GENERAL STRUCTURAL NOTES

1. IN ALL CASES, "CONTRACTOR" SHALL REFER TO THE CONTRACTOR OR SUB-CONTRACTOR RESPONSIBLE FOR THE TRADE SPECIFICALLY REFERRED TO IN THE NOTES (i.e. STEEL, CONCRETE, MASONRY). THE "CONTRACTOR" SHALL MEET ALL NOTE REQUIREMENTS AND SHALL INCLUDE THE COSTS ASSOCIATED WITH THESE REQUIREMENTS IN HIS/HER BID. THE GENERAL CONTRACTOR, OR CONSTRUCTION MANAGER, IS ULTIMATELY RESPONSIBLE FOR COMPLIANCE WITH ALL NOTE REQUIREMENTS.
2. THE CONTRACTOR SHALL PERFORM HIS/HER TRADE AND DUTIES IN A MANNER CONFORMING TO THE PROCEDURES AND REQUIREMENTS AS STATED IN THE 2015 INTERNATIONAL BUILDING CODE (IBC), AND/OR LATEST CODE ADOPTED BY THE LOCAL BUILDING OFFICIAL, AND ALL LOCAL ORDINANCES.
3. THE GENERAL CONTRACTOR, OR PROJECT MANAGER, SHALL COORDINATE THE WORK PERFORMED BY ALL TRADES.
4. THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND/OR ARCHITECT OF ANY DISCREPANCIES, OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR THE SPECIFICATIONS BEFORE PROCEEDING WITH ANY WORK INVOLVED. IN ALL CASES, UNLESS OTHERWISE DIRECTED, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN AND BE PERFORMED.
5. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS, DIMENSIONS, SLOPES AND ELEVATIONS, ETC., AT THE JOB SITE AND SHALL COORDINATE THESE WITH THE ARCHITECT AND WITH ALL TRADES. CONSTRUCTION DRAWINGS SHALL NOT BE SCALED FOR DIMENSIONS.
6. VISITS TO THE JOB SITE BY REPRESENTATIVES OF THE ENGINEER DO NOT CONSTITUTE APPROVAL OF THE WORK PERFORMED BY THE CONTRACTOR OR HIS SUBCONTRACTORS; THEY ARE MERELY FOR THE PURPOSE OF OBSERVATION.
7. SHOP DRAWINGS FOR ANY FABRICATED COMPONENTS OR COMPONENTS DESIGNED-BY-MANUFACTURER SHALL BE APPROVED BY THE ENGINEER AND ARCHITECT PRIOR TO FABRICATION AND ERECTION. SHOP DRAWINGS SHALL BE STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT.
8. THE CONTRACTOR SHALL VERIFY SIZES, LOCATIONS, LOADS, AND EQUIPMENT ANCHORAGE IN THE FIELD WITH THE EQUIPMENT MANUFACTURER (OR SUPPLIER) PRIOR TO FABRICATION OR INSTALLATION OF SUPPORTING STRUCTURES.
9. TEMPORARY SHORING (BRACING) SHALL BE PROVIDED WHERE NECESSARY. SHORING SHALL SUPPORT ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED (i.e. WIND). SHORING SHALL REMAIN IN PLACE AS LONG AS MAY BE REQUIRED FOR SAFETY OR UNTIL ALL THE STRUCTURAL ELEMENTS ARE COMPLETED. ALL SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR
10. DURING AND AFTER CONSTRUCTION, THE CONTRACTOR AND OWNER SHALL KEEP LOADS ON THE STRUCTURE WITHIN THE LIMITS OF THE DESIGN LOADS FOR THE OCCUPANCY. SEE STRUCTURAL PLANS AND CALCULATIONS FOR STRUCTURAL DESIGN LOADINGS AND CRITERIA.
11. ANY SPECIAL INSPECTION REQUIRED BY THE CONSTRUCTION DOCUMENTS, OR BY THE BUILDING OFFICIAL, OR BY THE IBC, IS THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE ON BEHALF OF THE OWNER.
12. CONTRACTOR SHALL BE RESPONSIBLE FOR SAFETY AND PROTECTION WITHIN AND ADJACENT TO THE JOB SITE.
13. PRIOR APPROVAL, IN WRITING, FROM THE ENGINEER IS REQUIRED FOR ANY DEVIATION FROM THE STRUCTURAL PLANS AND/OR CONSTRUCTION DOCUMENTS. OPTIONAL MEMBER SIZES AND VARIATIONS IN THE FRAMING REQUIRE PRIOR APPROVAL OF THE ENGINEER, ARCHITECT AND OWNER. FAILURE TO FOLLOW PLANS AND CONSTRUCTION DOCUMENTS CONSTITUTES CHANGE IN PROJECT SCOPE.
14. SEE STRUCTURAL PLANS FOR ADDITIONAL STRUCTURAL NOTES AND REQUIREMENTS.
15. THE ENGINEER RESERVES THE RIGHT TO REQUEST REPLACEMENT OF ANY PORTION OF THE STRUCTURE DEVIATING FROM THE PLANS WHERE WRITTEN PRIOR APPROVAL HAS NOT BEEN OBTAINED AND WHERE INSPECTION BY THE ENGINEER PRIOR TO CONSTRUCTION OF THE CHANGED PORTION HAS NOT HAPPENED.
16. ALL SITE WORK, GRADING, COMPACTION AND BACKFILL, ETC. SHALL BE DONE IN COMPLIANCE WITH A GEOTECHNICAL REPORT SPECIFIC TO THE SITE. IT IS THE GENERAL CONTRACTORS RESPONSIBILITY TO OBTAIN A GEOTECHNICAL REPORT, IF ONE HAS NOT ALREADY BEEN OBTAINED, AND SUBMIT A COPY TO THE ENGINEER FOR VERIFICATION.
17. ALL ANCHORING ADHESIVE SHALL BE SIMPSON SET-XP EPOXY OR HILTI HIT-HY200 MAX-SD ADHESIVE. ANCHORS SHALL BE INSTALLED PER MANUFACTURERS INSTRUCTIONS.
18. ALL NON-EPOXIED POST-INSTALLED ANCHORS TO BE SIMPSON STRONG-BOLT 2 WEDGE ANCHORS, TITEN HD SCREW ANCHORS, HILTI KWIK HUS-EZ SCREW ANCHORS, OR HILTI KWIK BOLT TZ ANCHORS.
19. FASTENERS AND ANCHOR BOLTS USED IN PRESERVATIVE-TREATED WOOD SHALL BE HOT DIPPED ZINC-COATED GALVANIZED STEEL. THE COATING WEIGHTS SHALL BE IN ACCORDANCE WITH ASTM A 153.

GENERAL CONCRETE NOTES

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. ALL WORK SHALL BE IN STRICT ACCORDANCE WITH THE 2015 IBC, ACI 318, AND LOCAL ORDINANCES.
3. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS PRIOR TO PLACING CONCRETE.
4. CONTRACTOR SHALL COORDINATE WITH MECHANICAL, ELECTRICAL, AND ARCHITECTURAL PRIOR TO PLACING CONCRETE. PROVIDE SLEEVES, BLOCK OUTS, ETC... AS REQUIRED.
5. CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER PLACEMENT OF ALL ANCHOR BOLTS, SEISMIC ANCHORS OR STRAPS, ETC.. INSTALL PER MANUFACTURER'S SPECIFICATIONS.
6. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL FORM WORK, POUR STOPS, ETC. REQUIRED TO CONSTRUCT ALL CONCRETE WORK. SUCH FORM WORK IS NOT NECESSARILY SHOWN ON THE STRUCTURAL PLANS OR DETAILS. THE CONTRACTOR SHALL SPECIFY ALL FORM WORK AND SHALL INCLUDE THE COST FOR SUCH IN HIS/HER ORIGINAL BID.
7. CONTRACTOR SHALL PROVIDE ALL SHORING AS REQUIRED.
8. FOOTINGS, FOUNDATION AND SLABS ON GRADE SHALL BE CONSTRUCTED ON PROPERLY COMPACTED NATURAL SOIL, OR ON STRUCTURAL FILL.
9. SEE FOUNDATION PLAN FOR ADDITIONAL NOTES AND REQUIREMENTS.

CONCRETE & REINFORCEMENT

10. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI IN 28 DAYS. FLAT SLABS, FOUNDATION WALLS, AND CONCRETE RETAINING WALLS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI. A COMPRESSIVE STRENGTH OF 2500 PSI HAS BEEN USED FOR CONCRETE DESIGN.
11. SEE PROJECT SPECIFICATIONS FOR CONCRETE DESIGN REQUIREMENTS.
12. ALL REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO THE STANDARD SPECIFICATIONS ASTM A615 GRADE 60. REINFORCING STEEL SHALL BE PROPERLY TIED INTO PLACE PRIOR TO PLACING CONCRETE.
13. ALL REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH THE ACI DETAILING MANUAL AND ACI STANDARDS (LATEST EDITION).
14. ALL SPLICES IN CONTINUOUS CONCRETE REINFORCING BARS SHALL LAP A MINIMUM OF 40 BARdiaS. ALL SPLICES SHALL BE MADE IN A COMPRESSION ZONE UNLESS NOTED. ALL CONTINUOUS REINFORCING SHALL TERMINATE WITH A 90 DEG. BEND OR WITH SEPARATE CORNER BARS.

FOUNDATION WALLS

15. SEE FOUNDATION WALL SCHEDULE, OR FOUNDATION PLAN, FOR SPECIFICATION OF FOUNDATION WALL REINFORCEMENT.
16. FOUNDATION WALLS HAVE BEEN DESIGNED USING AN EQUIVALENT FLUID PRESSURE. SEE STRUCTURAL PLANS AND CALCULATIONS FOR ACTUAL FLUID PRESSURE USED.
17. BACKFILL ADJACENT TO FOUNDATION WALLS OR IN LANDSCAPED AREAS SHALL BE PLACED IN LOOSE LIFTS A MAXIMUM OF EIGHT INCHES (8"). FILL SHALL HAVE A MOISTURE CONTENT WITHIN 2% OF OPTIMUM AND SHALL BE COMPACTED TO AT LEAST 90% MAXIMUM DENSITY (ASTM D 1557). HEAVY EQUIPMENT SHALL NOT BE USED TO BACKFILL WITHOUT PRIOR CONSENT OF THE ENGINEER.
18. THE CONTRACTOR SHALL COORDINATE STEPS IN WALLS WITH THE ARCHITECT, AND SHALL VERIFY WITH THE ENGINEER.

FOOTINGS

19. SEE FOOTING SCHEDULE FOR FOOTING SIZES AND REINFORCING REQUIREMENTS.
20. FOOTINGS HAVE BEEN DESIGNED USING AN ALLOWABLE BEARING PRESSURE. SEE STRUCTURAL PLANS AND CALCULATIONS FOR ACTUAL BEARING PRESSURE USED.
21. ALL EXTERIOR FOOTINGS SHALL BEAR BELOW FROST DEPTH. CONTRACTOR TO VERIFY.
22. THE CONTRACTOR SHALL COORDINATE STEPS IN FOOTINGS WITH THE ARCHITECT, AND SHALL VERIFY WITH THE ENGINEER.

CONCRETE EXPOSURE CLASS	
ELEMENT	EXP. CLASS
FOOTINGS	F1, S0, W0, C1
WALLS	F1, S0, W0, C1

1

DEFERRED SUBMITTALS

1. THE CONTRACTOR SHALL SUBMIT THE FOLLOWING DOCUMENTS TO THE ARCHITECT AND ENGINEER OF RECORD FOR REVIEW AND APPROVAL. THE DOCUMENTS MUST BE PREPARED AND STAMPED BY AN ENGINEER LICENSED IN THE STATE OF UTAH. THE DOCUMENTS MAY BE SUBMITTED AFTER THE BUILDING PERMIT IS ISSUED, BUT MUST BE SUBMITTED AND APPROVED PRIOR TO COMMENCING FABRICATION OR CONSTRUCTION OF THE COMPONENTS.
   a. CONCRETE MIX DESIGN
   b. WOOD AND ROOF TRUSSES

1

GENERAL STEEL NOTES

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. ALL WORK TO BE IN STRICT ACCORDANCE WITH THE 2015 IBC, AISC, AND LOCAL ORDINANCES.
3. ALL DIMENSIONS AND CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND ERECTION.
4. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
5. SEE ARCHITECTURAL SHEETS FOR DECK BEARING ELEVATIONS. STRUCTURAL STEEL DETAILER SHALL DETERMINE ALL BEARING PLATE ELEVATIONS FROM ARCHITECTURAL DECK ELEVATIONS
6. SEE ARCHITECTURAL SHEETS FOR ADDITIONAL DIMENSIONS.
7. SEE ARCHITECTURAL FOR ACCESS HATCHES, DRAFT STOPS, ETC.
8. SUBMIT SHOP DRAWINGS OF ALL STRUCTURAL STEEL, STEEL JOISTS, STEEL DECKING & MISCELLANEOUS STEEL TO ENGINEER FOR APPROVAL PRIOR TO FABRICATION.
9. SEE FRAMING PLANS FOR ADDITIONAL NOTES AND REQUIREMENTS.

