

SPECIAL INSPECTIONS SCHEDULE

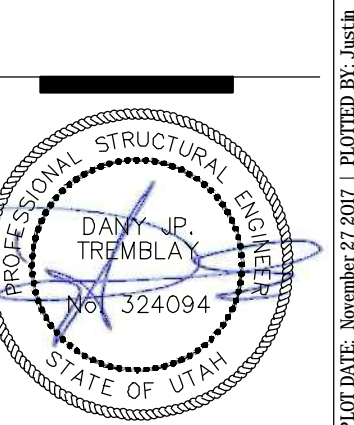
SPECIAL INSPECTIONS: GENERAL

- CONTRACTOR SHALL READ AND UNDERSTAND THEIR DUTIES IN THE SPECIFICATION AND UNDER THE BUILDING CODE FOR SPECIAL INSPECTIONS AND COORDINATE AS NECESSARY THE OWNERS RESPONSIBILITIES.
- THE SPECIAL INSPECTORS SHALL BE PROVIDED AND SHALL ONLY USE APPROVED SHOP DRAWINGS. SPECIAL INSPECTION REPORTS ARE TO BE SUBMITTED IMMEDIATELY TO THE SER, ARCHITECT, AND CONTRACTOR DAILY WHEN INSPECTIONS ARE PERFORMED.
- THE GENERAL CONTRACTOR SHALL PROVIDE TIMELY NOTICE TO THE SPECIAL INSPECTOR AND SUFFICIENT TIME FOR THE INSPECTOR TO PERFORM THEIR INSPECTION.
- SEE GENERAL STRUCTURAL NOTES SPECIAL INSPECTIONS SECTION, NOTE "C" FOR ALL PROJECT-SPECIFIC SPECIAL INSPECTIONS. ADJACENT CHART FOR THOSE ITEMS ONLY SHALL BE USED FOR THE PROJECT.

SPECIAL INSPECTIONS STATEMENT

- SPECIAL INSPECTIONS AND STRUCTURAL TESTING SHALL BE PROVIDED BY AN INDEPENDENT AGENCY EMPLOYED BY THE OWNER FOR THE ITEMS IDENTIFIED IN THIS SECTION AND IN OTHER AREAS OF THE APPROVED CONSTRUCTION PLANS AND SPECIFICATIONS, UNLESS WAIVED BY THE BUILDING OFFICIAL (SEE IBC CHAPTER 17).
- THE NAMES AND CREDENTIALS OF THE SPECIAL INSPECTORS TO BE USED SHALL BE SUBMITTED TO THE BUILDING OFFICIAL FOR APPROVAL.
- DUTIES OF THE SPECIAL INSPECTOR:
 - THE SPECIAL INSPECTOR SHALL REVIEW ALL WORK LISTED ABOVE FOR CONFORMANCE WITH THE APPROVED CONSTRUCTION PLANS AND SPECIFICATIONS AND THE 2015 IBC.
 - THE SPECIAL INSPECTOR SHALL FURNISH SPECIAL INSPECTION REPORTS TO THE EOR, CONTRACTOR, OWNER AND BUILDING OFFICIAL ON A WEEKLY BASIS, OR MORE FREQUENTLY AS REQUIRED BY THE BUILDING OFFICIAL. ALL ITEMS NOT IN COMPLIANCE SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, AND IF UNCORRECTED, TO THE EOR AND THE BUILDING OFFICIAL.
 - ONCE CORRECTIONS HAVE BEEN MADE BY THE CONTRACTOR, THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT TO THE BUILDING OFFICIAL STATING THAT THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE SPECIAL INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED CONSTRUCTION PLANS AND SPECIFICATIONS AS WELL AS THE APPLICABLE WORKMANSHIP PROVISIONS OF THE 2015 IBC.
 - DUTIES AND RESPONSIBILITIES OF THE CONTRACTOR:
 - THE CONTRACTOR SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE OWNER AND THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF WORK. IN ACCORDANCE WITH IBC 1704.4, THE STATEMENT OF RESPONSIBILITY SHALL CONTAIN ACKNOWLEDGEMENT OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED WITHIN THIS "STATEMENT OF SPECIAL INSPECTIONS".
 - THE CONTRACTOR SHALL NOTIFY THE RESPONSIBLE SPECIAL INSPECTOR THAT WORK IS READY FOR INSPECTION AT LEAST ONE WORKING DAY (24 HOURS MINIMUM) BEFORE SUCH INSPECTION IS REQUIRED.
 - ALL WORK REQUIRING SPECIAL INSPECTION SHALL REMAIN ACCESSIBLE AND EXPOSED UNTIL IT HAS BEEN OBSERVED BY THE SPECIAL INSPECTOR.
- PLEASE SEE THE "SPECIAL INSPECTION SCHEDULE" FOR THE TYPES, EXTENTS, AND FREQUENCY OF SPECIFIC ITEMS REQUIRING SPECIAL INSPECTIONS AND STRUCTURAL TESTS AS PART OF THIS PROJECT.

SPECIAL INSPECTIONS SCHEDULE			
AREAS REQUIRING SPECIAL INSPECTION:	FREQUENCY		COMMENTS:
	CONTINUOUS	PERIODIC	
FABRICATORS (IBC 1704.2.5)			
	◆		IF FABRICATOR IS APPROVED, ON-SITE INSPECTION IS NOT REQUIRED BUT A CERTIFICATE MUST BE PROVIDED TO THE B.O. (IBC 1704.2.5.1)
SOILS (IBC 1705.6)			
VERIFY ADEQUATE MATERIALS BELOW FOOTINGS		◆	PRIOR TO PLACEMENT OF CONCRETE.
EXCAVATION EXTEND TO PROPER DEPTH AND MATERIALS		◆	PRIOR TO PLACEMENT OF COMPACT FILL OR CONCRETE.
CLASSIFICATION AND TESTING OF FILL MATERIALS		◆	CHECK CLASSIFICATION AND GRADATIONS AT EACH LIFT, BUT NOT LESS THAN ONCE FOR EACH 10,000 FT ² OF SURFACE AREA.
VERIFY PROPER FILL MATERIALS, LIFT THICKNESS AND IN-PLACE DENSITIES	◆		-
VERIFY PROPERLY PREPARED SITE AND SUB-GRADE		◆	PRIOR TO PLACEMENT OF CONCRETE.
CONCRETE CONSTRUCTION (IBC 1705.3)			
REINFORCING STEEL PLACEMENT		◆	VERIFY SIZE, CLEARANCES, SPLICES, AND PROPER TIES.
EMBEDDED BOLTS OR PLATES		◆	-
VERIFY REQUIRED DESIGN MIX		◆	VERIFY MIX DESIGN MEETS STRENGTH AND EXPOSURE REQUIREMENTS LISTED ON APPROVED PLANS.
CONCRETE PLACEMENT / SAMPLING	◆		INCLUDES SAMPLING FOR AIR, SLUMP, STRENGTH, AND TEMPERATURE TECHNIQUES.
INSPECT FORMWORK		◆	VERIFY SHAPE, LOCATION, AND MEMBER DIMENSIONS.
POST-INSTALLED ANCHORS	◆		IN ACCORDANCE WITH APPROVED ICC-ES REPORT. PERIODIC INSPECTIONS ALLOWED IF STATED IN ES REPORT.
STRUCTURAL STEEL CONSTRUCTION (IBC 1705.2, 1705.12, 1705.13)			
- PRIOR TO WELDING (TABLE N5.4-1, AISC 360-10)			
VERIFY WELDING PROCEDURES	◆		-
MATERIAL IDENTIFICATION		◆	VERIFY TYPE AND GRADE OF MATERIAL.
WELDER IDENTIFICATION		◆	VERIFY THERE IS A SYSTEM IN PLACE TO IDENTIFY THE WELDER WHO HAS WELDED A JOINT OR MEMBER.
FIT-UP GROOVE WELDS		◆	VERIFY JOINT PREPARATION, DIMENSIONS, CLEANLINESS, TACKING, AND BACKING.
ACCESS HOLES		◆	VERIFY CONFIGURATION AND FINISH.
FIT-UP FILLET WELDS		◆	VERIFY ALIGNMENT, GAPS AT ROOTS, CLEANLINESS OF STEEL SURFACES, TACK WELD QUALITY, AND LOCATION.
- DURING WELDING (TABLE N5.4-2, AISC 360-10)			
USE OF QUALIFIED WELDERS		◆	VERIFY THAT WELDERS ARE APPROPRIATELY QUALIFIED.
CONTROL AND HANDLING OF WELDING CONSUMABLES		◆	VERIFY PACKAGING AND EXPOSURE CONTROL.
CRACKED TACK WELDS		◆	VERIFY WELDING IS NOT OVER A CRACKED TACK WELD.
ENVIRONMENTAL CONDITIONS		◆	VERIFY WIND SPEED IS WITHIN LIMITS AS WELL AS PRECIPITATION AND TEMPERATURE.
WPS FOLLOWED		◆	VERIFY ITEMS SUCH AS WELDING EQUIPMENT SETTINGS, TRAVEL SPEED, WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED, AND PROPER POSITION.
WELDING TECHNIQUES		◆	VERIFY INTERPASS AND FINAL AND FINAL CLEANING, EACH PASS IS WITHIN PROFILE LIMITATIONS AND PROPER POSITION.
- AFTER WELDING (TABLE N5.4-3, AISC 360-10)			
WELDS CLEANED		◆	VERIFY THAT WELDS HAVE BEEN PROPERLY CLEANED.
SIZE, LENGTH, AND LOCATION OF WELDS	◆		-
WELDS MEET VISUAL ACCEPTANCE CRITERIA	◆		-
ARC STRIKES	◆		-
K-AREA	◆		-
BACKING AND WELDING TABS REMOVED	◆		-
REPAIR ACTIVITIES	◆		-
DOCUMENT ACCEPTANCE / REJECTION OF WELD	◆		-
- NONDESTRUCTIVE TESTING (N5.5, AISC 360-10)			
CJP WELDS (RISK CAT. II)		◆	ULTRASONIC TESTING SHALL BE PERFORMED ON 10% OF CJP GROOVE WELDS IN BUTT, T- AND CORNER JOINTS SUBJECT TO TRANSVERSELY APPLIED TENSION LOADING IN MATERIALS 5/16" THICK OR GREATER. TESTING RATE MUST BE INCREASED IF >5% OF WELDS HAVE UNACCEPTED DEFECTS.
ACCESS HOLE (FLANGES >2")	◆		-
WELDED JOINTS SUBJECT TO FATIGUE	◆		-
- OTHER STEEL INSPECTIONS (N5.7, AISC 360-10)			
STRUCTURAL STEEL DETAILS		◆	ALL FABRICATED STEEL AND THEIR CONNECTIONS SHALL BE INSPECTED TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN IN THE APPROVED PLANS.
ANCHOR ROD / EMBEDS SUPPORTING STRUCTURAL STEEL		◆	SHALL BE ON THE PREMISES DURING THE PLACEMENT OF ANCHOR RODS / EMBEDMENTS. VERIFY DIAMETER, GRADE, TYPE, AND LENGTH OF ELEMENTS AND THE EXTENT OR DEPTH OF EMBEDMENT PRIOR TO PLACEMENT OF CONCRETE.
REDUCED BEAM SECTIONS (RBS)		◆	VERIFY CONTOUR AND FINISH AS WELL AS DIMENSIONAL TOLERANCES (SEE TABLE J8-1 OF AISC 341).
PROTECTED ZONES		◆	VERIFY THAT NO HOLES OR UNAPPROVED ATTACHMENTS ARE MADE WITHIN THE PROTECTED ZONE (SEE TABLE J8-1 OF AISC 341).
WOOD CONSTRUCTION			
WOOD SHEARWALLS		◆	SHEARWALL NAIL SPACING FOR TYPE 'B', 'C', AND DOUBLE-SHEATHED WALLS



SHEET TITLE

SPECIAL INSPECTIONS SCHEDULE

ISSUE DATE

SEPTEMBER 15, 2017

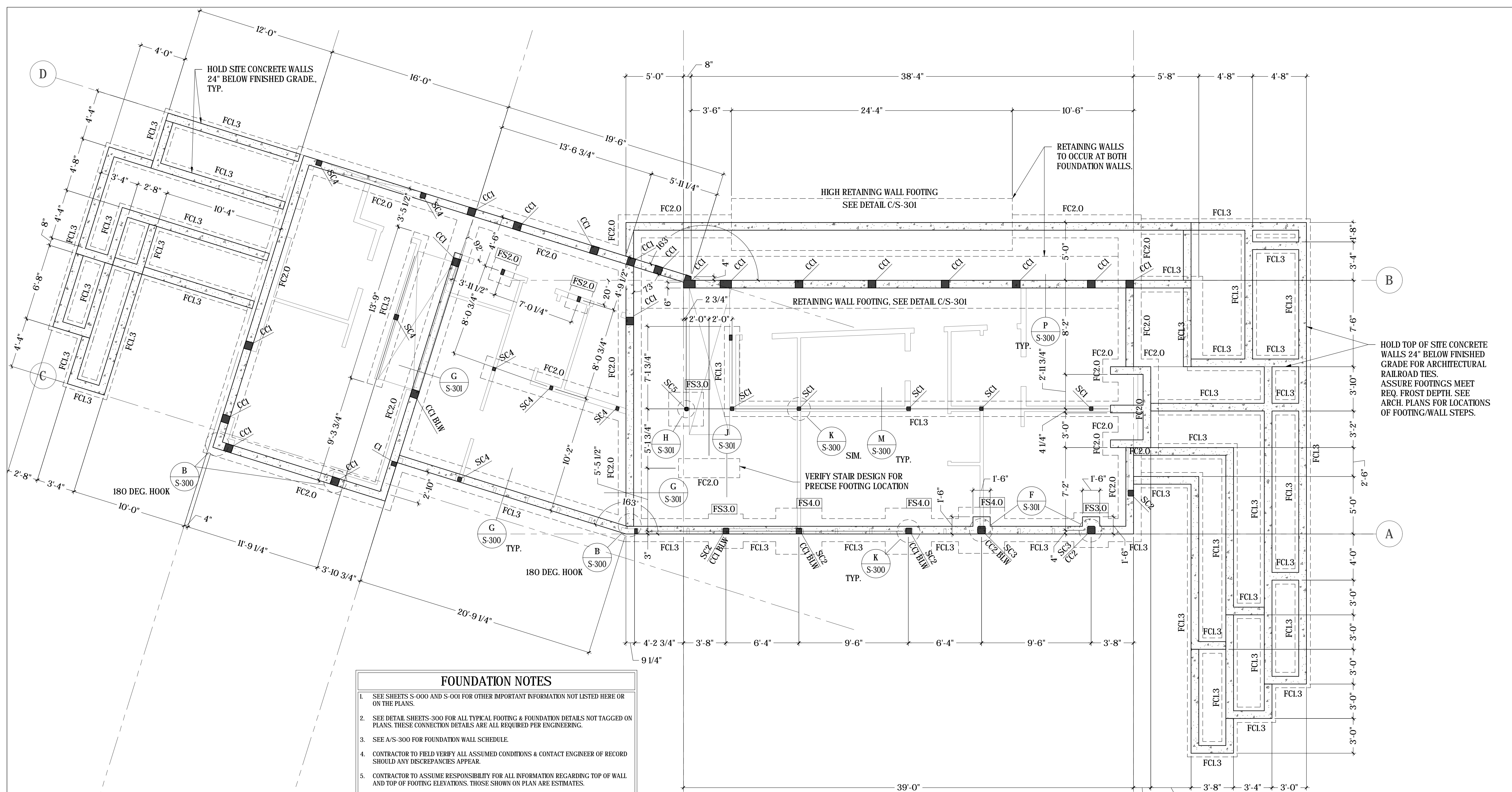
SHEET REVISION DATE

NOV. 27, 2017

SHEET NUMBER

S-001

BROWNING SKI LODGE
7977 HEARTWOOD DRIVE
EDEN, UTAH
WEBER COUNTY



- ### FOUNDATION NOTES
- SEE SHEETS S-000 AND S-001 FOR OTHER IMPORTANT INFORMATION NOT LISTED HERE OR ON THE PLANS.
 - SEE DETAIL SHEETS-300 FOR ALL TYPICAL FOOTING & FOUNDATION DETAILS NOT TAGGED ON PLANS. THESE CONNECTION DETAILS ARE ALL REQUIRED PER ENGINEERING.
 - SEE A/S-300 FOR FOUNDATION WALL SCHEDULE.
 - CONTRACTOR TO FIELD VERIFY ALL ASSUMED CONDITIONS & CONTACT ENGINEER OF RECORD SHOULD ANY DISCREPANCIES APPEAR.
 - CONTRACTOR TO ASSUME RESPONSIBILITY FOR ALL INFORMATION REGARDING TOP OF WALL AND TOP OF FOOTING ELEVATIONS. THOSE SHOWN ON PLAN ARE ESTIMATES.
 - WS INDICATES WALL STEP PER E/S-300. FS INDICATES FOOTING STEP PER E/S-300. COORDINATE w/ ARCH.
 - CENTER SPOT FOOTINGS ABOUT COLUMNS AND CENTER CONTINUOUS FOOTINGS ABOUT WALLS WHEREVER POSSIBLE.
 - FOR ALL FOUNDATION WALLS GREATER THAN 8 FT. IN HEIGHT, FORMS SHALL REMAIN OPEN UNTIL REBAR INSPECTION HAS BEEN COMPLETED.
 - LOCATE AND VERIFY ALL MOMENT FRAME COLUMNS TO ASSURE CONCRETE PIER DIMENSIONS AND CONCRETE PIER DETAILING IS PRECISE.
 - ALL FOOTINGS TO BE PLACED ON UNDISTURBED EARTH OR COMPACTED STRUCTURAL FILL PER GEOTECHNICAL REPORT.
 - ALL REINFORCING MATERIAL TO BE DEFORMED BARS, 60 GRADE.
 - FINISH GRADE 6" BELOW TOP OF FOUNDATION.
 - 36" MINIMUM FINISH GRADE TO BOTTOM OF FOOTING FOR EXTERIOR FOOTINGS.
 - 4" MINIMUM THICK CONCRETE SLAB ON GRADE. REINFORCE w/ 6x6xW4 WWF AT MID DEPTH. PLACE OVER 4" FREE DRAINING GRAVEL OVER PREPARED SUB GRADE. IF 6" THICK SLAB DESIRED, REINFORCE w/ #4 BARS @ 24" O.C., E.W. IN LIEU OF WWF.
 - CENTER SPOT FOOTINGS BELOW COLUMNS U.N.O.
 - REFER TO E/S-300 FOR STEPS IN FOUNDATION + COORDINATE w/ ARCH.
 - TYPICAL ANCHOR BOLTS TO BE 5/8" SIMPSON TITEN HD ANCHOR BOLTS w/ 5-1/2" EMBEDMENT PER A/S5.0 & 3x3x0.229" WASHERS. ENSURE STAINLESS NAILING INTO TYPICAL TREATED SILLPLATE, OR, AS AN ALTERNATIVE, USE BORATE TREATED SILLPLATES w/ TYPICAL NAILING. 5/8" DIA. CAST-IN-PLACE ANCHOR BOLTS w/ 7" EMBEDMENT ARE A VIABLE ALTERNATIVE.
 - ALL WOOD IN CONTACT w/ CONCRETE OR MASONRY MUST BE TREATED OR REDWOOD LUMBER. THIS INCLUDES, BUT IS NOT LIMITED TO, SILLPLATES AND LEDGERS.

HOLDOWN SCHEDULE

MARK	HARDWARE	DESCRIPTION	MIN. POST
HD1	CS14	WALL-TO-WALL STRAP	(2) 2x
HD2	CMST14	WALL-TO-WALL STRAP	(2) 2x
HD3	HDU5	USE 5/8" OSIMPS ON EPOXY-SET XP ANCHORS. MIN. 10" EMBED. 8" EDGE DISTANCE	(2) 2x

HOLDOWN NOTES:

- HOLDOWN SHALL BE ALIGNED WITH END, CORNER OR EDGE OF OPENING OF SHEARWALL ABOVE. ATTACH TO DOUBLE STUD MINIMUM FOR FULL LOAD VALUES.
- ALL HOLDOWNS SHALL BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS & RECOMMENDATIONS.
- WHERE BOLTS & HARDWARE COME IN CONTACT WITH PRESSURE TREATED LUMBER, PROVIDE STAINLESS OR CORROSION PROTECTION APPROVED FOR USE WITH PRESSURE TREATED LUMBER.
- INCREASE FOOTING THICKNESS AND/OR WIDTHS AS NECESSARY TO ACCOMMODATE ANCHOR EMBED AND EDGE DISTANCES.

FOOTING SCHEDULE

MARK	WIDTH	LENGTH	DEPTH	REINFORCING - (q = 3000 psf)
FCL3	1'-4"	CONT.	12"	(2) #4 CONTINUOUS
FC2.0	2'-0"	CONT.	12"	(3) #4 CONTINUOUS
FC2.5	2'-6"	CONT.	12"	(4) #4 CONTINUOUS
FSL3	1'-4"	1'-4"	10"	(3) #4 EACH WAY
FS2.0	2'-0"	2'-0"	10"	(3) #4 EACH WAY
FS2.5	2'-6"	2'-6"	10"	(3) #4 EACH WAY
FS3.0	3'-0"	3'-0"	10"	(4) #4 EACH WAY
FS3.5	3'-6"	3'-6"	10"	(5) #4 EACH WAY
FS4.0	4'-0"	4'-0"	12"	(4) #5 EACH WAY
FS4.5	4'-6"	4'-6"	15"	(5) #5 EACH WAY
FS6.0	6'-0"	6'-0"	18"	(6) #6 EACH WAY

CONCRETE COLUMN SCHEDULE

MARK	SIZE - (OPTIONAL SIZE(S))
CCI	(4) VERT. BARS TO MATCH CONCRETE WALL REINF. SEE DETAIL T/S3.0
CC2	MOMENT FRAME CONCRETE PIER, SEE DETAIL F/S3.0

CONCRETE COLUMN NOTE:

- SEE A/S3.0 FOR CONCRETE WALL SCHEDULE.
- FOR 'CCI' ADJACENT TO LINTEL SKYLIGHT OPENINGS, SEE H/S3.01

COLUMN SCHEDULE

MARK	SIZE - (OPTIONAL SIZE(S))
C	(2) 2x6 IN 2x6 WALLS ((2) 2x4 IN 2x4 WALLS - (REQ'D @ ALL BEAM & HDR ENDS, U.N.O.))
C1	(3) 2x6 - (DOUGLAS FIR LARCH #2)
SC1	HSS5x5x1/2
SC2	HSS6x6x1/2
SC3	HSS8x8x5/8 - MOMENT FRAME COLUMN
SC4	HSS4x4x1/4
SC5	PIPE4xS (4" STRONG PIPE)

COLUMN NOTE:

- WHERE WOOD COLUMNS ARE NOT DESIRED, THE CONTRACTOR MAY SUBSTITUTE 'SC1' STEEL COLUMNS FOR ALL WOOD COLUMN CALL-OUTS ON PLANS.
- NO STEEL ELEMENTS TO BE FABRICATED UNTIL STEEL SHOP DRAWINGS HAVE BEEN APPROVED BY ENGINEER OF RECORD.

PROFESSIONAL ENGINEER
DANN J. TREMBLA
324094
STATE OF UTAH

SHEET TITLE
FOOTING & FOUNDATION PLAN

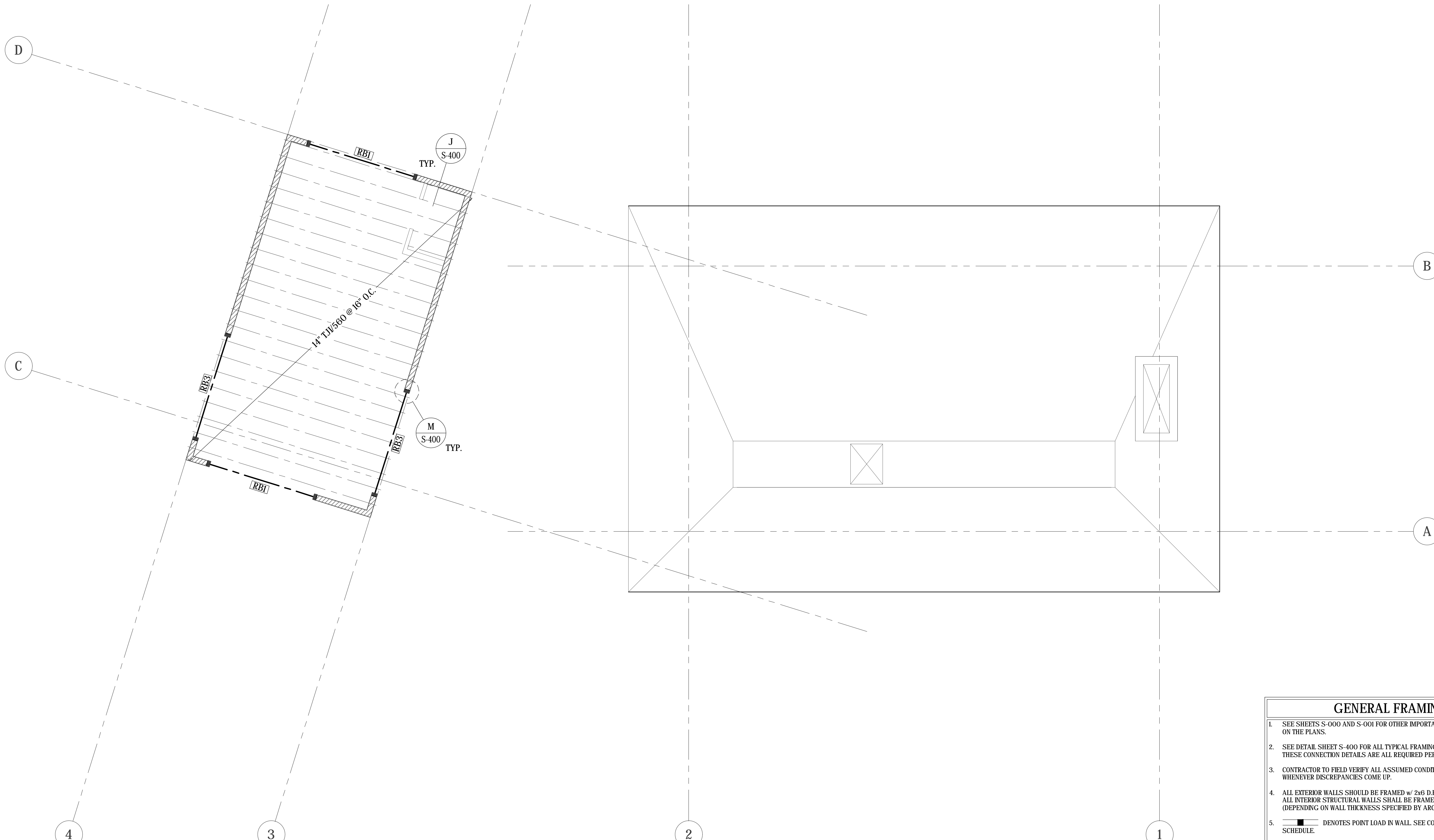
ISSUE DATE
SEPTEMBER 15, 2017

SHEET REVISION DATE
NOV. 27, 2017

SHEET NUMBER
S-101

REVISIONS: THE CLIENT HAS REQUESTED REVISIONS TO THE FOUNDATION PLAN. THE REVISIONS ARE: 1. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 2. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 3. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 4. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 5. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 6. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 7. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 8. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 9. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 10. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 11. ADDITIONAL FOOTINGS TO BE PLACED AT THE CORNERS OF THE FOUNDATION WALLS. 12. 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BROWNING SKI LODGE
7977 HEARTWOOD DRIVE
EDEN, UTAH
WEBER COUNTY



ROOF BEAM SCHEDULE

MARK	SIZE - (OPTIONAL SIZE(s))
HDR	(2) 2x8 - ((3) 2x6)
RB1	(2) 2x10 - ((3) 2x8)
RB2	(2) 1-3/4" x 7-1/2" LVL - (W5x16 STL C6x13 STL)
RB3	(2) 1-3/4" x 14" LVL - ((3) 1-3/4" x 11-7/8" LVL W8x15 STL)
RB4	C12x20.7 STL
RB5	W8x28 STL
RB6	W8x58 STL
RB7	W12x58 STL
RB8	W10x45 STL
RB9	W5x19 STL - (C6x13 STL)
RB10	(3) 1-3/4" x 7-1/2" LVL

ROOF BEAM NOTES:

- USE DOUGLAS FIR LARCH #2 FOR NOMINAL LUMBER
- USE DOUGLAS FIR LARCH DENSE #1 FOR R.S.
- USE DOUGLAS FIR LARCH 24F-V4 FOR SIMPLE SPAN GLULAMS & 24F-V8 FOR CONTINUOUS OR CANTILEVERS.
- UPSIZING BEAMS AS NECESSARY PER ARCHITECTURE
- NO STEEL ELEMENTS TO BE FABRICATED UNTIL STEEL SHOP DRAWINGS HAVE BEEN APPROVED BY ENGINEER OF RECORD.

