 1'-0"



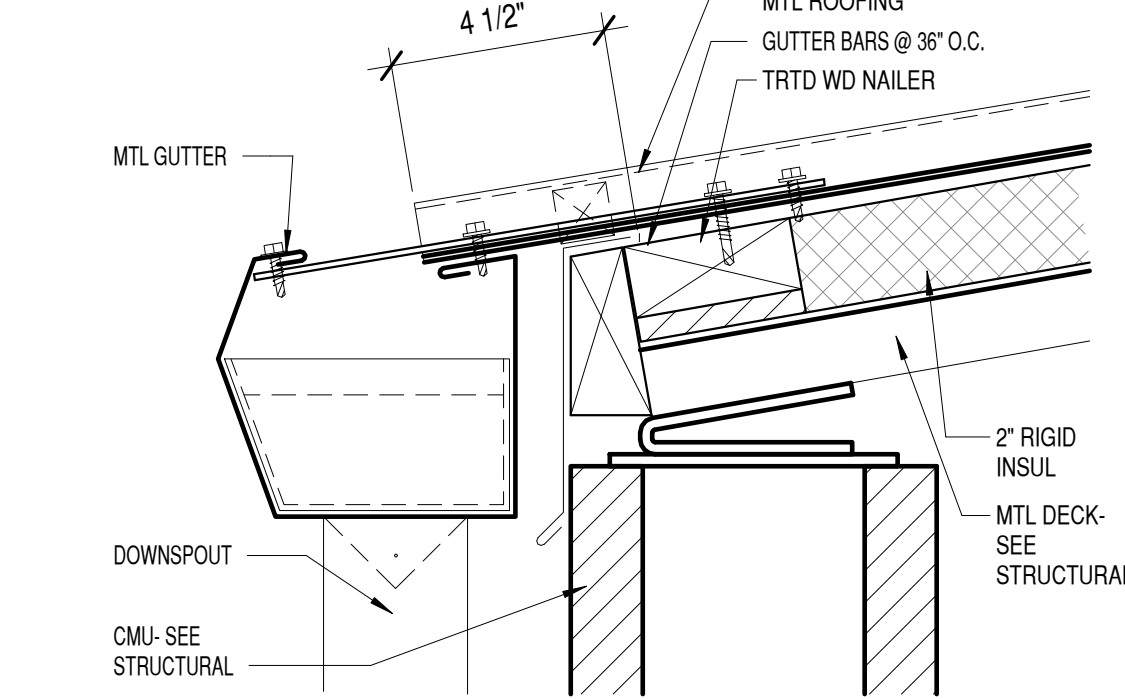
A3 DOOR HM (E) CMU JAMB
1 1/2" = 1'-0"



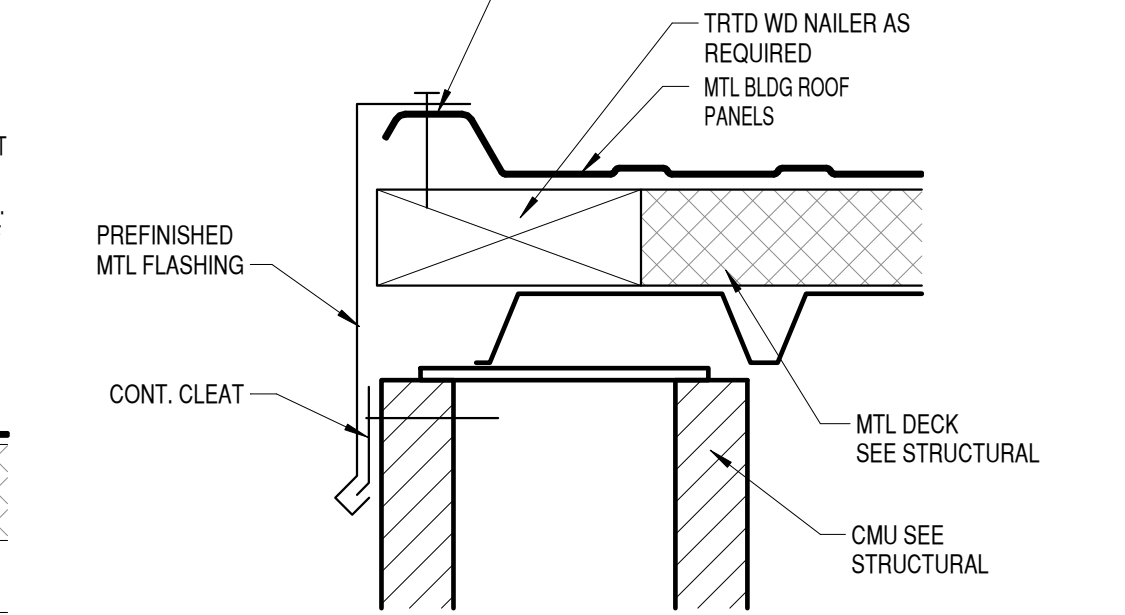
B4 DOOR THRESHOLD
3" = 1'-0"



A4 ROOF PENT
3" = 1'-0"

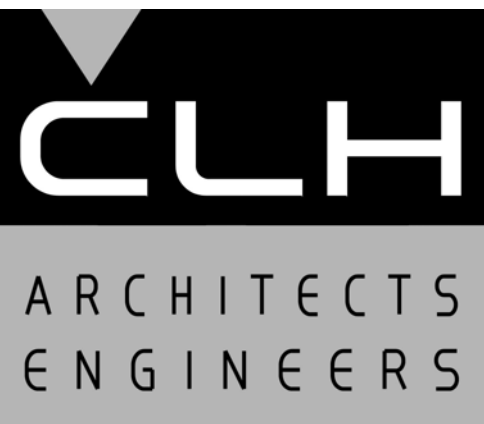


B5 MTL GUTTER
3" = 1'-0"



A5 END WALL FASCIA
3" = 1'-0"

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27 MARCH, 2018

SHEET TITLE

DETAILS

SHEET NO:

A501

DOOR SCHEDULE																																											
DOOR NUMBER	TYPE	DOOR				FRAME		DETAILS			GLAZING	FIRE RATING	COMMENTS	HINGES						LOCKSETS						STOPS		DOOR SEAL					MISC.										
		MATERIAL	WIDTH	HEIGHT	THICKNESS	TYPE	MATERIAL	HEAD	JAMB	THRESHOLD				NUMBER	PIVOTS	BALL BEARING	BRONZE/BRASS	STAINLESS STEEL	N.R. PINS	ENTRANCE	EXIT DEVICE	OFFICE	PASSAGE	PRIVACY	STORE ROOM	FLUSH BOLTS	SURFACE BOLTS	WALL STOP	FLOOR STOP	SMOKE STRIP	WEATHER STRIP	ASTRAGAL	SWEEP	THRESHOLD	CLOSER	PUSH/PULL	COORDINATOR	KICK PLATE	ARMOR PLATE				
D	101 A	F	HM	3' - 0"	7' - 0"	1 3/4"	M1	HM	D4/A501	C4/A501	B4/A501						X		X	X	X							X		X		X	X	X									
	101 B	OH1	STL	16' - 0"	10' - 0"			STL	D3/A501	C3/A501				ALL HARDWARE BY DOOR MANUF																													
	101 C	F	HM	3' - 0"	7' - 0"	1 3/4"	M1	HM	D4/A501	C4/A501	B4/A501						X		X	X	X							X		X		X	X										
	101 D	F	HM	3' - 0"	7' - 0"	1 3/4"	M2	HM	B3/A501	A3/A501				PAIR OF DOORS IN (E) WALL												X					X		X		X	X		X				X	
	101 E	F	HM	3' - 0"	7' - 0"	1 3/4"	M2	HM	B3/A501	A3/A501				PAIR OF DOORS IN (E) WALL												X					X		X		X	X		X				X	

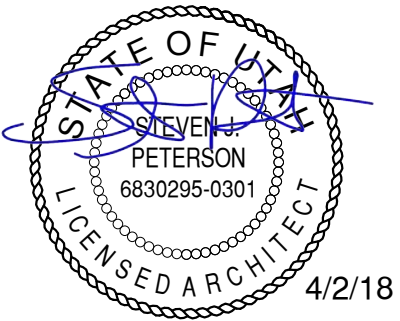
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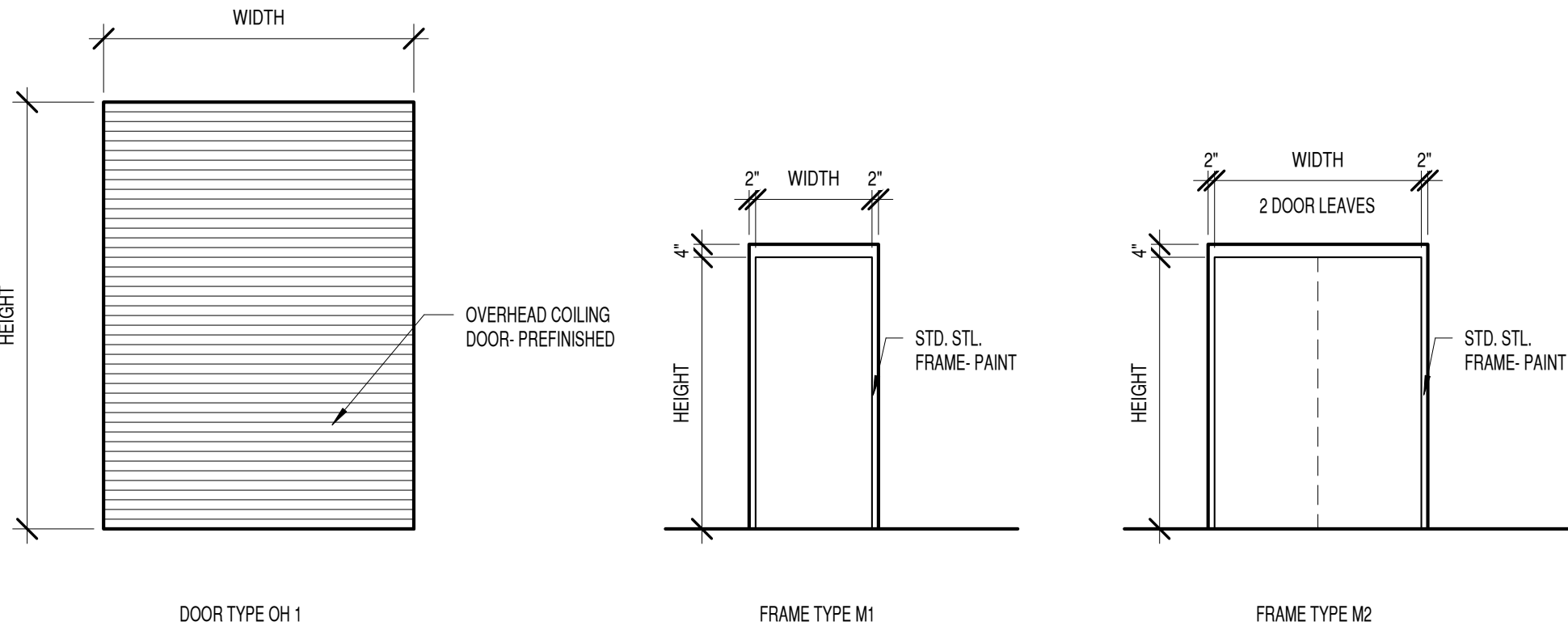
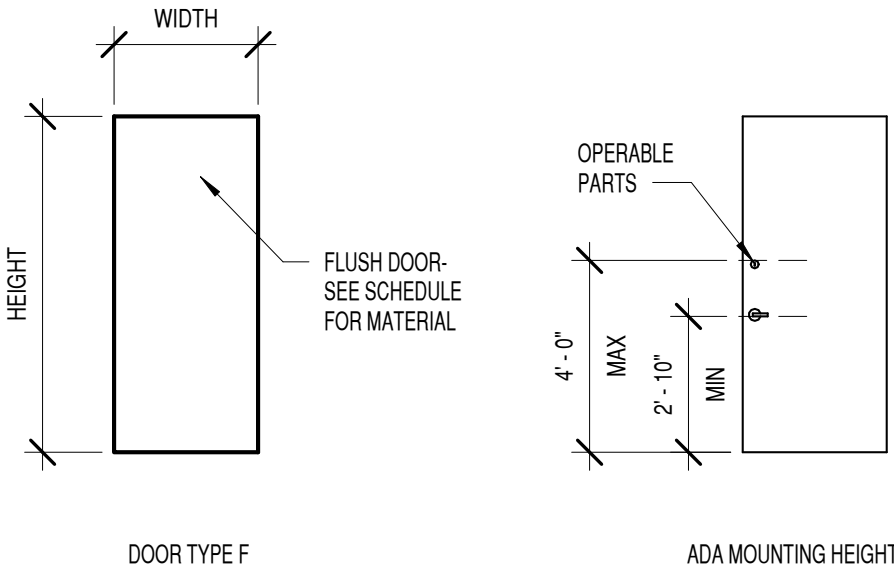
27 MARCH, 2018