STRUCTURAL STEEL

10. ALL WIDE FLANGE MEMBERS TO BE MANUFACTURED UNDER ASTM A992.
11. ALL STRUCTURAL PLATES, CHANNELS & ANGLES TO BE MANUFACTURED UNDER ASTM A36
12. ALL HSS MEMBERS TO BE MANUFACTURED UNDER ASTM A500 GRADE B.
13. ALL PIPE COLUMNS TO BE MANUFACTURED UNDER ASTM A53 GRADE B.
14. ALL BOLTS FOR STEEL TO STEEL CONNECTIONS TO BE 3/4" DIA. MIN. A325-N HIGH STRENGTH BOLTS, UNLESS NOTED OTHERWISE. BOLTS EMBEDDED IN CONCRETE OR MASONRY SHALL BE F1554 GRADE 36 UNLESS NOTED OTHERWISE.
15. ALL JOIST WELDS TO BE E7024. ALL DECK WELDS TO BE E6022. ALL WELDS FOR SEISMIC SPECIFIC CONNECTIONS TO BE E7018. ALL OTHER WELDS TO BE 70 KSI MIN. ALL WELDS SHALL BE BY A CERTIFIED WELDER.
16. ALL WELDS AND BOLTING TO MEET APPROVAL OF SPECIAL INSPECTOR AS REQUIRED BY BUILDING OFFICIAL.
17. ALL STEEL SHALL BE PROPERLY PRIMED EXCEPT AREAS THAT REQUIRE FIELD WELDING (i.e. TOP OF BEAMS).
18. ALL STEEL BEAMS USED AS GIRDERS SHALL HAVE WEB STIFFENERS EACH SIDE OF WEB AT BEARING ENDS AND AT CONCENTRATED LOADS. STIFFENER TO BE SAME THICKNESS AS WEB OF BEAM-TYPICAL. STEEL BEAMS USED AS JOISTS DO NOT REQUIRE WEB STIFFENERS.
19. SEE ARCHITECTURAL, MECHANICAL & ELECTRICAL FOR ADDITIONAL STEEL MEMBERS (BRACKETS, ANGLES, ETC...) REQUIRED.
20. STEEL MEMBERS SHALL NOT BE CUT, DRILLED OR TORCHED FOR PIPES, ETC. UNLESS SPECIFICALLY DETAILED.
21. ANY MODIFICATION OF STRUCTURAL MEMBERS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS IS NOT PERMITTED WITHOUT PRIOR APPROVAL.
22. ANY CONNECTIONS NOT DETAILED ON STRUCTURAL PLANS SHALL BE PROVIDED BY THE STEEL DETAILER. SHOP DRAWINGS FOR ALL FABRICATED STEEL CONNECTIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO FABRICATION AND INSTALLATION.

DESIGN CRITERIA

1. GOVERNING BUILDING CODE: 2015 INTERNATIONAL BUILDING CODE (IBC)
2. ROOF LIVE LOADING:
   a. ROOF LIVE LOAD . . . . . 20 PSF
   b. ROOF SNOW LOAD . . . . . 192 PSF
      1. GROUND SNOW LOAD, P_G . . . . . 274 PSF
      2. SNOW EXPOSURE FACTOR, C_E 1.0
      3. IMPORTANCE FACTOR, I_S 1.0
      3. THERMAL FACTOR, C_T 1.0
3. ROOF DEAD LOADS:
   a. GABLE ROOF . . . . . 15 PSF
4. EARTHQUAKE:
   a. RISK CATEGORY II
   b. SEISMIC DESIGN CATEGORY . . . . . D
   c. SPECTRAL RESPONSE ACCELERATIONS:
      S_S = 0.898g
      S_{DS} = 0.683g
      S_1 = 0.304g
      S_{D1} = 0.363g
   d. SOIL SITE CLASS: . . . . . B
      F_A = 1.1
      F_V = 1.8
   e. IMPORTANCE FACTOR, I_E 1.0
   f. DESIGN BASE SHEAR . . . . . C_S x W
   g. SEISMIC RESPONSE COEFFICIENT, C_S 0.105, 0.210
   h. ANALYSIS PROCEDURE . . . . . EQUIV. LATERAL FORCE
   i. BASIC SEISMIC FORCE RESISTING SYSTEM . . . . . WOOD SHEAR WALLS. SOCBF
   j. RESPONSE MODIFICATION FACTOR, R 6.5/3.25
5. WIND:
   a. BASIC WIND SPEED (3 SECOND GUST) . . . . . 115 MPH (ULTIMATE)
      90 MPH (NOMINAL)
   b. EXPOSURE . . . . . B
   c. INTERNAL PRESSURE COEFFICIANT, GC P_I 0.18
   d. COMPONENTS AND CLADDING PRESSURE . . . . . VARIES
6. FOUNDATION:
   a. SOILS REPORT BY. . . . . IGES
      DATED . . . . . AUG. 03, 2016
   b. SOIL BEARING PRESSURE. . . . . 2,600 PSF

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GENERAL WOOD FRAMING NOTES

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. ALL WORK TO BE IN STRICT ACCORDANCE WITH THE 2015 IBC, NDS, AND LOCAL ORDINANCES.

DIMENSIONAL LUMBER

3. DIMENSIONAL LUMBER USED AS STRUCTURAL FRAMING (i.e. JOISTS, RAFTERS, HEADERS) SHALL BE DOUGLAS FIR-LARCH № 2 OR EQUAL.
4. DIMENSIONAL LUMBER USED FOR STUD WALLS SHALL BE STUD GRADE UNLESS NOTED OTHERWISE. STUDS SHALL BE SPACED AT 16" O.C. MINIMUM, WITH A DOUBLE TOP PLATE. SPLICES IN THE DOUBLE TOP PLATE SHALL ALTERNATE TOP & BOTTOM AND SHALL LAP 48" MIN.
5. ROUGH CUT TIMBER USED AS STRUCTURAL FRAMING SHALL BE AS SPECIFIED IN THE CONSTRUCTION DOCUMENTS.

ENGINEERED LUMBER

6. GLU-LAMINATED BEAMS FOR SIMPLE SPANS SHALL BE 24F-V4 DF/DF. GLU-LAMINATED BEAMS FOR CONTINUOUS SPANS AND CANTILEVERS SHALL BE 24F-V8 DF/DF. DO NOT INSTALL GLU-LAMINATED BEAMS UPSIDE DOWN.
7. LAMINATED VENEER LUMBER AND THE LIKE SHALL BE INSTALLED PER MANUFACTURERS RECOMMENDATIONS AND SPECIFICATIONS. LVL BEAMS SHALL BE BUILT UP w/ 1 3/4" MEMBERS. SEE FRAMING PLANS FOR NUMBER OF MEMBERS REQUIRED.
8. I-JOISTS SHALL BE TJI OR EQUIVALENT, AND SHALL BE INSTALLED PER MANUFACTURERS RECOMMENDATIONS AND SPECIFICATIONS.
9. ENGINEERED LUMBER, WITH THE EXCEPTION OF EXTERIOR GRADE GLU-LAMINATED LUMBER, SHALL NOT BE USED IN EXTERIOR APPLICATIONS.
10. USE PRESSURE TREATED LUMBER FOR ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY IN CONTACT WITH EARTH (i.e. MUD SILL). IN SOME SITUATIONS, 26 GAUGE GALVANIZED SHEET METAL MAY BE PROVIDED AS AN APPROVED MOISTURE BARRIER. SEE ENGINEER FOR APPROVAL OF THIS OPTION.

BLOCKING, BRIDGING & MISCELLANEOUS

11. DIMENSIONAL JOISTS AND RAFTERS SHALL HAVE FULL-HEIGHT SOLID BLOCKING AT THEIR BEARING POINTS. EACH RAFTER AND/OR ROOF TRUSS SHALL BE ANCHORED WITH SIMPSON H1 ANCHORS AT EACH END.
12. I-JOISTS AND RAFTERS SHALL HAVE FULL-HEIGHT SOLID BLOCKING AT THEIR BEARING POINTS. CONNECT EACH BLOCK TO TOP OF EXTERIOR WALLS WITH SIMPSON A34 CLIPS EACH JOIST OR RAFTER SHALL BE ANCHORED WITH SIMPSON H2.5 ANCHORS AT EACH END.
13. WOOD MEMBERS SHALL NOT BE CUT FOR PIPES, ETC. UNLESS SPECIFICALLY DETAILED
14. BIRDS MOUTHS AND/OR NOTCHING OF STRUCTURAL MEMBERS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS IS NOT PERMITTED WITHOUT PRIOR APPROVAL.

COLUMNS & STUDS

15. ALL COLUMNS SHALL EXTEND DOWN THROUGH THE STRUCTURE TO THE FOUNDATION. COLUMNS SHALL BE BRACED AT EACH FLOOR LEVEL. COLUMNS SHALL BE AS WIDE AND DEEP AS THE MEMBER THEY SUPPORT IN ORDER TO PROVIDE FULL BEARING.
16. STAND ALONE POSTS SHALL BE DOUGLAS FIR-LARCH № 1OR EQUAL.
17. ALL EXTERIOR WALLS SHALL BE 2 x 6's AT 16" O.C.
18. ALL INTERIOR BEARING WALLS SHALL BE 2 x 6'S AT 16" O.C. UNLESS NOTED OTHER-WISE ON PLANS.

FLOOR, ROOF & WALL SHEATHING

19. ALL ROOF SHEATHING SHALL BE 5/8" APA EXP. 1 RATED SHEATHING OR EQUAL WITH 10d COMMON NAILS AT 6" O.C. PERIMETER, 6" O.C. PANEL EDGES AND AT 12" O.C. IN THE FIELD. PANEL EDGES ARE UNBLOCKED UNLESS NOTED OTHERWISE ON THE STRUCTURAL PLANS.
20. ALL FLOOR SHEATHING TO BE 3/4" THICK T&G SHEATHING GLUED AND NAILED WITH 10d COMMON NAILS OR EQUAL AT 6" O.C. PERIMETER, 6" O.C. PANEL EDGES AND AT 10" O.C. IN THE FIELD. PANEL EDGES ARE UNBLOCKED UNLESS NOTED OTHERWISE ON THE STRUCTURAL PLANS.
21. ALL EXTERIOR WALLS SHALL BE SHEATHED WITH 7/16" APA EXP. 1 RATED SHEATHING OR EQUAL WITH 8d COMMON NAILS AT 6" O.C. EDGES AND AT 12" O.C. IN THE FIELD - FLAT BLOCKED AT ALL PANEL EDGES, UNLESS NOTED OTHERWISE IN SHEAR WALL SCHEDULE.

STRUCTURAL CONNECTIONS

22. THE CONTRACTOR IS ULTIMATELY RESPONSIBLE TO PROVIDE ADEQUATE STRUCTURAL CONNECTIONS. CONNECTIONS MUST CARRY THE BEARING CAPACITY OF THE MEMBER AND ANY UPLIFT OR SEISMIC FORCES GENERATED IN THE MEMBER. SPECIAL CONSIDERATION SHALL BE GIVEN TO PREVENT CRUSHING OF THE MEMBER AT BEARING, SPLITTING AND/OR CRACKING OF THE WOOD, AND THE LIKE.
23. THE CONTRACTOR SHALL STRICTLY ADHERE TO THE CONNECTION DETAILS SPECIFIED ON THE PLANS OR INCLUDED WITH THE CONSTRUCTION DOCUMENTS. PRIOR APPROVAL IS REQUIRED FOR ANY DEVIATION FROM THE CONSTRUCTION DOCUMENTS.
24. SUBSTITUTION OF CONNECTIONS OTHER THAN THOSE SPECIFIED ON THE PLANS REQUIRES PRIOR APPROVAL. THE ENGINEER IS NOT RESPONSIBLE FOR CONNECTIONS NOT APPROVED PRIOR TO CONSTRUCTION OR INSTALLATION.
25. IF CONNECTION DETAILS, APPROVED BY THE ENGINEER, HAVE NOT BEEN PROVIDED IN THE CONSTRUCTION DOCUMENTS, IT IS THE CONTRACTORS RESPONSIBILITY TO SPECIFY AND PROVIDE ALL STRUCTURAL CONNECTIONS. IF OTHER THAN STANDARD CONNECTIONS ARE REQUIRED, SEE ENGINEER FOR ADDITIONAL ASSISTANCE.
26. USE SIMPSON CONNECTIONS OR EQUIVALENT. INSTALL PER MANUFACTURERS SPECIFICATIONS.
27. SHOP DRAWINGS FOR ALL FABRICATED STEEL CONNECTIONS SHALL BE SUBMITTED FOR REVIEW & APPROVAL PRIOR TO FABRICATION AND INSTALLATION. SEE GENERAL STEEL NOTES.
28. SEE GENERAL CONCRETE NOTES FOR SPECIFICATION OF ANCHOR BOLTS, ETC. IN NO CASE SHALL THE MUD SILL BE NOTCHED FOR THE INSTALLATION OF PLATE WASHERS, OR FOR ANY OTHER REASON.
29. ALL STRUCTURAL MEMBERS SHALL HAVE 1 3/4" MINIMUM BEARING.
30. FOR ADDITIONAL NAILING PATTERN, SEE SCHEDULES IN THE INTERNATIONAL BUILDING CODE (IBC).