COLUMN SCHEDULE

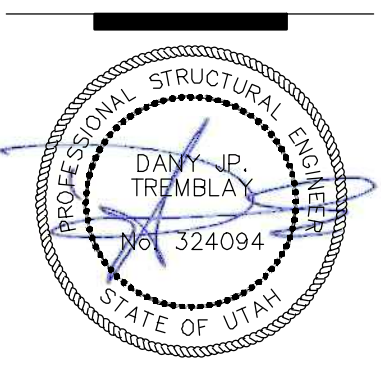
MARK	SIZE - (OPTIONAL SIZE(s))
C	(2) 2x6 IN 2x6 WALLS ((2) 2x4 IN 2x4 WALLS - (REQ'D @ ALL BEAM & HDR ENDS, U.N.O.)
CI	(3) 2x6 - (DOUGLAS FIR LARCH #2)
SC1	HSS5x5x1/2
SC2	HSS6x6x1/2
SC3	HSS8x8x5/8 - MOMENT FRAME COLUMN
SC4	HSS4x4x1/4
SC5	PIPE4xS (4" STRONG PIPE)

COLUMN NOTE:

- WHERE WOOD COLUMNS ARE NOT DESIRED, THE CONTRACTOR MAY SUBSTITUTE 'SC1' STEEL COLUMNS FOR ALL WOOD COLUMN CALL-OUTS ON PLANS.
- NO STEEL ELEMENTS TO BE FABRICATED UNTIL STEEL SHOP DRAWINGS HAVE BEEN APPROVED BY ENGINEER OF RECORD.

GENERAL FRAMING NOTES

- SEE SHEETS S-000 AND S-001 FOR OTHER IMPORTANT INFORMATION NOT LISTED HERE OR ON THE PLANS.
- SEE DETAIL SHEET S-400 FOR ALL TYPICAL FRAMING DETAILS NOT FLAGGED ON PLANS. THESE CONNECTION DETAILS ARE ALL REQUIRED PER OUR ENGINEERING.
- CONTRACTOR TO FIELD VERIFY ALL ASSUMED CONDITIONS & CONTACT ENGINEER OF RECORD WHENEVER DISCREPANCIES COME UP.
- ALL EXTERIOR WALLS SHOULD BE FRAMED w/ 2x6 D.F. #2 STUDS @ 16" O.C., TYPICAL U.N.O. ALL INTERIOR STRUCTURAL WALLS SHALL BE FRAMED w/ 2x4 D.F. #2 OR 2x6 D.F. #2 STUDS (DEPENDING ON WALL THICKNESS SPECIFIED BY ARCH.) @ 16" O.C., TYPICAL U.N.O
- DENOTES POINT LOAD IN WALL. SEE COLUMN CALL-OUTS ON PLAN & COLUMN SCHEDULE.
- DENOTES POINT LOAD ABOVE. SEE COLUMN CALL-OUTS ON PLAN & COLUMN SCHEDULE.
- ALL COLUMNS IN 2x6 WALLS TO BE (2) 2x6 MIN & ALL COLUMNS IN 2x4 WALLS TO BE (2) 2x4 MIN., U.N.O.
- ALL WALL OPENINGS 8FT OR GREATER IN WIDTH REQUIRE (2) 2x KING STUDS. OPENINGS 12FT OR GREATER REQUIRE (3) 2x KING STUDS.
- ALL VERTICAL WINDOW MULLIONS IN WALLS w/ AN UNBRACED HEIGHT GREATER THAN 12'-0" SHALL BE FRAMED w/ (2) 2x6 D.F. #2 STUDS. U.N.O. ON PLANS.
- ALL BEAM AND HEADER ENDS TO BEAR ON (1) CRIPPLE AND HAVE (1) TRIMMER U.N.O.
- ALL HEADERS TO BE (2) 2x8 MINIMUM AT EXTERIOR AND INTERIOR LOCATIONS, TYP., U.N.O..
- FLUSH FRAME BEAMS WHEREVER POSSIBLE. U.N.O. IN DETAILING.
- USE TOP-MOUNT HANGERS PER GSN WHEREVER POSSIBLE.
- ATTACH BEAMS TO BEAMS USING SIMPSON CONNECTORS TO SUIT JOIST SIZE PER GSN, OR PER TYPICAL BEAM TO BEAM DETAIL ON SHEET S-400.
- TYPICAL FLOOR FRAMING TO BE 11-7/8" TJI/360 JOISTS @ 16" O.C. (OR EQUIVALENT), TYPICAL U.N.O. ON PLANS.
- TYPICAL ROOF FRAMING TO BE 14" TJI/210 RAFTERS @ 16" O.C. (OR EQUIVALENT), TYPICAL U.N.O. ON PLANS.
- ROOF DEAD LOAD.....15 PSF
ROOF SNOW LOAD.....125 PSF + SNOWDRIFT PER IBC 2015
- NO TJs or LVLs SHALL BE EXPOSED IN AN EXTERIOR APPLICATION.



SHEET TITLE
ROOF FRAMING PLAN (PART B)

ISSUE DATE
SEPTEMBER 15, 2017

SHEET REVISION DATE
NOV. 27, 2017

SHEET NUMBER
S-221

REVISIONS: 1. THE COUNTY ENGINEER HAS REVIEWED THIS DRAWING FOR CONFORMANCE WITH THE UTAH PROFESSIONAL ENGINEERING ACT AND THE UTAH PROFESSIONAL ENGINEERING BOARD RULES. THE COUNTY ENGINEER'S REVIEW IS LIMITED TO THE TECHNICAL ASPECTS OF THE DRAWING AND DOES NOT CONSTITUTE AN ENDORSEMENT OF THE PROJECT OR A GUARANTEE OF THE ACCURACY OF THE INFORMATION PROVIDED HEREON. THE COUNTY ENGINEER'S REVIEW IS LIMITED TO THE TECHNICAL ASPECTS OF THE DRAWING AND DOES NOT CONSTITUTE AN ENDORSEMENT OF THE PROJECT OR A GUARANTEE OF THE ACCURACY OF THE INFORMATION PROVIDED HEREON.

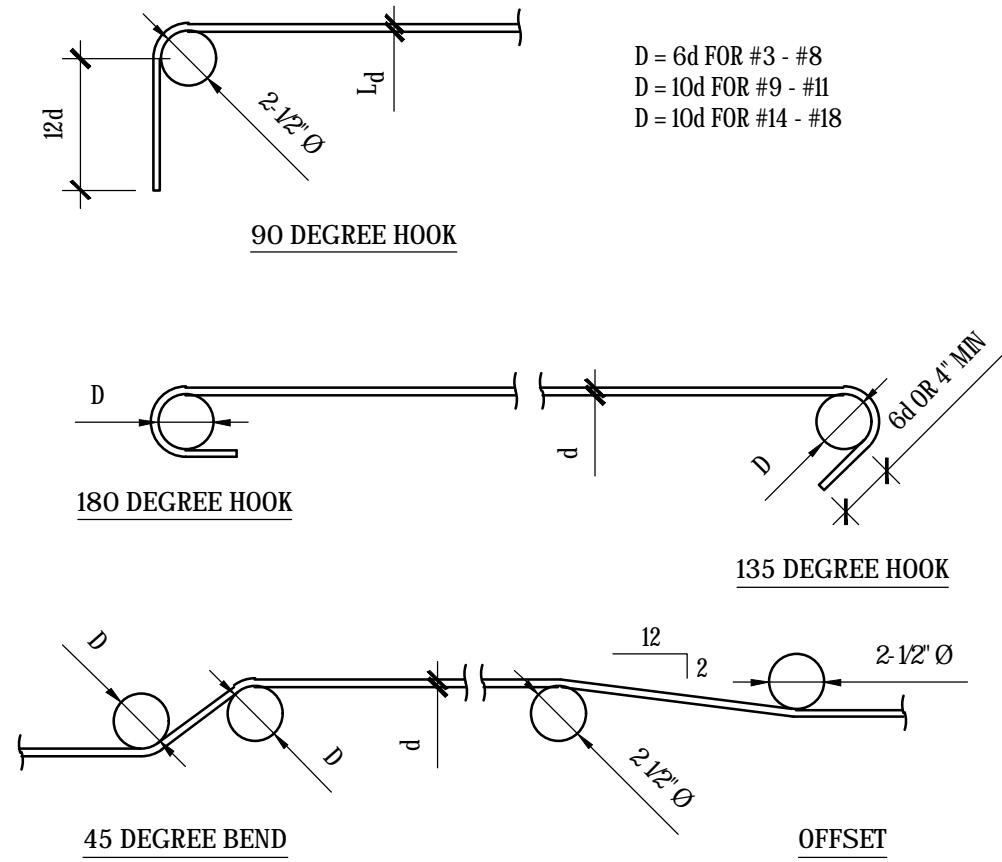
WALL HEIGHT	WALL THICK	VERTICAL STEEL	HORIZONTAL STEEL	STEEL PLACEMENT
7'-0" OR LESS	8"	#4 @ 24" O.C.	#4 @ 24" O.C. MAX	CENTER OF WALL
8'-0" MAX	8"	#4 @ 18" O.C.	#4 @ 12" O.C. MAX	CENTER OF WALL
9'-0" MAX	8"	#4 @ 12" O.C.	#4 @ 12" O.C. MAX	CENTER OF WALL
10'-0" MAX	8"	#5 @ 12" O.C.	#4 @ 12" O.C. MAX	CENTER OF WALL
11'-0" MAX	10"	#5 @ 8" O.C.	#5 @ 12" O.C. MAX	CENTER OF WALL
12'-0" MAX	10"	#5 @ 8" O.C.	#5 @ 12" O.C. MAX	CENTER OF WALL
14'-0" MAX	10"	#6 @ 12" O.C.	#5 @ 12" O.C. MAX	INSIDE FACE OF WALL

*NOTE: SEE S.O.I FOR CONCRETE LAP SPLICE SCHEDULE.

- (1) HORIZONTAL BAR MINIMUM REQUIRED 4" FROM TOP & BOTTOM.
- SPACE HORIZONTAL BARS EVENLY BETWEEN TOP & BOTTOM BARS NOT TO EXCEED MAX SPACING LISTED.
- STEEL LITELS TO BE 12" DEEP MINIMUM, HAVE (2) #4 @ BOTTOM AND BE 6'-0" LONG MAX.
- $F_c = 2500$ PSL. PROVIDE 3000 PSI FOR QUALITY CONTROL. $f_y = 60$ KSI.
- ACTIVE PRESSURE = 40 PCF.
- WALLS MUST BE RESTRAINED AT TOP AND BOTTOM.
- BACKFILL TO WITHIN 6" OF T.O.W.
- "WALL HEIGHT" FOR SCHEDULE PURPOSES IS TOP OF CONCRETE SLAB TO TOP OF CONCRETE WALL REGARDLESS OF HOW FAR THE WALL EXTENDS BELOW THE CONCRETE SLAB ON GRADE.
- SEE SCHEDULE ABOVE FOR STEEL PLACEMENT.
- TYPICAL ANCHOR BOLTS FOR STRUCTURAL WALLS TO BE 1/2" x 8" SIMPSON TITEN HD ANCHORS AND 3x3x0.229" WASHERS WITH SPACING PER ANCHOR BOLT SCHEDULE. SEE DETAIL C.S31.

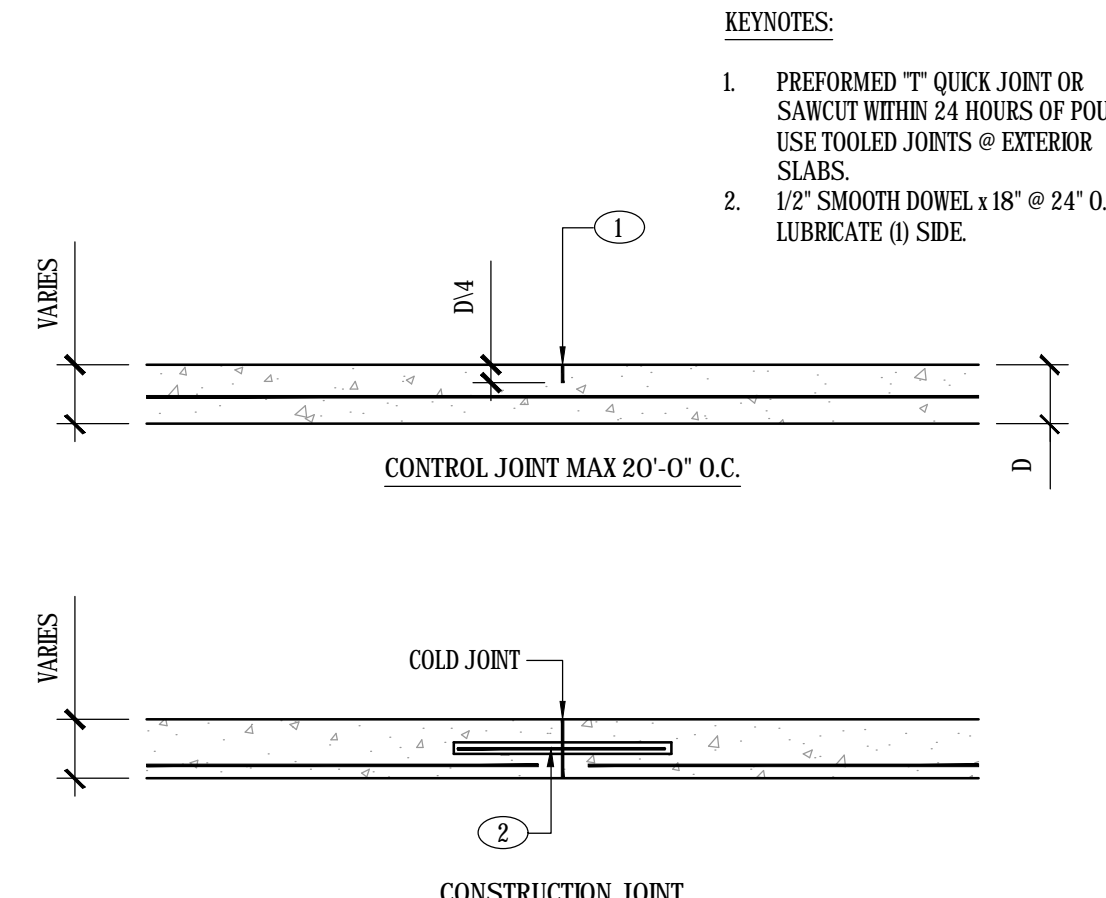
A FOUNDATION WALL SCHEDULE

S-300 NO SCALE



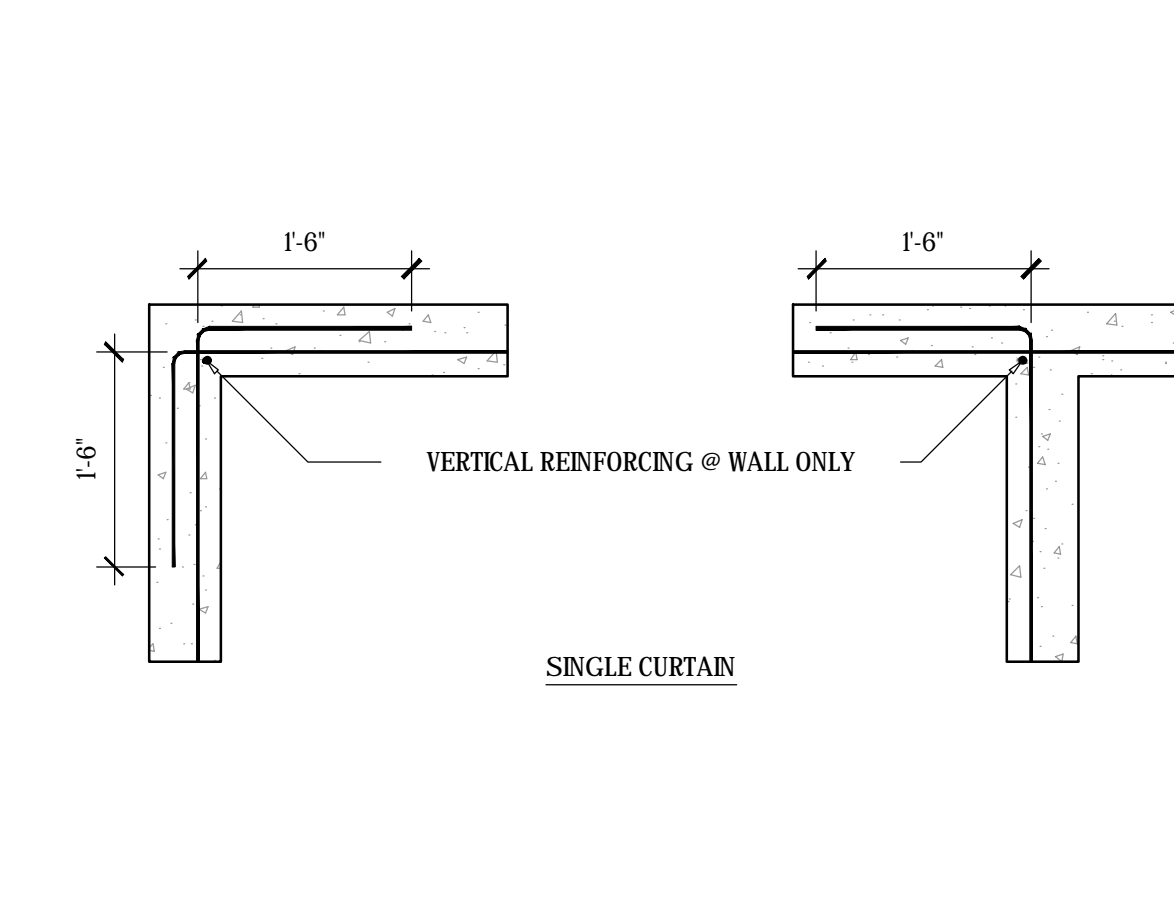
B BAR BENDS, HOOKS, AND OFFSETS

S-300 NO SCALE



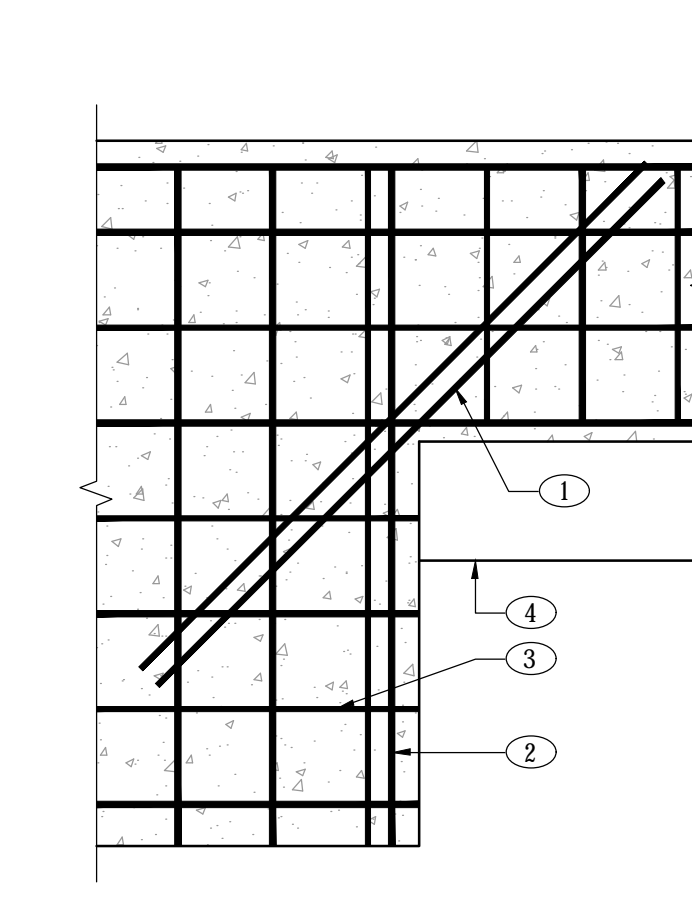
C SLAB ON GRADE JOINT

S-300 NO SCALE



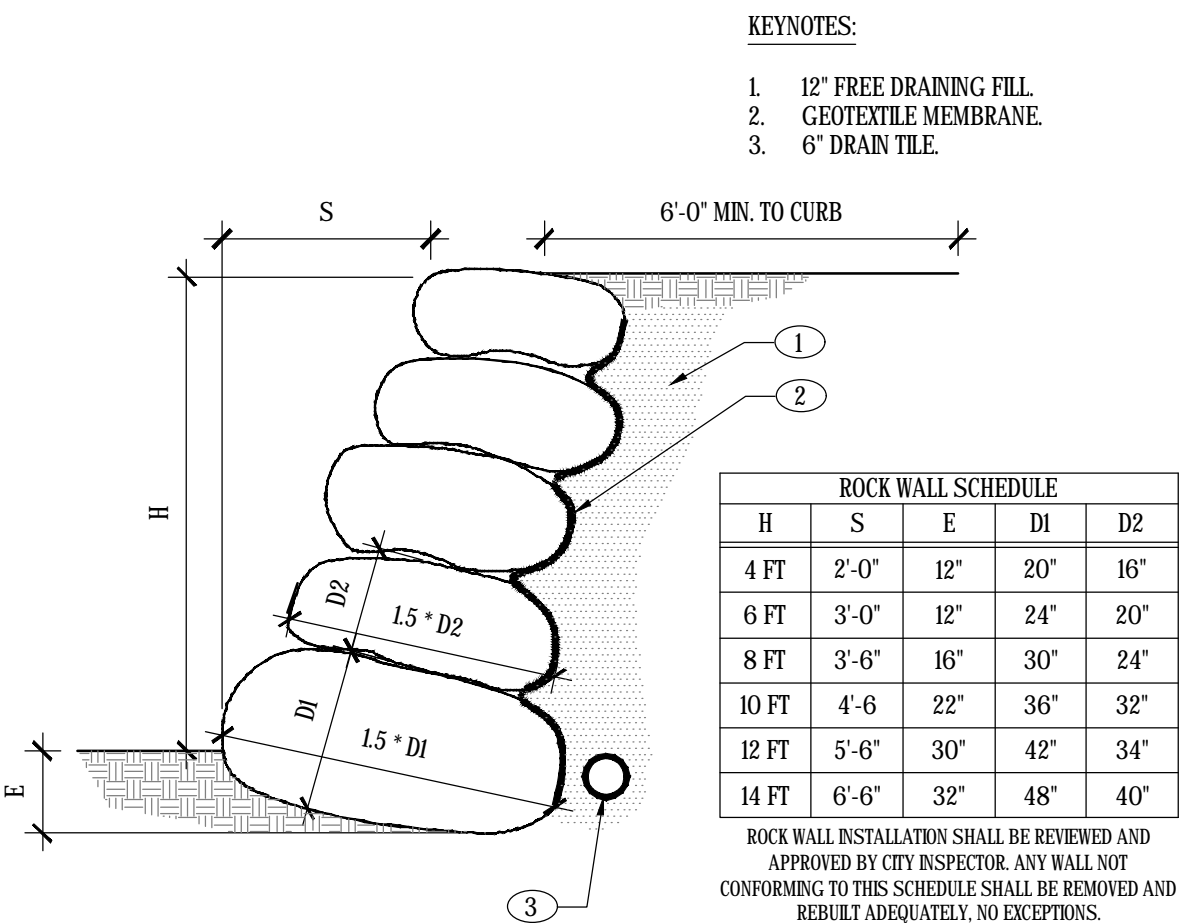
D INTERSECTIONS OF WALLS AND FOOTINGS

S-300 NO SCALE



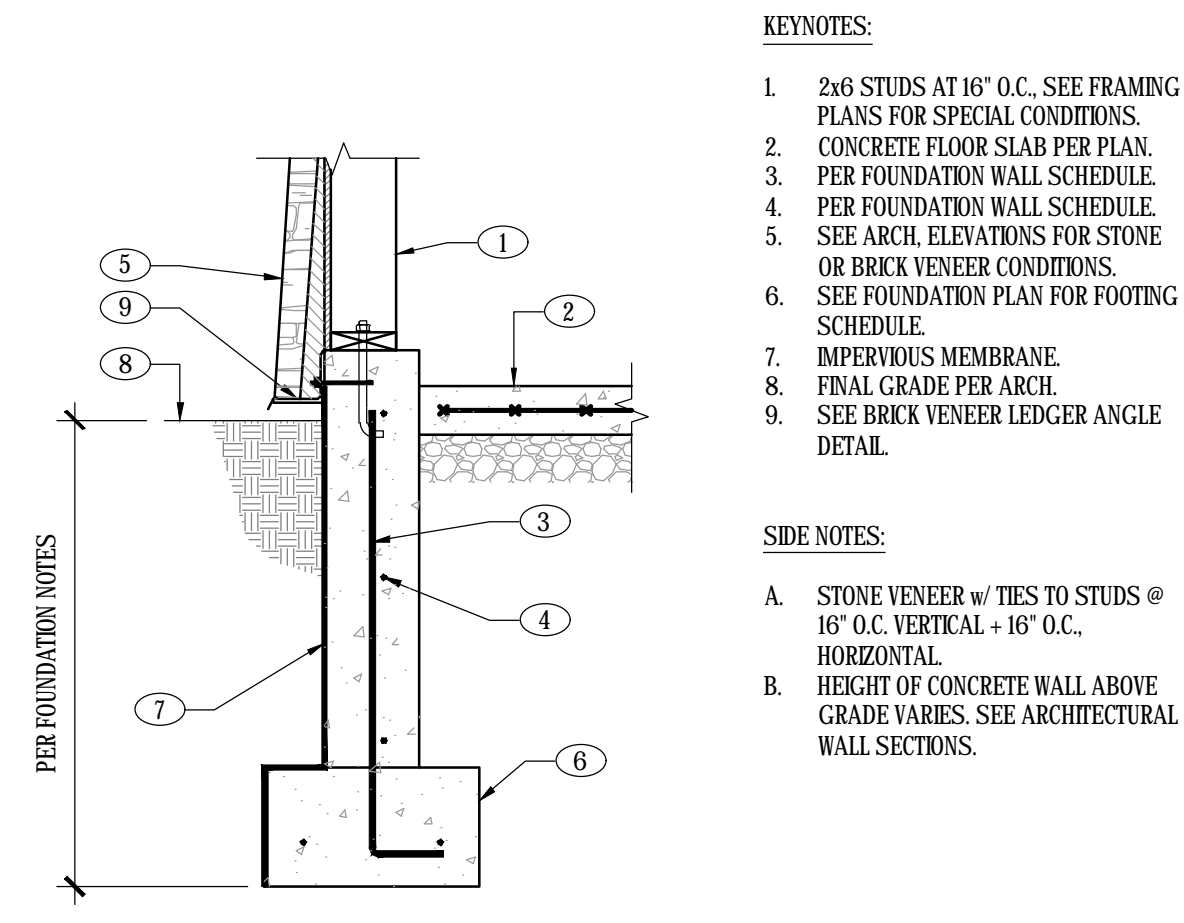
E TYPICAL FOOTING / WALL STEP

S-300 NO SCALE



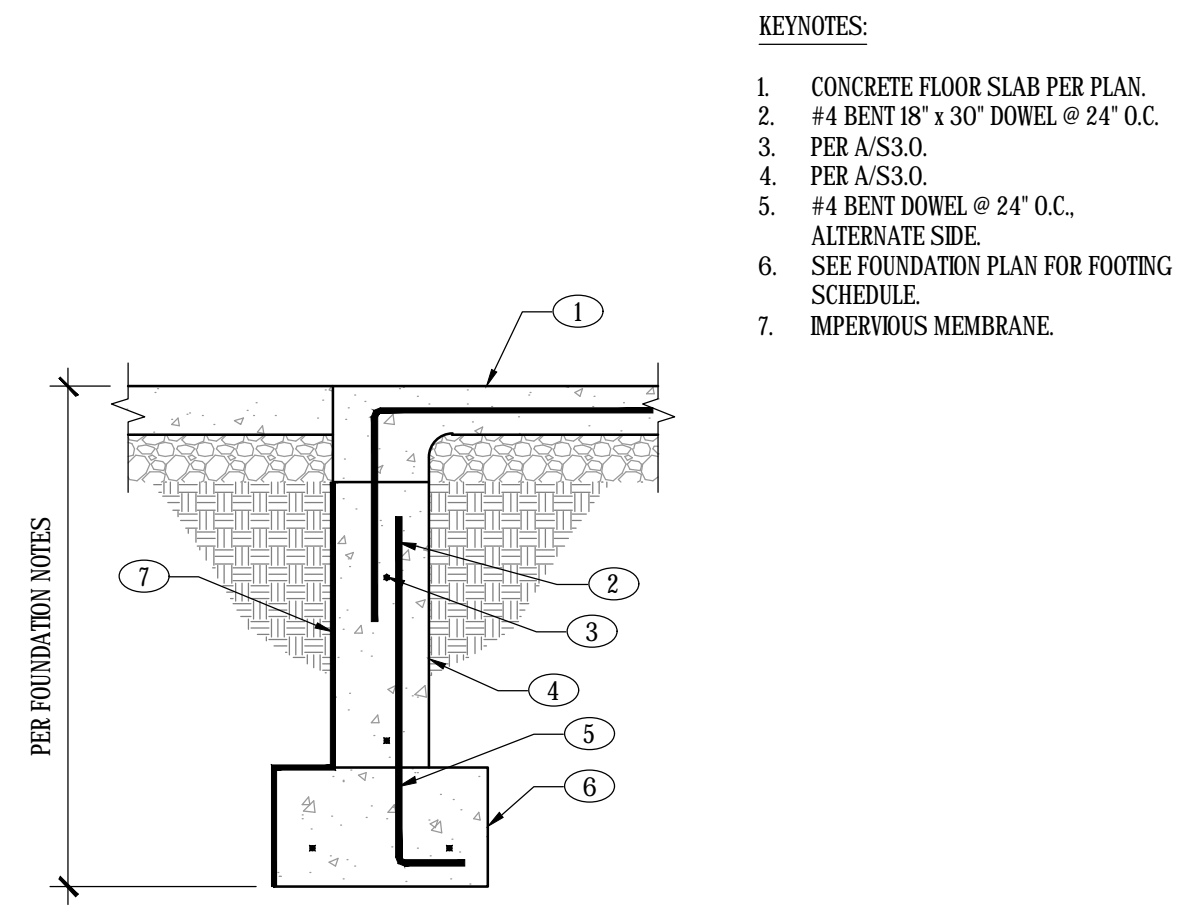
F ROCK PROTECTED SLOPES - SINGLE TIER (NO SURCHARGE)