SHEET TITLE

DOOR TYPE AND
SCHEDULE

SHEET NO:

A601



A3 DOOR AND FRAME TYPES
1/4" = 1'-0"

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1

2

3

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5

PACKAGE HVAC UNIT SCHEDULE

MARK	MANUFACTURER & MODEL	DESCRIPTION	AREA SERVED	REFRIG	ACFM	OUTSIDE AIR CFM	EXTERNAL STATIC PRESSURE (IN)	FAN RPM	HEATING ELECTRICAL			UNIT ELECTRICAL (NOT INCLUDING ELECTRIC HEAT)			COOLING COIL			DIMENSIONS LxWxH (IN)	WEIGHT (LBS)	NOTES					
									EDB	LDB	KW	TOTAL MBH	VOLTS/PHASE	MCA	MOP	VOLTS/PHASE	EER RATING				ROWS	FPI	EDB/EWB (°F)	LDB/LWB (°F)	TOTAL MBH
RTU 1	CARRIER 50TC-D28D1A6	ROOF TOP UNIT	PUMP ROOM	R-410A	7,800	--	0.5	774	60.0	70.9	23	785	460 / 3	51.9	60	460 / 3	10.0	--	--	75 / 60	48 / 47.9	238.6	142x87x57	2877	2 STAGE ELECTRIC HEAT. SUPPLY AND RETURN DUCT DETECTORS, SEISMIC RATED SLOPED ROOF CURB, ECONOMIZER w/ BAROMETRIC RELIEF, NON-FUSED DISCONNECT, 2-SPEED INDOOR FAN MOTOR CONTROLLED BY VFD, LOW AMBIENT CONTROL, WINTER START PACKAGE, COMFORT PRO PROGRAMMABLE THERMOSTAT, 16x25x2 FILTERS - QTY (9).

NOTE: ALL PERFORMANCE BASED ON SITE ELEVATION OF 4400 FT ABOVE SEA LEVEL.

MECHANICAL GENERAL NOTES:

1. ALL EQUIPMENT MANUFACTURES SHOWN AS A BASIS OF DESIGN. NOT INTENDED TO SOLE SOURCE EQUIPMENT MANUFACTURER.

2. ALL EQUIPMENT SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST STATE ADOPTED EDITION OF THE INTERNATIONAL MECHANICAL CODE AND SMACNA.

3. MECHANICAL PLANS ARE DIAGRAMMATIC ONLY. CONTRACTOR SHALL COORDINATE THEIR WORK WITH OTHER TRADES, AND ACTUAL JOB SITE CONDITIONS. CONTRACTOR TO FIELD VERIFY QUANTITIES AND DIMENSIONS.

4. CONTRACTOR TO PROVIDE ALL NECESSARY MATERIALS, DUCTWORK, HANGERS, FITTINGS, OFFSETS, INSULATION AND ACCESSORIES LOGICALLY REQUIRED FOR A COMPLETE FUNCTIONAL AIR DELIVERY SYSTEM.

5. DUCT DIMENSIONS ON DRAWINGS ARE INSIDE DIMENSIONS. MINIMUM DUCTWORK GAUGE TO BE 26 GAUGE.

6. CONTRACTOR SHALL COORDINATE ALL SUPPLY DIFFUSER PLACEMENTS.

7. ALL SQUARE ELBOWS IN SUPPLY AND RETURN DUCTWORK SHALL HAVE SINGLE THICKNESS TURNING VANES.

8. CONNECTIONS TO SUPPLY DIFFUSERS TO BE MADE WITH A RIDGED CONNECTION SO THAT CLEAR AND UNOBSTRUCTED AIRFLOW IS ACHIEVED.

9. MOUNT BOTTOM OF THERMOSTAT 48 INCHES ABOVE FINISHED FLOOR. RUN WIRING FROM THERMOSTAT LOCATION TO AIR HANDLER AND TERMINATE TO PROVIDE FOR A FULLY FUNCTIONAL SYSTEM. REFER TO THE CONTROL DRAWINGS FOR SPECIFICS.

10. CONTRACTOR TO FURNISH FILTERS.

11. CONTRACTOR TO FURNISH AND INSTALL CONDENSATE P-TRAP ON ALL NEW AIR HANDLERS PER DETAILS SHOWN ON DRAWING.

12. ALL PIPING THAT COMES IN CONTACT WITH A DISSIMILAR METAL TO BE PROTECTED AGAINST GALVANIC CORROSION.

13. SEISMIC SUPPORTS ARE NOT REQUIRED FOR HVAC DUCTWORK IF DUCTS ARE SUSPENDED FROM HANGARS 12 INCH OR LESS IN LENGTH.

14. REFER TO STRUCTURAL DETAILS FOR ALL EQUIPMENT AND DUCT PENETRATIONS THROUGH ROOF. IF DETAIL IS NOT PRESENT THEN CONTACT ENGINEER.

15. ALL EXPOSED DUCTWORK TO HAVE ALL LABELS AND WRITING REMOVED FROM DUCT.

16. AFTER AIR AND HYDRONIC SYSTEM BALANCING HAS BEEN COMPLETED, MARK ALL BALANCING DAMPER AND BALANCING VALVES TO PERMANENTLY INDICATE FINAL POSITION; IE AN ARROW OR DRAWING AN OUTLINE OF BALANCING HANDLE POSITION.

17. PIPING SEISMIC SUPPORTS SHALL NOT BE REQUIRED IF PIPING IS SUPPORTED BY ROD HANGERS; HANGERS IN THE PIPE RUN 12 INCH OR LESS IN LENGTH FROM TOP OF THE PIPE TO THE SUPPORTING STRUCTURE. PIPING WITH ROD HANGERS; HANGERS IN THE PIPE RUN OVER 12 INCHES IN LENGTH FROM THE TOP OF THE PIPE TO THE SUPPORT STRUCTURE WILL BE REQUIRED TO BE SEISMICALLY RESTRAINED.

18. EXISTING UTILITY LOCATIONS AND SIZES ARE ESTIMATED. CONTRACTOR TO FIELD VERIFY.

PUMP SCHEDULE

MARK	MANUFACTURER & MODEL	SYSTEM SERVED	TYPE	GPM	HEAD (FT)	FLUID	FLUID TEMP (°F)	IMPELLER DIA (IN)	ELECTRICAL		WEIGHT	NOTES	
									RPM	HP			VOLT/PHASE
P 1	ARMSTRONG 4280-4x3x8-7.5	CT 1	CENTRIFUGAL	225	57.75	WATER	75	8	1765	7.5	460 / 3	256	4" INLET, 3" OUTLET, SG-44 SUCTION GUIDE PREMIUM EFFICIENT MOTOR, PROVIDE YASKAWA VFD.
P 2	ARMSTRONG 4280-4x3x8-7.5	CT 1	CENTRIFUGAL	225	57.75	WATER	75	8	1765	7.5	460 / 3	256	4" INLET, 3" OUTLET, SG-44 SUCTION GUIDE PREMIUM EFFICIENT MOTOR, PROVIDE YASKAWA VFD.

PIPING AND EQUIPMENT INSULATION SCHEDULE

SYMBOL	SYSTEM	EXPOSED	CONCEALED	ABOVE CEILING	EXTERIOR EXPOSED	THICKNESS	INDOOR FIELD APPLIED JACKET	OUTDOOR FIELD APPLIED JACKET
CS CR	CONDENSER	PRE-FORMED MINERAL FIBER	PRE-FORMED MINERAL FIBER	PRE-FORMED MINERAL FIBER	--	SEE THICKNESS TABLE BELOW	--	ALUMINUM

MINIMUM PIPE INSULATION THICKNESS TABLE

FLUID DESIGN OPERATING TEMP. RANGE (°F)	INSULATION CONDUCTIVITY		NOMINAL PIPE OR TUBE SIZE (in.)				
	CONDUCTIVITY Btu-in/(h-ft²-°F)	MEAN RATING TEMP. °F	<1"	1" TO <1 1/2"	1 1/2" TO <4"	4" TO <8"	≥8"
HEATING SYSTEMS (HOT WATER HEATING)							
141-200	0.25-0.29	125	1.5	1.5	2.0	2.0	2.0
105-140	0.22-0.28	100	1.0	1.0	1.5	1.5	1.5
COOLING SYSTEMS (CHILLED WATER, AND REFRIGERANT)							
40-60	0.21-0.27	75	0.5	0.5	1.0	1.0	1.0
<40	0.20-0.26	50	0.5	1.0	1.0	1.0	1.5

NOTES:
1. ANY PIPE ON THIS SCHEDULE WITHIN THE FACILITY TO BE INSULATED AS DEFINED.
2. PIPING INSULATION IS NOT REQUIRED BETWEEN THE CONTROL VALVE AND COIL ON RUN-OUTS WHEN THE CONTROL VALVE IS LOCATED WITHIN 4 FT OF THE COIL AND THE PIPE SIZE IS 1 IN. OR LESS.
3. THESE THICKNESSES ARE BASED ON ENERGY EFFICIENCY CONSIDERATIONS ONLY. ISSUES SUCH AS WATER VAPOR PERMEABILITY OR SURFACE CONDENSATION SOMETIMES REQUIRE VAPOR RETARDERS OR ADDITIONAL INSULATION.