STAIR FRAMING

31. STAIR STRINGERS SHALL BE 11 7/8" LVL's AT 16" O.C. (MAX.) w/ A MAXIMUM HORIZONTAL RUN OF 12'-0". USE 14" LVL UP TO 16'-0" RUN

GENERAL WOOD TRUSS NOTES

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. THE TRUSSES SHALL ALSO BE DESIGNED PER THE 2015 INTERNATIONAL BUILDING CODE, AND LOCAL ORDINANCES. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE DESIGN AND FABRICATION OF THE PRE-ENGINEERED TRUSSES, PER THE DESIGN CRITERIA ABOVE. DESIGN MUST TAKE INTO ACCOUNT UNBALANCED SNOW LOADS, SNOW DRIFTING, INCREASED SNOW LOADS ON EAVES AND IN VALLEYS, IMPACT LOADS FROM FALLING SNOW AND ICE, ETC.
3. THE PROJECT ENGINEER, OR ENGINEER OF RECORD, IS NOT RESPONSIBLE FOR THE DESIGN OF THE PRE-ENGINEERED TRUSSES, NOR FOR THE INSTALLATION, ETC. OF THE TRUSSES. TRUSS DESIGN DRAWINGS FOR ALL WOOD TRUSSES SHALL BE SUBMITTED TO THE ENGINEER AND ARCHITECT FOR REVIEW AND APPROVAL PRIOR TO FABRICATION AND INSTALLATION.
4. THE TRUSS DESIGN DRAWINGS NEED TO INCLUDE ALL SPECIFIC REQUIREMENTS DESCRIBED IN INTERNATIONAL BUILDING CODE. AS REQUIRED IN ABOVE MENTIONED CODE A LICENSED DESIGN PROFESSIONAL LICENSED IN JURISDICTION WHERE PROJECT IS LOCATED WILL NEED TO STAMP TRUSS DESIGN DRAWINGS. THE DESIGN PROFESSIONAL CANNOT BE THE ENGINEER OF RECORD FOR THE PROJECT AND NEEDS EXPERIENCE DESIGNING PRESS PLATE WOOD TRUSSES.
5. ALL TRUSS TO TRUSS AND TRUSS TO STRUCTURAL BEAM CONNECTORS SHALL BE SPECIFIED BY THE TRUSS MANUFACTURER, AND CALLED OUT AT THE PROPER LOCATION ON TRUSS PLACEMENT DIAGRAM.
6. THE TRUSSES SHALL BE DESIGNED TO CARRY ANY ADDITIONAL LOADS DUE TO MECHANICAL UNITS, OVERHEAD DOORS, ROOF OVERBUILDS, ETC. SEE STRUCTURAL PLANS FOR ADDITIONAL REQUIREMENTS.
7. ALL MEMBERS SHALL BE DESIGNED FOR COMBINED STRESSES, BASED ON THE WORST LOADING CONDITION.
8. BOTTOM CHORDS OF TRUSSES, ACTING AS CEILING MEMBERS, MUST BE ABLE TO SUPPORT A 10 PSF LIVE LOAD PER IBC REQUIREMENTS.
9. EACH CHORD SECTION SHALL BE ENGAGED IN TWO PANEL POINTS BEFORE BEING SPLICED. SPLICE NEEDS TO OCCUR AT PANEL POINT, OR ZERO FORCE LOCATION.
10. PROVIDE 1/8" CAMBER FOR EACH 6 FEET OF TRUSS UNLESS OTHERWISE INDICATED.

TRUSS BRACING & BLOCKING

11. THE TRUSS MANUFACTURER SHALL SPECIFY PROPER BRACING OF COMPRESSION CHORD MEMBERS 6'-0" LONG (OR LONGER), AS WELL AS BRACING REQUIRED FOR TRUSS ERECTION, AND ANY OTHER BRACING.
12. THE TRUSS MANUFACTURER SHALL SPECIFY ALL REQUIRED TRUSS BLOCKING. TRUSS BLOCKING SHALL BE DESIGNED FOR LATERAL LOADINGS.

FABRICATION & INSTALLATION

13. ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO FABRICATION.
14. FABRICATE TRUSSES FROM SHOP DRAWINGS REVIEWED AND APPROVED BY THE ENGINEER AND ARCHITECT.
15. FABRICATE TRUSSES IN JIGS WITH MEMBERS ACCURATELY CUT TO PROVIDE GOOD BEARING AT JOINTS. JOINTS SHALL BE ACCEPTABLE IF THE AVERAGE OPENING BETWEEN ENDS OF MEMBERS IMMEDIATELY AFTER FABRICATION IS LESS THAN 1/16", EXCEPT THAT TRUSS COMPRESSION CHORD JOINTS AT SPLICES AND RIDGES SHALL HAVE FULL CONTACT BETWEEN MEMBERS.
16. TRUSS FABRICATORS USING METAL PLATES SHALL HAVE PLANT INSPECTED FOUR TIMES PER YEAR BY AN INDEPENDENT TESTING LABORATORY IN ACCORDANCE WITH TPI REGULATIONS AND COPIES OF INSPECTIONS MADE AVAILABLE TO OWNER UPON REQUEST.
17. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE INSTALLATION OF THE TRUSSES PER THE TRUSS MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS. NO WEB OR CHORD MEMBERS SHALL BE MODIFIED IN THE FIELD.

METAL GUSSET PLATES

18. GUSSET PLATES SHALL BE SPECIFIED FOR GREATER OF EITHER THE MEMBER FORCES SHOWN ON DRAWINGS OR THE MEMBER FORCES DERIVED FROM STRUCTURAL ANALYSIS. PLUS OR MINUS 6%.
19. NO PANEL POINT SHALL HAVE MORE THAN ONE PLATE PER TRUSS SIDE.
20. PRESS PLATES INTO MEMBERS TO OBTAIN FULL PENETRATION WITHOUT CRUSHING OUT SURFACE OF WOOD. PLATE EMBEDMENT IS ACCEPTABLE IF OPENING BETWEEN PLATE AND WOOD SURFACE IS LESS THAN 1/32".
21. LUMBER DEFECTS AND PLATE MISPLACEMENT, IN COMBINATION, SHALL NOT REDUCE PLATE AREA OR NUMBER OF AFFECTIVE TEETH, PRONGS, OR NAILS BY MORE THAN 10%.
22. DO NOT APPLY METAL GUSSET PLATES AFTER SHOP FABRICATION.
23. ALL LOADS SPECIFICALLY CALLED OUT ON PLANS TO BE USED IN DESIGNING TRUSSES, ARE ALLOWABLE STRESS DESIGN (ASD) FORMAT w/ NO LOAD FACTORS INCLUDED.
24. WHEN TRUSSES ARE CALLED OUT AS LATERAL DRAG STRUTS. THE PHYSICAL TRUSS DESIGNATED AS SUCH NEEDS TO BE PHYSICALLY AND PERMANENTLY MARKED DIFFERENT FROM NORMAL TRUSSES.

DESIGN LOADS FOR ROOF TRUSSES:

TOP CHORD LIVE LOAD	= 192 PSF
TOP CHORD DEAD LOAD	= 10 PSF
BOT CHORD LIVE LOAD	= 0 PSF
BOT CHORD DEAD LOAD	= 5 PSF
TOTAL DESIGN LOAD	= 207 PSF

DEFLECTION CRITERIA ROOF TRUSSES:

TOTAL LOAD DEFLECTION = L/ 240

DYNAMIC STRUCTURES
1887 NORTH 1120 WEST PROVO, UTAH 84604
PH: (801)356-1140 FAX: (801)356-0001

Structural Plans for:
POWDER MOUNTAIN CABIN 1500+

PROFESSIONAL STRUCTURAL ENGINEER
NO. 190917
JAY D. ADAMS
STATE OF UTAH
9/22/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	AS SHOWN
DATE:	JULY 28, 2017
JOB No.	17-089

GENERAL NOTE SHEET

SHEET No.
S0.1

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017

SPECIAL INSPECTION SCHEDULE

SOILS (IBC 1705.6)

REQ'D	TASK	INSPECTION FREQUENCY CONT.	INSPECTION FREQUENCY PERIODIC	COMMENTS:
X	VERIFY ADEQUATE MATERIALS BELOW FOOTINGS		◊	PRIOR TO PLACEMENT OF CONCRETE.
X	EXCAVATION EXTEND TO PROPER DEPTH AND MATERIALS		◊	PRIOR TO PLACEMENT OF COMPACTED FILL OR CONCRETE.
X	CLASSIFICATION AND TESTING OF FILL MATERIALS		◊	CHECK CLASSIFICATION AND GRADATIONS AT EACH LIFT, BUT NOT LESS THAN ONCE FOR EACH 10,000 FT^2 OF SURFACE AREA.
X	VERIFY PROPER FILL MATERIALS, LIFT THICKNESSES AND IN-PLACE DENSITIES	◊		
X	VERIFY PROPERLY PREPARED SITE AND SUBGRADE		◊	PRIOR TO PLACEMENT OF CONCRETE.

CONCRETE CONSTRUCTION (IBC 1705.3)

REQ'D	TASK	INSPECTION FREQUENCY CONT.	INSPECTION FREQUENCY PERIODIC	COMMENTS:
X	REINFORCING STEEL PLACEMENT		◊	VERIFY SIZE, CLEARANCES, SPLICES AND PROPER TIES.
X	REINFORCING BAR WELDING a. WELDABILTY OF NON ASTM A706 BARS b. SINGLE PASS FILLED WELDS < 5/16" c. ALL OTHER WELDS	◊	◊ ◊	
X	CAST IN ANCHORS		◊	VERIFY MIX DESIGN MEETS STRENGTH AND EXPOSURE REQUIREMENTS LISTED ON APPROVED PLANS.
X	POST-INSTALLED ANCHORS a. ADHESIVE ANCHORS INSTALLED HORIZONTALLY OR UPWARDLY INCLINED RESISTING SUSTAINED TENSION LOADS b. POST INSTALLED ANCHORS NOT DEFINED IN a.	◊	◊	IN ACCORDANCE WITH APPROVED ICC-ES REPORT. PERIODIC INSPECTIONS ALLOWED IF STATED IN ES REPORT.
X	VERIFY REQUIRED DESIGN MIX		◊	VERIFY MIX DESIGN MEETS STRENGTH AND EXPOSURE REQUIREMENTS LISTED ON APPROVED PLANS.
X	SLUMP, AIR + TEMPERATURE TESTS. PREPARE STRENGTH TEST SAMPLES	◊		
X	CONCRETE PLACEMENT	◊		INCLUDES SAMPLING FOR AIR, SLUMP, STRENGTH AND TEMPERATURE TECHNIQUES.
X	CURING TEMPERATURE MAINTENANCE		◊	
	PRESTRESSED CONCRETE a. PRESTRESSING FORCES b. GROUTING OF BONDED TENDONDS	◊ ◊		
	ERECTION OF PRECAST MEMBERS		◊	
	POST-TENSIONED CONCRETE STRENGTH		◊	
X	INSPECT FORMWORK		◊	

COLD-FORMED STEEL CONSTRUCTION (IBC 1705.11.2 & 1705.12.3)

REQ'D	TASK	INSPECTION FREQUENCY CONT.	INSPECTION FREQUENCY PERIODIC	COMMENTS:
	COMPONENTS OF WIND AND SEISMIC-FORCE RESISTING SYSTEMS		◊	VERIFY PROPER SCREW ATTACHMENT, BOLTING AND ANCHORING OF SHEAR WALLS, BRACES AND HOLDOWNS HAVING A FASTENER SPACING ≤ 4" O.C.
	FIELD WELDING OF ELEMENTS OF MAIN LATERAL FORCE RESISTING SYSTEM.		◊	

OTHER THAN STRUCTURAL STEEL (IBC 1705.2.2)

REQ'D	TASK	INSPECTION FREQUENCY CONT.	INSPECTION FREQUENCY PERIODIC	COMMENTS:
	STEEL ROOF & FLOOR DECK:			
	MATERIAL VERIFICATION OF STEEL DECK		◊	IDENTIFICATION MARKINGS PER APPLICABLE ASTM STANDARD
	ROOF AND DECK WELDS		◊	VERIFY THAT WELDS CONFORM TO AWS D1.3.
	WELDING OF REINFORCING STEEL:			
	VERIFICATION OF WELDABILITY (EXCEPT A706 BAR)		◊	VERIFY MATERIAL IS ABLE TO CONFORM TO AWS D1.4.

INSTALLATION OF OPEN-WEB STEEL JOISTS AND GIRDERS (IBC 1705.2.3)

REQ'D	TASK	INSPECTION FREQUENCY CONT.	INSPECTION FREQUENCY PERIODIC	COMMENTS:
	END CONNECTIONS		◊	SJI 2207.1
	BRIDGING - HORIZONTAL OR DIAGONAL a. STANDARD BRIDGING b. NON-STANDARD BRIDGING		◊ ◊	SJI 2207.1

MASONRY CONSTRUCTION (IBC 1705.4)