S-300 NO SCALE



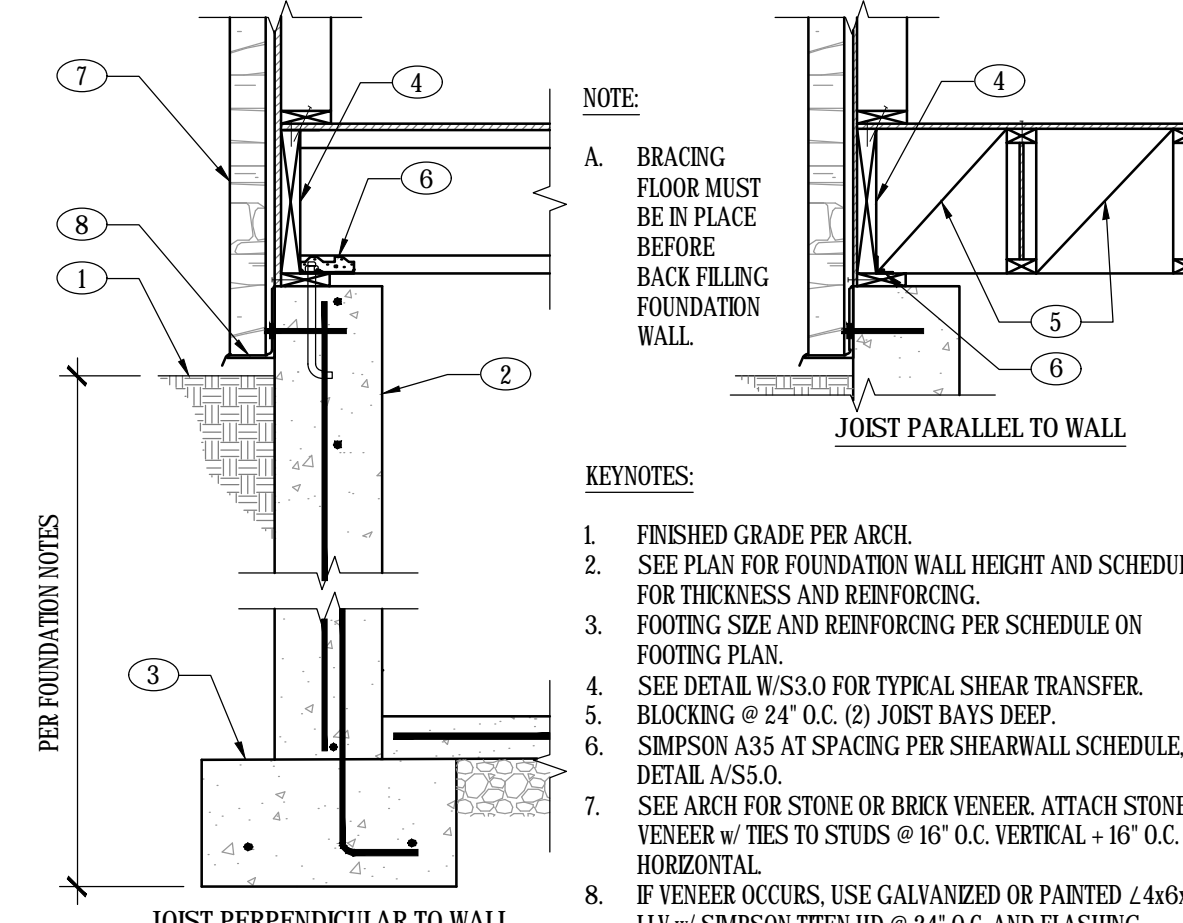
G TYPICAL FROST-PROTECTED FOUNDATION

S-300 NO SCALE



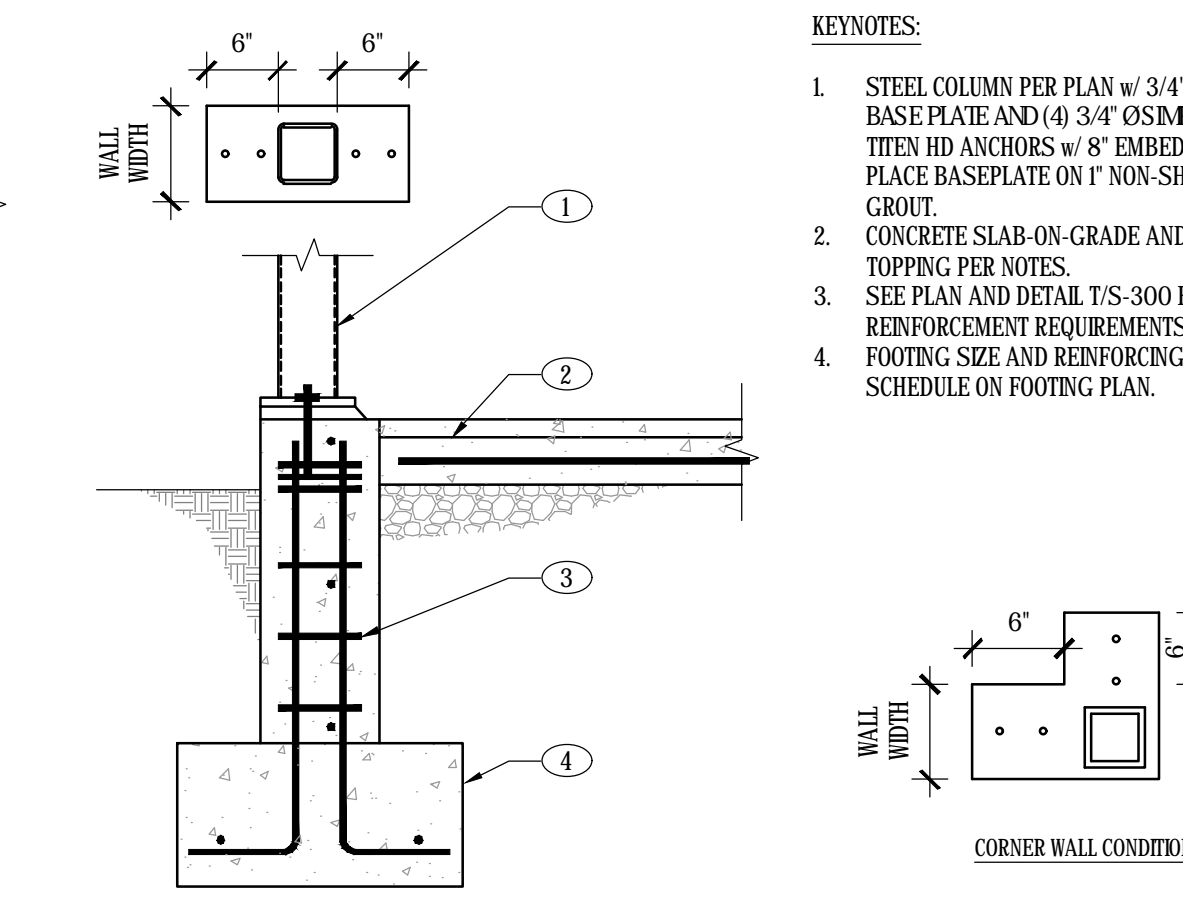
H OPENING @ FOUNDATION

S-300 NO SCALE



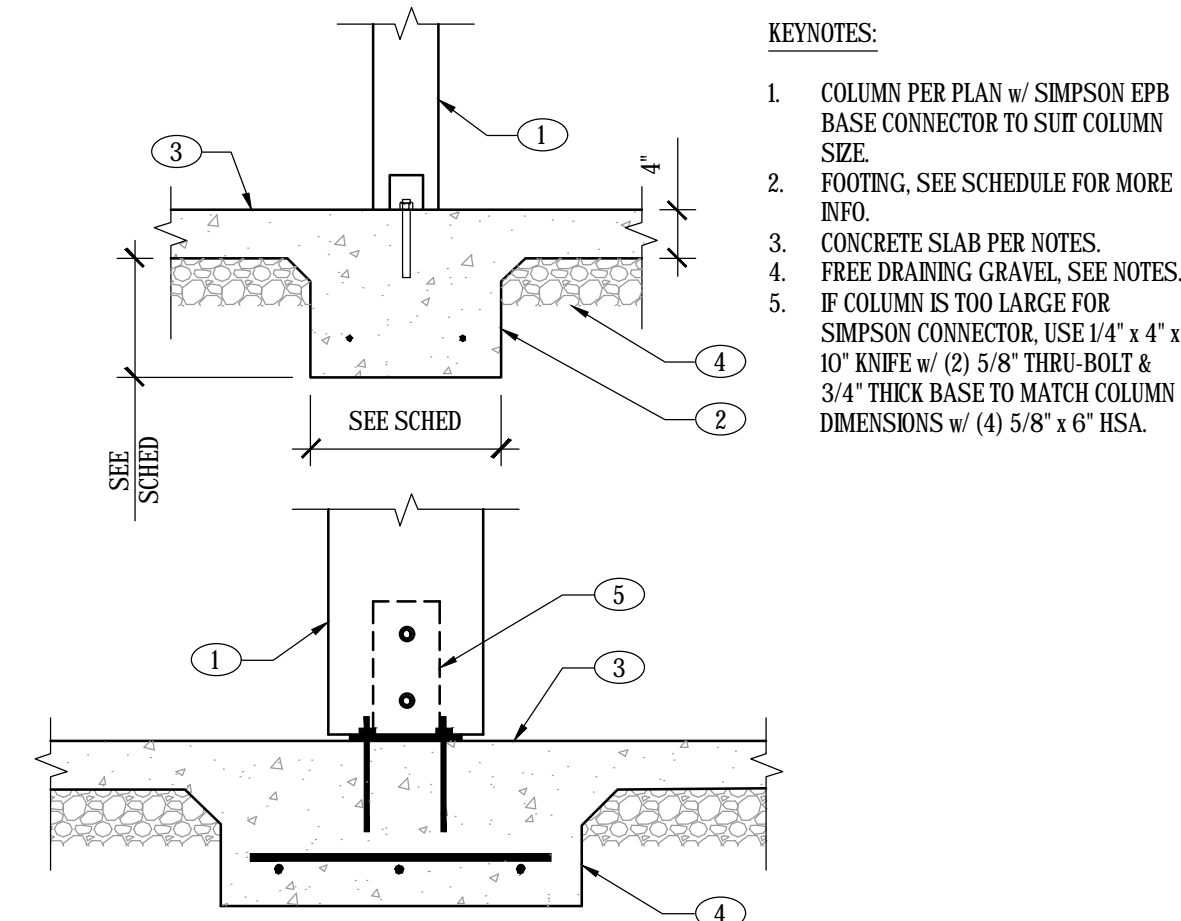
J FLOOR JOISTS ON TOP OF FOUNDATION WALL

S-300 NO SCALE



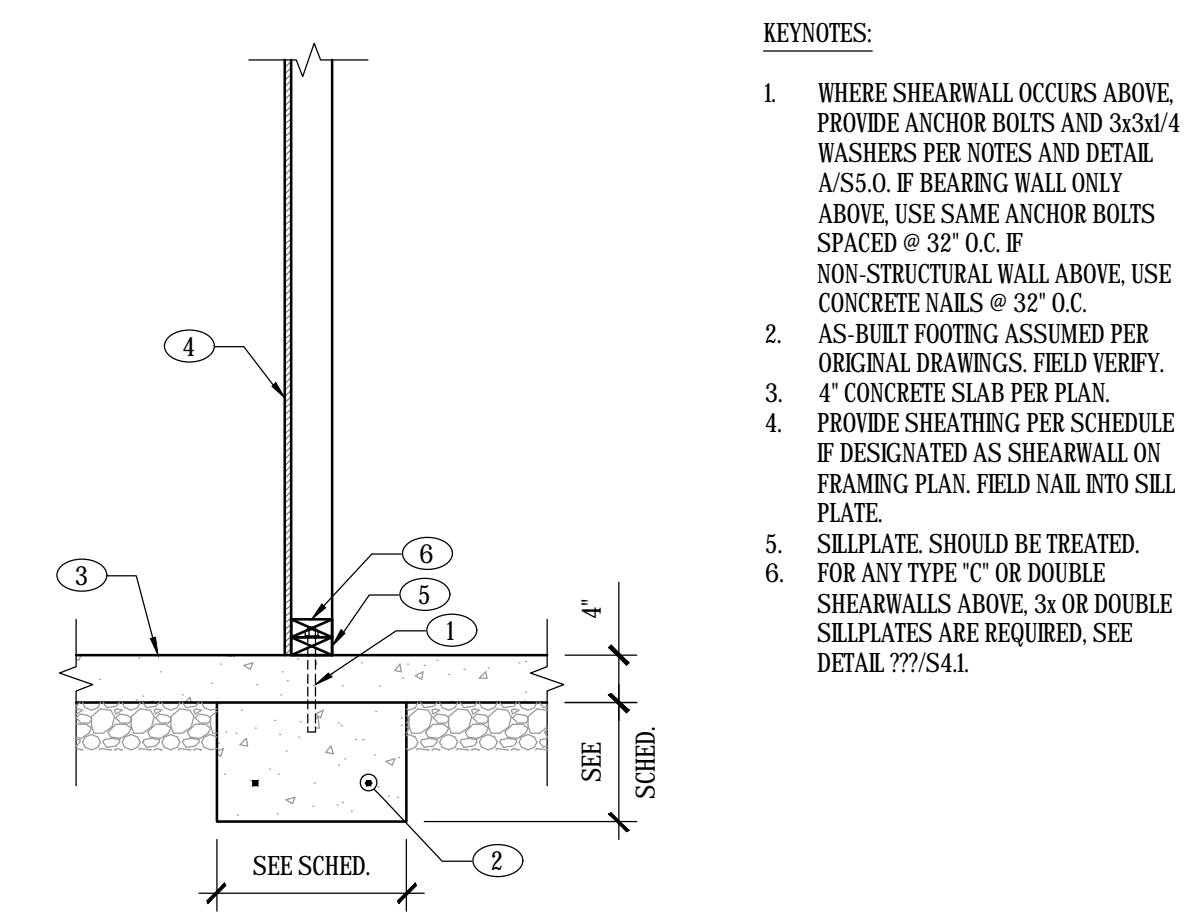
K STEEL COLUMN AT PERIMETER FOUNDATION WALL

S-300 NO SCALE



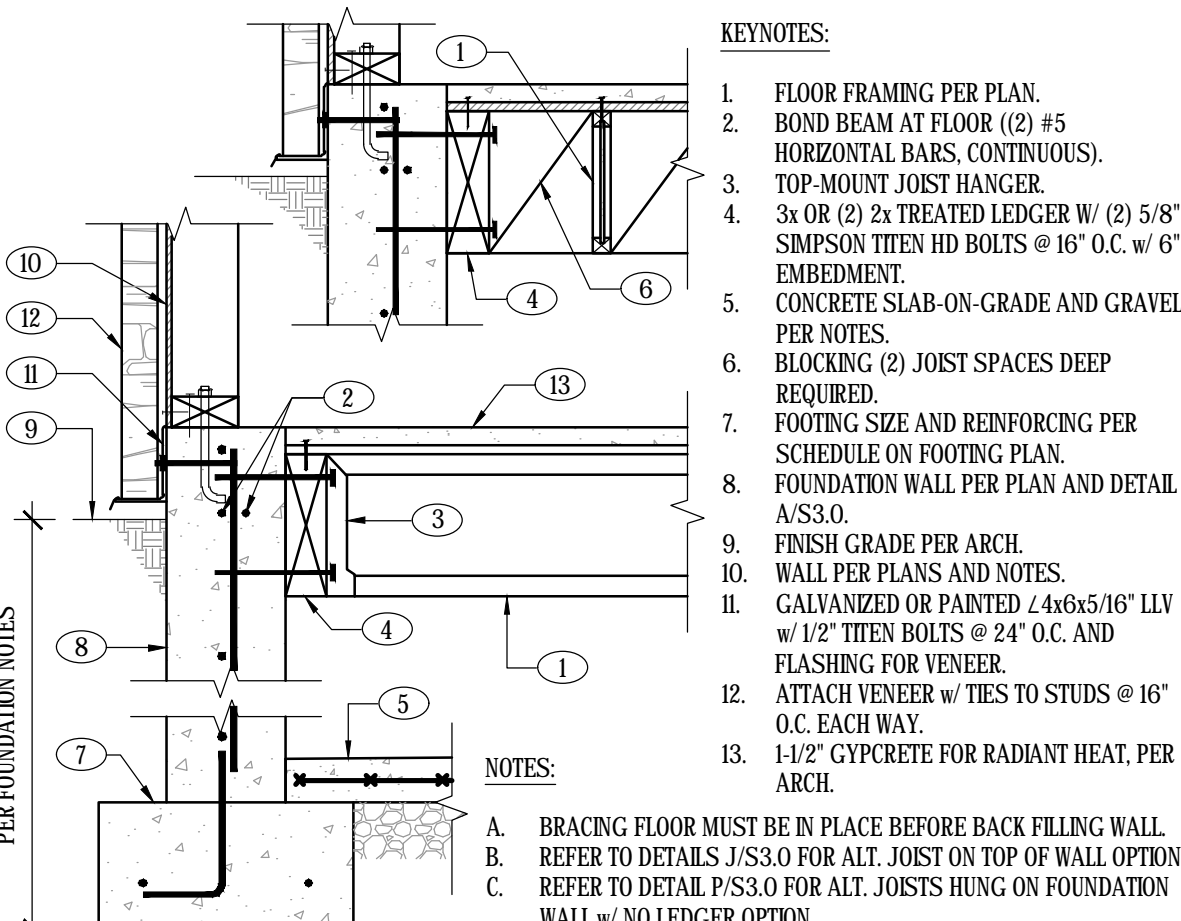
L INTERIOR WOOD COLUMN AT FOUNDATION

S-300 NO SCALE



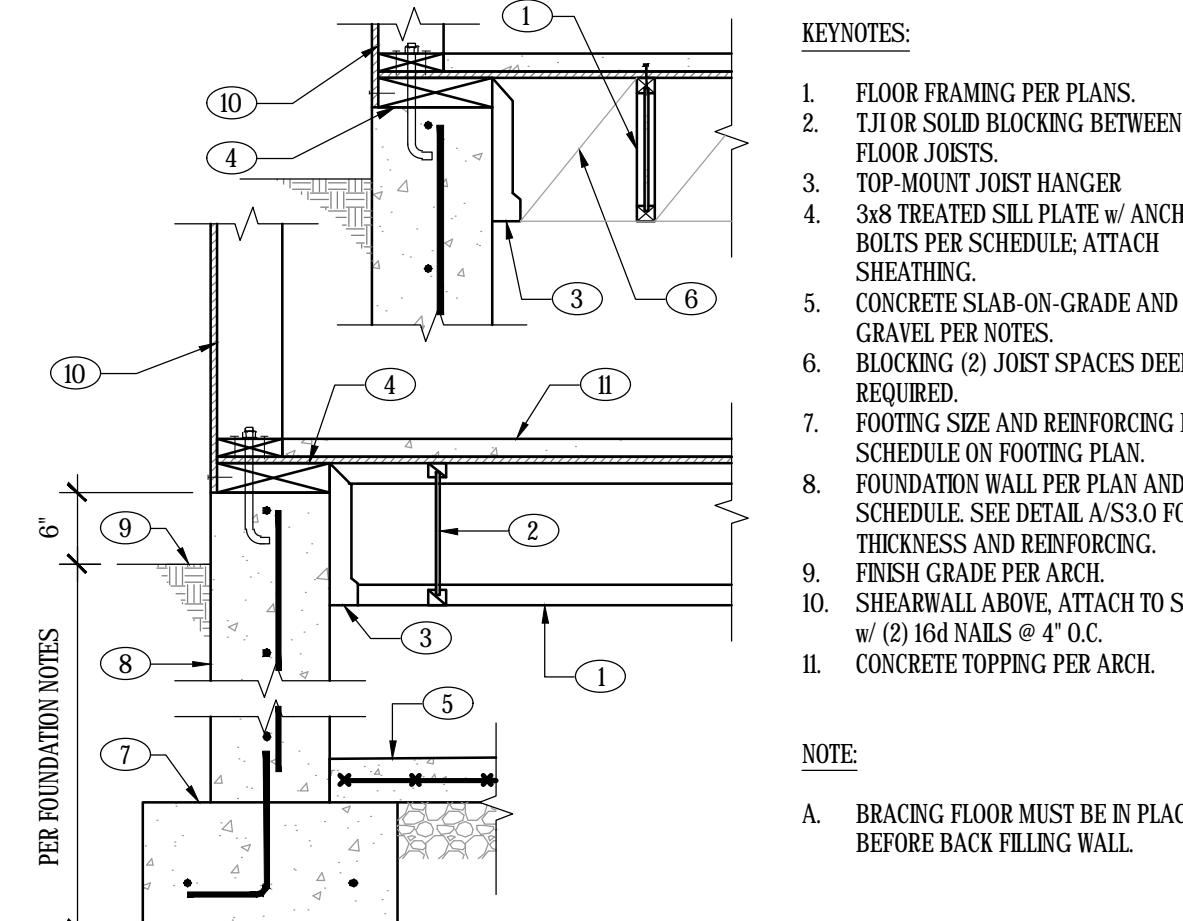
M INTERIOR FOOTING

S-300 NO SCALE



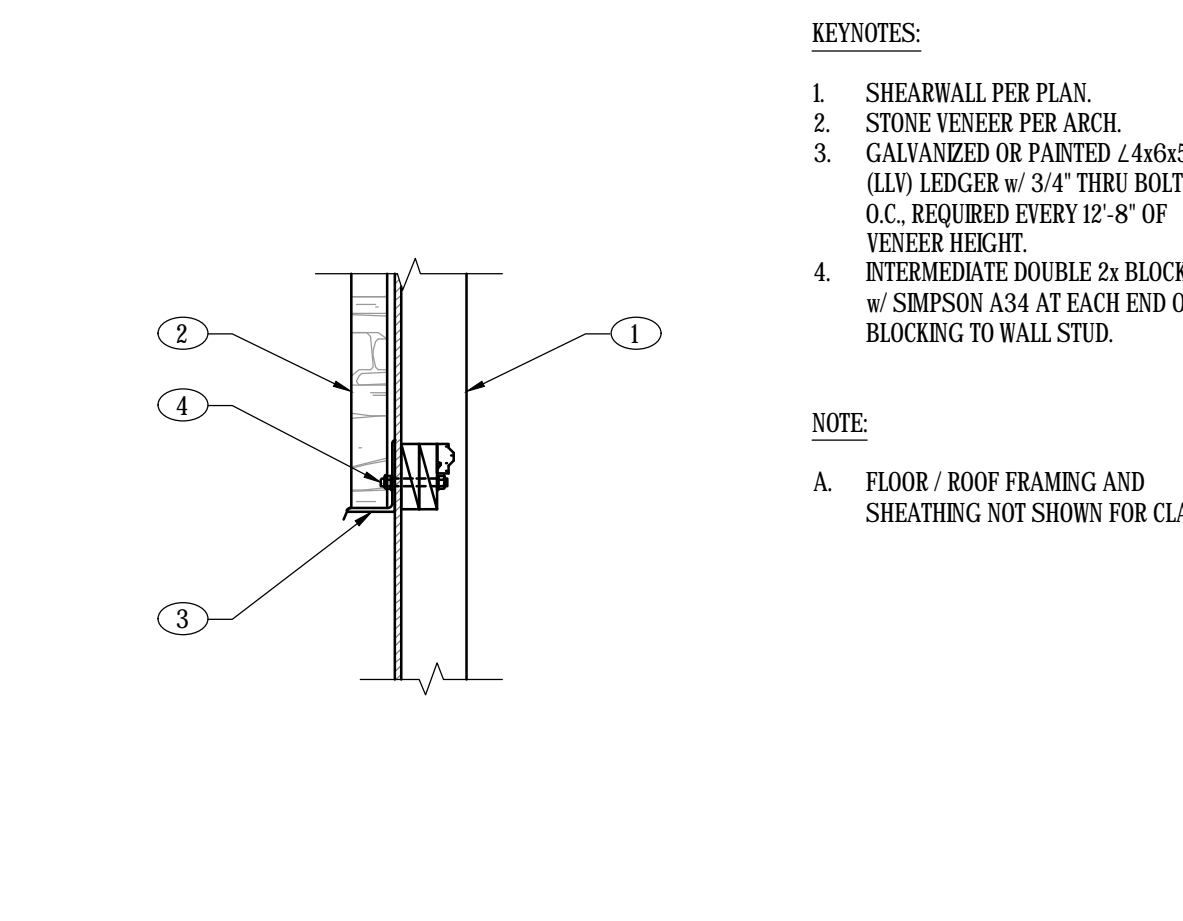
N FLOOR JOISTS LEDGERED TO FOUNDATION WALL