PIPING MATERIAL SCHEDULE

SYSTEM	MATERIAL	JOINTS	NOTES
DOMESTIC HOT & COLD WATER	TYPE 'L' COPPER	3" & SMALLER BELOW GRADE BRAZED 3" & SMALLER ABOVE GRADE SOLDERED	OR POLYPROPYLENE OR PRO PRESS
CONDENSER WATER	SCH 40 BLACK STEEL	GROVED COUPLING (VICTAULIC OR EQUAL)	
COMPRESSED AIR	GALVANIZED STEEL	THREADED	
CONDENSATE DRAIN	OUTDOOR: COPPER INDOOR: PVC	O: PRO PRESS I: SOLVENT CEMENT	

COOLING TOWER SCHEDULE

MARK	MANUFACTURER & MODEL	HEAT REJECTION MBH	GPM	EWT/LWT (°F)	AMB WB (°F)	MAKE-UP WATER SIZE (IN)	PIPING CONN SIZE (IN)	ELECTRICAL			DIMENSIONS LxWxH (FT)	OPERATING WEIGHT (LBS)	NOTES	
								FAN		BASIN HEATER				
								RPM	MOTOR HP					VOLT/PHASE
CT 1	MARLEY 495K	810	225	82.2 / 75	67	3/4	6	1800	5	460 / 3	4.5	8 x 6.5 x 9	3853	PROVIDE YASKAWA VFD.

SUMP PUMP SCHEDULE

MARK	MANUFACTURER & MODEL	SYSTEM SERVED	TYPE	GPM	HEAD (FT)	FLUID	FLUID TEMP	MOTOR		WEIGHT (LBS)	NOTES	
								RPM	HP			VOLT/PHASE
SP 1	LIBERTY 40S-LTS	COOLING TOWER BLOWDOWN	DRAIN PUMP	23	25	WATER	AMBIENT	3450	1/2	120 / 1	23	10 FT CORD, 2 IN DISCHARGE, AUTOMATIC OPERATION, 2 IN INLET, 2 IN VENT

MECHANICAL LEGEND

SYMBOL	DESCRIPTION
	DOMESTIC COLD WATER PIPING (DCW)
	COMPRESSED AIR PIPING
	CONDENSER SUPPLY
	CONDENSER RETURN
	MANUAL VOLUME CONTROL DAMPER
	EXHAUST AIR DUCT - UP / DOWN
	RETURN/OUTSIDE AIR DUCT - UP / DOWN
	SUPPLY AIR DUCT - UP / DOWN
	MITERED ELBOW W/ TURNING VANES
	GATE VALVE
	GLOBE VALVE
	BALL VALVE
	BUTTERFLY VALVE
	CALIBRATED BALANCING VALVE
	CHECK VALVE
	2-WAY CONTROL VALVE
	3-WAY CONTROL VALVE
	STRAINER WITH BLOWDOWN
	CONCENTRIC REDUCER
	ECCENTRIC REDUCER
	VENTURI
	THREADED END FOR HOSE CONNECTION
	BALL VALVE W/ NIPPLE AND CAP
	FLEX CONNECTION
	DIRECTION OF FLOW
	BLIND FLANGE OR CAP
	PIPING DOWN
	PIPING UP
	PIPING TEE DOWN
	FLANGE
	UNION
	THERMOMETER
	PRESSURE GAUGE
	AIR LINE QUICK CONNECT
	PUMP
	THERMOSTAT
	EQUIPMENT SYMBOL
	DETAIL SYMBOL
	SHEET KEYNOTE
	POINT OF CONNECTION
	AIRFLOW DIRECTION ABOVE FINISH FLOOR
	EXISTING
	NOT IN CONTRACT
	NORMALLY OPEN
	NORMALLY CLOSED
	TYPICAL

CLH ARCHITECTS ENGINEERS

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Parker

ADDITION TO THE CONTROL SYSTEMS DIV. PLANT PARKER/HANNIFIN CORP. 1425 WEST 2675 NORTH OGDEN, UTAH

MARK	DATE	DESCRIPTION
ISSUE DATE: 27 MARCH, 2018		
PROJECT NO: 18110		
CAD DWG FILE:		
DRAWN BY: GM, JAC		
CHK'D BY: MPM		

PERMIT SET

27 MARCH, 2018

SHEET TITLE

MECHANICAL SCHEDULES

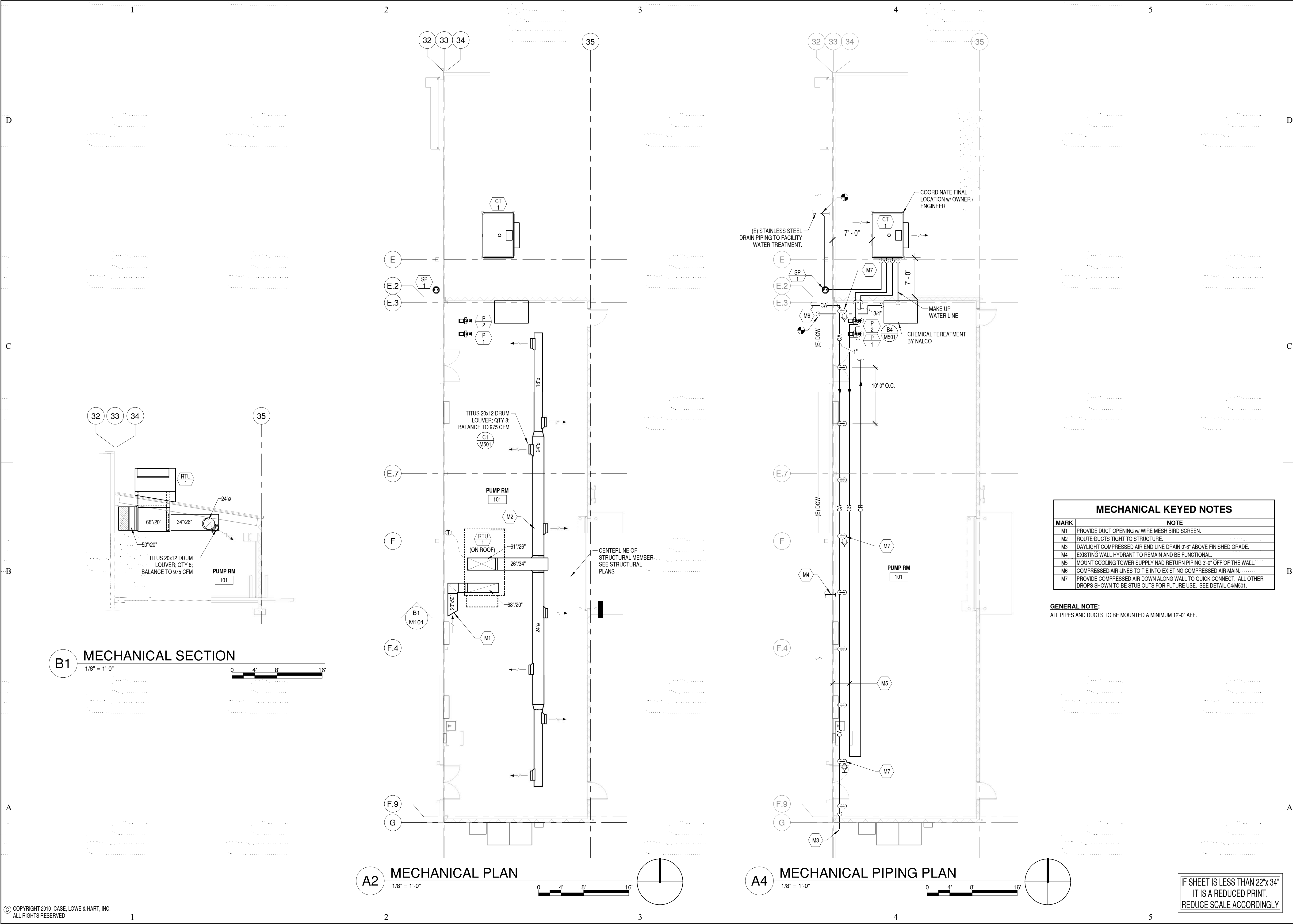
SHEET NO:

M001

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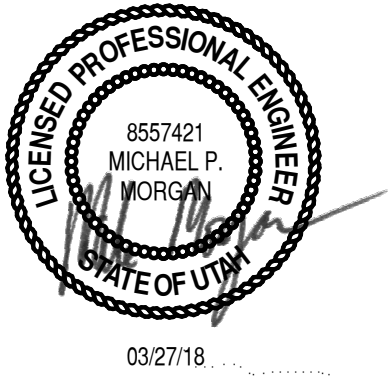
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CONTROL SYSTEMS DIV.
PLANT
PARKER/HANNIFIN CORP.

1425 WEST 2675 NORTH OGDEN, UTAH

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SHEET TITLE

MECHANICAL
PLANS

SHEET NO:

M101

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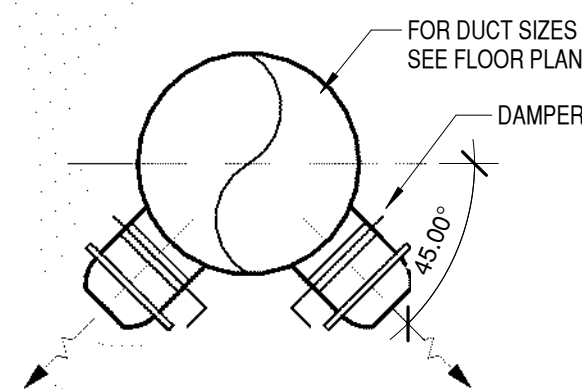
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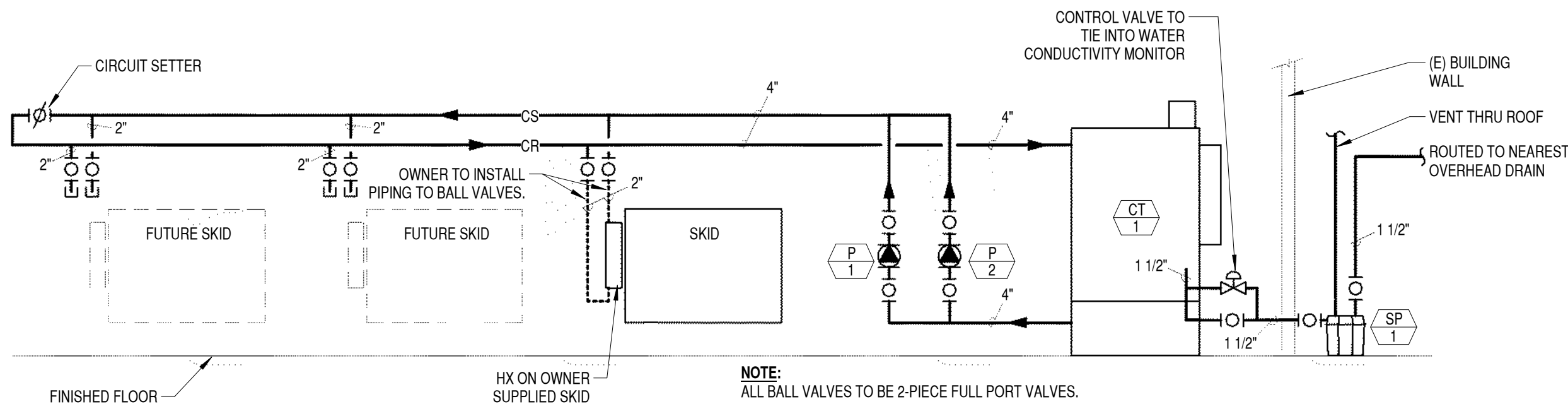
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NO SCALE