REQ'D	TASK	INSPECTION FREQUENCY CONT.	INSPECTION FREQUENCY PERIODIC	COMMENTS:
	MINIMUM TESTING (TABLE 1.19.2, TMS - 402/ACI 530-11):			
	VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) FOR SELF-CONSOLIDATING GROUT.		◊	COMPRESSIVE STRENGTH TESTS PER ASTM C 1019 FOR SLUMP FLOW AND ASTM C 1611 FOR VSI.
	VERIFICATION OF F'_M		◊	DETERMINE COMPRESSIVE STRENGTH PER "UNIT STRENGTH" OR "PRISM TEST" AS SPECIFIED IN ARTICLE 1.4.B OF ACI 530.1 PRIOR TO CONSTRUCTION.
	PRIOR TO CONSTRUCTION (ARTICLE 1.15, TMS-602/ACI 530.1-11):			
	REVIEW MATERIAL CERTIFICATES, MIX DESIGNS, TEST RESULTS AND CONSTRUCTION PROCEDURES		◊	VERIFY MATERIALS CONFORM TO APPROVED CONSTRUCTION DOCUMENTS. MIX DESIGN, TEST RESULTS, MATERIAL CERTIFICATES, AND CONSTRUCTION PROCEDURES SHOULD BE SUBMITTED FOR REVIEW. MORTAR MIX DESIGNS SHALL CONFORM TO ASTM C 270 WHILE GROUT SHALL CONFORM TO ASTM C 476. MATERIAL CERTIFICATES SHALL BE PROVIDED FOR THE FOLLOWING: REINFORCEMENT; ANCHORS, TIES, FASTENERS, AND METAL ACCESSORIES; MASONRY UNITS; MORTAR AND GROUT MATERIALS. REVIEW COLD-WEATHER OR HOT-WEATHER CONSTRUCTION PROCEDURES.
	AS CONSTRUCTION BEGINS (TABLE 1.19.2, TMS-402/ACI 530-11):			
	PROPORTIONS OF SITE-PREPARED MORTAR		◊	VERIFY THAT MORTAR IS TYPE AND COLOR SPECIFIED ON APPROVED PLANS, IT CONFORMS TO ASTM C 270, AND IS MIXED PER ARTICLE 2.6.A OF ACI 530.1.
	CONSTRUCTION OF MORTAR JOINTS		◊	VERIFY MORTAR JOINTS MEET ARTICLE 3.3.B OF ACI 530.1.1
	GRADE AND SIZE OF PRE-STRESSING TENDONS AND ANCHORAGES		◊	VERIFY THAT PRE-STRESSING TENDONS CONFORM TO REQUIREMENTS OF ARTICLE 2.4B AND 2.4H OF ACI530.1
	LOCATION OF REINFORCEMENT, CONNECTORS AND ANCHORAGES.		◊	VERIFY REINFORCEMENT IS PLACED IN ACCORDANCE WITH ARTICLE 3.4 OF 530.1.
	PRE-STRESSING TECHNIQUE		◊	VERIFY PRE-STRESSING TECHNIQUE CONFORMS TO ARTICLE 3.6B OR ACI 530.1
	PROPERTIES OF THIN BED MORTAR FOR AAC MASONRY	◊	◊	VERIFY REINFORCEMENT IS PLACED IN ACCORDANCE WITH ARTICLE 3.4 OF 530.1.
	PRIOR TO GROUTING (TABLE 1.19.2, TMS-402/ACI 530-11):			
	GROUT SPACE		◊	VERIFY GROUT SPACE IS FREE OF MORTAR DROPPINGS, DEBRIS, LOOSE AGGREGATE, AND OTHER DELETERIOUS MATERIALS AND THAT CLEANOUTS ARE PROVIDED PER ARTICLE 3.2D AND 3.2F OF ACI 530.1
	GRADE, TYPE AND SIZE OF REINFORCEMENT, ANCHOR BOLTS AND ANCHORAGES.		◊	VERIFY REINFORCEMENT, JOINT REINFORCEMENT, ANCHOR BOLTS AND VENEER ANCHORS COMPLY WITH APPROVED PLANS AND SECTIONS 1.6 OF ACI 530.
	PLACEMENT OF REINFORCEMENT, CONNECTORS AND ANCHORAGES.		◊	VERIFY REINFORCEMENT, JOINT REINFORCEMENT, ANCHOR BOLTS AND VENEER ANCHORS ARE INSTALLED PER APPROVED PLANS AND ARTICLES 3.2.E, 3.4, AND 3.6.A OF ACI 530.1.
	PROPORTIONS OF SITE-PREPARED GROUT.		◊	VERIFY GROUT PROPORTIONS MEET ASTM C 476 AND A SLUMP BETWEEN 8-11 INCHES. SELF-CONSOLIDATED GROUT SHALL NOT BE PROPORTIONED ONSITE.
	CONSTRUCTION OF MORTAR JOINTS		◊	VERIFY MORTAR JOINTS PLACED IN ACCORDANCE WITH ARTICLE 3.3.B OF ACI 530.1.
	DURING CONSTRUCTION (TABLE 1.19.2, TMS-402/ACI 530-11):			
	SIZE AND LOCATION OF STRUCTURAL ELEMENTS		◊	VERIFY LOCATIONS OF STRUCTURAL ELEMENTS PER APPROVED PLANS AND CONFIRM TOLERANCES MEET ARTICLE 3.3.F OF ACI 530.1.
	TYPE, SIZE AND LOCATION OF ANCHORS, FRAMES, ETC.		◊	VERIFY CORRECT ANCHORAGES AND CONNECTIONS ARE PROVIDED PER APPROVED PLANS AND SECTIONS 1.16.4.3 AND 1.17.1 OF ACI 530.
	WELDING OF REINFORCEMENT	◊		VERIFY CONFORMANCE WITH SECTIONS 2.1.7.7.2, 3.3.3.4 (c) AND 8.3.3.4 (b) OF ACI 530
	APPLICATION AND MEASUREMENT OF PRE-STRESSING FORCE	◊		VERIFY CONFORMANCE WITH ARTICLE 3.6B OF ACI 530.1
	PLACEMENT OF GROUT	◊		
	PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (<40°F) OR HOT WEATHER (>90°F).		◊	VERIFY COLD-WEATHER CONSTRUCTION COMPLIES WITH ARTICLE 1.8.C OF ACI 530.1 AND HOT WEATHER CONSTRUCTION PER ARTICLE 1.8.D OF ACI 530.1.
	PLACEMENT OF GROUT AND PRE-STRESSING GROUT FOR BONDED TENDONS	◊		VERIFY COMPLIANCE WITH ARTICLE 3.5, 3.6C OF ACI 530.1
	OBSERVATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND / OR PRISMS.		◊	CONFIRM SPECIMENS/ PRISMS ARE PERFORMED AS REQUIRED BY ARTICLE 1.4 OF ACI 530.1.

WOOD CONSTRUCTION (IBC 1705.11.2)

REQ'D	TASK	INSPECTION FREQUENCY CONT.	INSPECTION FREQUENCY PERIODIC	COMMENTS:
X	COMPONENTS OF WIND AND SEISMIC-FORCE RESISTING SYSTEMS		◊	VERIFY PROPER SCREW ATTACHMENT, BOLTING AND ANCHORING OF SHEAR WALLS, BRACES AND HOLDOWNS HAVING A FASTENER SPACING ≤ 4" O.C.
	FIELD GLUING OF MAIN LATERAL FORCE RESISTING SYSTEM	◊		

STATEMENT OF SPECIAL INSPECTIONS

1. THE PROJECT OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED BELOW. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS REQUIRED BY THE BUILDING DEPARTMENT OF THE LOCAL JURISDICTION.

2. SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS DONE IN CONFORMANCE WITH APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALE BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT A PHASE OF THE WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE START OF WORK.

3. SPECIAL INSPECTIONS FOR EACH TASK SHALL BE CARRIED OUT IN COMPLIANCE WITH REQUIREMENTS PER THE CURRENT IBC AND OTHER MATERIAL STANDARDS.

FABRICATION SHOP REQUIREMENTS

4. WHERE FABRICATION OF STRUCTURAL LOAD BEARING MEMBERS AND ASSEMBLIES IS BEING PERFORMED ON THE PREMISES OF A FABRICATORS SHOP, SPECIAL INSPECTIONS REQUIRED BELOW SHALL BE PROVIDED IN THE SHOP DURING THE FABRICATION PROCESS. THIS REQUIREMENT MAY BE EXCEPTED IF THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. A CERTIFICATE SHALL BE REQUIRED TO VERIFY SUCH APPROVAL. AT COMPLETION OF THE FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DRAWINGS.

DYNAMIC STRUCTURES
1887 NORTH 1120 WEST PROVO, UTAH 84604
PH: (801)356-1140 FAX: (801)356-0001

Structural Plans for:
POWDER MOUNTAIN CABIN 1500+

PROFESSIONAL STRUCTURAL ENGINEER
NO. 190917
JAY D. ADAMS
STATE OF UTAH
7/27/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	
DATE:	JULY 28, 2017
JOB No.	17-089

SPECIAL INSPECTION SHEET

SHEET No.
S0.2

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017

STRUCTURAL STEEL CONSTRUCTION (IBC 1705.2, 1705.11, 1705.12)

REQ'D	TASK	INSPECTION TYPE Q.C.	INSPECTION TYPE Q.A.	COMMENTS:
	PRIOR TO WELDING (TABLE N5.4-1, AISC 360-10):			
X	VERIFY WELDING PROCEDURES	P	P	
X	MANUFACTURER CERTIFICATIONS	P	P	
X	MATERIAL IDENTIFICATION	O	O	VERIFY TYPE AND GRADE OF MATERIAL.
X	WELDER IDENTIFICATION	O	O	VERIFY THERE IS A SYSTEM IN PLACE TO IDENTIFY THE WELDER WHO HAS WELDED A JOINT OR MEMBER.
	FIT-UP GROOVE WELDS	O	O	VERIFY JOINT PREPARATION, DIMENSIONS, CLEANLINESS, TACKING AND BACKING.
	ACCESS HOLES	O	O	VERIFY CONFIGURATION AND FINISH.
	FIT-UP FILLET WELDS	O	O	VERIFY ALIGNMENT, GAPS AT ROOT, CLEANLINESS OF STEEL SURFACES, TACK WELD QUALITY AND LOCATION.
X	CHECK WELDING EQUIPMENT	O	O	
	DURING WELDING (TABLE N5.4-2, AISC 360-10):			
X	USE OF QUALIFIED WELDERS	O	O	VERIFY THAT WELDERS ARE APPROPRIATELY QUALIFIED.
X	CONTROL AND HANDLING OF WELDING CONSUMABLES	O	O	VERIFY PACKAGING AND EXPOSURE CONTROL.
X	CRACKED TACK WELDS	O	O	VERIFY WELDING IS NOT OVER A CRACKED TACK WELD.
X	ENVIRONMENTAL CONDITIONS	O	O	VERIFY WIND SPEED IS WITHIN LIMITS AS WELL AS PRECIPITATION AND TEMPERATURE.
X	WPS FOLLOWED	O	O	VERIFY ITEMS SUCH AS WELDING EQUIPMENT SETTINGS, TRAVEL SPEED, WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED, AND PROPER POSTITION.
X	WELDING TECHNIQUES	O	O	VERIFY INTERPASS AND FINAL CLEANING, EACH PASS IS WITHIN PROFILE LIMITATIONS, AND QUALITY OF EACH PASS.
	AFTER WELDING (TABLE N5.4-3, AISC 360-10):			
X	WELDS CLEANED	O	O	VERIFY THAT WELDS HAVE BEEN PROPERLY CLEANED.
X	SIZE, LENGTH AND LOCATION OF WELDS	P	P	
X	WELDS MEET VISUAL ACCEPTANCE CRITERIA	P	P	
	ARC STRIKES	P	P	
	PRIOR TO BOLTING (TABLE N5.6-1 AISC 360-10):			
X	MANUFACTURERS CERTIFICATIONS FOR FASTENERS	O	P	
X	FASTENERS MARKED w/ ASTM REQUIREMENTS	O	O	
X	PROPER FASTENERS SELECTED FOR DETAIL	O	O	
X	PROPER PROCEDURE FOR DETAIL	O	O	
X	CONNECTING ELEMENTS	O	O	
X	PRE-INSTALLATION VERIFICATION TESTING	P	O	
X	PROPER STORAGE OF FASTENERS	O	O	
	DURING BOLTING (TABLE N5.6-2 AISC 360-10):			
X	FASTENER ASSEMBLIES	O	O	
X	JOINTS SNUG TIGHT PRIOR TO PRETENSIONING	O	O	
X	PROPER WRENCH USAGE	O	O	
X	FASTENERS PRETENSIONED	O	O	
	AFTER BOLTING (TABLE N5.6-3, AISC 360-10):			
X	STRUCTURAL STEEL DETAILS	P	P	

O- OBSERVE THESE ITEMS ON A RANDOM BASIS.

P- PERFORM THESE TASKS FOR EACH WELDED / BOLTED JOINT OR MEMBER (AISC 360-10 N5.4)

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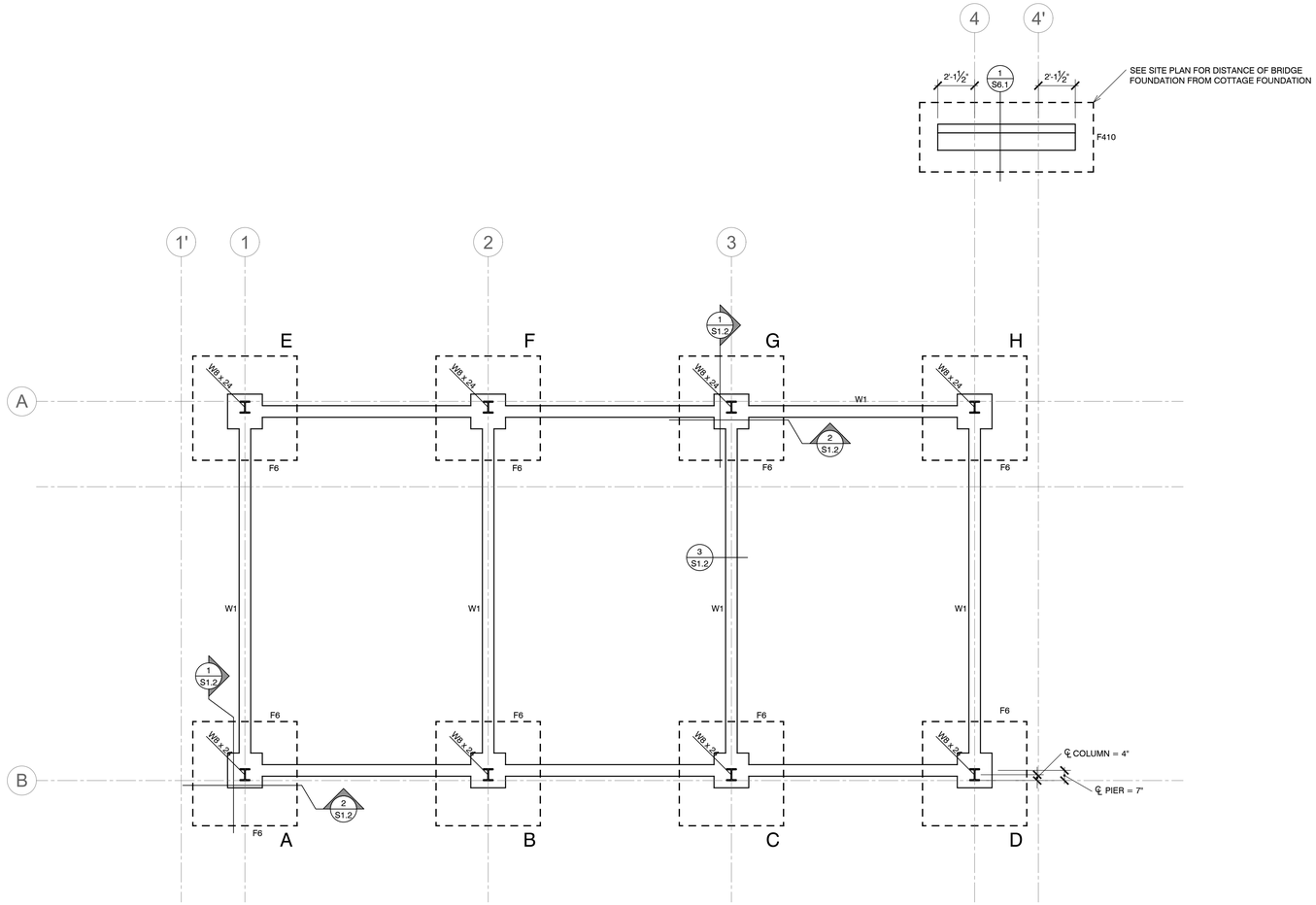
SPECIAL INSPECTION SHEET