S-300 NO SCALE



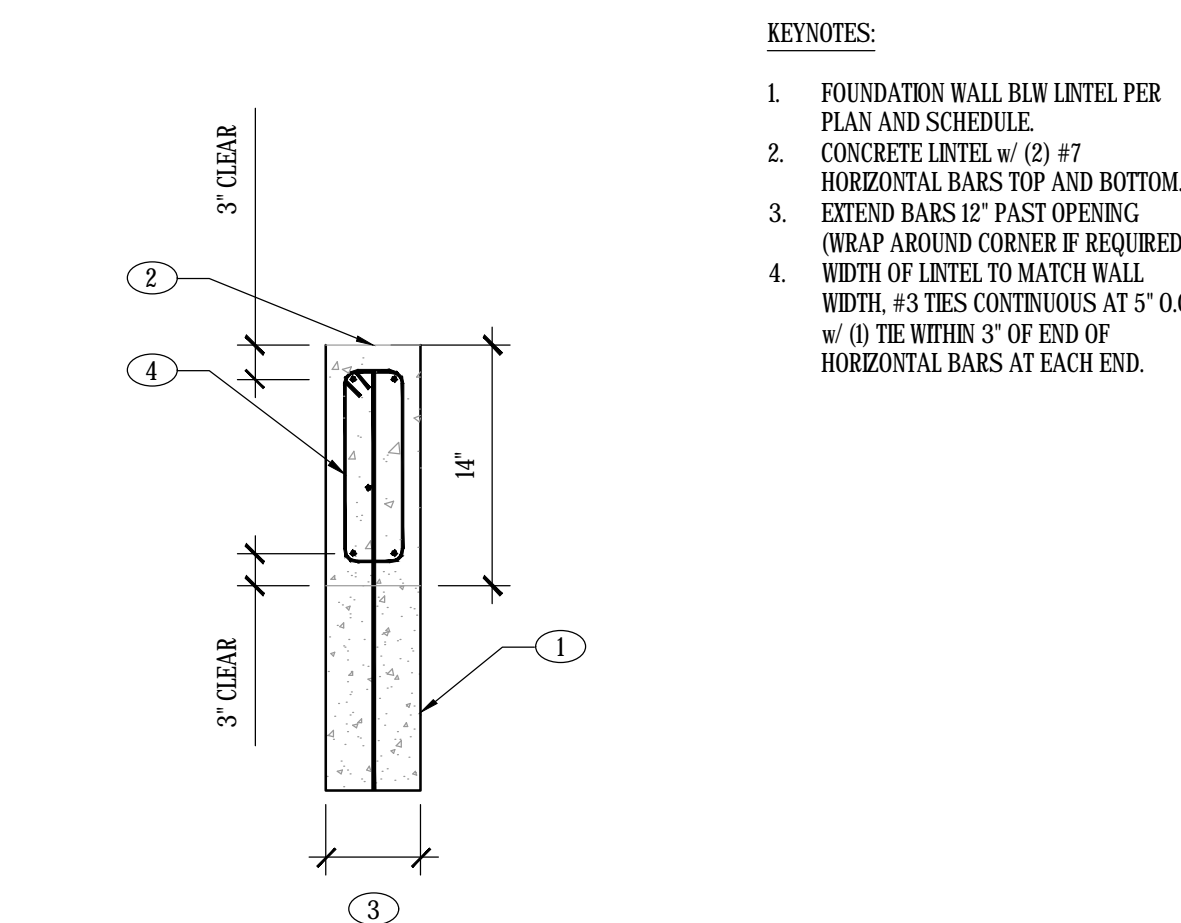
P JOISTS / RAFTERS AT HANGER HUNG OFF SILL PLATE

S-300 NO SCALE



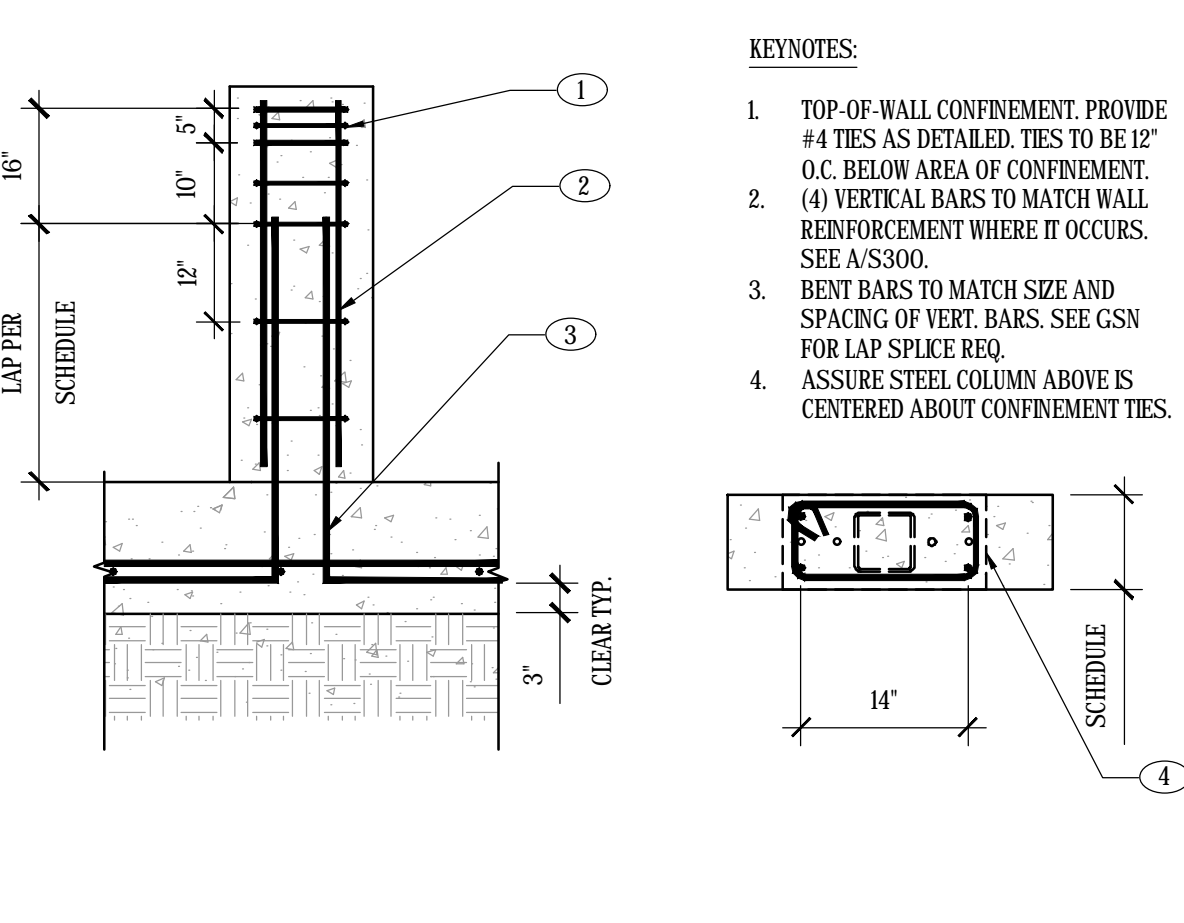
R TYPICAL BRICK VENEER LEDGER ANGLE

S-300 NO SCALE



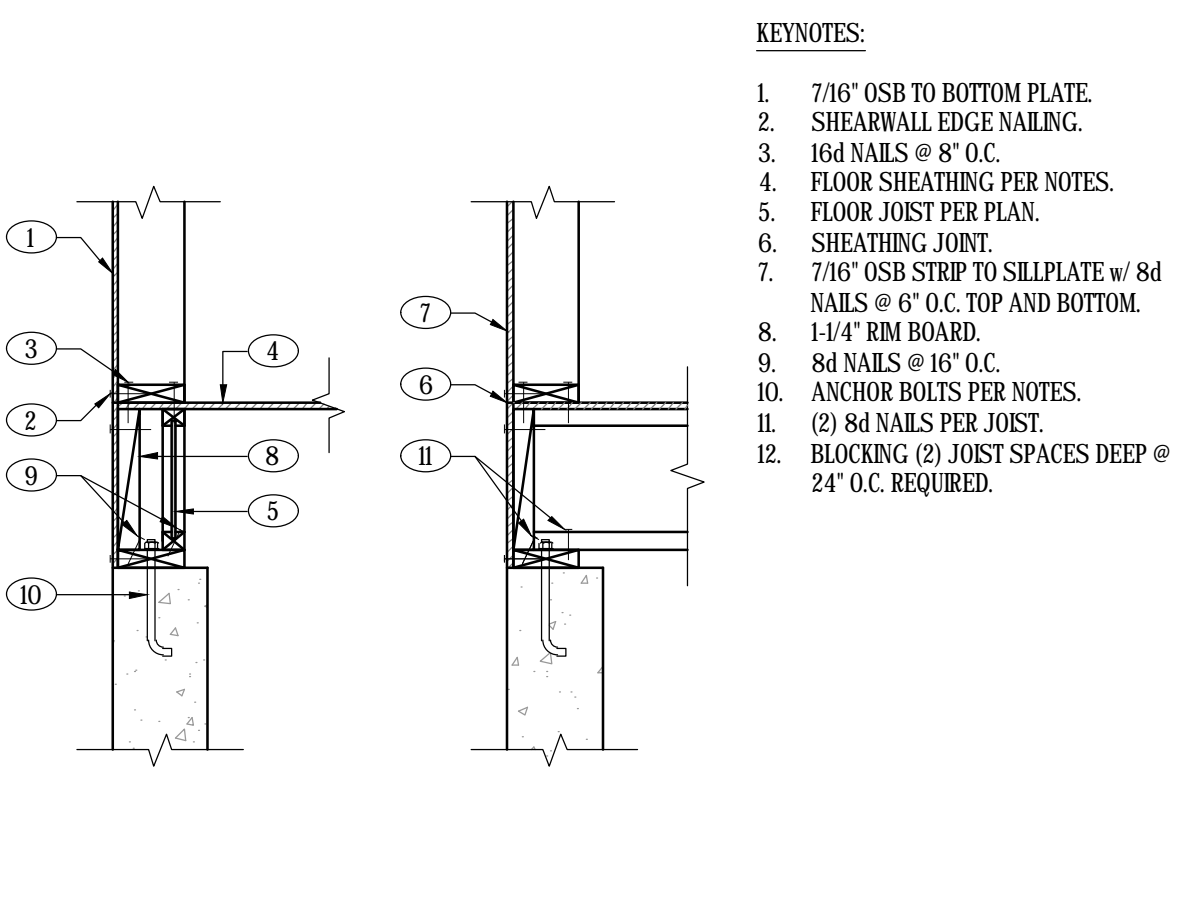
S CONCRETE LINTEL

S-300 NO SCALE



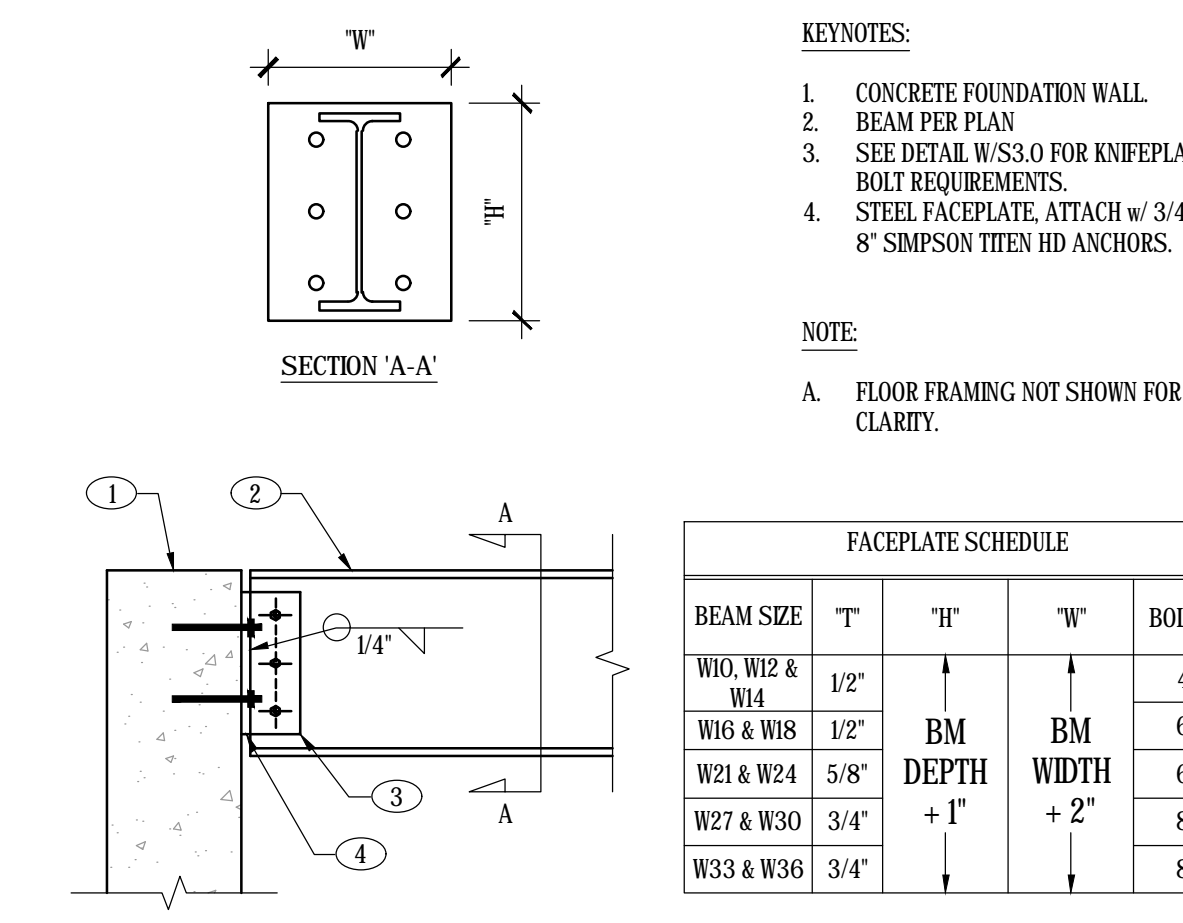
T WALL CONFINEMENT DETAIL

S-300 NO SCALE



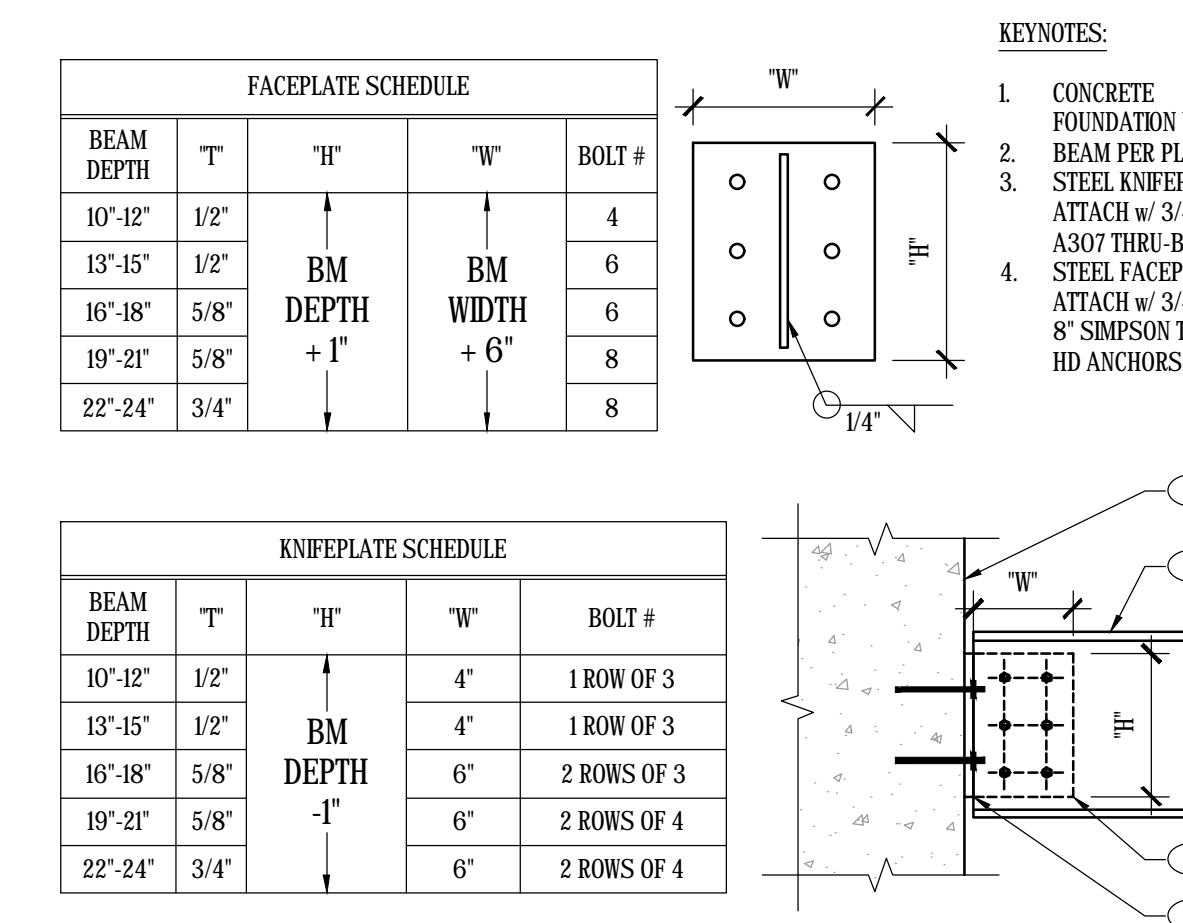
W TYPICAL SHEAR TRANSFER

S-300 NO SCALE



V STEEL BEAM TO CONCRETE WALL

S-300 NO SCALE



W WOOD BEAM TO CONCRETE WALL

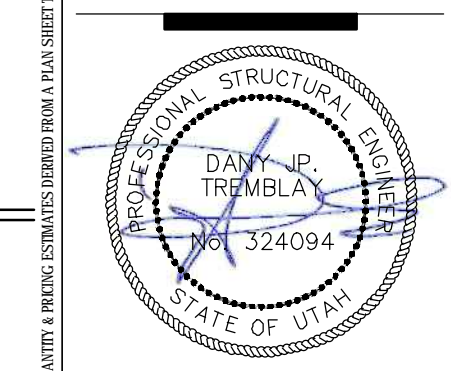
S-300 NO SCALE

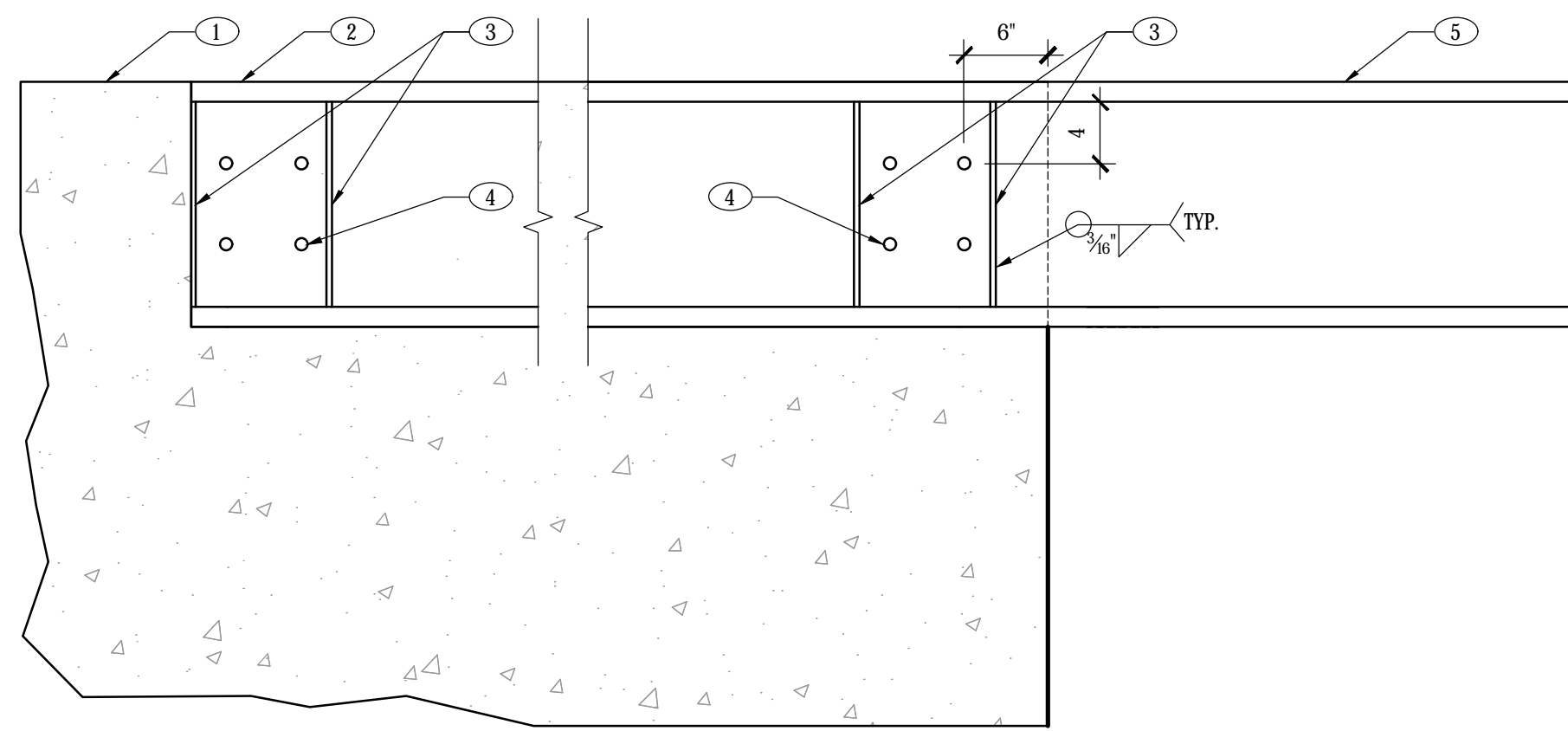
- KEYNOTES:**
- (2) #4 x 5'-0"
 - (2) #4 VERTICAL
 - WALL REINFORCING PER SCHEDULE
 - STRIP FOOTING PER PLAN AND SCHEDULE

- KEYNOTES:**
- STEEL COLUMN PER PLAN w/ 3/4" BASE PLATE AND (4) 3/4" OSMIPSON TITEN HD ANCHORS w/ 8" EMBEDMENT. PLACE BASEPLATE ON 1" NON-SHRINK GROUT.
 - CONCRETE SLAB-ON-GRADE AND TOPPING PER NOTES.
 - SEE PLAN AND DETAIL T/S-300 FOR REINFORCEMENT REQUIREMENTS. FOOTING SIZE AND REINFORCING PER SCHEDULE ON FOOTING PLAN.

- KEYNOTES:**
- SHEARWALL PER PLAN.
 - STONE VENEER PER ARCH.
 - GALVANIZED OR PAINTED 4x6x5/16" (LLV) LEDGER w/ 3/4" THRU BOLT @ 16" O.C. REQUIRED EVERY 12'-8" OF VENEER HEIGHT.
 - INTERMEDIATE DOUBLE 2x BLOCKING w/ SIMPSON A34 AT EACH END OF BLOCKING TO WALL STUD.
- NOTE:**
- A. FLOOR / ROOF FRAMING AND SHEATHING NOT SHOWN FOR CLARITY.

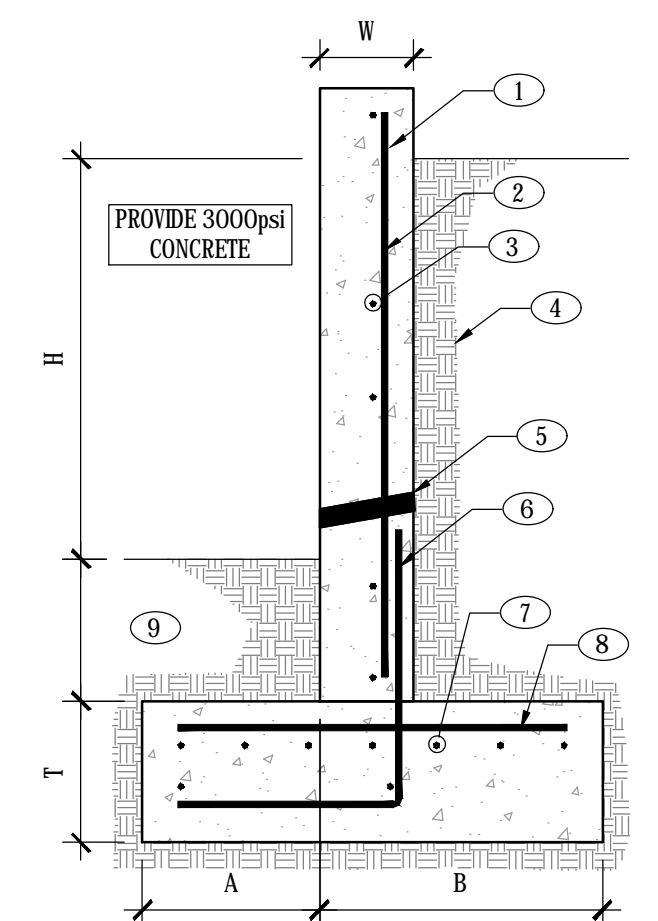
- KEYNOTES:**
- CONCRETE FOUNDATION WALL.
 - BEAM PER PLAN.
 - STEEL KNIFEPLATE & STEEL REQUIREMENTS.
 - STEEL FACEPLATE, ATTACH w/ 3/4" x 8" SIMPSON TITEN HD ANCHORS.
- NOTE:**
- A. FLOOR FRAMING NOT SHOWN FOR CLARITY.