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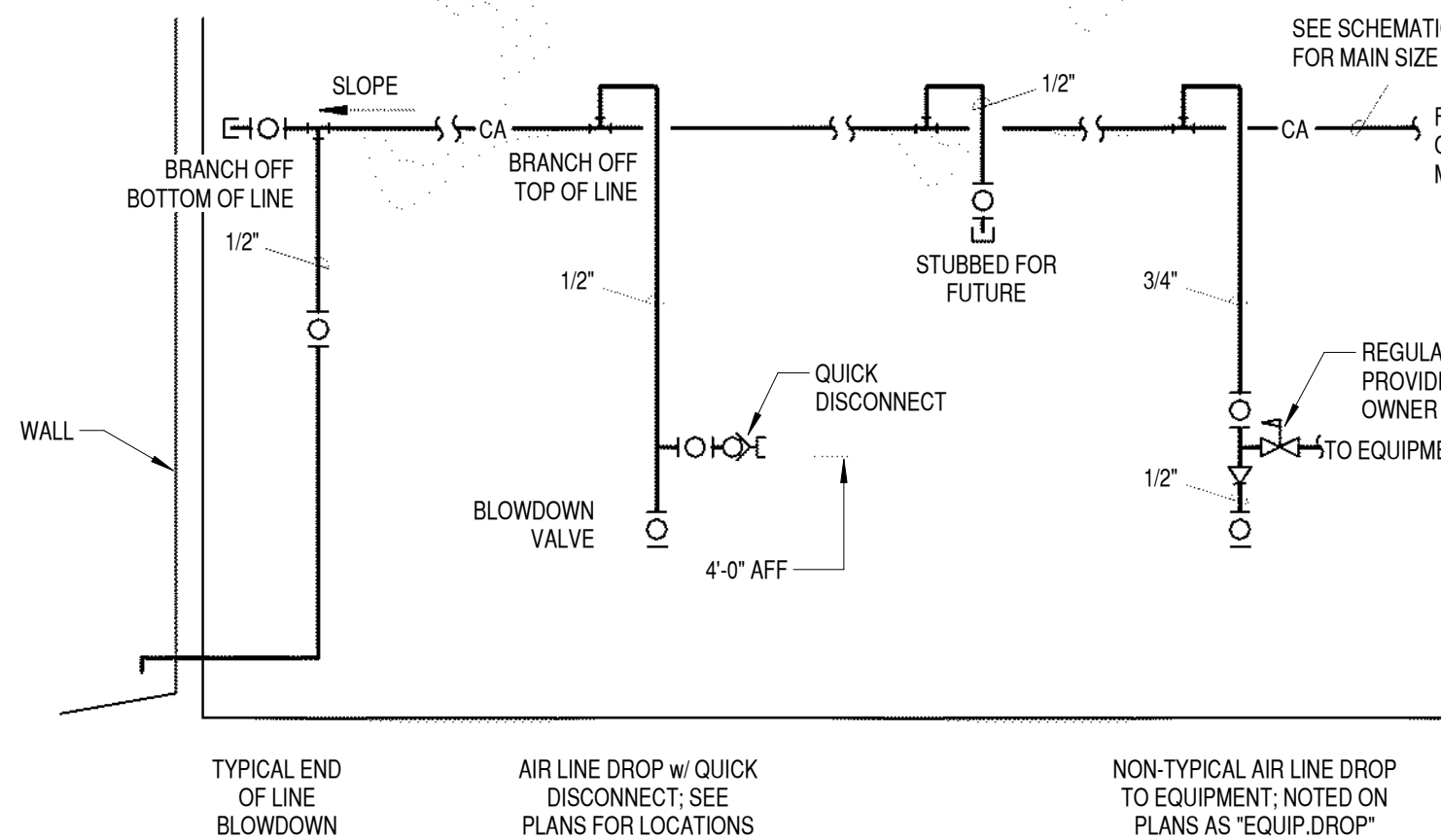
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CHILLED WATER PIPING SCHEMATIC

C2

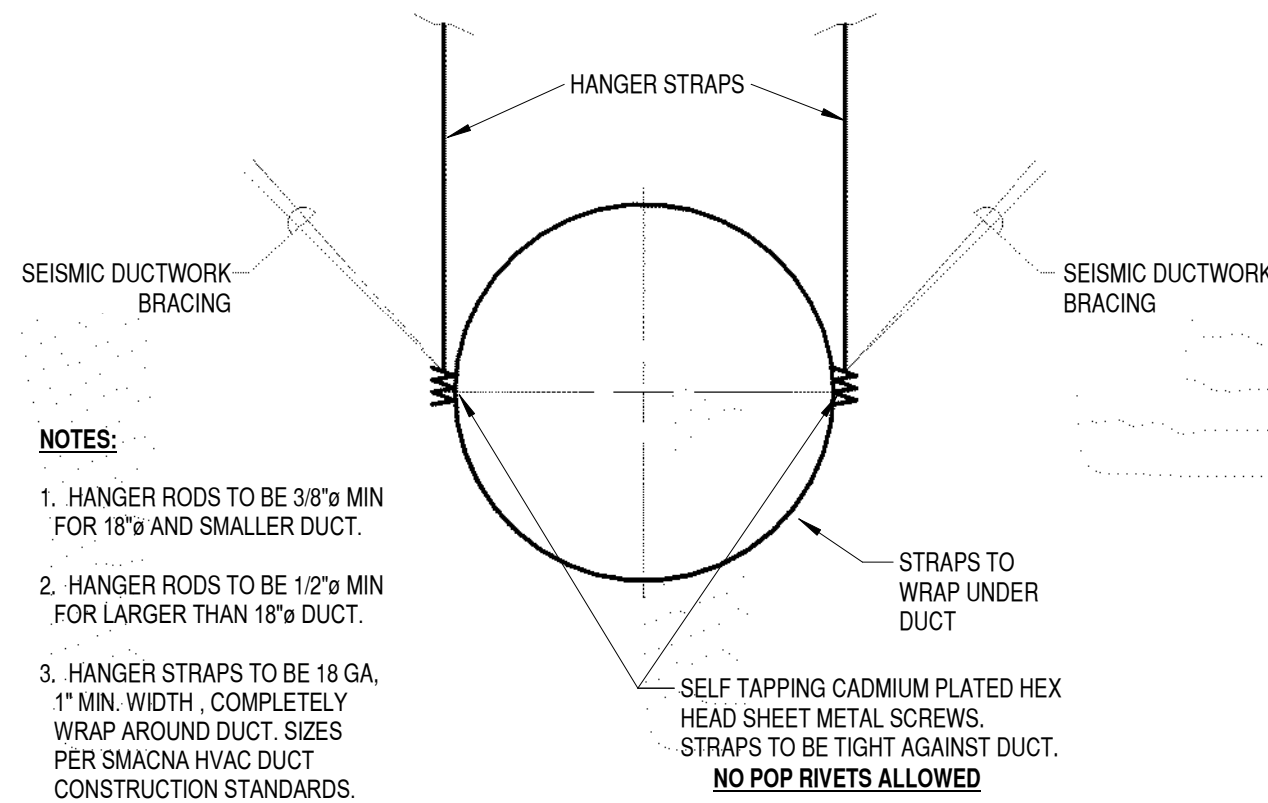
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COMPRESSED AIR LINE BLOWDOWN AND AIR DROP DETAILS

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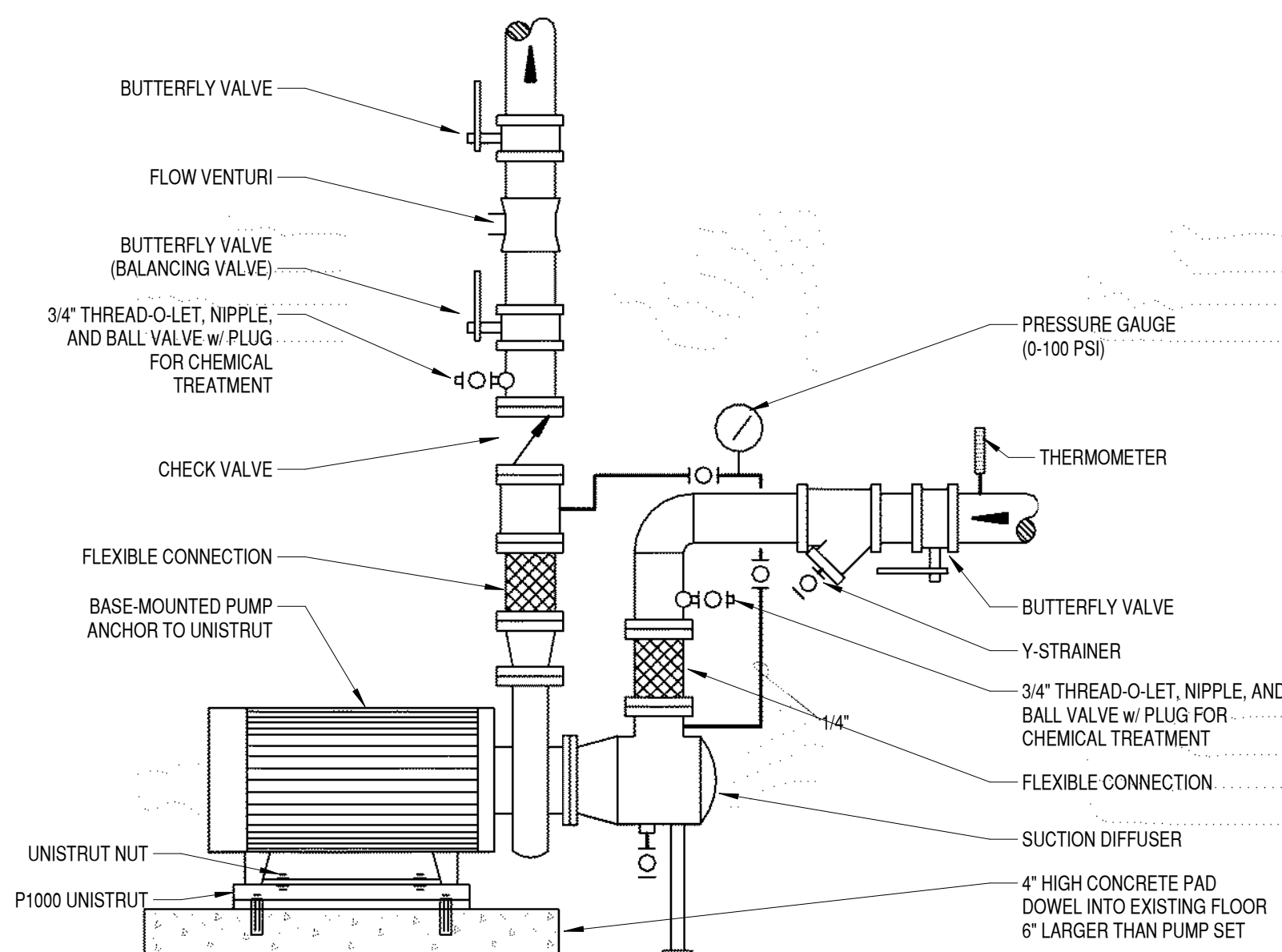
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ROUND DUCT HANGER WITH SEISMIC BRACING DETAIL