SHEET No.
S0.3

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017

Horizon Village Finished Grade at Center of Pier - 2017.07.13

Unit Type	Pier Number	Finished Grade at Pier	Top of Pier Elevation	Bottom of Footing Elevation
1500SF	2A	8760.38	8760.50	8753.38
	2B	8758.30	8760.50	8751.30
	2C	8765.44	8766.00	8758.44
	2D	8763.22	8766.00	8756.22
	2E	8769.40	8770.00	8762.40
	2F	8768.27	8770.00	8761.27
1500SF	3A	8727.42	8728.00	8720.42
	3B	8727.58	8728.00	8720.58
	3C	8732.46	8733.00	8725.46
	3D	8732.07	8733.00	8725.07
	3E	8734.16	8734.50	8727.16
	3F	8734.15	8734.50	8727.15
2500SF	4A	8744.49	8745.00	8737.49
	4B	8744.42	8745.00	8737.42
	4C	8743.88	8745.00	8736.88
	4D	8743.81	8744.00	8736.81
	4E	8743.05	8744.00	8736.05
	4F	8750.23	8750.50	8743.23
	4G	8750.23	8750.50	8743.23
	4H	8750.23	8750.50	8743.23
	4I	8750.23	8750.50	8743.23
	4J	8750.23	8750.50	8743.23
2500SF	5A	8810.67	8811.00	8803.67
	5B	8809.82	8811.00	8802.82
	5C	8809.20	8811.00	8802.20
	5D	8807.71	8808.00	8800.71
	5E	8806.26	8806.50	8799.26
	5F	8816.22	8816.50	8809.22
	5G	8816.19	8816.50	8809.19
	5H	8816.13	8816.50	8809.13
	5I	8816.07	8816.50	8809.07
	5J	8814.80	8815.00	8807.80
1500plusSF	6A	8776.88	8777.00	8769.88
	6B	8775.53	8776.00	8768.53
	6C	8772.86	8773.00	8765.86
	6D	8769.93	8770.50	8762.93
	6E	8782.72	8783.00	8775.72
	6F	8782.62	8783.00	8775.62
	6G	8779.71	8780.00	8772.71
	6H	8776.66	8777.00	8769.66
1500SF	7A	8792.86	8793.25	8785.86
	7B	8790.27	8791.00	8783.27
	7C	8797.24	8797.75	8790.24
	7D	8795.19	8795.50	8788.19
	7E	8800.45	8801.00	8793.45
	7F	8799.47	8800.00	8792.47
2500SF	8A	8778.16	8778.75	8771.16
	8B	8777.50	8777.75	8770.50
	8C	8777.42	8777.75	8770.42
	8D	8777.01	8777.75	8770.01
	8E	8775.99	8776.50	8768.99
	8F	8785.10	8785.50	8778.10
	8G	8785.25	8785.50	8778.25
	8H	8785.24	8785.50	8778.24
	8I	8784.88	8785.50	8777.88
	8J	8784.10	8784.50	8777.10
1500plusSF	9A	8767.24	8767.50	8760.24
	9B	8766.41	8767.50	8759.41
	9C	8764.06	8764.50	8757.06
	9D	8762.41	8763.00	8755.41
	9E	8773.00	8773.50	8766.00
	9F	8772.18	8772.75	8765.18
	9G	8770.40	8771.00	8763.40
	9H	8767.71	8768.00	8760.71
1500plusSF	10A	8748.17	8748.75	8741.17
	10B	8746.78	8747.00	8739.78
	10C	8745.71	8746.00	8738.71
	10D	8745.41	8746.00	8738.41
	10E	8754.64	8755.00	8747.64
	10F	8752.94	8753.50	8745.94
	10G	8751.18	8751.50	8744.18
	10H	8750.67	8751.50	8743.67
1000SF	11A	8706.47	8707.00	8699.47
	11B	8706.12	8707.00	8699.12
	11C	8709.49	8710.00	8702.49
	11D	8709.45	8710.00	8702.45
	11E	8711.84	8712.25	8704.84
	11F	8711.71	8712.25	8704.71
1500plusSF	12A	8800.77	8801.00	8793.77
	12B	8797.93	8798.25	8790.93
	12C	8795.36	8796.00	8788.36
	12D	8793.20	8793.75	8786.20
	12E	8805.59	8806.00	8798.59
	12F	8803.88	8804.25	8796.88
	12G	8801.65	8802.00	8794.65
	12H	8799.21	8799.75	8792.21
1000SF	13A	8779.35	8780.00	8772.35
	13B	8779.70	8780.00	8772.70
	13C	8784.01	8784.50	8777.01
	13D	8783.44	8784.50	8776.44
	13E	8788.19	8788.50	8781.19
	13F	8783.19	8783.75	8776.19
1500SF	14A	8783.38	8784.00	8776.38
	14B	8780.27	8780.75	8773.27
	14C	8787.42	8788.00	8780.42
	14D	8783.88	8784.25	8776.88
	14E	8790.59	8791.00	8783.59
	14F	8787.93	8788.50	8780.93
1500SF	15A	8759.93	8760.50	8752.93
	15B	8758.21	8758.75	8751.21
	15C	8763.93	8764.50	8756.93
	15D	8761.11	8761.50	8754.11
	15E	8767.87	8768.00	8760.87
	15F	8764.89	8765.25	8757.89
1000SF	16A	8735.82	8736.50	8728.82
	16B	8735.93	8736.50	8728.93
	16C	8738.81	8739.50	8731.81
	16D	8739.00	8739.50	8732.00
	16E	8742.08	8742.50	8735.08
	16F	8742.23	8742.50	8735.23
1500plusSF	17A	8792.79	8793.25	8785.79
	17B	8792.37	8793.25	8785.37
	17C	8791.22	8792.00	8784.22
	17D	8791.59	8792.00	8784.59
	17E	8799.78	8801.00	8792.78
	17F	8800.38	8801.00	8793.38
	17G	8799.28	8799.75	8792.28
	17H	8799.26	8799.75	8792.26
1500SF	18A	8821.10	8821.50	8814.10
	18B	8816.37	8817.00	8809.37
	18C	8824.41	8825.00	8817.41
	18D	8820.85	8821.25	8813.85
	18E	8826.58	8827.00	8819.58
	18F	8825.48	8826.00	8818.48
1000SF	19A	8801.37	8802.00	8794.37
	19B	8800.49	8801.00	8793.49
	19C	8808.22	8808.75	8801.22
	19D	8807.69	8808.75	8800.69
	19E	8814.76	8815.50	8807.76
	19F	8814.40	8815.50	8807.40
1500plusSF	20A	8725.56	8726.00	8718.56
	20B	8725.58	8726.00	8718.58
	20C	8725.24	8726.00	8718.24
	20D	8724.88	8726.00	8717.88
	20E	8728.40	8728.75	8721.40
	20F	8728.27	8728.75	8721.27
	20G	8727.97	8728.75	8720.97
	20H	8727.73	8728.75	8720.73
1000SF	21A	8720.19	8720.75	8713.19
	21B	8720.17	8720.75	8713.17
	21C	8721.60	8722.00	8714.60
	21D	8721.36	8722.00	8714.36
	21E	8723.40	8723.75	8716.40
	21F	8723.05	8723.75	8716.05
1500plusSF	22A	8727.00	8727.50	8720.00
	22B	8727.07	8727.50	8720.07
	22C	8726.42	8727.00	8719.42
	22D	8724.48	8725.50	8717.48
	22E	8724.48	8730.00	8717.48
	22F	8729.50	8730.00	8722.50
	22G	8729.00	8730.00	8722.00
	22H	8727.94	8728.50	8720.94
1500plusSF	23A	8714.65	8715.00	8707.65
	23B	8714.46	8715.00	8707.46
	23C	8714.32	8715.00	8707.32
	23D	8714.16	8715.00	8707.16
	23E	8717.72	8718.00	8710.72
	23F	8716.96	8717.25	8709.96
	23G	8716.29	8717.25	8709.29
	23H	8715.40	8716.00	8708.40
2500SF	24A	8699.52	8700.00	8692.52
	24B	8698.44	8699.00	8691.44
	24C	8697.97	8699.00	8690.97
	24D	8698.02	8699.00	8691.02
	24E	8697.70	8698.50	8690.70
	24F	8704.68	8704.75	8697.68
	24G	8703.89	8704.75	8696.89
	24H	8704.20	8704.75	8697.20
	24I	8704.13	8704.75	8697.13
	24J	8703.20	8704.00	8696.20
1500SF	25A	8717.42	8718.00	8710.42
	25B	8717.41	8718.00	8710.41
	25C	8719.32	8720.00	8712.32
	25D	8719.02	8720.00	8712.02
	25E	8722.75	8723.00	8715.75
	25F	8722.01	8723.00	8715.01
1000SF	26A	8687.97	8688.25	8680.97
	26B	8687.27	8688.25	8680.27
	26C	8691.80	8692.25	8684.80
	26D	8691.19	8692.25	8684.19
	26E	8694.67	8695.00	8687.67
	26F	8694.27	8695.00	8687.27
1000SF	27A	8708.73	8709.25	8701.73
	27B	8708.19	8709.25	8701.19
	27C	8710.75	8711.25	8703.75
	27D	8709.91	8710.50	8702.91
	27E	8713.16	8713.75	8706.16
	27F	8711.71	8712.25	8704.71
1500plusSF	28A	8750.80	8751.25	8743.80
	28B	8750.63	8751.25	8743.63
	28C	8750.73	8751.25	8743.73
	28D	8750.37	8751.25	8743.37
	28E	8756.11	8757.00	8749.11
	28F	8756.21	8757.00	8749.21
	28G	8756.81	8757.00	8749.81
	28H	8756.71	8757.00	8749.71
1000SF	29A	8743.44	8744.00	8736.44
	29B	8744.43	8745.00	8737.43
	29C	8746.72	8747.75	8739.72
	29D	8747.38	8747.75	8740.38
	29E	8750.93	8751.50	8743.93
	29F	8750.96	8751.50	8743.96
1500SF	30A	8731.96	8732.50	8724.96
	30B	8731.82	8732.50	8724.82
	30C	8734.89	8735.50	8727.89
	30D	8735.08	8735.50	8728.08
	30E	8738.23	8738.75	8731.23
	30F	8738.20	8738.75	8731.20
2500SF	31A	8740.17	8740.75	8733.17
	31B	8740.32	8740.75	8733.32
	31C	8741.13	8741.75	8734.13
	31D	8741.26	8741.75	8734.26
	31E	8741.71	8742.25	8734.71
	31F	8745.58	8746.50	8738.58
	31G	8745.79	8746.50	8738.79
	31H	8746.17	8746.50	8739.17
	31I	8746.60	8747.25	8739.60
	31J	8747.38	8747.75	8740.38



FOUNDATION SCHEDULE

MARK	WALL WIDTH	WALL HEIGHT	VERT. REINFORCING SIZE	VERT. REINFORCING SPACING	HORIZ. REINFORCING SIZE	HORIZ. REINFORCING SPACING
* W1	10"	VARIES	#5	12" O.C.	#5	12" O.C.

PLACE (2) # 4 HORIZ. BARS AT TOP & BOTTOM OF WALL CONTINUOUS, TYPICAL.
RECESS TOP OF WALL AT OPENINGS & POUR SLAB THROUGH, SEE DETAILS

* THIS WALL REQUIRES (2) MATS OF REINFORCING (1) MAT 2" OFF EA. FACE AS SPECIFIED ABOVE

FOOTING SCHEDULE

MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSS-WISE No.	SIZE	LENGTH	SPACING	REINFORCING LENGTH-WISE No.	SIZE	LENGTH	SPACING
F6	6'-0"	6'-0"	14"	(7)	#5	5'-6"	EQUAL	(7)	#5	5'-6"	EQUAL
F410	4'-0"	10'-0"	12"	(11)	#5	3'-6"	EQUAL	(5)	#5	9'-6"	EQUAL

DYNAMIC STRUCTURES
1887 NORTH 1120 WEST PROVO, UTAH 84604
PH: (801)356-1140 FAX: (801)356-0001

Structural Plans for:

POWDER MOUNTAIN CABIN 1500+

PROFESSIONAL STRUCTURAL ENGINEER
NO. 190917
JAY D. ADAMS
STATE OF UTAH
9/22/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	1/4" = 1'-0"
DATE:	JULY 28, 2017
JOB No.	17-089

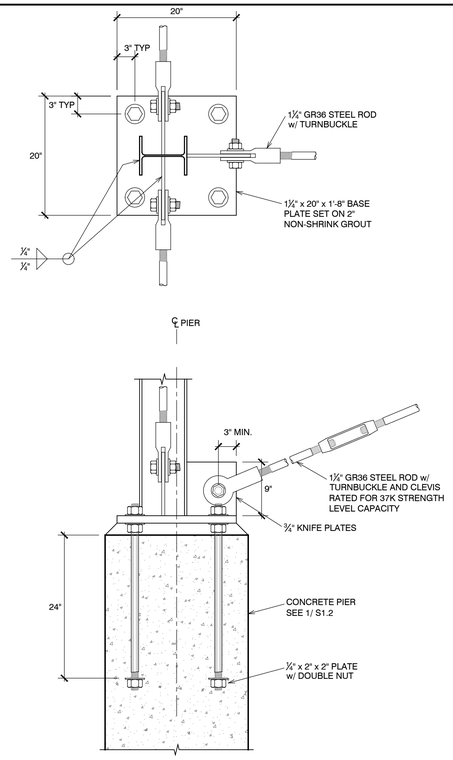
FOUNDATION PLAN

SHEET No.