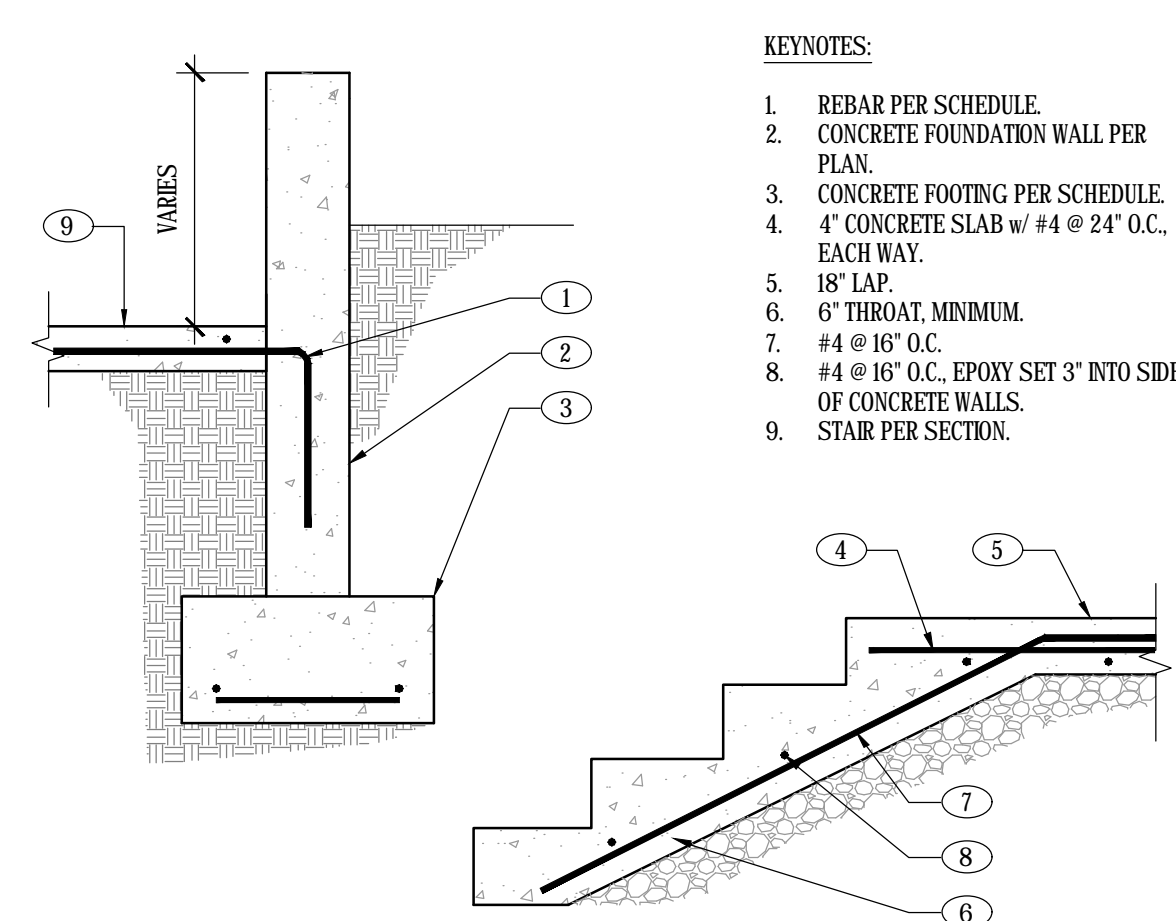
A CHANNEL BEAM BOLTED TO FACE OF CONCRETE
S-301 NO SCALE

- KEYNOTES:**
1. CONCRETE FOUNDATION WALL
 2. STEEL CHANNEL BEAM PER PLAN
 3. 3/8" WEB STIFFENERS
 4. 4- 3/4" x 6" SIMPSON TITEN HD ANCHORS
 5. CANTILEVERED END LENGTH NOT TO EXCEED LENGTH SHOWN ON STRUCTURAL FRAMING PLANS.
- NOTE:**
- A. FLOOR FRAMING NOT SHOWN FOR CLARITY.
 - B. SEE L/S-400 FOR TYP. WOOD NAILER CONNECTION
 - C. B-DECKING MAY OCCUR AT TOP FLANGE OF CHANNEL BEAM, IF SO, CONNECT DECKING TO CHANNEL PER E/S-301.
 - D. VERIFY ELEVATION OF CHANNEL BEAM WITH ARCHITECTURAL PLANS.



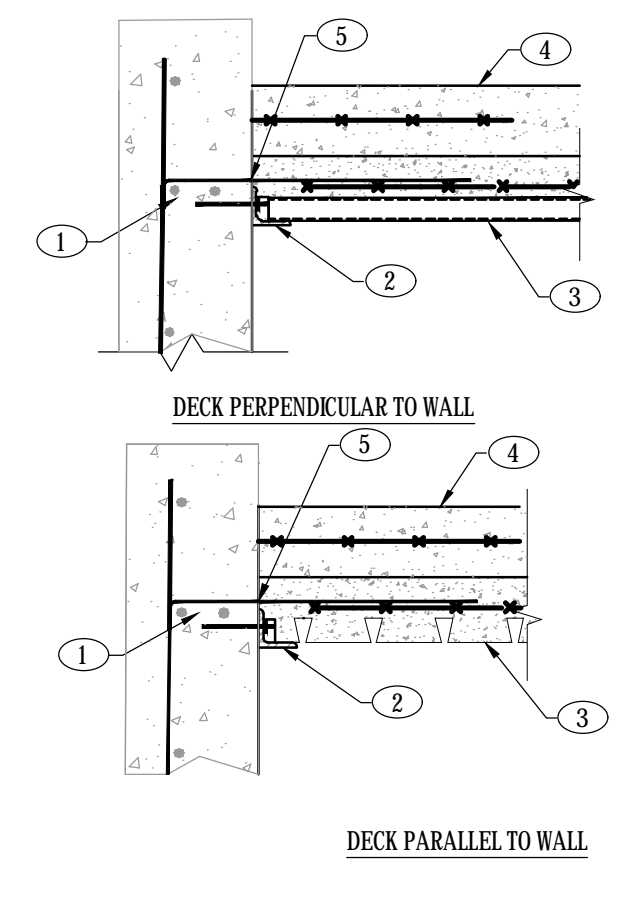
C TYPICAL RETAINING WALL AND SCHEDULE
S-301 NO SCALE

- KEYNOTES:**
1. 2" COVER TO VERTICAL
 2. "C" VERTICAL @ "D" O.C.
 3. #4 HORIZONTAL @ 16" O.C.
 4. FREE DRAINING BACKFILL MATERIAL (NO SILTY OR SANDY SOILS)
 5. 1-1/2" WEEP HOLE @ 30" O.C. w/ 1/2" ROUND GRAVEL TRENCH RUNNING BEHIND, NOT REQUIRED FOR WALLS < 4 ft
 6. "C" DOWEL @ "D" O.C. w/ 30" LAP.
 7. (7) #4 CONTINUOUS AS SHOWN.
 8. "C" TRANSVERSE @ "D" O.C.
 9. MIN. 18" EXCEPT 6" ALLOWED FOR 2 ft WALLS.



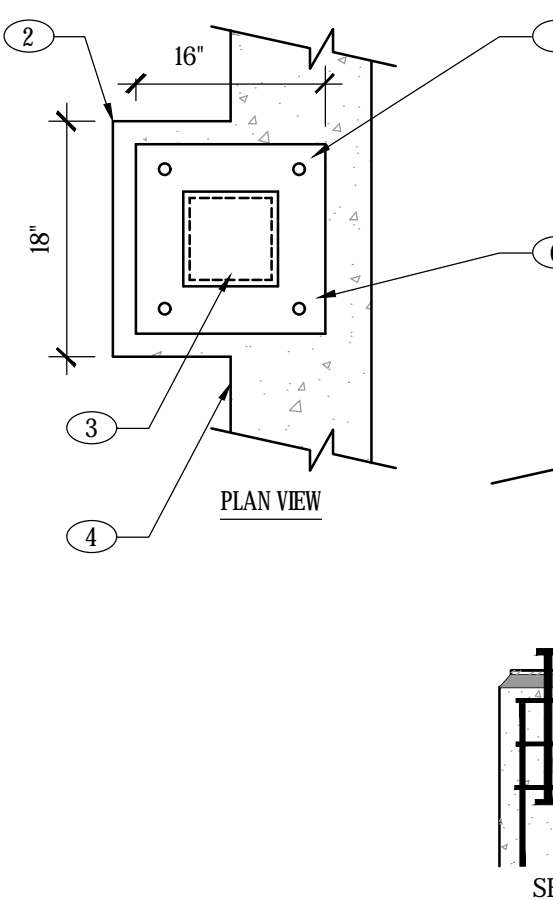
D CONCRETE STAIRS (IF OCCUR)
S-301 NO SCALE

- KEYNOTES:**
1. REBAR PER SCHEDULE
 2. CONCRETE FOUNDATION WALL PER PLAN
 3. CONCRETE FOOTING PER SCHEDULE
 4. 4" CONCRETE SLAB w/ #4 @ 24" O.C. EACH WAY.
 5. 18" LAP.
 6. 6" THROAT, MINIMUM
 7. #4 @ 16" O.C.
 8. #4 @ 16" O.C. EPOXY SET 3" INTO SIDE OF CONCRETE WALLS.
 9. STAIR PER SECTION.



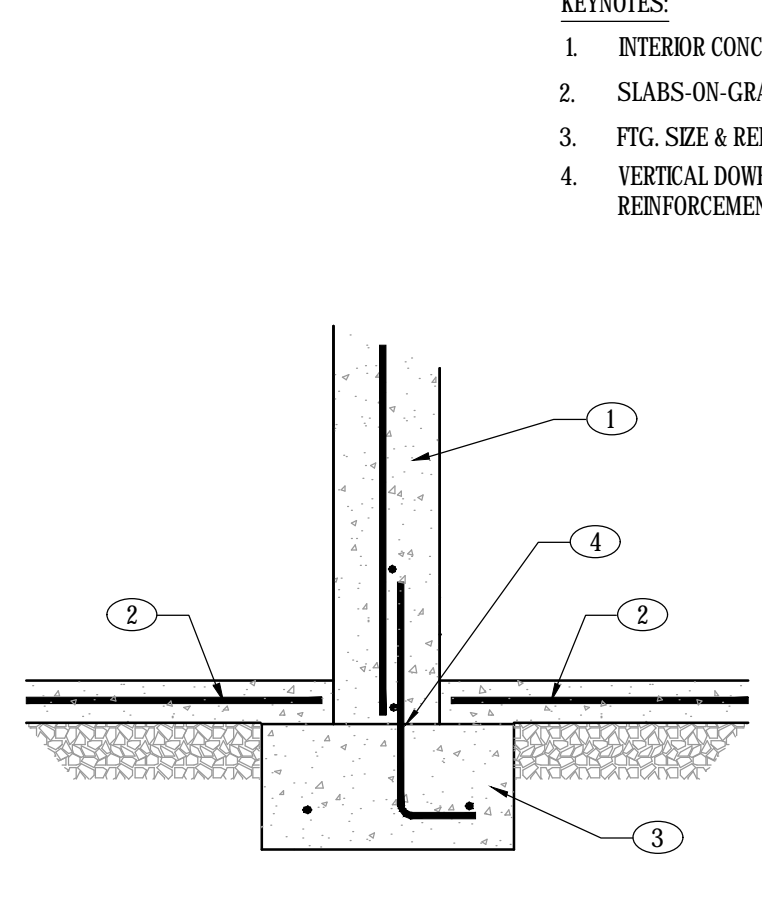
E B-DECK TO FOUNDATION WALL
S-301 NO SCALE

- KEYNOTES:**
1. CONCRETE WALL PER PLANS & SCHEDULES.
 2. 13x3/4" CONTINUOUS STEEL ANGLE LEDGER W/ 3/4" DIA SIMPSON TITEN HD (OR EQUIV) @ 24" O.C. WITH 6" MIN. EMBED. DECK MAY SLOPE PER ARCH. SEE PLANS.
 3. VERO W2 FORMLOCK COMPOSITE METAL DECK SYSTEM (4" TOTAL SLAB DEPTH) W/ 6x6-WL4xWL4 WWF. 3/4" PUDDLE WELDS @ 12" O.C. AT ALL STL CONNECTIONS.
 4. CONC. TOPPING/SURFACE TREATMENT & WATERPROOFING PER ARCH.
 5. #4 REBAR DOWELS @ 24" O.C. EXTEND 18" INTO SLAB & WALL EACH.
- SIDENOTES:**
- A. SEE PLANS FOR SURFACE CONDITIONS.
 - B. GREEN ROOF MAY OCCUR. SEE ARCH FOR WATERPROOFING & MORE INFORMATION.
 - C. CURB MAY OR MAY NOT OCCUR. SEE ARCH PLANS.



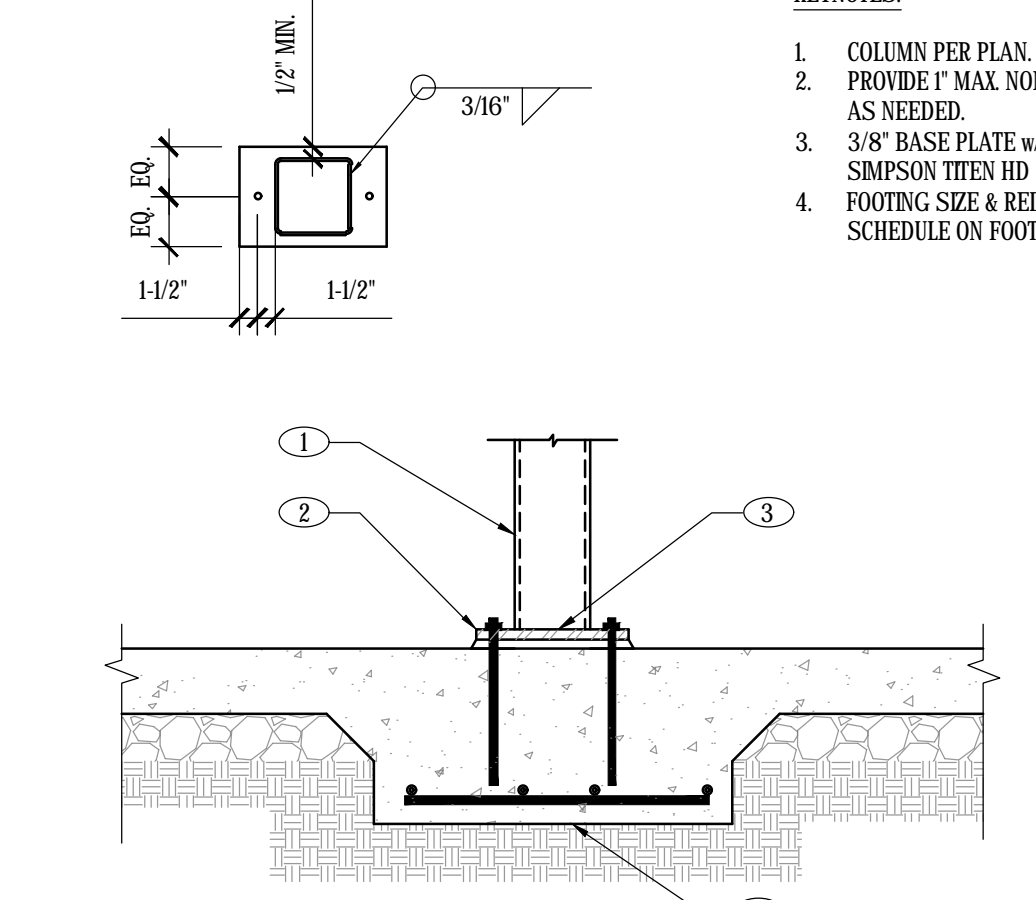
F MOMENT FRAME COLUMN CONNECTION
S-301 NO SCALE

- KEYNOTES:**
1. 16" x 16" x 1" THICK PL. w/ (4) 3/4" HEAVY HEX BOLTS CAST IN-PLACE. MIN. 12" EMBEDMENT ON 1" NON-SHRINK GROUT. OR EPOXY SET ANCHOR BOLT EMBEDDED 14" INTO CONC.
 2. 18" x 18" CONCRETE COLUMN & FOUNDATION BELOW. SEE PLAN AND SCHEDULE FOR WALL REINFORCING
 3. MOMENT FRAME COLUMN PER PLAN
 4. CONCRETE WALL PER PLAN
 5. 1" NON-SHRINK GROUT
 6. 16" GAGE W/4-#4 VERTICAL BARS AT EA CORNER W/TOP OF WALL CONFINEMENT PER U/S-300



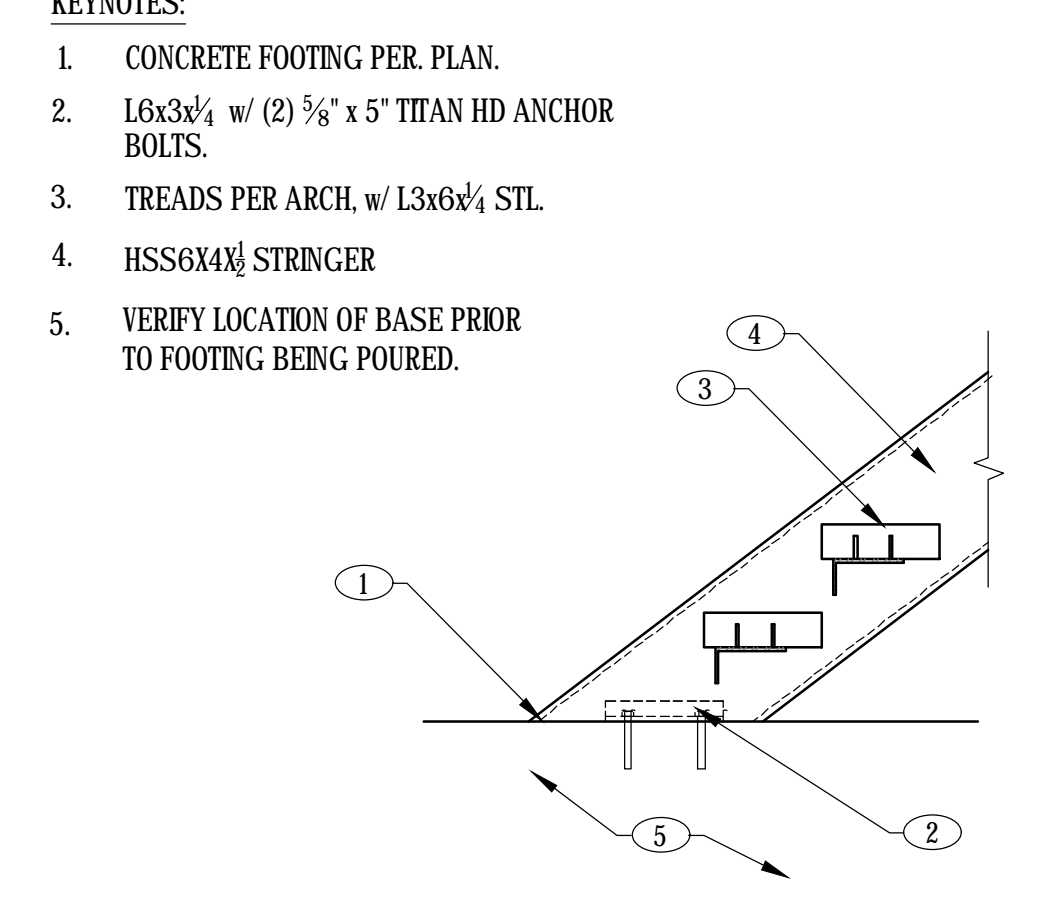
G INTERIOR CONCRETE WALL
S-301 NO SCALE

- KEYNOTES:**
1. INTERIOR CONCRETE WALL, REINFORCED PER A/S-31
 2. SLABS ON-GRADE. PER FOUNDATION PLAN NOTES.
 3. FTG. SIZE & REINF. PER SCHEDULE ON FTG. PLAN
 4. VERTICAL DOWEL INTO FOOTING TO MATCH VERTICAL WALL REINFORCEMENT.

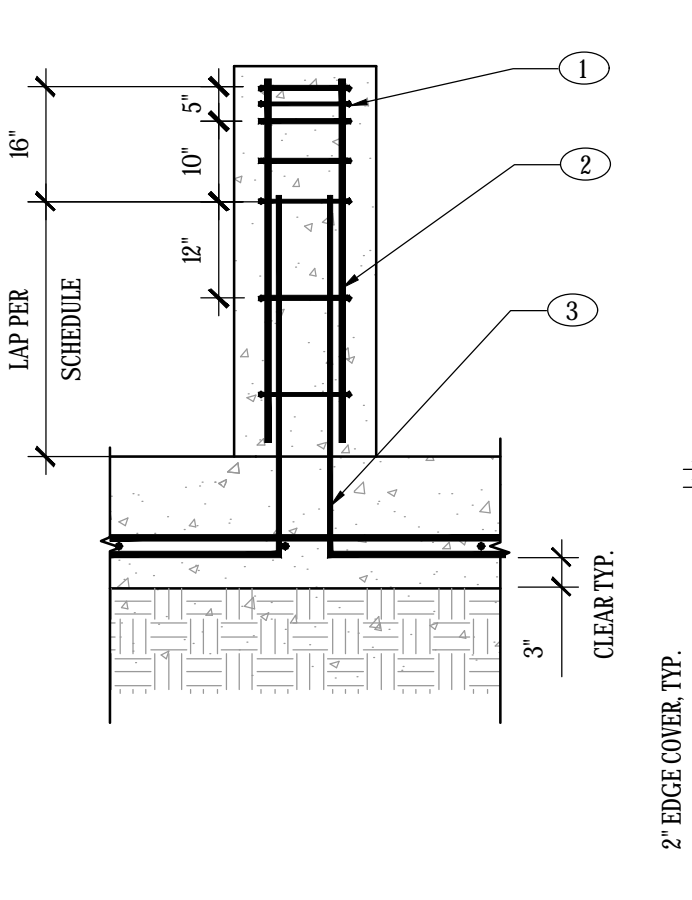


H TALL FOUNDATION WALL
S-301 NO SCALE

- KEYNOTES:**
1. COLUMN PER PLAN.
 2. PROVIDE 1" MAX. NON-SHRINK GROUT AS NEEDED.
 3. 3/8" BASE PLATE w/ (4) 1/2" x 8" SIMPSON TITEN HD (5-1/2" EMBED).
 4. FOOTING SIZE & REINFORCING PER SCHEDULE ON FOOTING PLAN.



J STEEL STRING AT FOUNDATION CONNECTION
S-301 NO SCALE



H CONCRETE COLUMN (CC2) AT FLOOR SKY LIGHT OPENINGS
S-301 NO SCALE

- KEYNOTES:**
1. TOP-OF-WALL CONFINEMENT. PROVIDE #4 TIES AS DETAILED. TIES TO BE 12" O.C. BELOW AREA OF CONFINEMENT.
 2. (4) VERTICAL BARS TO MATCH WALL REINFORCEMENT WHERE IT OCCURS. SEE A/S-300. BENT BARS TO MATCH SIZE AND SPACING OF VERT. BARS. SEE GSN FOR LAP SPLICE REQ.
 3. ASSURE STEEL COLUMN ABOVE IS CENTERED ABOUT CONFINEMENT TIES.

L NOT USED
S-301 NO SCALE

M NOT USED
S-301 NO SCALE

N NOT USED
S-301 NO SCALE

P NOT USED
S-301 NO SCALE

R NOT USED
S-301 NO SCALE

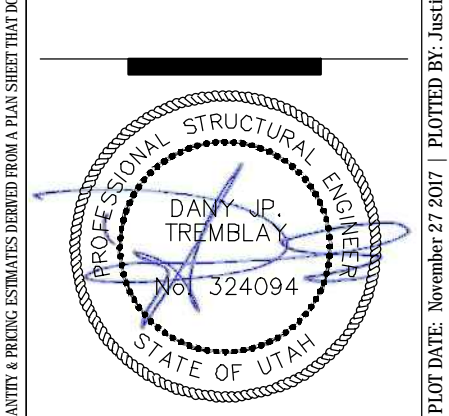
S NOT USED
S-301 NO SCALE

T NOT USED
S-301 NO SCALE

U NOT USED
S-301 NO SCALE

Y NOT USED
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W NOT USED
S-301 NO SCALE



SHEET TITLE
FOOTING & FOUNDATION DETAILS
ISSUE DATE

SEPTEMBER 15, 2017

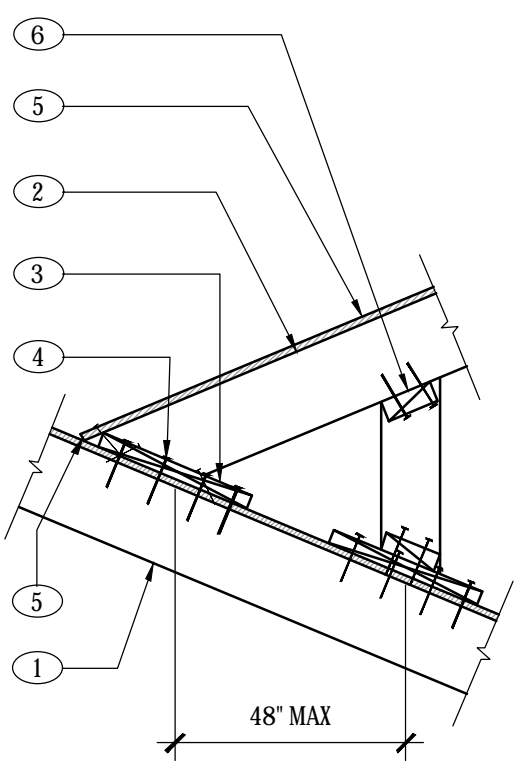
SHEET REVISION DATE
NOV. 27, 2017
SHEET NUMBER

S-301

IMPORTANT NOTICE: THE CITY OF EDEN HAS REVIEWED THESE PLANS AND APPROVED THEM FOR CONSTRUCTION. HOWEVER, THE CITY OF EDEN IS NOT PROVIDING ANY GUARANTEE OR WARRANTY FOR THE ACCURACY OF THESE PLANS. THE USER OF THESE PLANS SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE CITY OF EDEN AND THE STATE OF UTAH. THE USER OF THESE PLANS SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE CITY OF EDEN AND THE STATE OF UTAH. THE USER OF THESE PLANS SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE CITY OF EDEN AND THE STATE OF UTAH.

KEYNOTES:

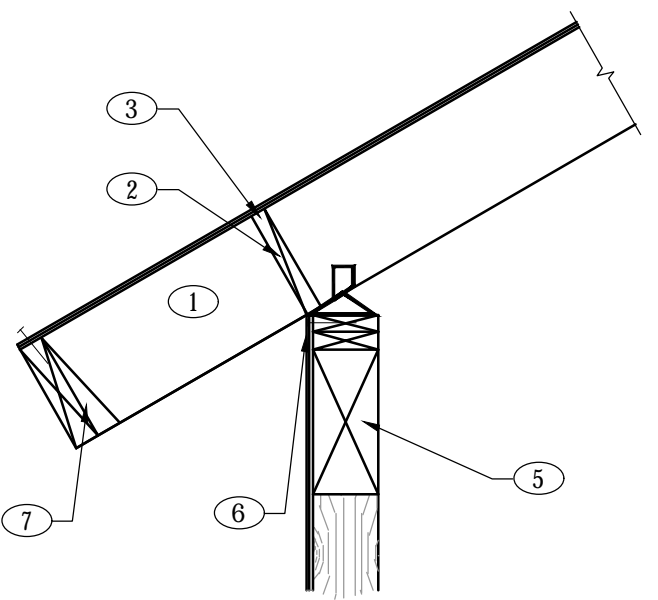
- FRAMING PER PLAN
- 2x12 @ 16" O.C. BRACE @ 48" O.C. ON PONYWALL. PROVIDE 2x8 RIDGE / HIP NAILER.
- 2x12 VALLEY SLEEPER w/ (4) 16d PER RAFTER
- (3) 16d TOENAIL & (1) 16d FACENAIL FROM JACK RAFTER @ VALLEY SLEEPER TYPICAL
- SHEATHING PER NOTES w/ 8d @ 6" O.C. EDGES.
- 2x4 PONY WALL PERPENDICULAR TO STRUCTURAL ROOF FRAMING w/ 2x10 NAILER @ 36" O.C.



A OVERBUILD PARALLEL TO ROOF FRAMING
S-400 NO SCALE

KEYNOTES:

- ROOF RAFTER w/ SIMPSON VPA CLIP @ EA. END. TAPER RAFTER PER ARCH.
- CONTINUOUS 2x BLOCKING BETWEEN ROOF FRAMING
- NAIL SHEATHING TO BLOCKING & FRAMING PER NOTES
- CONNECT BLOCKING TO TOP PLATE OR BEAM w/ SIMPSON A35 @ SPACING PER SW SCHEDULE.
- BEAM/HDR. MAY OCCUR. SEE PLAN. FOR STEEL BEAM. WOOD NAILER PER L/S400 REQUIRED.
- WOOD SHEARWALL MAY OCCUR BELOW. EXTEND SHEATHING TO TOP PLATE & EDGE NAIL PER SW SCHED.
- CONNECT RAFTERS TO FASCIA BEAMS w/ TOP MOUNT HANGERS.