C4

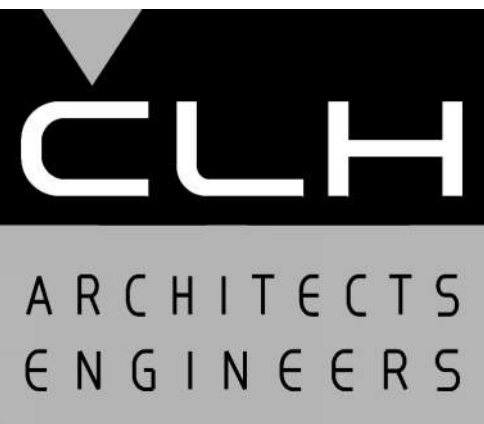
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PUMP PIPING DETAIL

B4

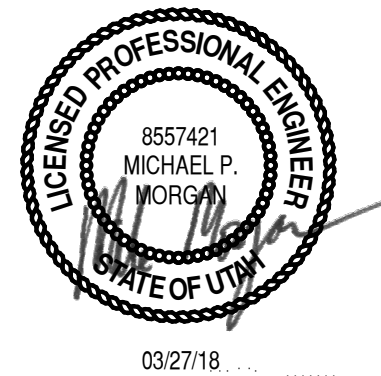
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MECHANICAL DETAILS

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
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


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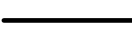
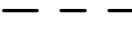


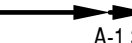
5

- LIGHTING**
-  FIXTURE - CHAIN, RECESSED, SURFACE OR PENDANT HUNG
NUMBER INDICATES FIXTURE TYPE
-  FIXTURE - RECESSED OR SURFACE MOUNT WITH EMERGENCY
BATTERY PACK, NUMBER INDICATES FIXTURE TYPE
-  WALL MOUNTED LIGHT FIXTURE,
NUMBER INDICATES TYPE OF FIXTURE










SWITCHES (+48" UNLESS NOTED)

-  SINGLE POLE SWITCH
-  THREE-WAY SWITCH
-  FOUR-WAY SWITCH











CIRCUITING

-  WIRING CONCEALED IN CEILING OR WALL
-  WIRING CONCEALED IN FLOOR
-  WIRING EXISTING
-  CROSSLINES INDICATE NUMBER OF #12 CONDUCTORS.
GROUND IS REPRESENTED BY CROSSLINE WITH DOT
ON TOP, OTHER CONDUCTORS AND 1/2" CONDUIT AS
INDICATED.
-  BRANCH CIRCUIT HOMERUN TO PANELBOARD; NUMBER OF ARROWS
INDICATE NUMBER OF CIRCUITS. LETTER AND NUMBER NOTATION
IDENTIFIES PANEL AND CIRCUIT NUMBER(S).







PANELBOARDS AND POWER EQUIPMENT

-  SURFACE MOUNTED PANELBOARD AND CABINET
-  SWITCHBOARD OR DISTRIBUTION PANELBOARD
(AS NOTED)
-  MANUAL MOTOR STARTER
-  MAGNETIC MOTOR STARTER
-  COMBINATION STARTER & DISCONNECT SWITCH
-  FUSED DISCONNECT
-  MOTOR OUTLET, HORSEPOWER AS INDICATED
-  PUSHBUTTON STATION
-  REFERS TO MECHANICAL OR OWNERS EQUIPMENT ITEM
SEE SCHEDULES


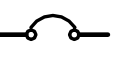
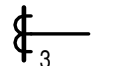
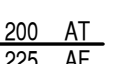
RECEPTACLES (+18" UNLESS NOTED)

-  DUPLEX RECEPTACLE, NEMA 5-20R, GROUNDING TYPE
-  QUAD RECEPTACLE (DOUBLE DUPLEX),
NEMA 5-20R, GROUNDING TYPE
-  SPECIAL PURPOSE OUTLET - 208 V., 1 PHASE - NUMBER INDICATES
AMPERAGE. NO NUMBER INDICATES 20 AMPS
-  SPECIAL PURPOSE OUTLET - 208 V., 3 PHASE - NUMBER INDICATES
AMPERAGE. NO NUMBER INDICATES 20 AMPS
-  SPECIAL PURPOSE OUTLET - 480 V., 1 PHASE - NUMBER
INDICATES AMPERAGE. NO NUMBER INDICATES 20 AMP
-  SPECIAL PURPOSE OUTLET - 480 V., 3 PHASE - NUMBER
INDICATES AMPERAGE. NO NUMBER INDICATES 20 AMPS
-  JUNCTION BOX
-  TRANSIENT VOLTAGE SURGE SUPPRESSOR
-  GROUND ROD - 3/4" x 10'-0"
-  THERMOSTAT

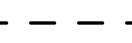
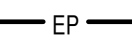

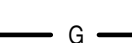

FIRE ALARM/MNS SYSTEM

-  MANUAL PULL STATION (+48" AFF)
-  FIRE ALARM - STROBE SPEAKER/HORN,
NUMBER INDICATES STROBE CANDELLA RATING
MOUNT NO HIGHER THAN 6'-6" AFF
-  CEILING MOUNTED FIRE ALARM STROBE SPEAKER/HORN,
NUMBER INDICATES STROBE CANDELLA RATING
-  SMOKE DETECTOR
-  SMOKE DETECTOR SLEEPING ROOM
-  DUCT SMOKE DETECTOR


ONE-LINE DIAGRAM


-  TRANSFORMER
-  MOLDED CASE CIRCUIT BREAKER
-  CURRENT TRANSFORMER, NUMBER
AS INDICATED
-  CIRCUIT BREAKER TRIP SETTING
CIRCUIT BREAKER FRAME SIZE

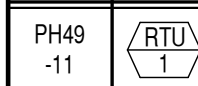
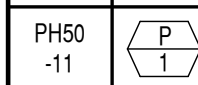
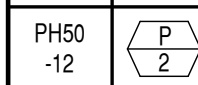
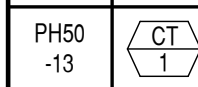
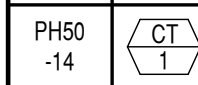
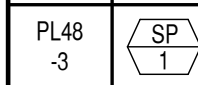
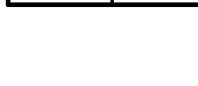
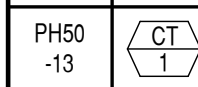
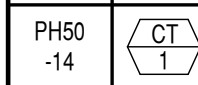
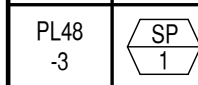
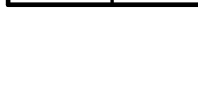

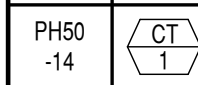
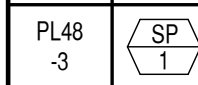
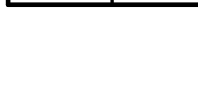


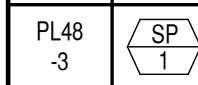
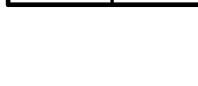
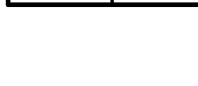
EXTERIOR (NEW)

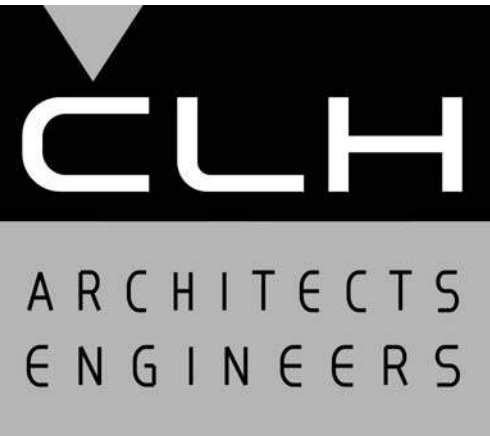
-  UNDERGROUND DUCT LINE
-  PRIMARY CIRCUIT
-  SECONDARY CIRCUIT
-  4/0 BARE COPPER GROUND WIRE PLACED IN OR
ALONGSIDE FOOTINGS
-  TRANSFORMER

ABBREVIATIONS

-  KEYED NOTE CALLOUT - NUMBER AS INDICATED
- 3R NEMA 3R ENCLOSURE
- 12 NEMA 12 ENCLOSURE
- 4 NEMA 4 ENCLOSURE
- 4X NEMA 4X ENCLOSURE
- A AMPERE
- AFF ABOVE FINISHED FLOOR
- AIC AMPERES INTERRUPTING CAPACITY
- APPROX APPROXIMATELY
- BC BARE COPPER
- C CONDUIT
- CB CIRCUIT BREAKER
- CKT CIRCUIT
- CO CONDUIT ONLY
- CONC CONCRETE
- CT CURRENT TRANSFORMER
- CU COPPER
- (E) EXISTING
- EMCS ENERGY MONITORING AND CONTROL SYSTEM
- EMT ELECTRICAL METALLIC TUBING
- FA FIRE ALARM
- FLR FLOOR
- FT FEET
- GFI GROUND FAULT CIRCUIT-INTERRUPTER
- GND or GRD GROUND
- IN INCHES
- KVA KILOVOLT AMPERE
- KVAR KILOVOLT CAPACITANCE
- KWH KILOWATT-HOUR
- LED LIGHT EMITTING DIODE
- MAX MAXIMUM
- MIN MINIMUM
- (N) NEW
- NEC NATIONAL ELECTRICAL CODE
- NEMA NATIONAL ELECTRICAL MANUFACTURING
ASSOCIATION
- NIC NOT IN CONTRACT
- NL NIGHT LIGHT ON UNSWITCHED CIRCUIT
- OFOI OWNER FURNISHED OWNER INSTALLED
- OFCI OWNER FURNISHED CONTRACTOR INSTALLED
- O.C. ON CENTER
- O.H. OVERHEAD
- RM ROOM
- RGC RIGID GALVANIZED CONDUIT
- TYP TYPICAL
- UON UNLESS OTHERWISE NOTED
- V VOLT
- W WATT
- w/ WITH
- WP WEATHERPROOF
- XFMR TRANSFORMER
- +12" MOUNTING HEIGHT ABOVE FINISHED FLOOR
OR GRADE