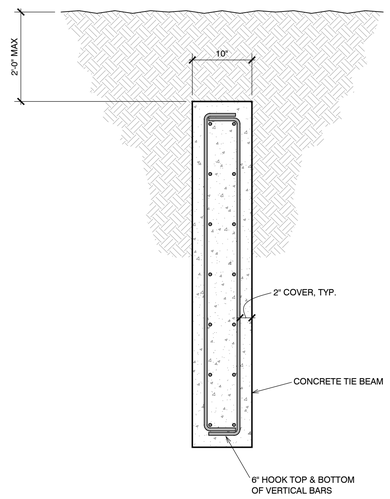
S1.1

2 PLAN REVIEW-09/22/2017

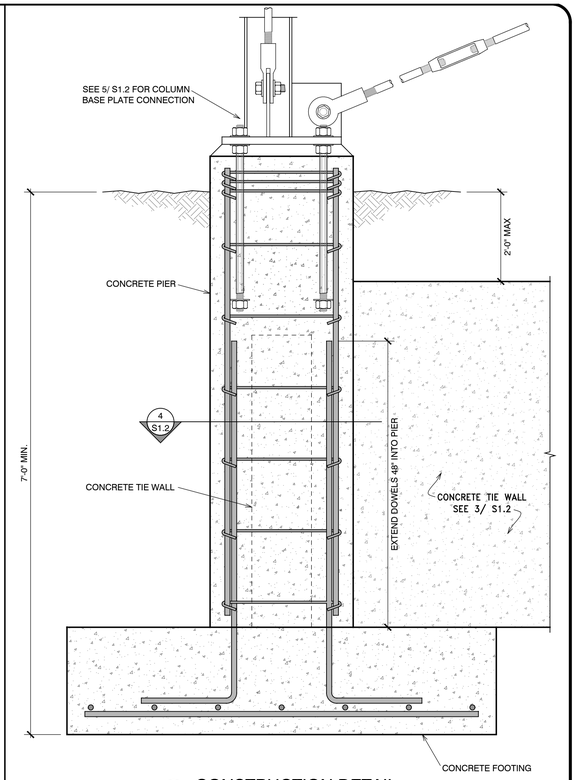
1 PLAN REVIEW-09/05/2017



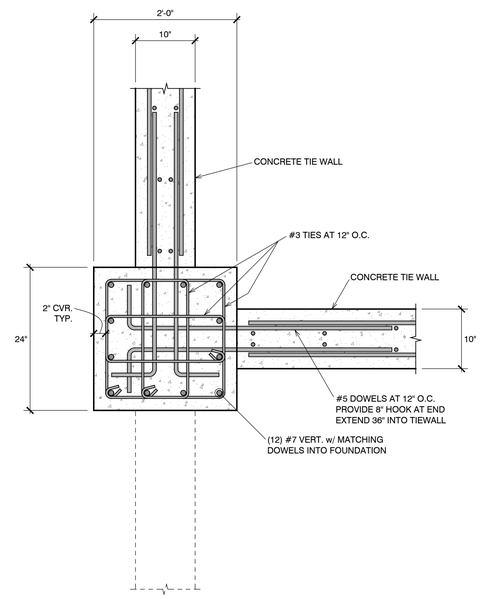
5 S1.2 CONSTRUCTION DETAIL
NO SCALE



3 S1.2 CONSTRUCTION DETAIL
NO SCALE



1 S1.2 CONSTRUCTION DETAIL
NO SCALE



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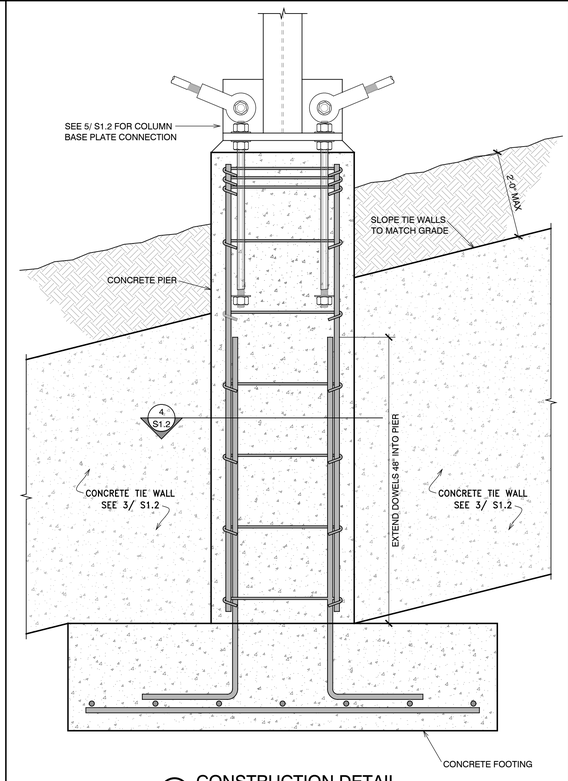
NO SCALE



2 S1.2 CONSTRUCTION DETAIL
NO SCALE

DYNAMIC STRUCTURES
1887 NORTH 1120 WEST PROVO, UTAH 84604
PH: (801)356-1140 FAX: (801)356-0001

Structural Plans for:

POWDER MOUNTAIN CABIN 1500+

PROFESSIONAL STRUCTURAL ENGINEER
NO. 190917
JAY D. ADAMS
STATE OF UTAH
9/22/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	AS SHOWN
DATE:	JULY 28, 2017
JOB No.	17-089

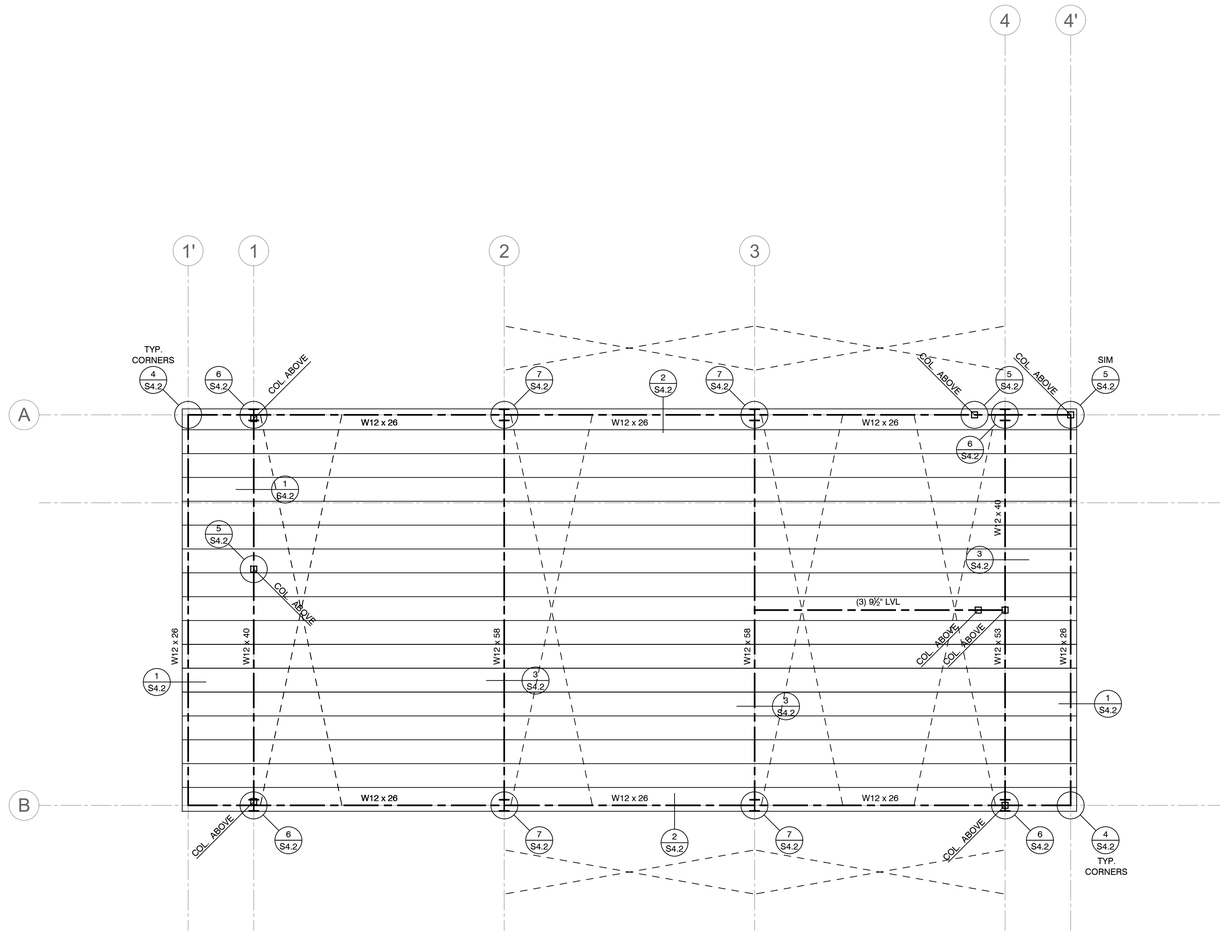
CONSTRUCTION DETAILS

SHEET No.

S1.2

2 PLAN REVIEW-09/22/2017

1 PLAN REVIEW-09/05/2017



FLOOR FRAMING NOTES

1. FRAME FLOOR w/ 9½" TJI/210 AT 16" O.C.
   PROVIDE 3" CONCRETE TOPPING ON FLOOR WHERE INDICATED IN ARCH. PLANS
2. SEE FRAMING NOTES ON S0.1 FOR FLOOR SHEATHING SPECIFICATIONS
3. REPRESENTS 1⅛" ROD CROSS BRACING BETWEEN FOUNDATION AND MAIN LEVEL FRAMING

DYNAMIC STRUCTURES
1887 NORTH 1120 WEST PROVO, UTAH 84604
PH: (801)356-1140 FAX: (801)356-0001

Structural Plans for:

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9/22/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	1/4" = 1'-0"
DATE:	JULY 28, 2017
JOB No.	17-089

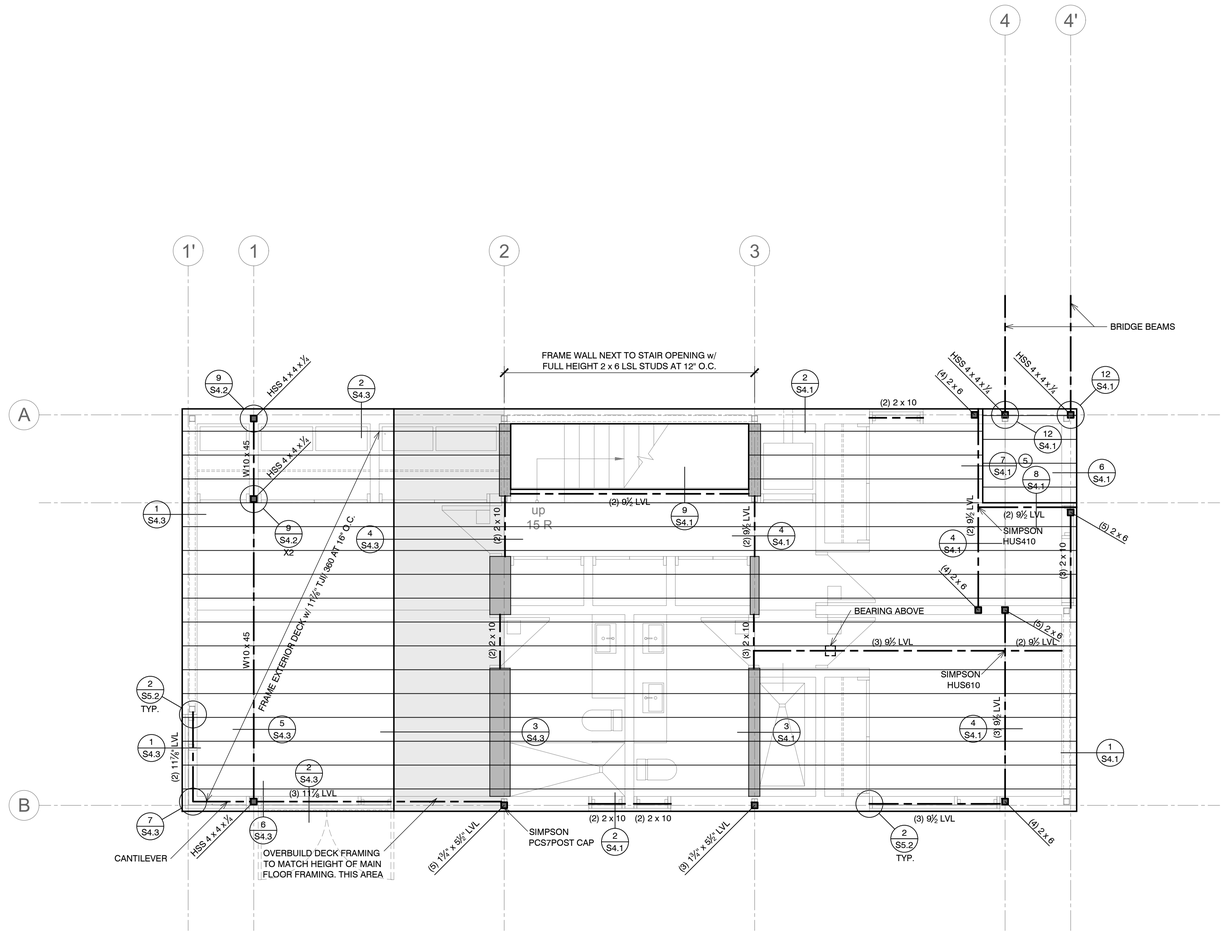
MAIN FLOOR FRAMING PLAN

SHEET No.

S2.1

2 PLAN REVIEW-09/22/2017

1 PLAN REVIEW-09/05/2017



DEAD LOAD= 50 PSF
LIVE LOAD =40 PSF

FLOOR FRAMING NOTES

1. FRAME FLOOR w/ 9½" TJI/210 AT 16" O.C.
   PROVIDE 3" CONCRETE TOPPING ON FLOOR WHERE INDICATED IN ARCH. PLANS
2. SEE FRAMING NOTES ON S0.1 FOR FLOOR SHEATHING SPECIFICATIONS
3. ▭ INDICATES INTERIOR BEARING WALL
4. FRAME EXTERIOR WALLS AND BEARING WALLS w/ 2 x 6 AT 16" O.C.
5. FRAME ENTRY FLOOR w/ 2 x 8 AT 16" O.C.