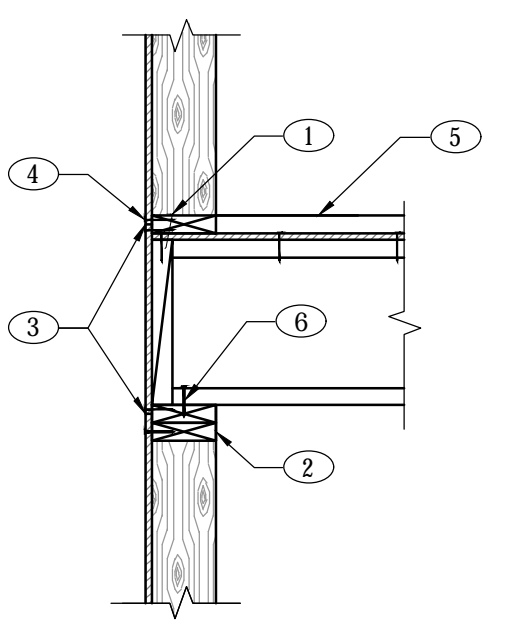


SIDENOTES:
A. SEE WALL SECTION FOR ADDITIONAL INFO.

B NOT USED
S-400 NO SCALE

KEYNOTES:

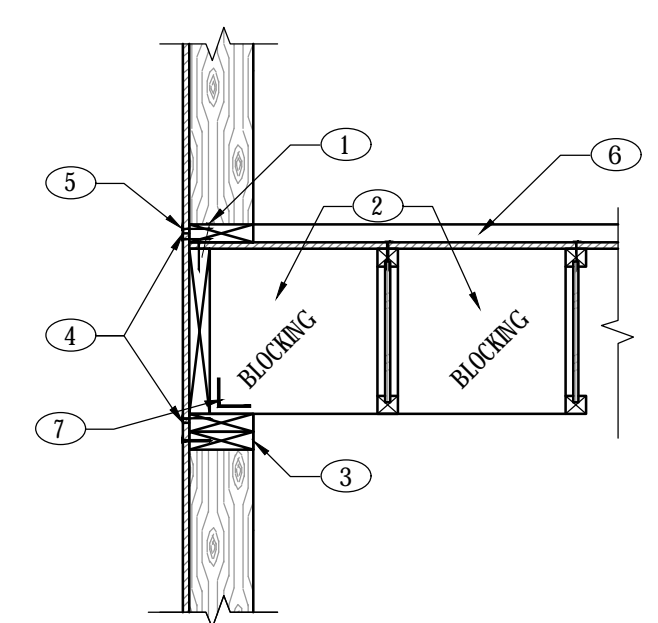
- 16d AT 6" O.C.
- DBL. TOP PLATE 4'-0" MIN. LENGTH OF SPLICE & USE 16d NAILS EACH SIDE OF SPLICE
- LAP SHEATHING ON WALL PLATES OR ACROSS RM BOARD
- SEE FRAMING NOTES FOR SHEATHING & NAILING
- FLOOR TOPPING PER ARCH.
- TOENAIL JOIST TO TOP PLATE PER GSN. SIMPSON A35 BLOCK TO TOP PLATE PER SW SCHEDULE, A/S5.0



C FLOOR BEARING AT EXTERIOR WALL
S-400 NO SCALE

KEYNOTES:

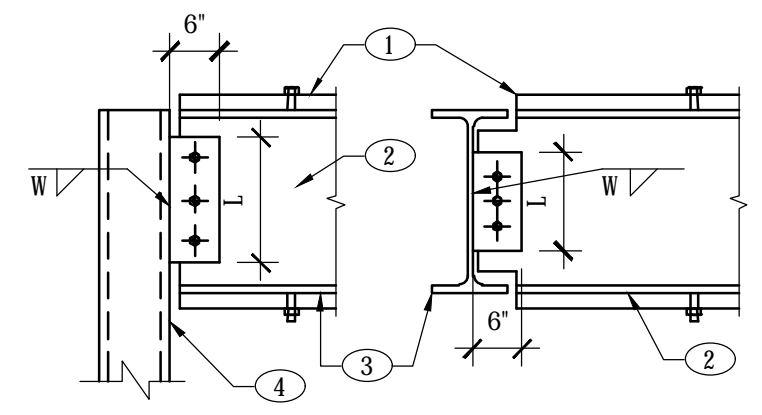
- 16d AT 6" O.C.
- BLOCKING (2) JOIST SPACES DEEP AT 24" O.C.
- DBL. TOP PLATE 4'-0" MIN. LENGTH OF SPLICE & USE 16d NAILS EACH SIDE OF SPLICE
- LAP SHEATHING ON WALL PLATES OR ACROSS RM BOARD
- SEE FRAMING NOTES FOR SHEATHING & NAILING
- FLOOR TOPPING PER ARCH.
- SIMPSON A35 RM BOARD TO TOP PLATE PER SW SCHEDULE, A/S5.0



D FLOOR PARALLEL TO EXTERIOR WALL
S-400 NO SCALE

KEYNOTES:

- 2x PLATE TOP & BOTTOM w/ 12" Ø STUDS @ 24" O.C. STAGGERED.
- BEAM (A). SEE SCHEDULE.
- BEAM PER PLAN.
- COLUMN PER PLAN.



KNIFE SCHEDULE - BEAM TO BEAM OR BEAM TO COLUMN

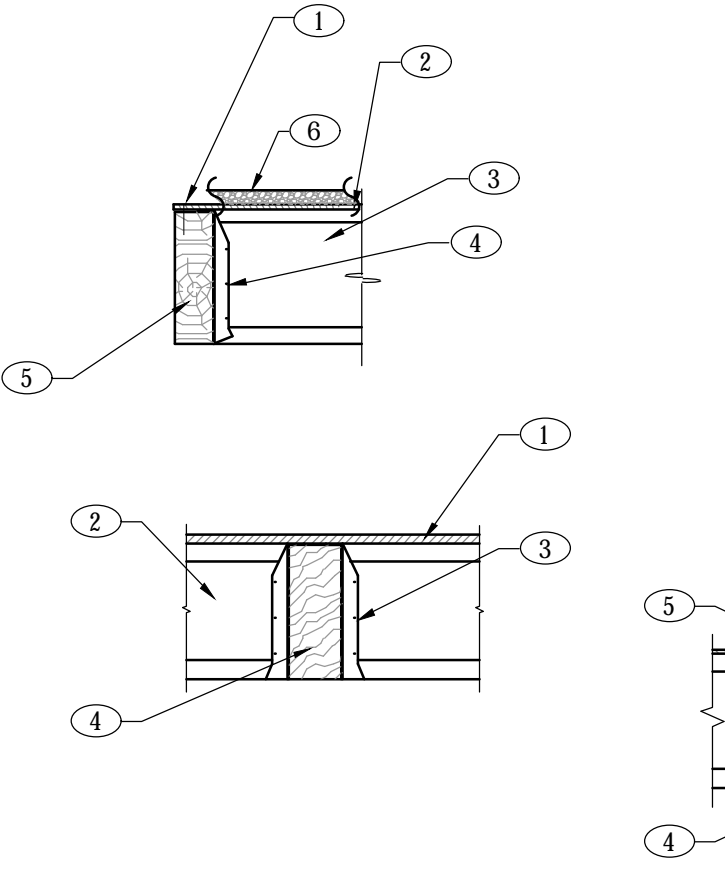
BEAM (A)	L	T	# BOLTS	BOLT ROWS	BOLT Ø	W
W10 OR LESS	8"	3/8"	2	1	3/4"	3/16"
W12 & W14	10"	3/8"	3	1	3/4"	3/16"
W16 & W18	14"	1/2"	4	2	3/4"	1/4"
W21 & W24	18"	1/2"	4	2	3/4"	1/4"
W27 & W30	24"	3/4"	6	2	3/4"	5/16"
W33 & W36	30"	3/4"	8	2	3/4"	5/16"

NOTE: FILLET WELD LENGTH IS FULL 'L' OF KNIFE PLATE & BOLTS SHALL BE A325 BOLTS.

E TYPICAL STEEL-TO-STEEL CONNECTIONS
S-400 NO SCALE

KEYNOTES:

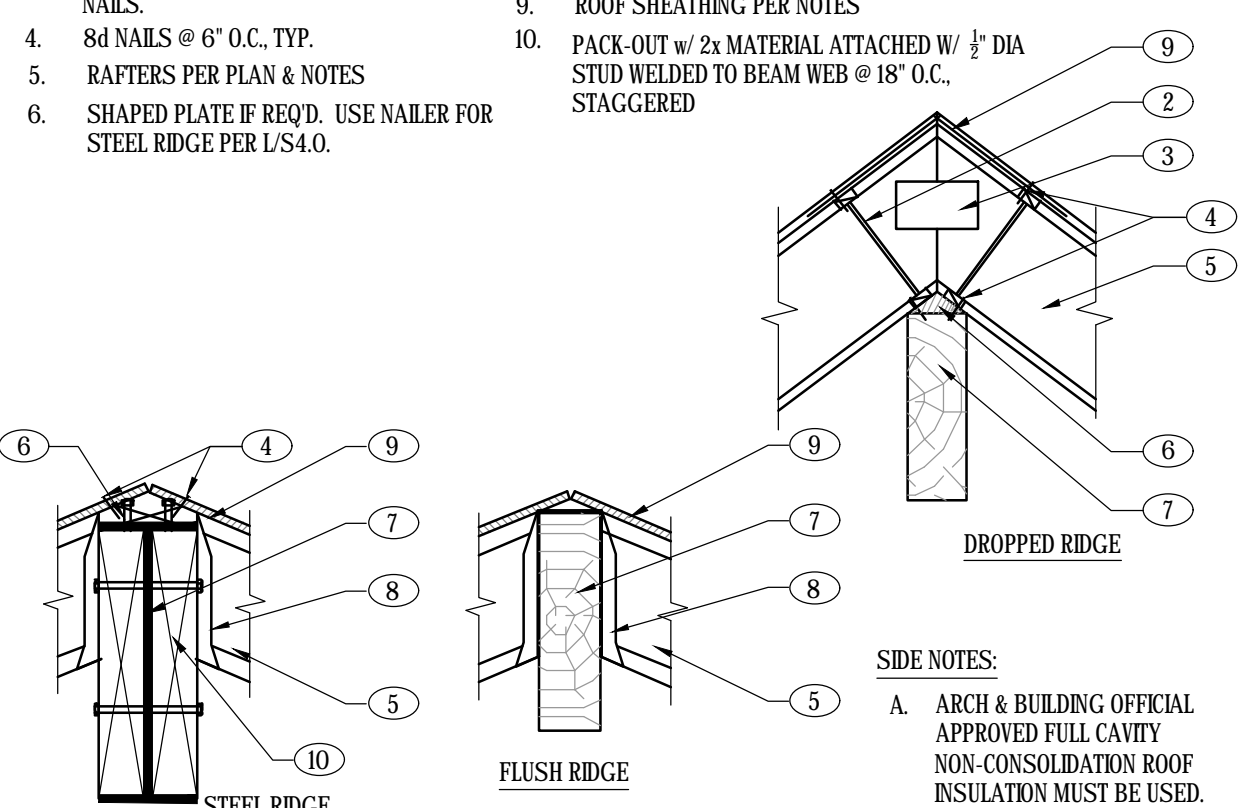
- SHEATHING NAILING PER NOTES.
- ROOF SHEATHING PER NOTES
- RAFTER PER PLAN
- HANGER PER S/S4.0
- FASCIA BEAM PER PLAN
- BALLAST AND ROOFING PER ARCH.



F FLUSH FRAMED BEAMS, INCLUDING FASCIA
S-400 NO SCALE

KEYNOTES:

- NOT USED
- ALTERNATE BLOCK SIDE TO SIDE
- 5'x9'x3/8" PLYWOOD GUSSET w/ (8) 10d NAILS
- 8d NAILS @ 6" O.C. TYP.
- RAFTERS PER PLAN & NOTES
- SHAPED PLATE IF REQ'D. USE NAILER FOR STEEL RIDGE PER L/S4.0.
- BEAM PER PLAN
- TOP MOUNT, SKEWED SIMPSON HANGER TO SUIT RAFTER SIZE. SEE S/S4.0
- ROOF SHEATHING PER NOTES
- PACK-OUT w/ 2x MATERIAL ATTACHED w/ 1/2" DIA STUD WELDED TO BEAM WEB @ 18" O.C. STAGGERED

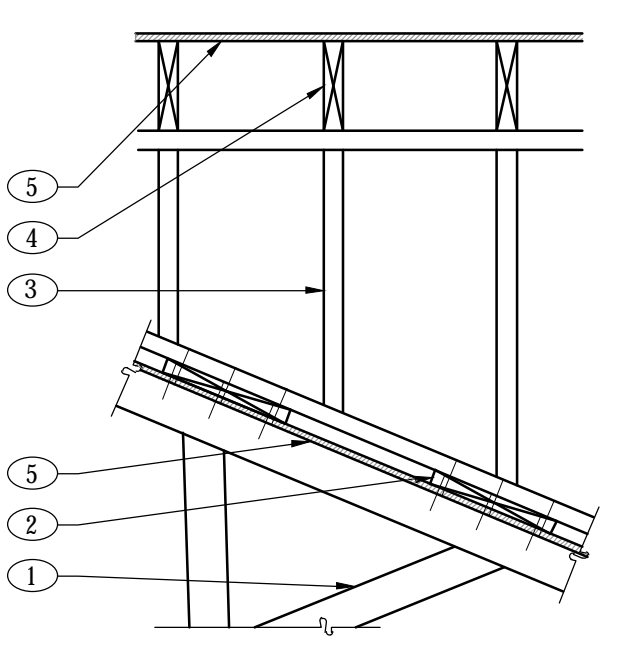


SIDE NOTES:
A. ARCH & BUILDING OFFICIAL APPROVED FULL CAVITY NON-CONSOLIDATION ROOF INSULATION MUST BE USED.

G TYPICAL RIDGE/HIP CONNECTION
S-400 NO SCALE

KEYNOTES:

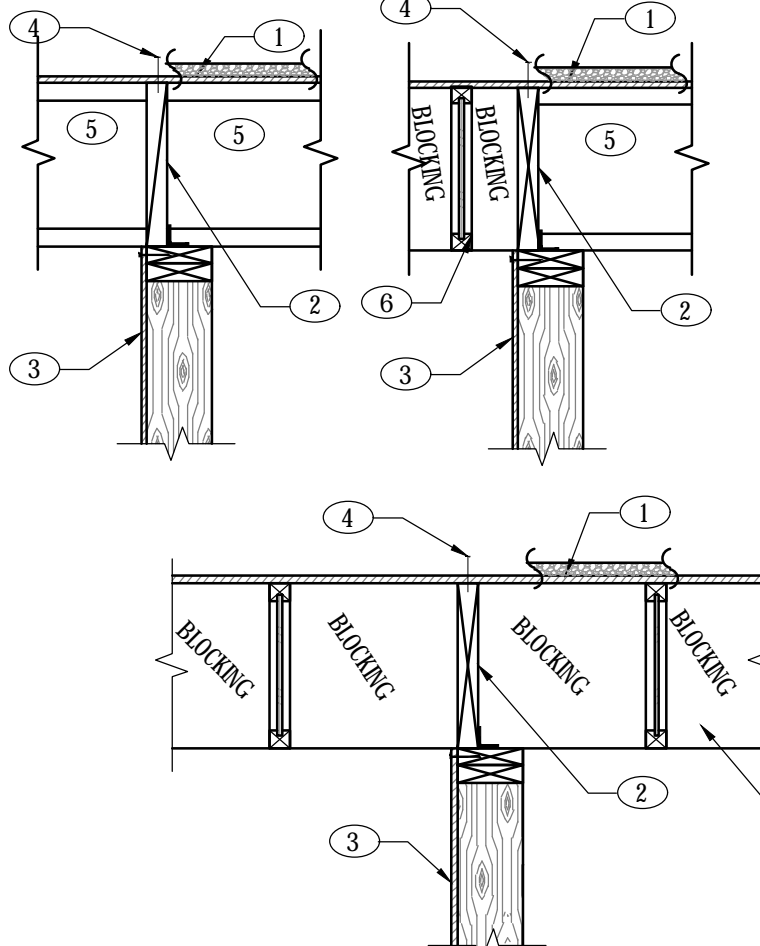
- ROOF TRUSSES OR RAFTERS PER PLAN
- CONTINUOUS 2x10 AT EACH TRUSS WEB MEMBER w/ (3) 16d NAILS.
- 2x4 PONY WALL @ 36" O.C. & MINIMUM OF (1) AT RIDGE LINE.
- OVERBUILD RAFTERS TO BE MINIMUM OF 2x12 @ 24" O.C.
- ROOF SHEATHING PER NOTES w/ 8d @ 6" O.C. EDGES, 12" O.C. FIELD.



H OVERBUILD PERPENDICULAR TO ROOF FRAMING
S-400 NO SCALE

KEYNOTES:

- SEE FRAMING NOTES FOR ROOF SHEATHING & NAILING. SEE ARCH FOR BALLAST & ROOFING.
- LVL SOLID BLOCKING BTWN JOIST. OR CONT. SOLID LVL RM BOARD. USE SIMPSON A34 AT 24" O.C. TO CONNECT TO TOP PLATE BELOW.
- SHEARWALL. SHEATH PER PLAN NOTES & SCHEDULE.
- 16d NAIL AT 4" O.C.
- FRAMING PER PLAN
- SOLID BLOCKING @ 24" O.C. TWO JOIST BAYS ON EITHER SIDE OF SHEARWALLS.

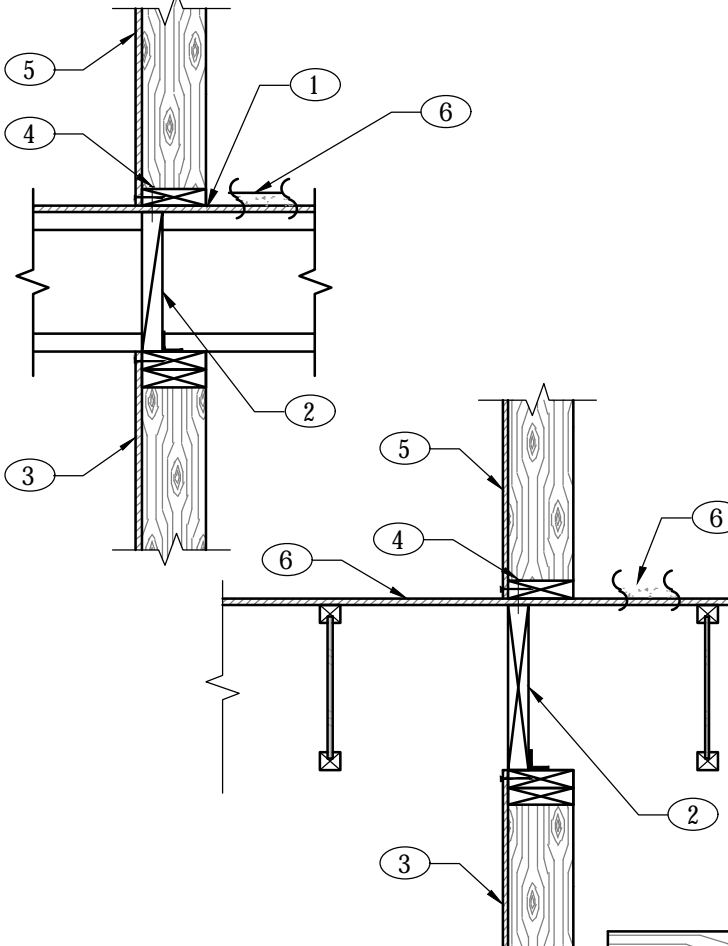


ADDITIONAL NOTES:
A. SHEATHING MAY OCCUR AT BOTH SIDES OF SHEAR WALL PER PLAN

I ROOF FRAMING EXTENDING OVER WOOD SHEARWALL
S-400 NO SCALE

KEYNOTES:

- SEE FRAMING NOTES FOR FLOOR SHEATHING & NAILING.
- 2x SOLID BLOCKING BTWN JOIST. USE SIMPSON A34 AT 32" O.C. TO CONNECT BLOCK TO TOP PLATE BELOW.
- SHEARWALL. SHEATH PER PLAN NOTES & SCHEDULE.
- 16d NAIL AT 4" O.C.
- WALL MAY OR MAY NOT OCCUR ABOVE. SEE PLANS.
- 1-1/2" CONCRETE TOPPING FOR RADIANT HEAT. NORMAL WEIGHT CONCRETE ACCOUNTED FOR.

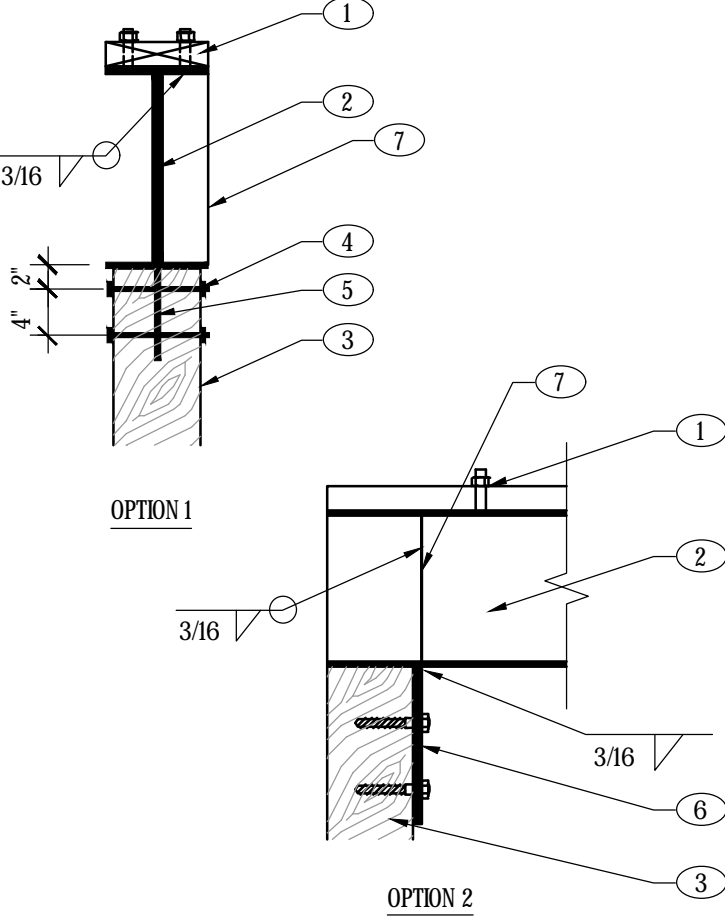


KEYNOTES:
1. BEAM
2. (5) 1/2" DIA. THRU-BOLTS
3. SIMPSON PC OR EPC POST CAP
4. WOOD COLUMN
5. 1/2" THICK STEEL BUCKET w/ 3/8" DIA THRU-BOLTS AS SHOWN
6. STEEL COLUMN PER PLANS.

J FLOOR BEARING ON BOTH SIDES OF WALL
S-400 NO SCALE

KEYNOTES:

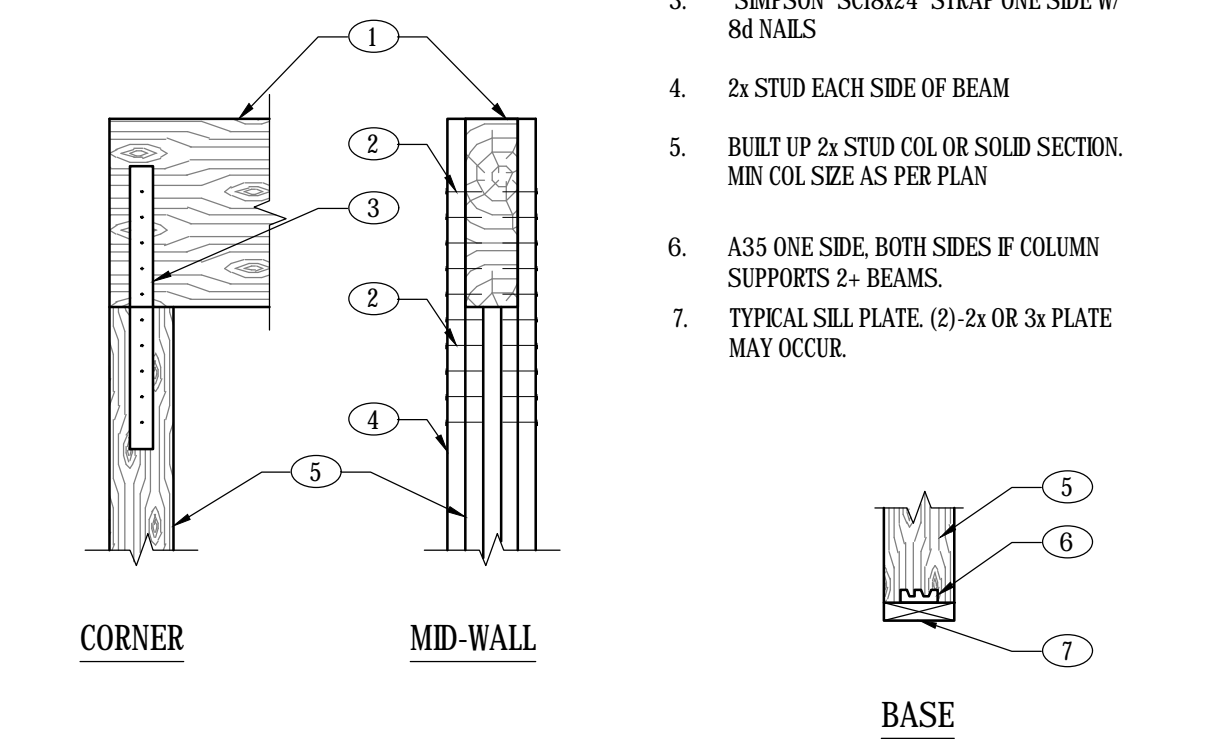
- 2x PLATE TOP & BOTTOM w/ 1/2" STUD @ 24" O.C. STAGGERED.
- STEEL BEAM PER PLAN
- COL PER PLAN
- (2) 1/2" BOLTS
- 5" Wx8" (Hx1/2") STEEL PLATE WELDED TO US OF BEAM
- 4"x10"x1/2" PLATE w/ (2) 1/2" DIA. x 4" LAG BOLTS
- 1/2" WEB STIFFENER REQUIRED AT BEARING. ONE SIDE ONLY.



K BEAM TO BEAM CONX
S-400 NO SCALE

KEYNOTES:

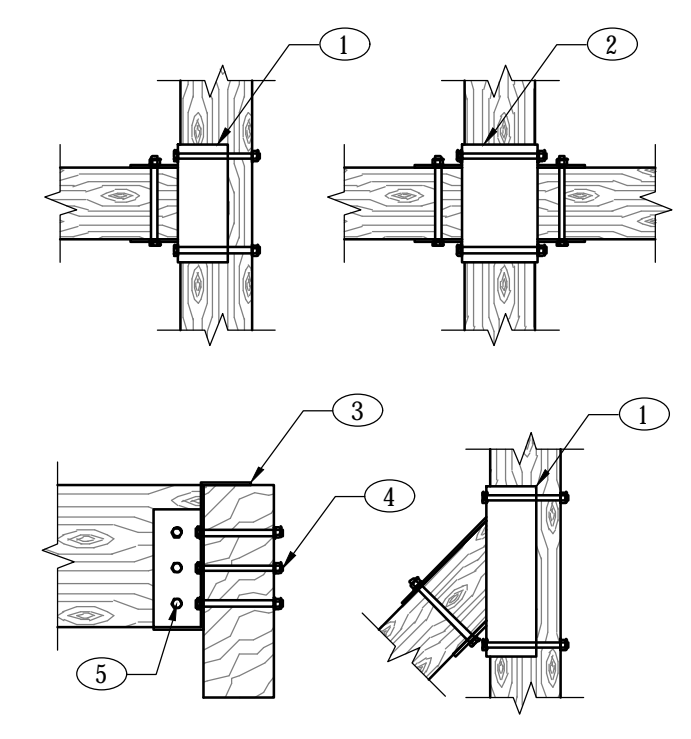
- BEAM AS PER PLANS
- (5) 16d NAILS EACH SIDE
- "SIMPSON" SC18x24" STRAP ONE SIDE w/ 8d NAILS
- 2x STUD EACH SIDE OF BEAM
- BUILT UP 2x STUD COL OR SOLID SECTION. MIN COL SIZE AS PER PLAN
- A35 ONE SIDE, BOTH SIDES IF COLUMN SUPPORTS 2+ BEAMS.
- TYPICAL SILL PLATE. (2)-2x OR 3x PLATE MAY OCCUR.