LIGHTING FIXTURE SCHEDULE															NOTE: ALL INTERIOR & EXTERIOR LIGHTING CONTROLS TO BE COMMISSIONED		
NO.	DESCRIPTION	VOLTS	MTG.	LENS	FINISH	LAMPS				BALLASTS				MAXIMUM INPUT WATTS	MANUFACTURER & CATALOG NUMBER (NO SUBSTITUTIONS WITHOUT PRIOR APPROVAL)	DETAILS	
						TYPE			NO. OF LAMPS	WATTS/LAMP TYPE	S	TYPE					NO. PER LUMINAIRE
						LED	F	H				E	O				
1	LED STRIPLIGHT	120/277	-	ACRYLIC	WHITE	*			1	LED 5000K		*		1	104	LITHONIA TZL1N-L96-14000LM-FST-MVOLT-50K-80CRI-CS93W-WH-ZACVH	
1E	LED STRIPLIGHT	120/277	-	ACRYLIC	WHITE	*			1	LED 5000K		*		1	104	LITHONIA TZL1N-L96-14000LM-FST-MVOLT-50K-80CRI-E7W-CS93W-WH-ZACVH	
2	LED WALLPACK	120/277	WALL + 15 FT.	ACRYLIC	DARK BRONZE	*			1	LED 5000K		*		1	85	CREE C-WP-A-TRAD-8L-50K-DB	
3	-	-	-	-	-		*		-	-		*		-	-	-	
4	-	-	-	-	-		*		-	-		*		-	-	-	
	EXIT LIGHT	120/277	WALL OR CEILING SURFACE	NONE	-		*		-	-		*		-	3	-	

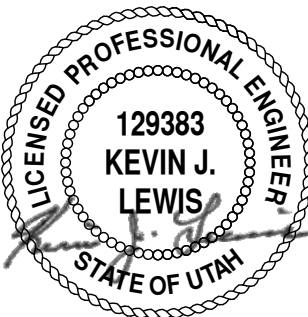
EQUIPMENT SCHEDULE																
CRKT.	EQUIP. NO.	DESCRIPTION	VOLTS	PHASE	WATTS H.P.	BRK	STARTERS SIZE	CONTROL		PILOT		CONTACTS		CONTROL TRANS.	REMARKS	
PH49 -11		ROOFTOP UNIT	480	3	43 KW	60									FUSE PER MANUFACTURER NEMA 3R	
PH50 -11		CIRC. PUMP	480	3	7.5 HP	20	1					2	2		YASKAWA VFD FURNISHED WITH PUMP	
PH50 -12		CIRC. PUMP	480	3	7.5 HP	20	1					2	2		YASKAWA VFD FURNISHED WITH PUMP	
PH50 -13		COOLING TOWER	480	3	5 HP	20	1					2	2		YASKAWA VFD FURNISHED WITH TOWER NEMA 3R	
PH50 -14		SUMP HEATER	480	3	4.5 KW	20									NEMA 3R	
PL48 -3		COOLING TOWER DRAIN PUMP	120	1	1/2 HP	20	\$								MOTOR RATED SWITCH	



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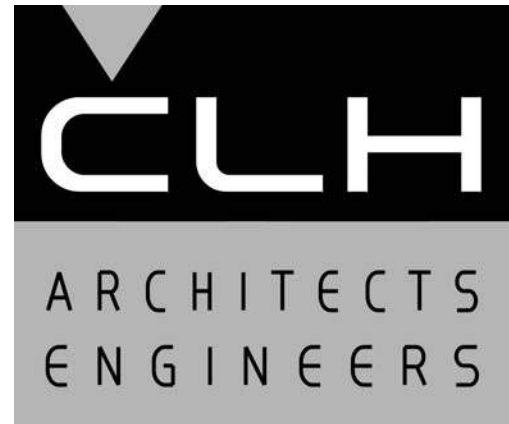
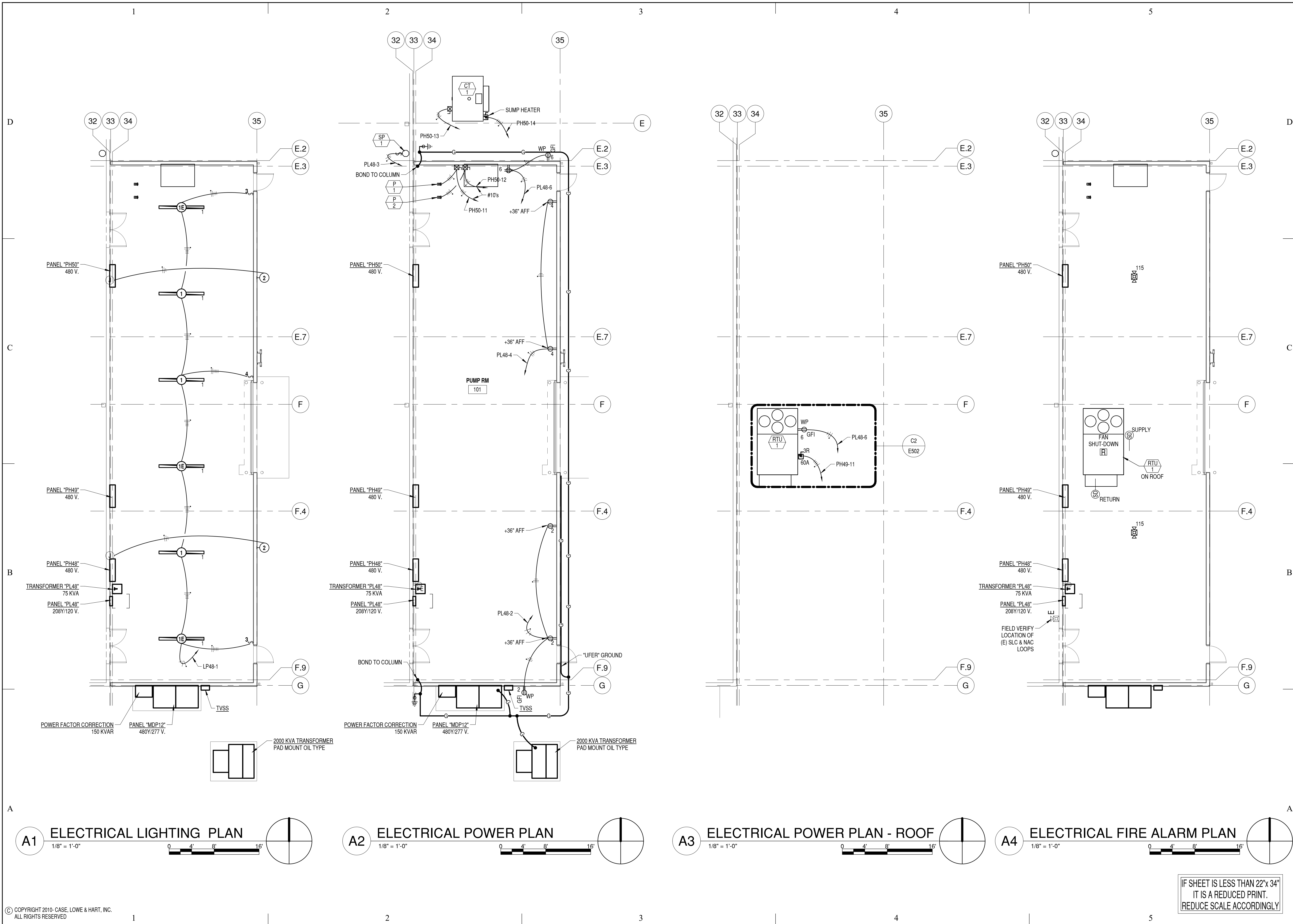
ELECTRICAL
LEGEND,
SCHEDULES &
DETAILS

SHEET NO:

E001

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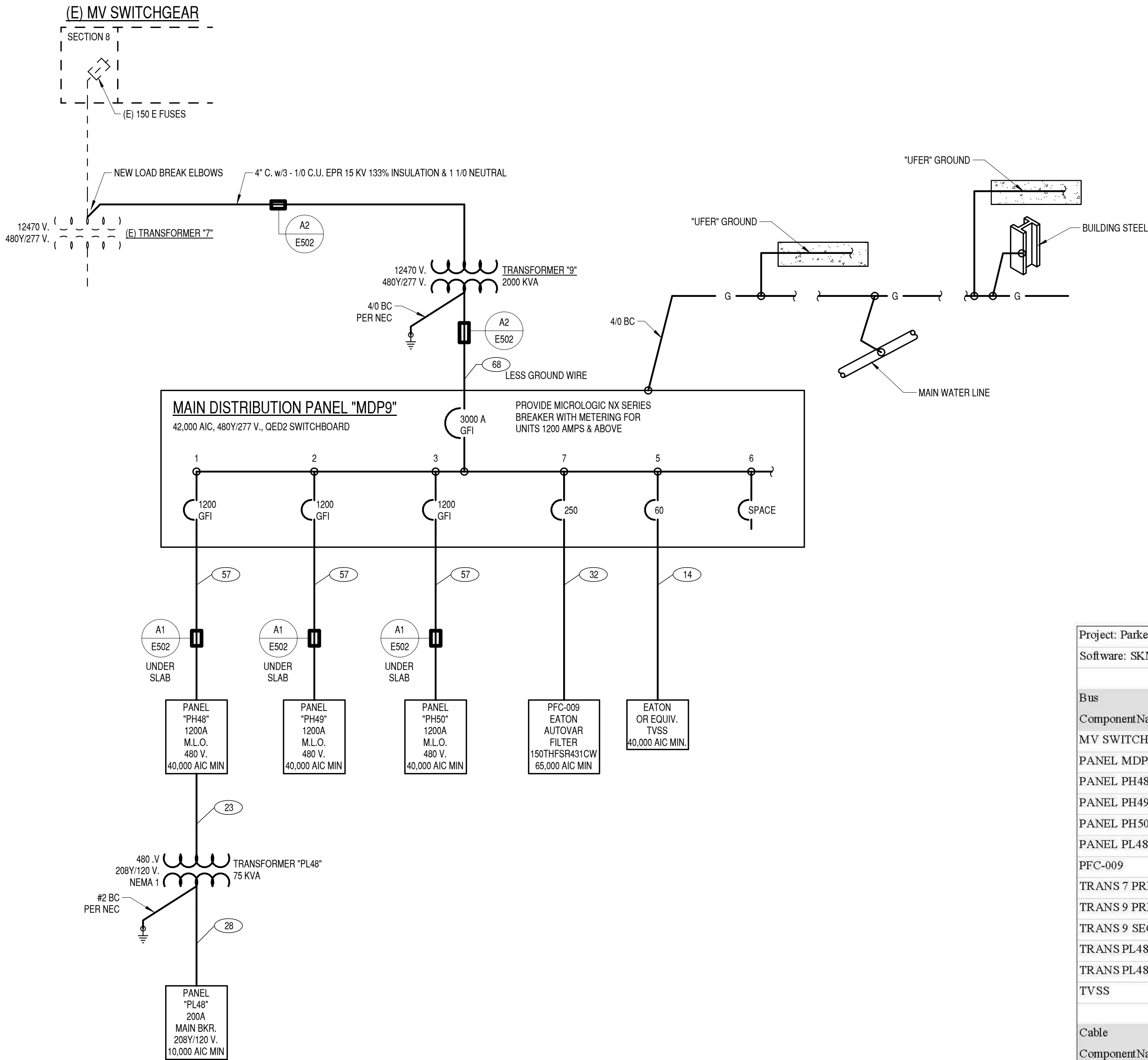
ENLARGED
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PLANS