DYNAMIC STRUCTURES
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Structural Plans for:
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STATE OF UTAH
9/22/17

DESIGNED BY:	J.D.A.
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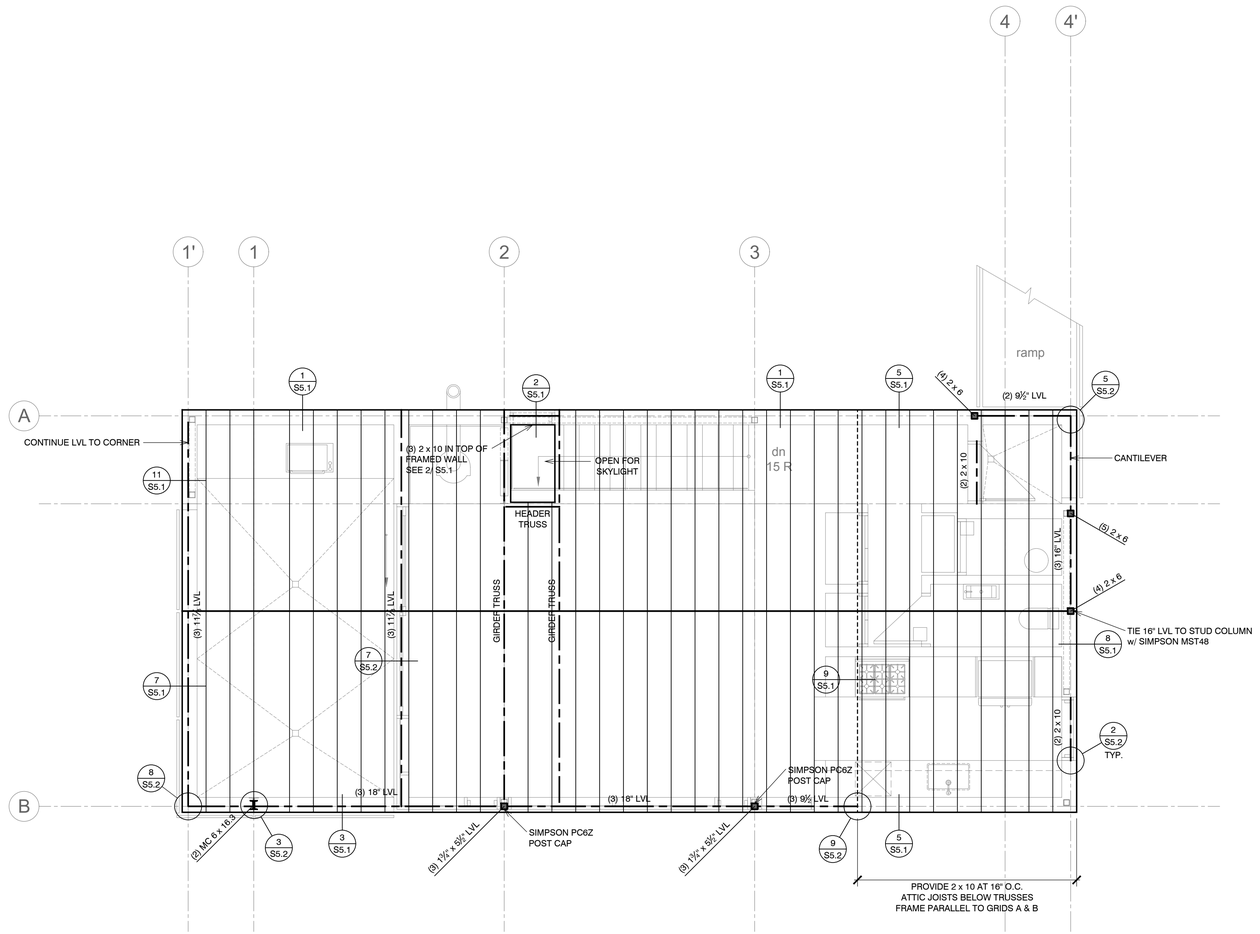
SCALE:	1/4" = 1'-0"
DATE:	JULY 28, 2017
JOB No.	17-089

UPPER FLOOR FRAMING PLAN

SHEET No.
S2.2

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017



ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

DEAD LOAD= 15 PSF
SNOW LOAD =192 PSF

ROOF FRAMING NOTES

1. FRAME ROOF w/ PRE-ENGINEERED ROOF TRUSSES AT 16" O.C.
2. SEE FRAMING NOTES ON S0.1 FOR ROOF SHEATHING SPECIFICATIONS
3. FRAME EXTERIOR WALLS w/ 2 x 6 AT 16" O.C.

DYNAMIC STRUCTURES
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Structural Plans for:
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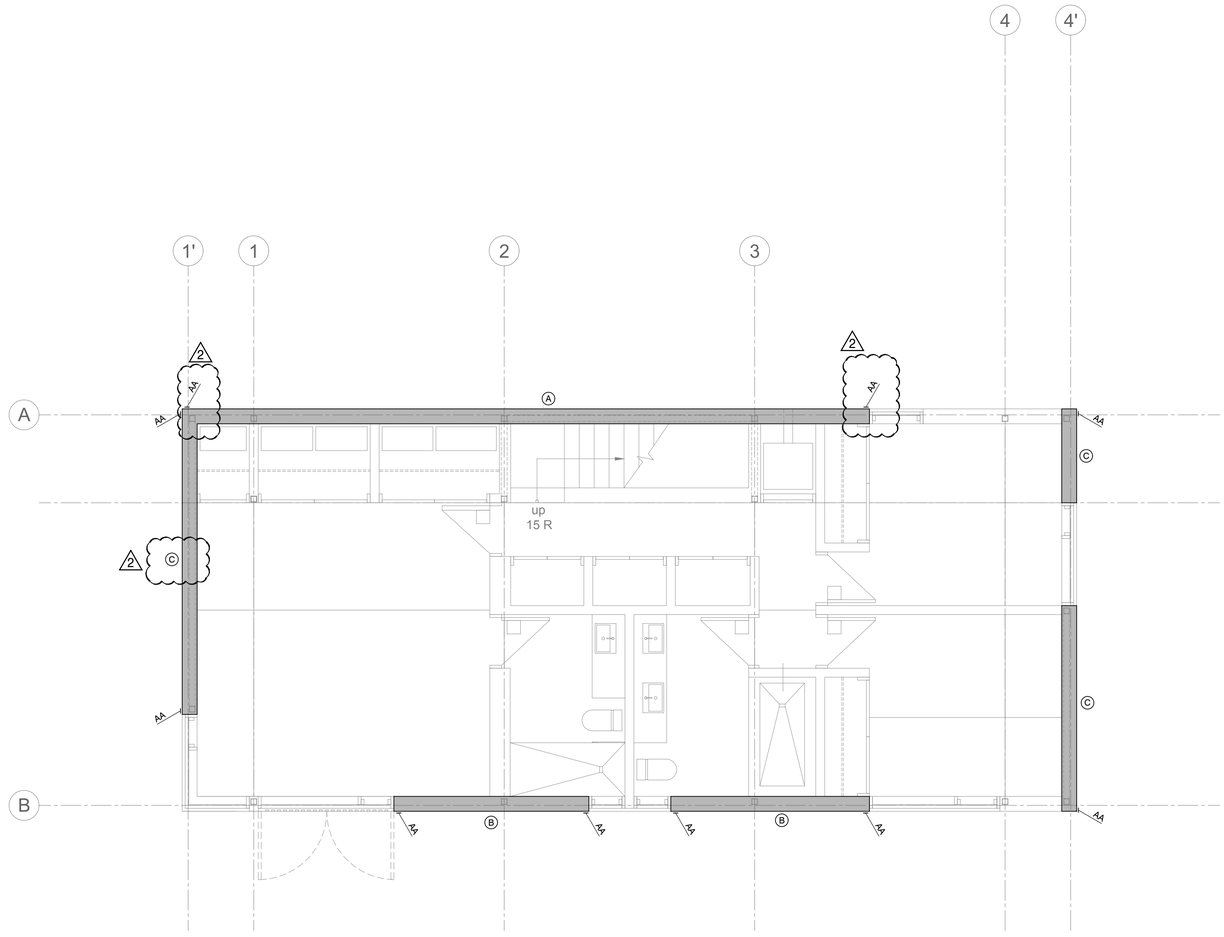
ROOF FRAMING PLAN

SHEET No.
S2.3

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017



SHEARWALL SCHEDULE

MARK	PANEL GRADE	PANEL THICKNESS	PANEL EDGE NAILING	PANEL FIELD NAILING	STUDS AT ADJOINING PANEL EDGES	ANCHOR BOLTS AT FOUNDATION LEVEL	SILL PLATE AT FOUNDATION
Ⓐ	APA EXP. 1	7/16"	8d AT 6" O.C.	8d AT 12" O.C.	2x	⅝"dia. x 10" AT 32" O.C.	2x TREATED
Ⓑ	APA EXP. 1	7/16"	8d AT 4" O.C.	8d AT 12" O.C.	2x	⅝"dia. x 10" AT 32" O.C.	2x TREATED
Ⓒ	APA EXP. 1	7/16"	8d AT 3" O.C.	8d AT 12" O.C.	2x	⅝"dia. x 10" AT 32" O.C.	2x TREATED
Ⓓ	APA EXP. 1	7/16"	8d AT 2" O.C.	8d AT 12" O.C.	3x	⅝"dia. x 10" AT 16" O.C.	2x TREATED

1. SEE GENERAL NOTES FOR ADDITIONAL INFORMATION
2. PLYWOOD, ORIENTED STRAND BOARD AND COMPOSITE BOARD (BUT NOT STRUCTURAL PARTICLE BOARD) ARE ACCEPTED AS EQUALS
3. ALL PANEL EDGES AT SHEAR WALLS SHALL BE BACKED WITH 2" NOMINAL FRAMING, EXCEPT WHERE INDICATED TO BE 3" NOMINAL ON SCHEDULE. 3x MATERIAL MAY BE REPLACED WITH 4x MATERIAL. MULTIPLE LAYERS OF 2x FRAMING SHALL NOT BE USED WHERE 3x FRAMING IS INDICATED.
4. ALL ANCHOR BOLTS TO HAVE A 3" x 3" x ¼" PLATE WASHER (SEE SEE SCHEDULE ABOVE FOR SPACING)
5. ALL STUDS IN SHEAR WALLS SHALL BE DOUGLAS FIR-LARCH
6. SHEAR WALL PANELS INDICATED ON SCHEDULE ARE TO BE SHEATHED FOR FULL HEIGHT OF THE WALL.
7. SEE SPECIAL INSPECTION PAGE FOR ADDITIONAL REQUIREMENTS
8. WHERE PANELS ARE APPLIED ON BOTH FACES OF A SHEAR WALL AND NAIL SPACING IS LESS THAN 6" ON CENTER ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS. ALTERNATIVELY, THE WIDTH OF THE NAILED FACE OF FRAMING MEMBERS SHALL BE 3" NOMINAL OR GREATER AT ADJOINING PANEL EDGES AND NAILS AT ALL PANEL EDGES SHALL BE STAGGERED.

HOLD DOWN SCHEDULE

MARK	HOLDOWN	ATTACHMENT TO STUDS	FOUNDATION ANCHORS	MINIMUM STUDS	REMARKS
AA	SIMPSON MST48	(34) 16d SINKERS	N. A.	(2) 2x	SEE DETAILS ON S3.3
BB	SIMPSON MST72	(62) 16d SINKERS	N. A.	(2) 2x	SEE DETAILS ON S3.3

1. ALL ANCHORS ARE SIMPSON STRONG-TIE. (OR EQUAL)
2. INSTALLATION OF ALL HOLDOWN ANCHORS AND STRAPS SHALL BE PER MANUFACTURES RECOMMENDATIONS AND SPECIFICATIONS
3. PROVIDE EDGE NAILING ALONG STUDS CONNECTED TO HOLDOWN ANCHORS AND STRAPS
4. SEE SPECIAL INSPECTION PAGE FOR ADDITIONAL REQUIREMENTS