L CORNER, MID-WALL, BASE
S-400 NO SCALE

KEYNOTES:

- 4" MIN TOP FLANGE
- SADDLE OVER SPORTING BEAM
- 1/4" STEEL BUCKET TOP MOUNT
- (6) 5/8" THRU BOLT
- (3) 5/8" THRU BOLT

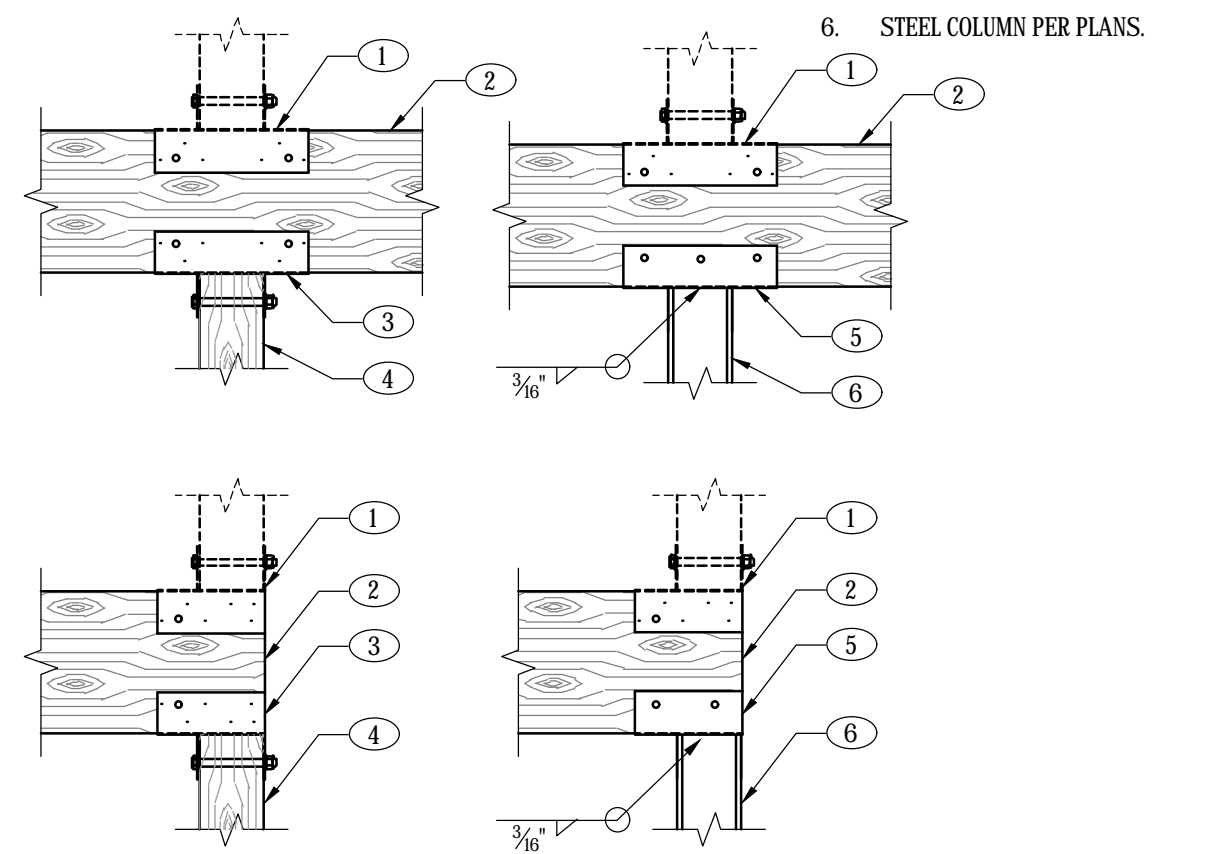


NOTE: IF SIMPSON TOP-MOUNT HANGERS CAN BE USED AT THIS CONNECTION, THEN USE HANGER TO SUIT JOIST SIZE

M STANDARD WOOD BM/COL CONX
S-400 NO SCALE

KEYNOTES:

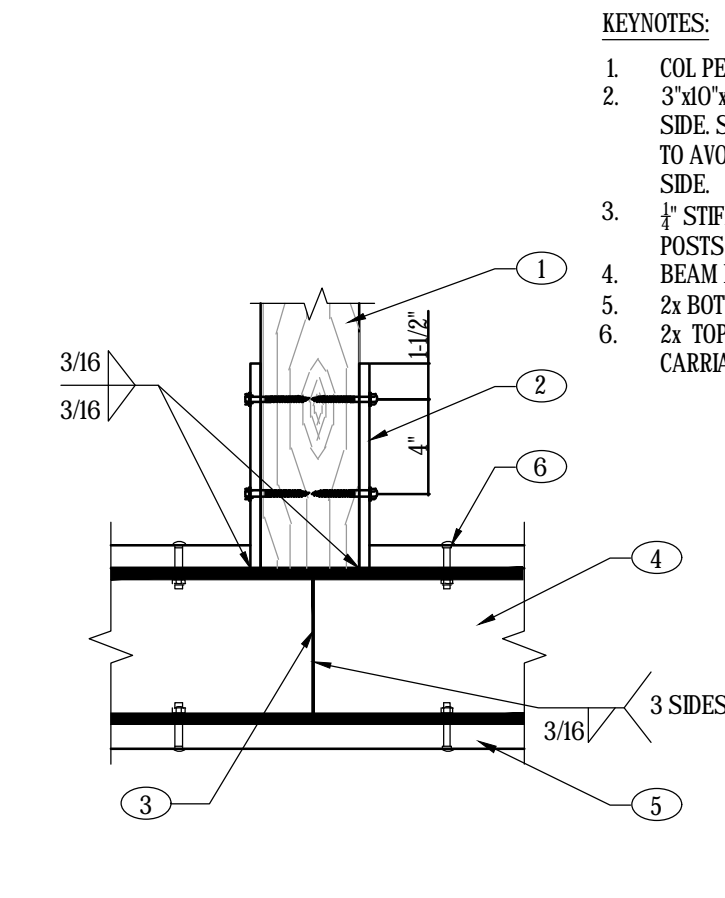
- REVERSED SIMPSON PC OR EPC POST CAP IF COL ABV.
- WOOD BEAM
- SIMPSON PC OR EPC POST CAP
- WOOD COLUMN
- 1/2" THICK STEEL BUCKET w/ 3/8" DIA THRU-BOLTS AS SHOWN
- STEEL COLUMN PER PLANS.



N TYP. WOOD BEAM TO WOOD BEAM CONNECTIONS
S-400 NO SCALE

KEYNOTES:

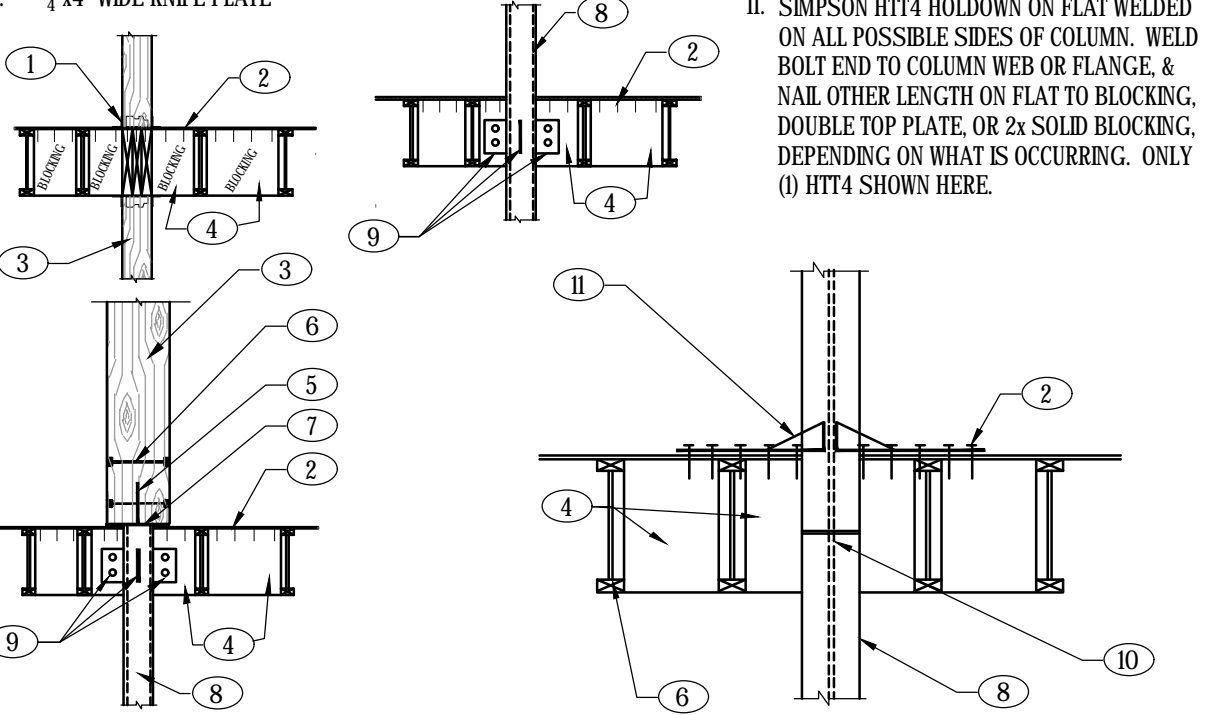
- COL PER PLAN
- 3"x10"x1/2" PL w/ (2) 1/2"x3-1/2" LAG SCREWS EA SIDE. STAGGER SCREW HOLES AS NEEDED TO AVOID CONTACT WITH SCREW ON OTHER SIDE
- 1/2" STIFFENER (ONLY REQUIRED FOR SOLID POSTS OR POSTS GREATER THAN 3 PLYS) BEAM PER PLAN
- 2x BOTTOM PLATE AS REQUIRED
- 2x TOP PLATE w/ 1/2" COUNTERSINK CARRIAGE BOLTS @ 24" O.C. STAGGERED
- (2) 1/2" BOLT PER COL END. COUNTER-SINK OPTIONAL
- 3/8" THICK CAP/BASEPLATE
- STEEL COLUMN PER PLAN (MAY BE CONTINUOUS THROUGH FLOOR)
- WOOD COLUMN PER PLAN
- (2) 2x BLOCK IN 4 DIRECTIONS EXTENDING MIN. 32" FROM EA. OPEN COLUMN FACE. (NOT REQ'D WHERE COLUMN IS ADJACENT FRAMED WALL)
- 6" TALL 1/2" KNIFE PL w/ (2) 1/2" THRU-BOLT ON EA. COL FACE
- 1/2" WEB STIFFENERS BOTH SIDES OF WEB AT MID-DEPTH OF FLOOR
- SIMPSON HIT4 HOLDDOWN ON FLAT WELDED ON ALL POSSIBLE SIDES OF COLUMN. WELD BOLT END TO COLUMN WEB OR FLANGE, & NAIL OTHER LENGTH ON FLAT TO BLOCKING, DOUBLE TOP PLATE, OR 2x SOLID BLOCKING, DEPENDING ON WHAT IS OCCURRING. ONLY (1) HIT4 SHOWN HERE.



O STEEL BEAMS TO WOOD POSTS
S-400 NO SCALE

KEYNOTES:

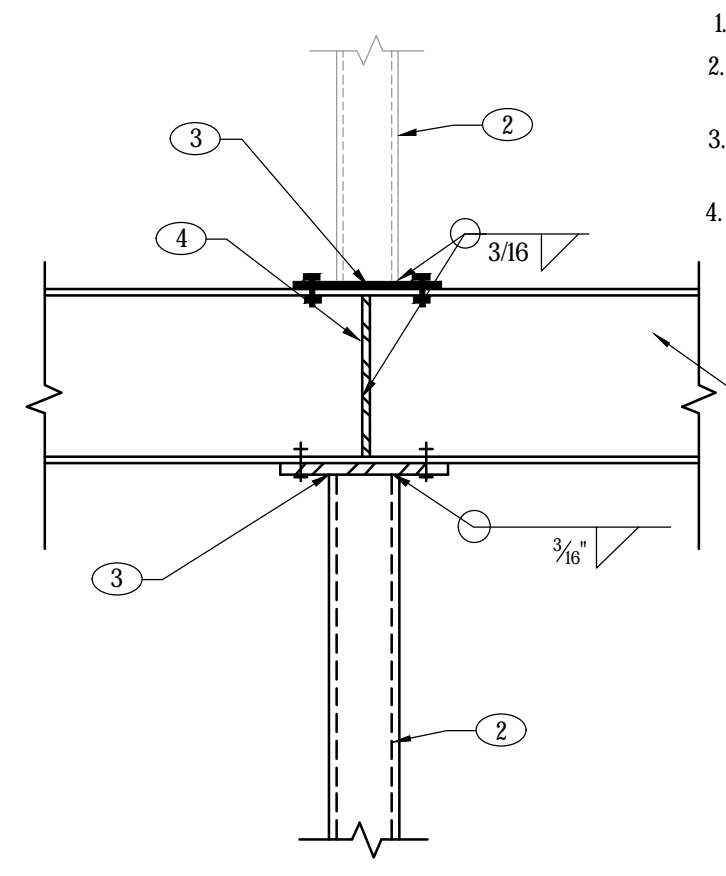
- A34 CLIP ON 4 FACES, TOP & BOTT.
- 16d NAILS @ 6" O.C. ON EA. BLOCK
- WOOD COLUMN PER PLAN
- (2) 2x BLOCK IN 4 DIRECTIONS EXTENDING MIN. 32" FROM EA. OPEN COLUMN FACE. (NOT REQ'D WHERE COLUMN IS ADJACENT FRAMED WALL)
- 1/4" WIDE KNIFE PLATE
- (2) 1/2" BOLT PER COL END. COUNTER-SINK OPTIONAL
- 3/8" THICK CAP/BASEPLATE
- STEEL COLUMN PER PLAN (MAY BE CONTINUOUS THROUGH FLOOR)
- WOOD COLUMN PER PLAN
- (2) 2x BLOCK IN 4 DIRECTIONS EXTENDING MIN. 32" FROM EA. OPEN COLUMN FACE. (NOT REQ'D WHERE COLUMN IS ADJACENT FRAMED WALL)
- 6" TALL 1/2" KNIFE PL w/ (2) 1/2" THRU-BOLT ON EA. COL FACE
- 1/2" WEB STIFFENERS BOTH SIDES OF WEB AT MID-DEPTH OF FLOOR
- SIMPSON HIT4 HOLDDOWN ON FLAT WELDED ON ALL POSSIBLE SIDES OF COLUMN. WELD BOLT END TO COLUMN WEB OR FLANGE, & NAIL OTHER LENGTH ON FLAT TO BLOCKING, DOUBLE TOP PLATE, OR 2x SOLID BLOCKING, DEPENDING ON WHAT IS OCCURRING. ONLY (1) HIT4 SHOWN HERE.



P TYP. WOOD BEAM TO WOOD BEAM CONNECTIONS
S-400 NO SCALE

KEYNOTES:

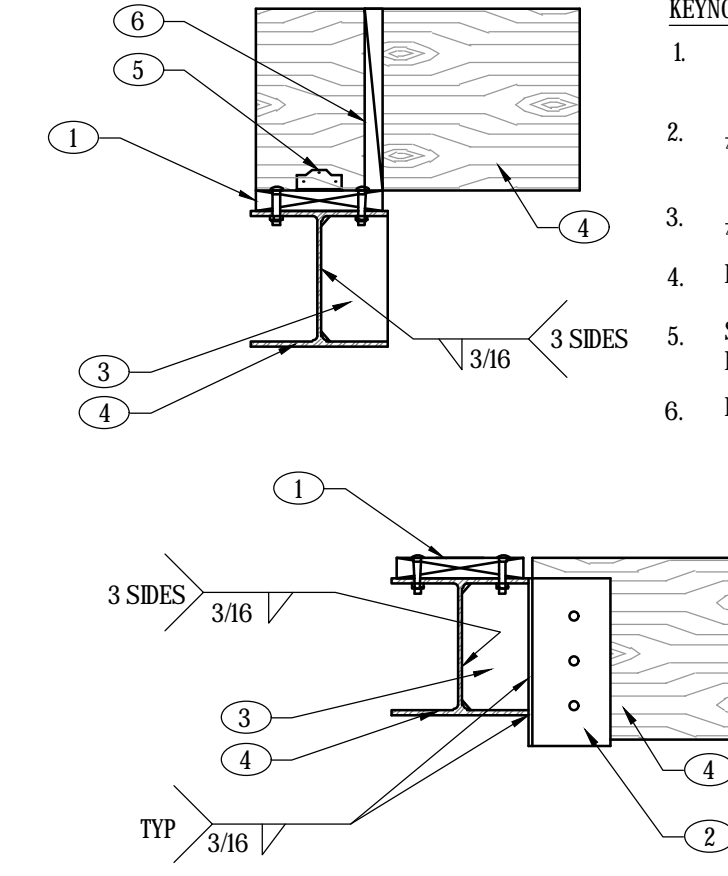
- STEEL BEAM PER PLAN.
- STEEL COLUMN PER PLAN (MAY NOT OCCUR ABV)
- 8"x4"x3/8" STEEL BASEPLATE w/ (4) 3/8" THRU-BOLTS.
- 1/2" WEB STIFFENERS BOTH SIDES



Q BM OR COL TRANSFER CONX - CONCEALED
S-400 NO SCALE

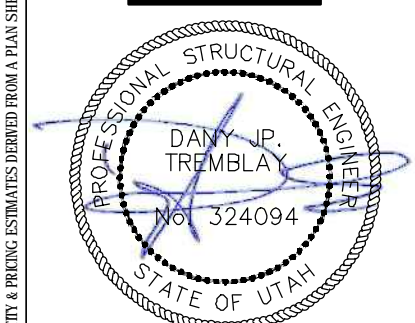
KEYNOTES:

- 2x TOP NAILER w/ 1/2" DIA. COUNTERSINK CARRIAGE BOLT @ 24" O.C. STAGGERED
- 1/2" PLATE BUCKET w/ (3) 1/2" DIA. THRU-BOLTS
- 1/2" STIFFENER ONE SIDE
- BEAMS PER PLAN
- SIMPSON A34 BOTH SIDES OF WOOD BEAM.
- BLOCKING PER FRAMING REQUIREMENTS

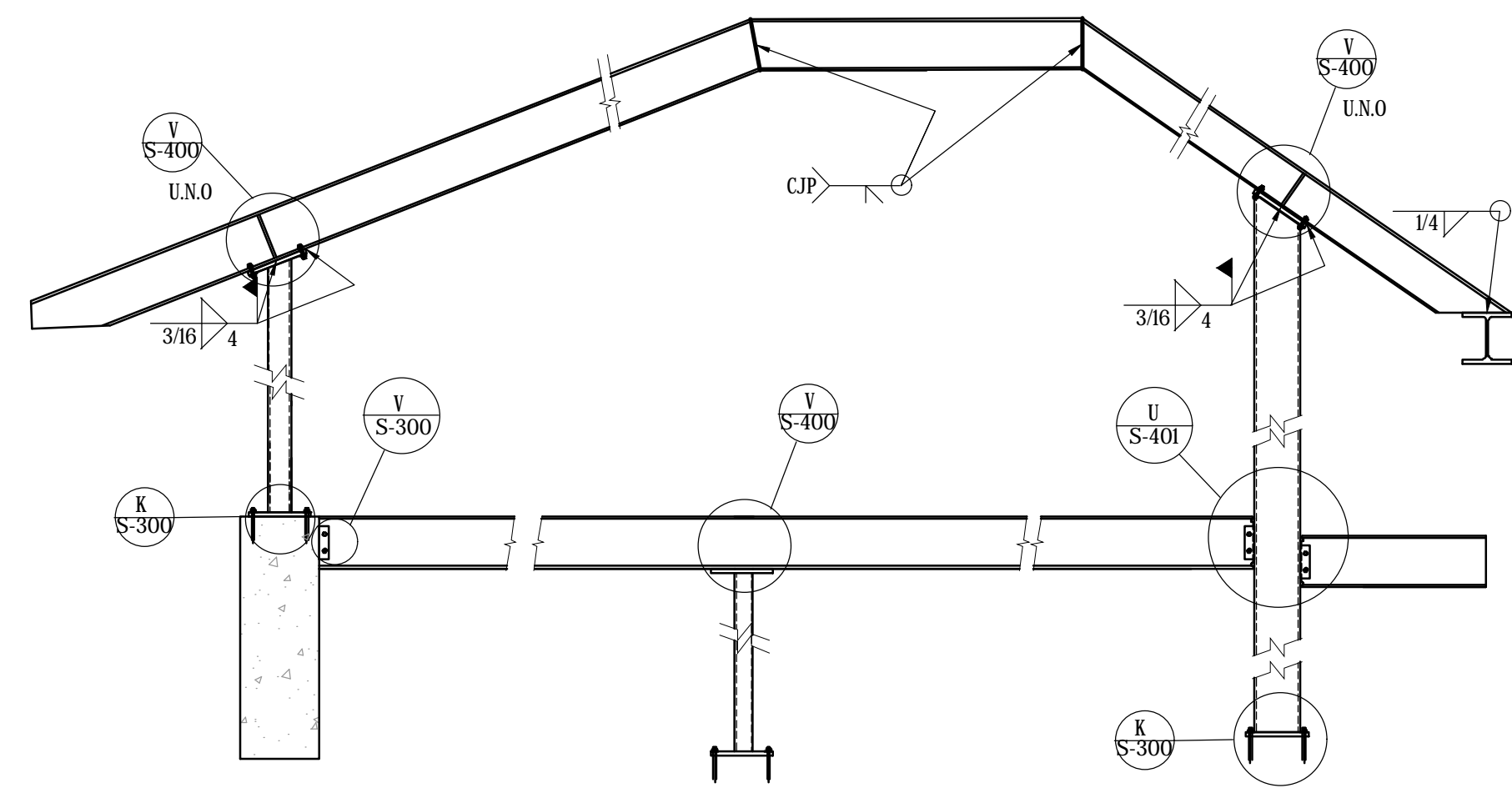


R WOOD BEAM TO STL BEAM
S-400 NO SCALE

BROWNING SKI LODGE
7977 HEARTWOOD DRIVE
EDEN, UTAH
WEBER COUNTY



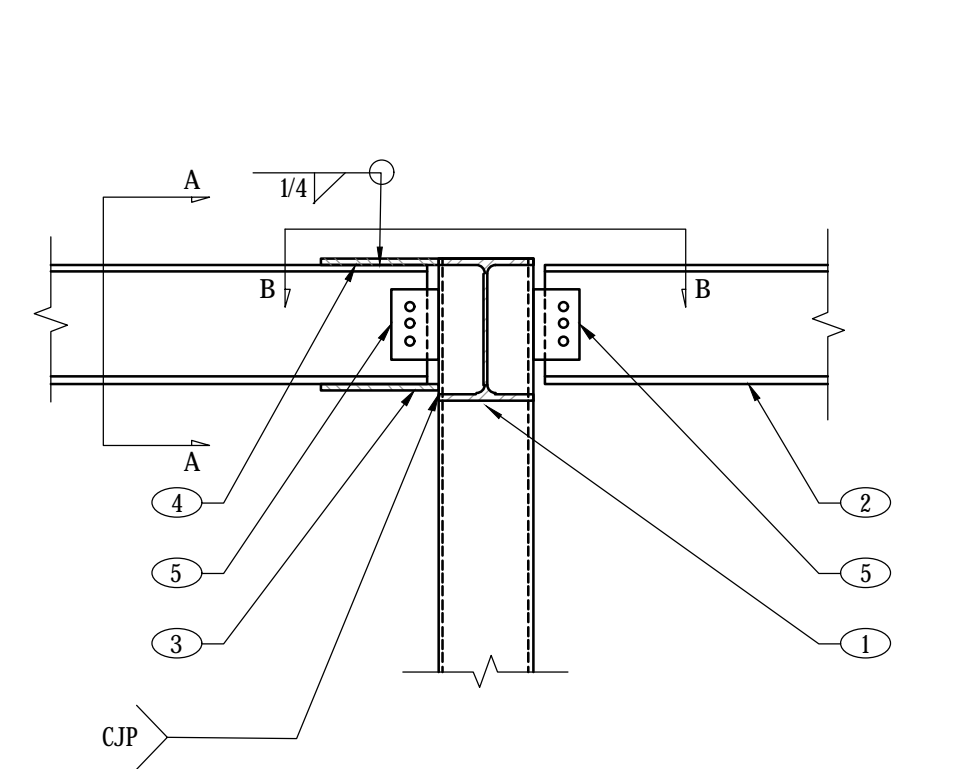
IMPORTANT NOTICE: THE CONTENTS OF THESE DRAWINGS ARE THE PROPERTY OF CANYONS STRUCTURAL CONSULTING. NO PART OF THESE DRAWINGS ARE TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM, WITHOUT THE WRITTEN CONSENT OF CANYONS STRUCTURAL CONSULTING.



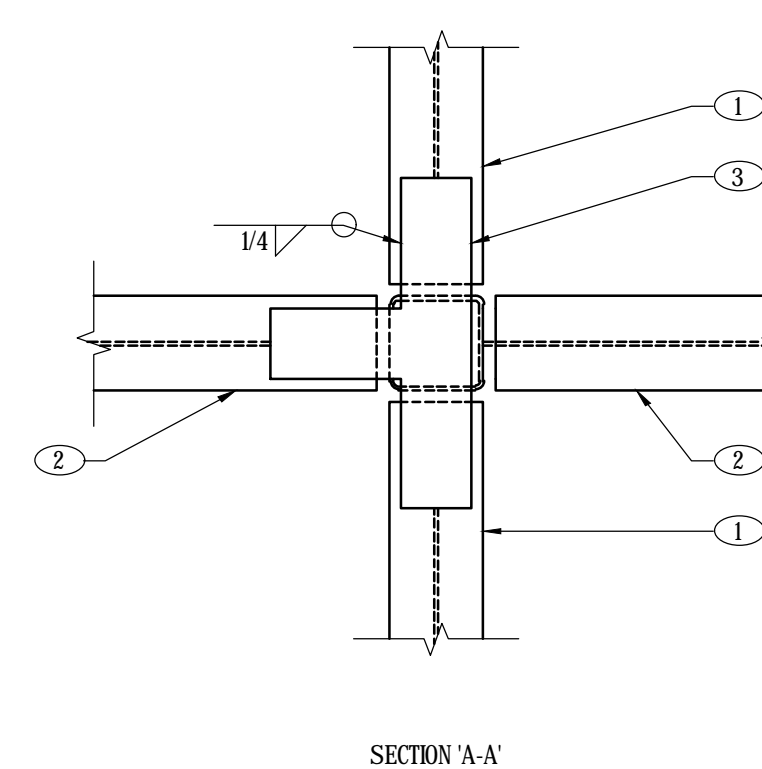
A TYP. CRANKED FRAME PROFILE
S-401 NO SCALE

- KEYNOTES:**
1. CRANKED STEEL ROOF BEAM PER PLAN.
 2. STEEL BEAM PER PLAN.
 3. 1/2" STEEL PLATE.
 4. 1/2" STEEL PLATE.
 5. TYP. STEEL-TO-STEEL CONNECTION.