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CONDUIT AND CONDUCTOR SCHEDULE (COPPER)						
TYPE	AMPS (1)	CONDUIT SIZE (3)	CONDUCTOR		NOTES	
			QTY	SIZE (1) GND (2)		
1	20	0.75	2	12 12	-	
2	20	0.75	3	12 12	-	
3	20	0.75	4	12 12	-	
4	30	0.75	2	10 10	-	
5	30	0.75	3	10 10	-	
6	30	0.75	4	10 10	-	
7	40	0.75	2	8 10	-	
8	40	0.75	3	8 10	-	
9	40	0.75	4	8 10	-	
10	55	0.75	2	6 8	-	
11	55	0.75	3	6 8	-	
12	55	1	4	6 8	-	
13	70	1	2	4 8	-	
14	70	1	3	4 8	-	
15	70	1.25	4	4 8	-	
16	85	1.25	2	3 8	-	
17	85	1.25	3	3 8	-	
18	85	1.25	4	3 8	-	
19	95	1.25	3	2 6	-	
20	95	1.25	4	2 6	-	
21	110	1.25	3	1 6	-	
22	110	1.5	4	1 6	-	
23	150	1.5	3	1/0 6	-	
24	150	2	4	1/0 6	-	
25	175	2	3	2/0 6	-	
26	175	2	4	2/0 6	-	
27	200	2	3	3/0 6	-	
28	200	2	4	3/0 6	-	
29	230	2.5	3	4/0 4	-	
30	230	2.5	4	4/0 4	-	
31	255	2.5	3	250 4	-	
32	255	2.5	4	250 4	-	
33	310	2.5	3	350 3	-	
34	310	3	4	350 3	-	
35	380	3	3	500 3	-	
36	380	4	4	500 3	-	
37	400	2 EA 2	3	3/0 3	-	
38	400	2 EA 2	4	3/0 3	-	
39	420	3	3	600 2	-	
40	420	4	4	600 2	-	
41	460	2 EA 2	3	4/0 2	-	
42	460	2 EA 2.5	4	4/0 2	-	
43	510	2 EA 2.5	3	250 1	-	
44	510	2 EA 2.5	4	250 1	-	
45	620	2 EA 2.5	3	350 1/0	-	
46	620	2 EA 3	4	350 1/0	-	
47	760	2 EA 3	3	500 1/0	-	
48	760	2 EA 4	4	500 1/0	-	
49	820	2 EA 3	3	600 2/0	-	
50	820	2 EA 4	4	600 2/0	-	
51	855	3 EA 2.5	3	300 2/0	-	
52	855	3 EA 3	4	300 2/0	-	
53	1000	3 EA 3	3	400 2/0	-	
54	1000	3 EA 3	4	400 2/0	-	
55	1140	3 EA 3	3	500 3/0	-	
56	1140	3 EA 4	4	500 3/0	-	
57	1240	4 EA 2.5	3	350 3/0	-	
58	1240	4 EA 3	4	350 3/0	-	
59	1260	3 EA 3	3	600 3/0	-	
60	1260	3 EA 4	4	600 3/0	-	
61	1675	5 EA 3	4	400 4/0	-	
62	1680	4 EA 4	4	600 4/0	-	
63	2010	6 EA 3	4	400 250	-	
64	2100	5 EA 4	4	600 250	-	
65	2520	6 EA 4	4	600 350	-	
66	2660	7 EA 4	4	500 350	-	
67	2940	7 EA 4	4	600 400	-	
68	3040	8 EA 4	4	500 400	-	
69	4180	11 EA 4	4	500 500	-	
70	4200	10 EA 4	4	600 500	-	
NOTES:						
1	CONDUCTOR SIZE USING NEC TABLE 310-16; 60 DEG. C UP TO #1 AWG PER NEC 110.14(C)(1)(A)					
2	SIZED USING NEC TABLE 250.122					
3	CONDUIT SIZED FOR THHN OR THWN OR THWN-2 IN RNC (SCH 40)					



ELECTRICAL POWER RISER
DIAGRAM

B2 NONE

Project: Parker-Hannifin East Pump Room			
Software: SKM			
Bus			
ComponentName	InitSymRMS 3P (A)	LF VD% (%)	
MV SWITCH GEAR SECTION 8	9259828.00	0.00	
PANEL MDP9	40768.00	2.72	
PANEL PH48	39043.34	2.79	
PANEL PH49	37426.86	2.84	
PANEL PH50	35913.13	2.92	
PANEL PL48	5501.01	1.83	
PFC-009	35893.07	2.72	
TRANS 7 PRIMARY	95449.31	0.05	
TRANS 9 PRIMARY	65678.98	0.08	
TRANS 9 SECONDARY	41408.39	2.66	
TRANS PL48 PRIMARY	32601.44	2.82	
TRANS PL48 SECONDARY	5872.70	1.73	
TVSS	36541.55	2.72	
Cable			
ComponentName	CableSize (kcmil)	Length (ft)	Rpos (Ohms/1000 ft)
CBL-MDP9	8-500	20.0	0.0276
CBL-PFC-009	250	15.0	0.0552
CBL-PNL PH48	4-350	30.0	0.0368
CBL-PNL PH49	4-350	60.0	0.0368
CBL-PNL PH50	4-350	90.0	0.0368
CBL-PNL PL48	3/0	15.0	0.0805
CBL-TRANS 7	1/0 AL 15 KV	350.0	0.2100
CBL-TRANS 9	1/0 CU 15 KV	250.0	0.1304
CBL-TRANS PL48	1/0	15.0	0.1280
CBL-TVSS	4	5.0	0.3210

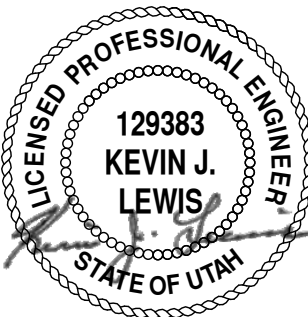
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03/27/2018



ADDITION TO THE
CONTROL SYSTEMS DIV.
PLANT
PARKER/HANNIFIN CORP.
1425 WEST 2675 NORTH OGDEN, UTAH

MARK DATE DESCRIPTION

ISSUE DATE: 27 MARCH, 2018
PROJECT NO: 18110
CAD DWG FILE:
DRAWN BY: J.M.S.
CHK'D BY: K.J.L.

PERMIT SET

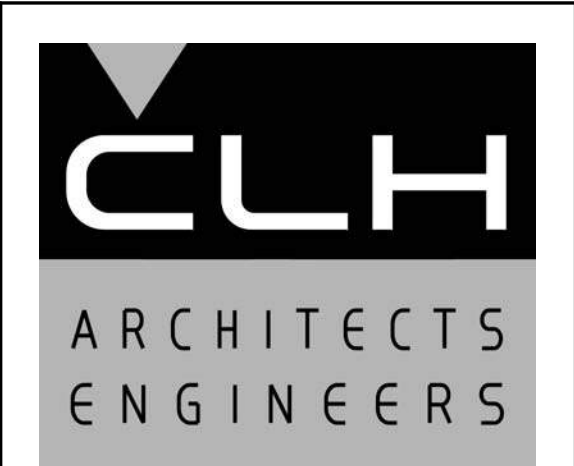
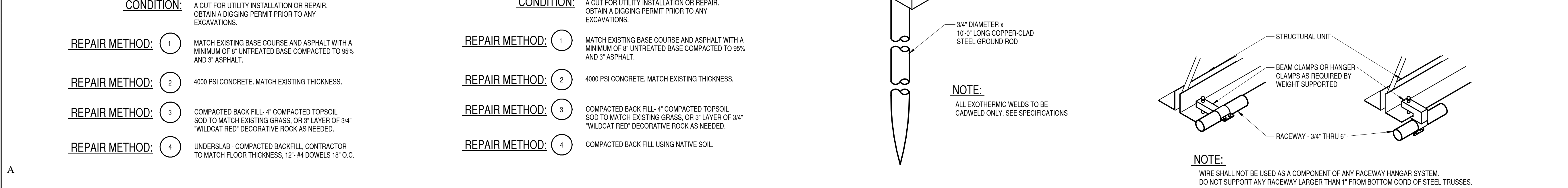
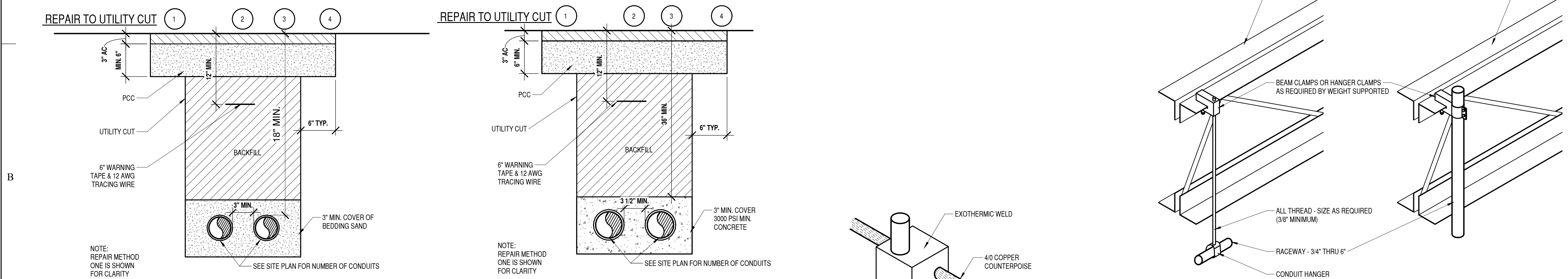
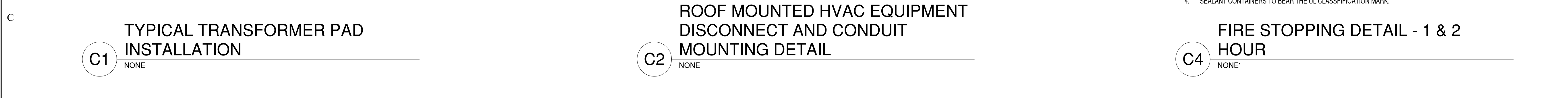
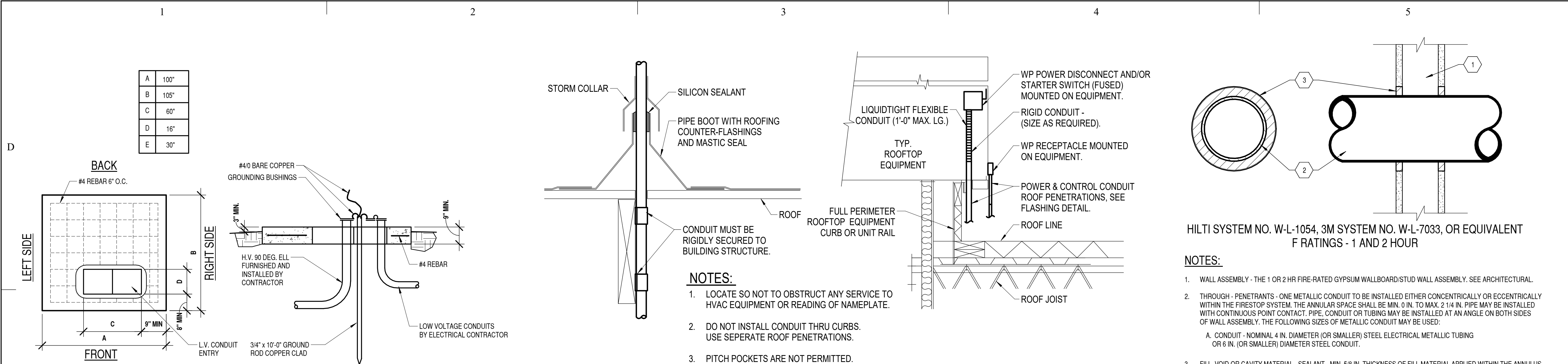
27 MARCH, 2018

SHEET TITLE

ELECTRICAL
DETAILS

SHEET NO:

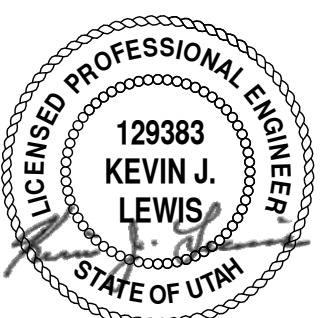
E501



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03/27/2018



ADDITION TO THE
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PERMIT SET
27 MARCH, 2018

SHEET TITLE

ELECTRICAL
DETAILS

SHEET NO:

E502

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REDUCE SCALE ACCORDINGLY

D

PANEL:	MDP9			MAIN BREAKER			3000			VOLTAGE:			480 Y/ 277			PHASE: 3		WIRE: 4		NEMA: 1		
MAINS:	3000 AMPS			MOUNTING:			PAD			LOCATION			SE CORNER PUMP RM			REMARKS:		NEMA 3R, SQUARE D I-LINE				
LOAD DESCRIPTION				WIRE	P	AMP	CKT	KVA			PHASE			KVA			CKT	AMP	P	WIRE	LOAD DESCRIPTION	
								PWR	LTS	C.O.	A	B	C	C.O.	LTS	PWR						
PANEL PH48				4-350	3	1200	1	748.0	6.4	18.0	515.1	515.1	515.1	0.0	0.0	773.0	2	1200	3	4-350	PANEL PH49	
PANLE PH50				4-350	3	1200	3	740.0	0.0	0.0	246.7	246.7	246.7				4	250	3	250	PFC-009	
TVSS				4	3	60	5				0.0	0.0	0.0				6		3		BLANK	
BLANK					3		7				0.0	0.0	0.0				8		3		BLANK	
BLANK					3		9				0.0	0.0	0.0				10		3		BLANK	
BLANK					3		11				0.0	0.0	0.0				12		3		BLANK	
BLANK					3		13				0.0	0.0	0.0				14		3		BLANK	
SUB-TOTAL (KVA)								1488.0	6.4	18.0	761.8	761.8	761.8	0.0	0.0	773.0						
										TYPE OF LOAD						CONNECTED			DIVERSITY		DEMAND	
										LIGHTING						6.4			100%		6.4	
										POWER						2261.0			50%		1130.5	
										C.O.						18.0			NEC 220.44		14.0	
PANEL LOADING				46%						TOTAL						2285.4			KVA		1150.9	
3/8/2018 9:17										TOTAL						2749			AMPS		1385	

PANEL:	PH48			MAIN BREAKER			NONE			VOLTAGE:			480				PHASE: 3			WIRE: 4		NEMA: 1			
MAINS:	1200 AMPS			MOUNTING:			PAD			LOCATION			EAST PUMP RM			REMARKS: SQUARE D I-LINE									
LOAD DESCRIPTION				WIRE	P	AMP	CKT	KVA		PHASE			KVA		CKT	AMP	P	WIRE	LOAD DESCRIPTION						
								PWR	LTS	C.O.	A	B	C	C.O.	LTS	PWR									
100 HP MOTOR				2/0	3	200	1	100.0			66.7	66.7	66.7			100.0	2	200	3	2/0	100 HP MOTOR				
100 HP MOTOR				2/0	3	200	3	100.0			66.7	66.7	66.7			100.0	4	200	3	2/0	100 HP MOTOR				
100 HP MOTOR				2/0	3	200	5	100.0			66.7	66.7	66.7			100.0	6	200	3	2/0	100 HP MOTOR				
40 HP MOTOR				6	3	90	7	40.0			21.7	21.7	21.7			25.0	8	70	3	8	25 HP MOTOR				
40 HP MOTOR				6	3	90	9	40.0			21.7	21.7	21.7			25.0	10	70	3	8	25 HP MOTOR				
SPARE					3	20	11	5.0			15.5	15.5	15.5	16.2	6.4	19.0	12	125	3	1/0	TRANSFORMER PL48				
SPARE					3	20	13	5.0			3.3	3.3	3.3			5.0	14	30	3		SPARE				
SUB-TOTAL (KVA)								390.0	0.0	0.0	262.2	262.2	262.2	16.2	6.4	374.0									
											TYPE OF LOAD					CONNECTED			DIVERSITY		DEMAND				
											LIGHTING					6.4			100%		6.4				
											POWER					764.0			50%		382.0				
											C.O.					16.2			NEC 220.44		13.1				
PANEL LOADING				40%										TOTAL					786.6			KVA		401.5	
3/22/2018 14:36											TOTAL					947			AMPS			483			

PANEL:		PH49		MAIN BREAKER		NONE			VOLTAGE:		480		PHASE: 3		WIRE: 4		NEMA: 1			
MAINS:		1200 AMPS		MOUNTING:		PAD			LOCATION		EAST PUMP RM		REMARKS: SQUARE D I-LINE							
LOAD DESCRIPTION		WIRE	P	AMP	CKT	PWR	LTS	C.O.	A	B	C	C.O.	LTS	PWR	CKT	AMP	P	WIRE	LOAD DESCRIPTION	
100 HP MOTOR		2/0	3	200	1	100.0			66.7	66.7	66.7			100.0	2	200	3	2/0	100 HP MOTOR	
100 HP MOTOR		2/0	3	200	3	100.0			66.7	66.7	66.7			100.0	4	200	3	2/0	100 HP MOTOR	
100 HP MOTOR		2/0	3	200	5	100.0			66.7	66.7	66.7			100.0	6	200	3	2/0	100 HP MOTOR	
40 HP MOTOR		6	3	90	7	40.0			21.7	21.7	21.7			25.0	8	70	3	8	25 HP MOTOR	
40 HP MOTOR		6	3	90	9	40.0			21.7	21.7	21.7			25.0	10	70	3	8	25 HP MOTOR	
RTU		6	3	60	11	43.0			16.0	16.0	16.0			5.0	12	30	3		SPARE	
SPARE			3	20	13	5.0			3.3	3.3	3.3			5.0	14	30	3		SPARE	
SUB-TOTAL (KVA)						428.0	0.0	0.0	262.7	262.7	262.7	0.0	0.0	360.0						
									TYPE OF LOAD					CONNECTED			DIVERSITY		DEMAND	
									LIGHTING					0.0			100%		0.0	
									POWER					788.0			50%		394.0	
									C.O.					0.0			NEC 220.44		0.0	
PANEL LOADING		40%							TOTAL					788.0			KVA		394.0	
3/22/2018 14:35							TOTAL					948			AMPS		474			

PANEL:		PH50		MAIN BREAKER		NONE			VOLTAGE:		480				PHASE:		3 WIRE:		4		NEMA:		1	
MAINS:		1200 AMPS		MOUNTING:		PAD			LOCATION		EAST PUMP RM				REMARKS:		SQUARE D I-LINE							
LOAD DESCRIPTION				WIRE	P	AMP	CKT	KVA		PHASE			KVA			CKT	AMP	P	WIRE	LOAD DESCRIPTION				
								PWR	LTS	C.O.	A	B	C	C.O.	LTS	PWR								
100 HP MOTOR				2/0	3	200	1				66.7	66.7	66.7			100.0	2	200	3	2/0	100 HP MOTOR			
100 HP MOTOR				2/0	3	200	3				66.7	66.7	66.7			100.0	4	200	3	2/0	100 HP MOTOR			
100 HP MOTOR				2/0	3	200	5				66.7	66.7	66.7			100.0	6	200	3	2/0	100 HP MOTOR			
40 HP MOTOR				6	3	90	7				21.7	21.7	21.7			25.0	8	70	3	8	25 HP MOTOR			
40 HP MOTOR				6	3	90	9				21.7	21.7	21.7			25.0	10	70	3	8	25 HP MOTOR			
PUMP 1				12	3	20	11		7.5		5.0	5.0	5.0			7.5	12	20	3	12	PUMP 2			
COOLING TOWER				5	3	20	13		4.0		2.8	2.8	2.8			4.5	14	30	3		CT SUMP HEATER			
SUB-TOTAL (KVA)								391.5	0.0	0.0	251.2	251.2	251.2	0.0	0.0	362.0								
											TYPE OF LOAD					CONNECTED			DIVERSITY		DEMAND			
											LIGHTING					0.0			100%		0.0			
											POWER					753.5			50%		376.8			
											C.O.					0.0			NEC 220.44		0.0			
PANEL LOADING				38%							TOTAL					753.5			KVA		376.8			
3/22/2018 14:40									TOTAL					907			AMPS		454					