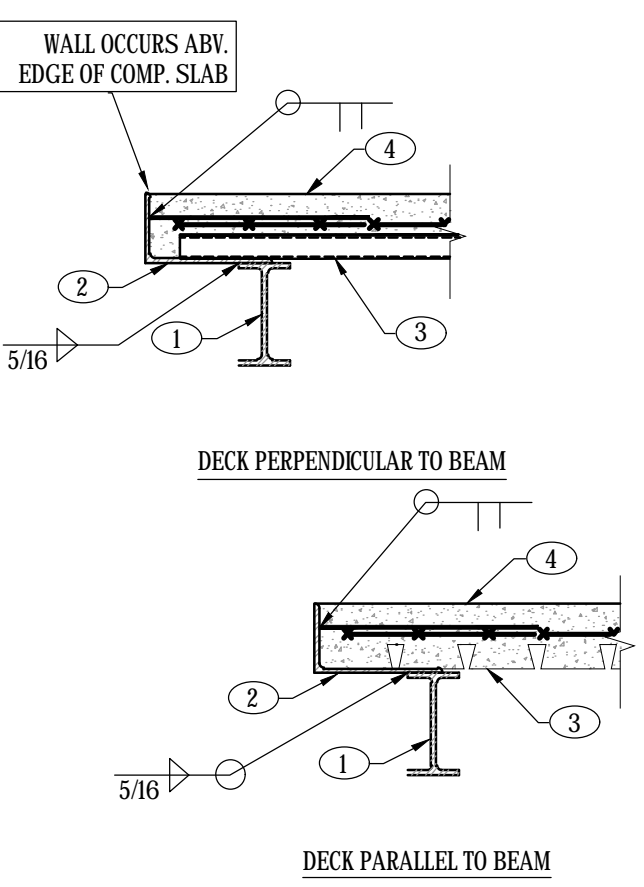
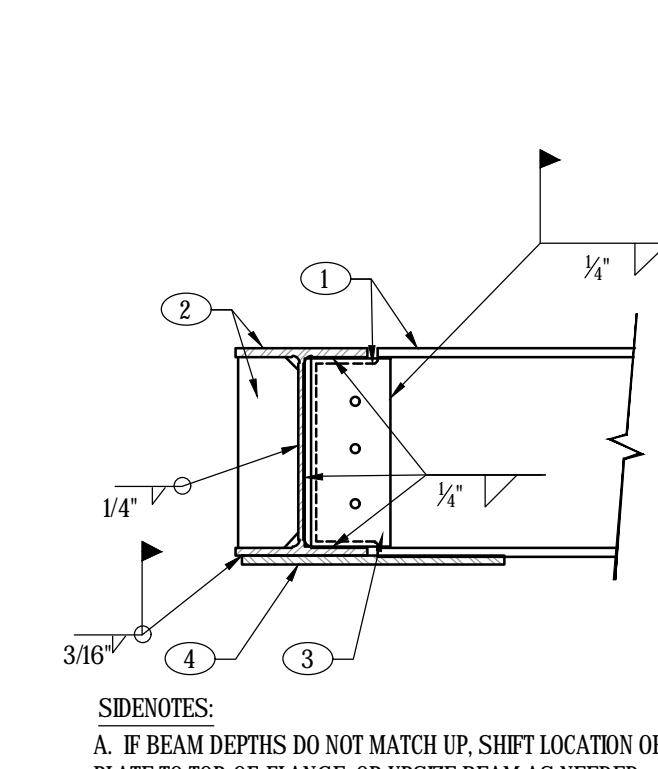
NOTE:
FULL PEN WELDS REQUIRED AT ALL HSS TENSION-COMPRESSION PLATES CONNECTIONS.



B MOMENT CONNECTION
S-401 NO SCALE



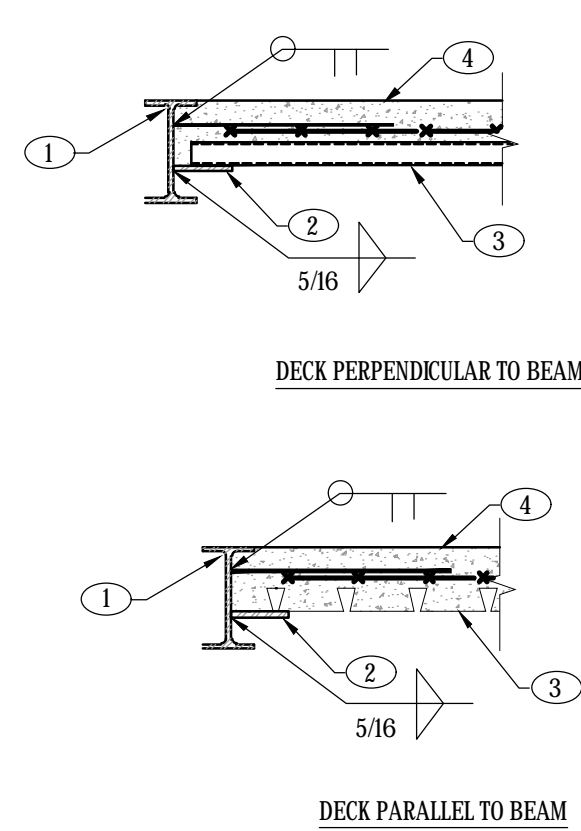
E RIGID STEEL-TO-STEEL CONNECTION
S-401 NO SCALE



F COMPOSITE DECK TO STEEL BEAM
S-401 NO SCALE

- KEYNOTES:**
1. STEEL BEAM PER PLAN, DROPPED.
 2. 1/2" CONTINUOUS BENT PLATE W/ #4 DBA @ 18" O.C. EXTENDING 24" INTO SLAB. BENT PLATE LEG LENGTHS TO ACCOMMODATE ARCH DESIGN & METAL DECK WELDING AS SHOWN.
 3. COMPOSITE METAL DECK & REINFORCEMENT PER NOTES. PUDDLE-WELD REQUIREMENTS PER E/S301 & PLAN NOTES. METAL DECK MAY SLOPE PER ARCH.
 4. SURFACE TREATMENT & WATERPROOFING PER ARCH.

SIDENOTES:
A. CONSTRUCTION SEQUENCING THE RESPONSIBILITY OF CONTRACTOR.

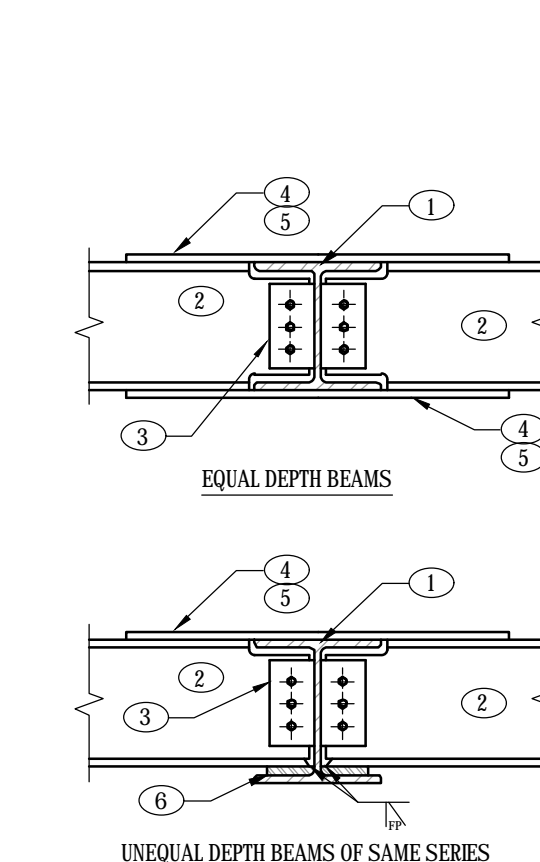
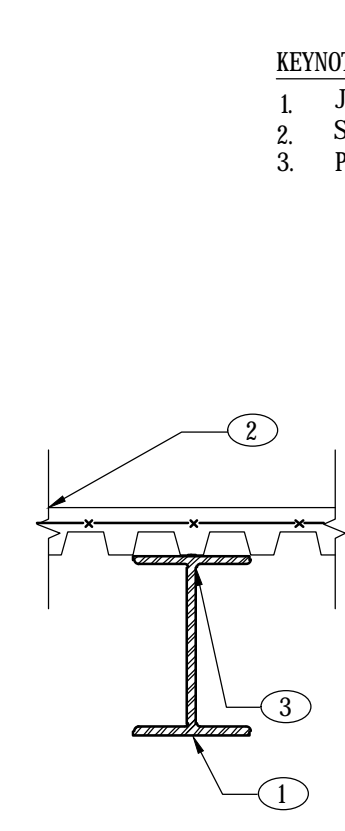


G COMPOSITE DECK TO STEEL BEAM (FLUSH OPTION)
S-401 NO SCALE

- KEYNOTES:**
1. STEEL BEAM PER PLAN, FLUSH, W/ #4 DBA @ 18" O.C. EXTENDING 24" INTO SLAB.
 2. 1/2" CONTINUOUS BEARING PLATE. PLATE WIDTH TO ACCOMMODATE ARCH DESIGN & METAL DECK WELDING AS SHOWN.
 3. COMPOSITE METAL DECK & REINFORCEMENT PER NOTES. PUDDLE-WELD REQUIREMENTS PER E/S301 & PLAN NOTES. METAL DECK MAY SLOPE PER ARCH.
 4. SURFACE TREATMENT & WATERPROOFING PER ARCH.

SEE H/S-401 FOR FLUSH OPTION

H DROPPED STEEL AT METAL DECKING (OPTION)
S-401 NO SCALE

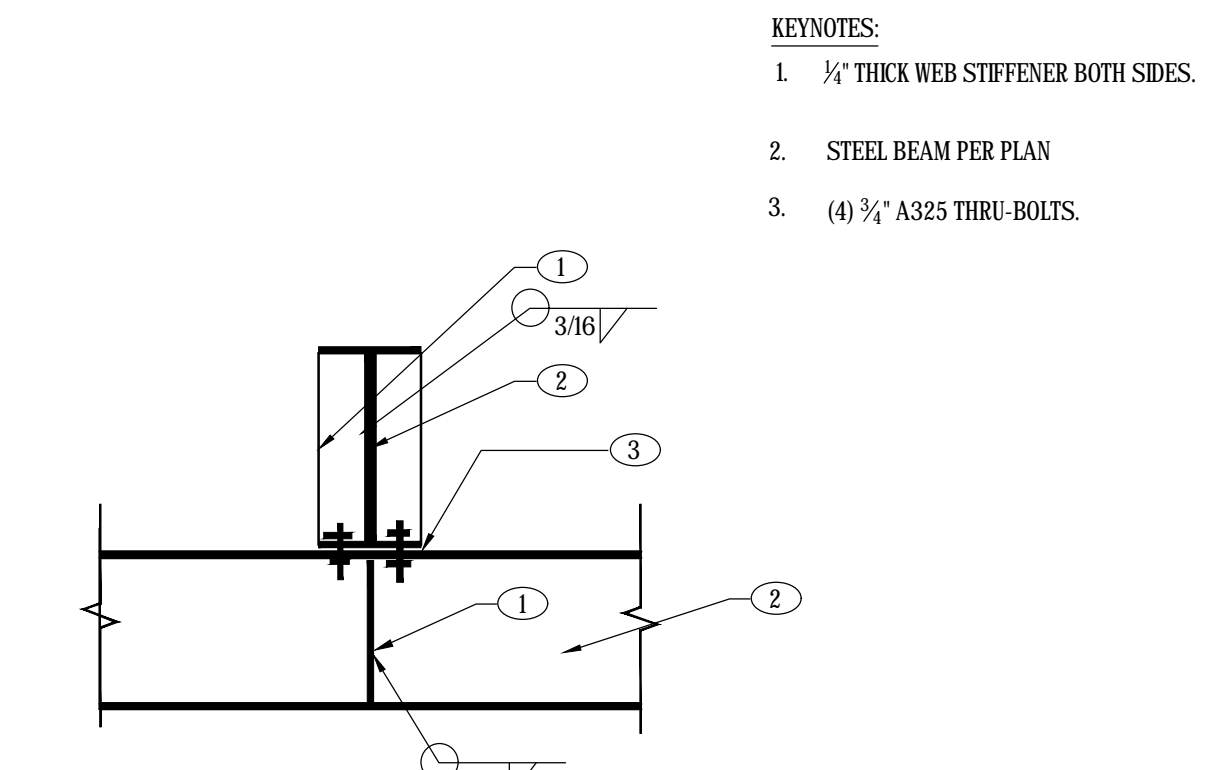


J NOT USED
S-401 NO SCALE

- KEYNOTES:**
1. SUPPORT BEAM PER PLAN
 2. THRU-BEAM PER PLAN
 3. STANDARD GUSSET PLATE BM/BM CONN. PER TYPICAL DETAIL ON E/S4.0
 4. SPLICE PLATE
 - 4.1. LENGTH = 3x THRU-BEAM DEPTH
 - 4.2. WIDTH = THRU-BEAM WIDTH - 1/2"
 - 4.3. THICKNESS = THRU-BEAM FLANGE THICKNESS + 1"
 5. 3/8" FILLET WELD ALL AROUND
 6. BACKER PLATE
 - 6.1. LENGTH = 2' MIN
 - 6.2. WIDTH = THRU-BEAM WIDTH + 1'
 - 6.3. THICKNESS = 1" MAX

IF DROPPED BEAM IS PREFERRED AT LOCATION OF DETAIL CUT, SEE DETAIL K/S-401

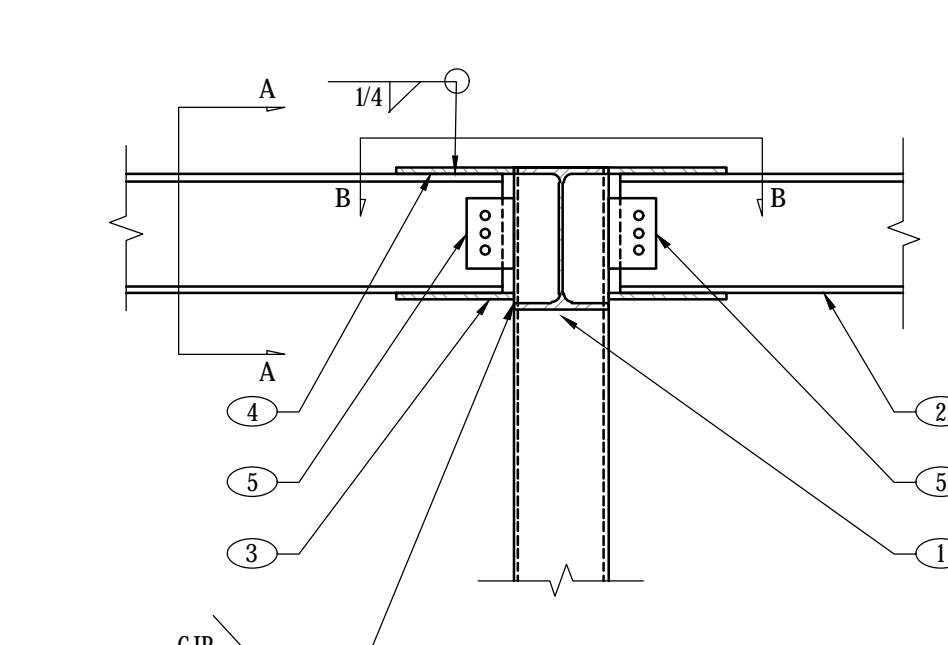
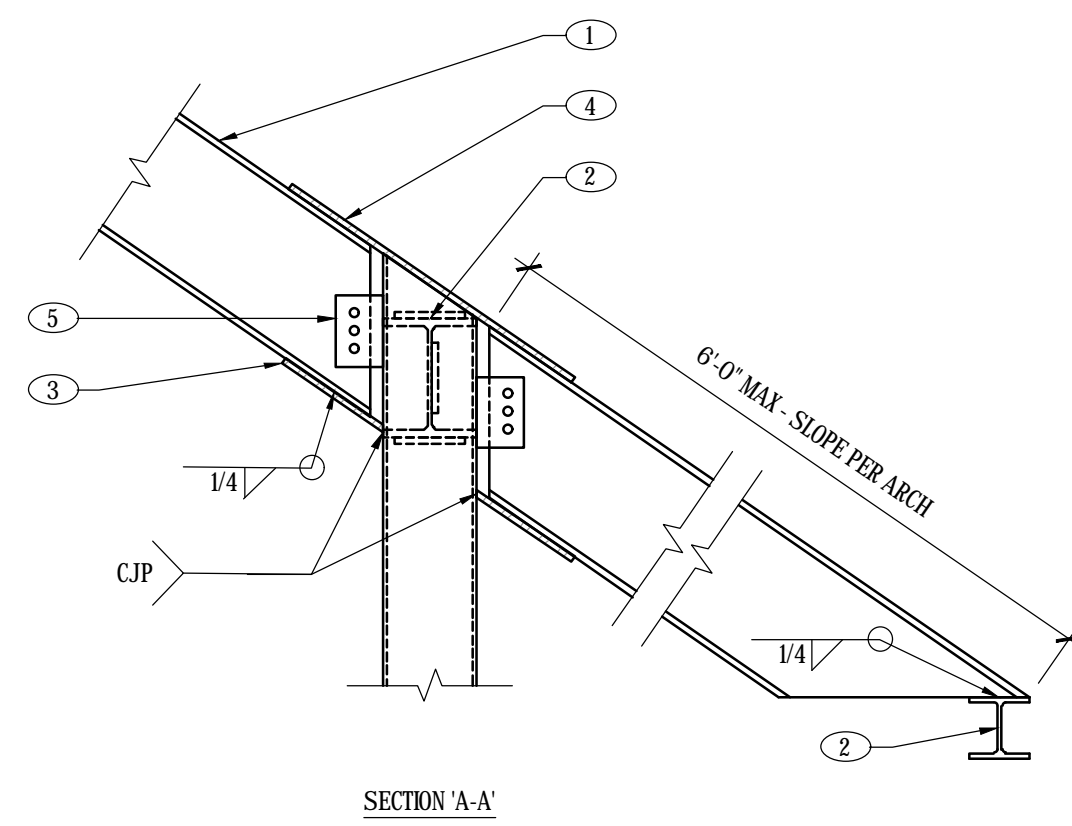
K STEEL BEAM OVER DROPPED STEEL BEAM
S-401 NO SCALE



- KEYNOTES:**
1. 1/2" THICK WEB STIFFENER BOTH SIDES.
 2. STEEL BEAM PER PLAN
 3. (4) 3/4" A325 THRU-BOLTS.

- KEYNOTES:**
1. CRANKED STEEL ROOF BEAM PER PLAN.
 2. STEEL BEAM PER PLAN.
 3. 1/2" STEEL PLATE.
 4. 1/2" STEEL PLATE.
 5. STEEL-TO-STEEL CONNECTION.

NOTE:
FULL PEN WELDS REQUIRED AT ALL HSS TENSION-COMPRESSION PLATES CONNECTIONS.

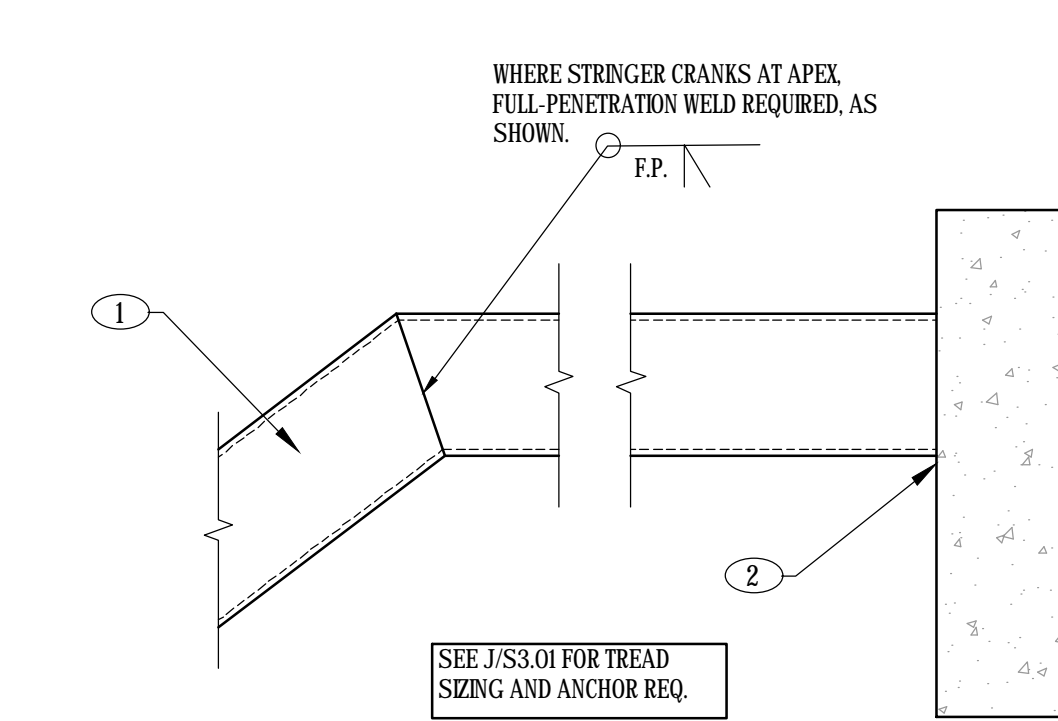


S SPLICED BEAMS WITH FIXED CONNECTION
S-401 NO SCALE

- KEYNOTES:**
1. 4 1/2" x 16" x 1/2" PLATE
 2. CANTILEVERED BEAM PER PLAN
 3. COLUMN PER PLAN.
 4. SEE E/S401 FOR BEAM-TO-COLUMN CONN.
 5. 4 1/2" x 12" x 1/2" PLATE
 6. 3/8" STUD @ 24" O.C. EA. COL. FACE AGAINST STUD WALL.
 7. STL. COL. PER PLAN w/ 3/4" BASE PL. & (4) 3/4" DIA. SIMPSON TITEN HD ANCHORS W/ 8" EMBEDMENT. PLACE BASEPLATE ON 1" NON-SHRINK GROUT.
 8. BEAM PER PLAN

U CANTILEVERED BEAM FIXED TO HSS COLUMN
S-401 NO SCALE

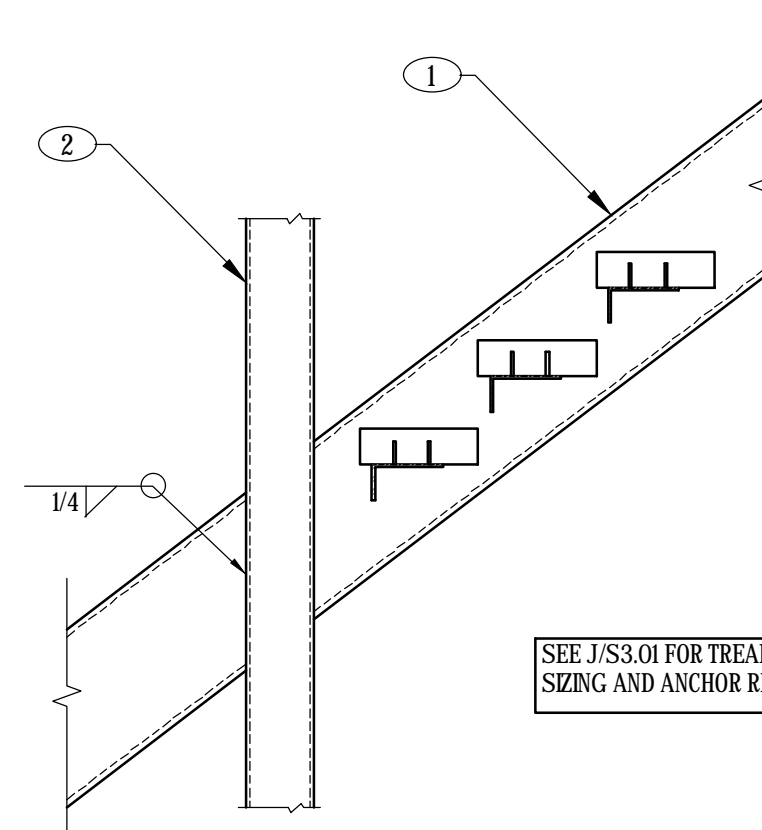
- KEYNOTES:**
1. HSS6x4 1/2" STRINGER
 2. CONNECT TO FOUNDATION WALL PER. VS-300



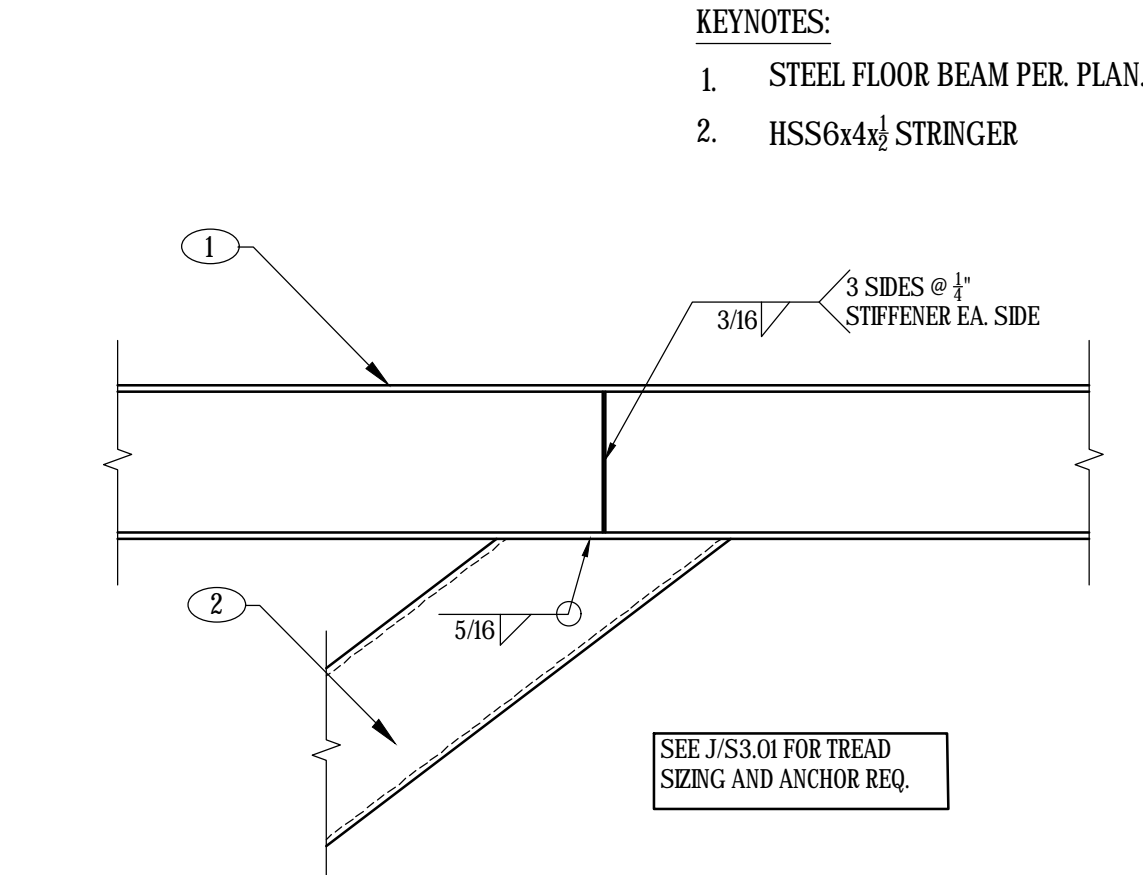
Q CRANKED STEEL STRINGER
S-401 NO SCALE

- KEYNOTES:**
1. PIPE4XS
 2. HSS6x4 1/2" STRINGER

Y PIPE COLUMN AT STAIR STRINGER
S-401 NO SCALE



R HSS STRINGER TO BOTTOM FLANGE FLOOR BEAM
S-401 NO SCALE



- KEYNOTES:**
1. STEEL FLOOR BEAM PER PLAN.
 2. HSS6x4 1/2" STRINGER