DYNAMIC STRUCTURES
1887 NORTH 1120 WEST PROVO, UTAH 84604
PH: (801)356-1140 FAX: (801)356-0001

Structural Plans for:
POWDER MOUNTAIN CABIN 1500+

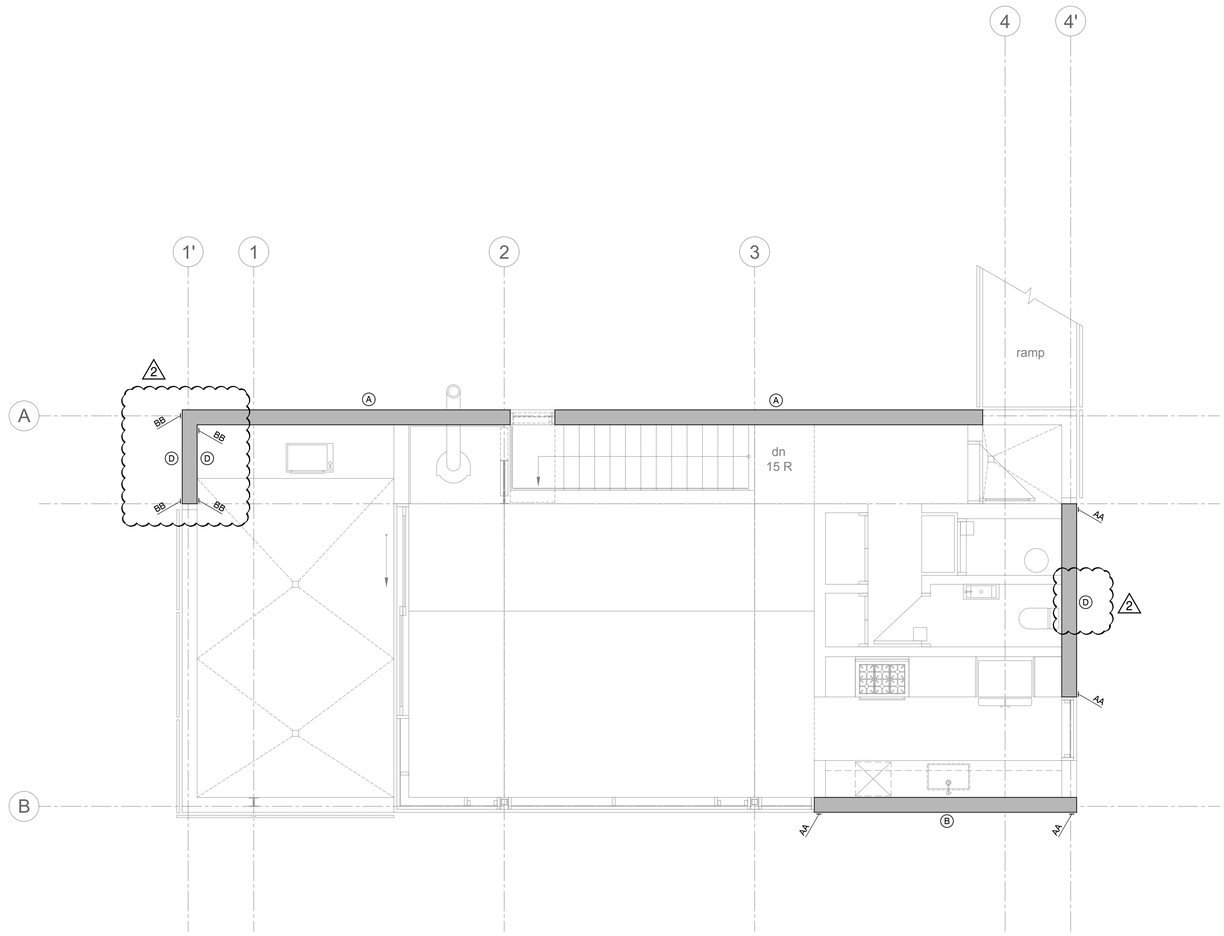
PROFESSIONAL STRUCTURAL ENGINEER
NO. 190917
JAY D. ADAMS
STATE OF UTAH
9/22/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	1/4" = 1'-0"
DATE:	JULY 28, 2017
JOB No.	17-089

MAIN FLOOR SHEARWALL PLAN

SHEET No.
S3.1

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017



SHEARWALL SCHEDULE

MARK	PANEL GRADE	PANEL THICKNESS	PANEL EDGE NAILING	PANEL FIELD NAILING	STUDS AT ADJOINING PANEL EDGES	ANCHOR BOLTS AT FOUNDATION LEVEL	SILL PLATE AT FOUNDATION
Ⓐ	APA EXP. 1	7/16"	8d AT 6" O.C.	8d AT 12" O.C.	2x	⅝"dia. x 10" AT 32" O.C.	2x TREATED
Ⓑ	APA EXP. 1	7/16"	8d AT 4" O.C.	8d AT 12" O.C.	2x	⅝"dia. x 10" AT 32" O.C.	2x TREATED
Ⓒ	APA EXP. 1	7/16"	8d AT 3" O.C.	8d AT 12" O.C.	2x	⅝"dia. x 10" AT 32" O.C.	2x TREATED
Ⓓ	APA EXP. 1	7/16"	8d AT 2" O.C.	8d AT 12" O.C.	3x	⅝"dia. x 10" AT 16" O.C.	2x TREATED

1. SEE GENERAL NOTES FOR ADDITIONAL INFORMATION
2. PLYWOOD, ORIENTED STRAND BOARD AND COMPOSITE BOARD (BUT NOT STRUCTURAL PARTICLE BOARD) ARE ACCEPTED AS EQUALS
3. ALL PANEL EDGES AT SHEAR WALLS SHALL BE BACKED WITH 2" NOMINAL FRAMING, EXCEPT WHERE INDICATED TO BE 3" NOMINAL ON SCHEDULE. 3x MATERIAL MAY BE REPLACED WITH 4x MATERIAL. MULTIPLE LAYERS OF 2x FRAMING SHALL NOT BE USED WHERE 3x FRAMING IS INDICATED.
4. ALL ANCHOR BOLTS TO HAVE A 3" x 3" x ¼" PLATE WASHER (SEE SEE SCHEDULE ABOVE FOR SPACING)
5. ALL STUDS IN SHEAR WALLS SHALL BE DOUGLAS FIR-LARCH
6. SHEAR WALL PANELS INDICATED ON SCHEDULE ARE TO BE SHEATHED FOR FULL HEIGHT OF THE WALL.
7. SEE SPECIAL INSPECTION PAGE FOR ADDITIONAL REQUIREMENTS
8. WHERE PANELS ARE APPLIED ON BOTH FACES OF A SHEAR WALL AND NAIL SPACING IS LESS THAN 6" ON CENTER ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS. ALTERNATIVELY, THE WIDTH OF THE NAILED FACE OF FRAMING MEMBERS SHALL BE 3" NOMINAL OR GREATER AT ADJOINING PANEL EDGES AND NAILS AT ALL PANEL EDGES SHALL BE STAGGERED.

HOLD DOWN SCHEDULE

MARK	HOLDOWN	ATTACHMENT TO STUDS	FOUNDATION ANCHORS	MINIMUM STUDS	REMARKS
AA	SIMPSON MST48	(34) 16d SINKERS	N. A.	(2) 2x	SEE DETAILS ON S3.3
BB	SIMPSON MST72	(62) 16d SINKERS	N. A.	(2) 2x	SEE DETAILS ON S3.3

1. ALL ANCHORS ARE SIMPSON STRONG-TIE. (OR EQUAL)
2. INSTALLATION OF ALL HOLDOWN ANCHORS AND STRAPS SHALL BE PER MANUFACTURES RECOMMENDATIONS AND SPECIFICATIONS
3. PROVIDE EDGE NAILING ALONG STUDS CONNECTED TO HOLDOWN ANCHORS AND STRAPS
4. SEE SPECIAL INSPECTION PAGE FOR ADDITIONAL REQUIREMENTS

DYNAMIC STRUCTURES
1887 NORTH 1120 WEST PROVO, UTAH 84604
PH: (801)356-1140 FAX: (801)356-0001

Structural Plans for:
POWDER MOUNTAIN CABIN 1500+

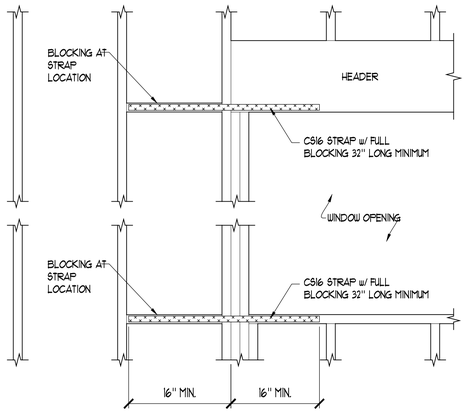
PROFESSIONAL STRUCTURAL ENGINEER
NO. 190917
JAY D. ADAMS
STATE OF UTAH
10/22/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	1/4" = 1'-0"
DATE:	JULY 28, 2017
JOB No.	17-089

UPPER FLOOR SHEARWALL PLAN

SHEET No.
S3.2

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017



4 S3.3 CONSTRUCTION DETAIL
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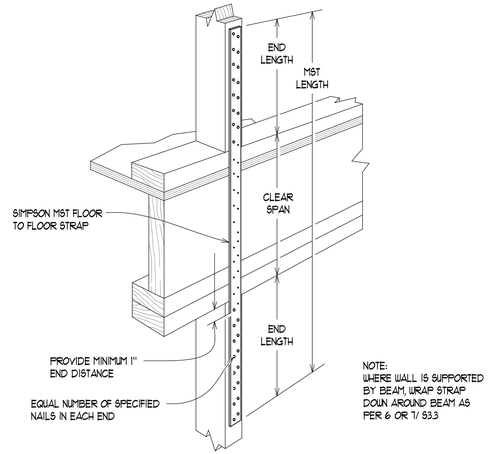

1 S3.3 CONSTRUCTION DETAIL
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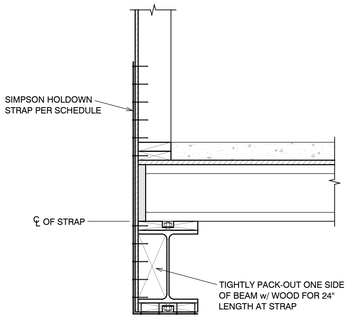


2 S3.3 CONSTRUCTION DETAIL
NO SCALE

NOT USED

3 S3.3 CONSTRUCTION DETAIL
NO SCALE

DYNAMIC STRUCTURES
1887 NORTH 1120 WEST PROVO, UTAH 84604
PH: (801)356-1140 FAX: (801)356-0001

Structural Plans for:
POWDER MOUNTAIN CABIN 1500+

PROFESSIONAL STRUCTURAL ENGINEER
NO. 190917
JAY D. ADAMS
STATE OF UTAH
9/22/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	AS SHOWN
DATE:	JULY 28, 2017
JOB No.	17-089

CONSTRUCTION DETAILS

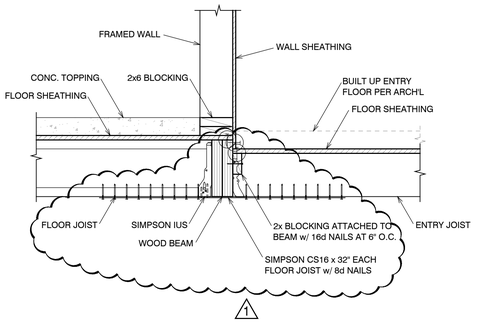
SHEET No.
S3.3

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017

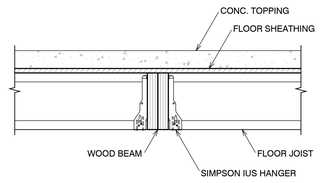
NOT USED

10 S4.1 CONSTRUCTION DETAIL
NO SCALE

= DIAPHRAGM EDGE NAILING SEE FRAMING NOTES
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN

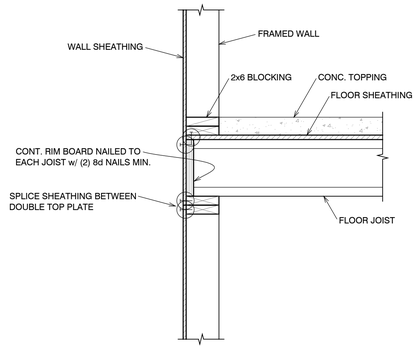


7 S4.1 CONSTRUCTION DETAIL
NO SCALE



4 S4.1 CONSTRUCTION DETAIL
NO SCALE

= DIAPHRAGM EDGE NAILING SEE FRAMING NOTES
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN

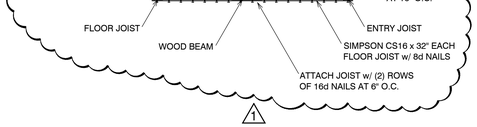


1 S4.1 CONSTRUCTION DETAIL
NO SCALE

NOT USED

11 S4.1 CONSTRUCTION DETAIL
NO SCALE

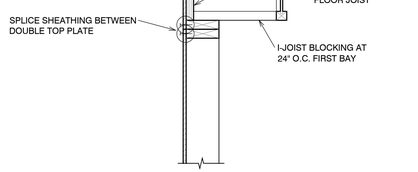
= DIAPHRAGM EDGE NAILING SEE FRAMING NOTES
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



8 S4.1 CONSTRUCTION DETAIL
NO SCALE

5 S4.1 CONSTRUCTION DETAIL
NO SCALE

= DIAPHRAGM EDGE NAILING SEE FRAMING NOTES
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



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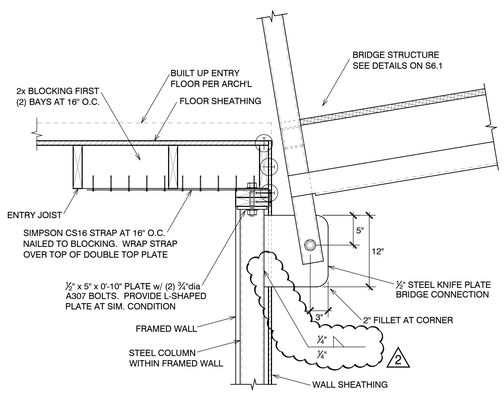
= DIAPHRAGM EDGE NAILING SEE FRAMING NOTES
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



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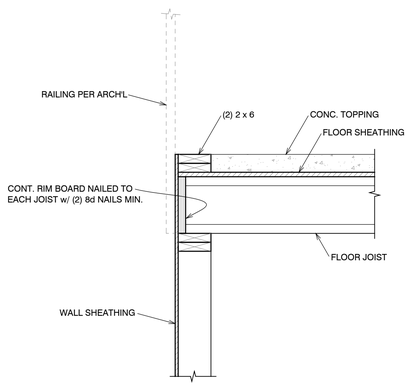

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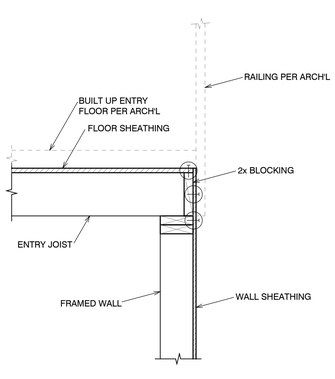
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



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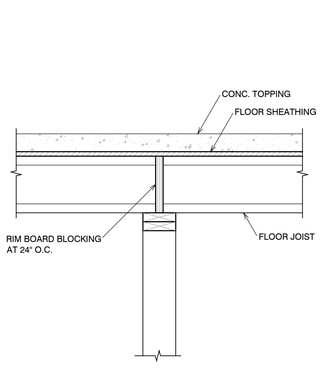


3 S4.1 CONSTRUCTION DETAIL
NO SCALE

DYNAMIC STRUCTURES
1887 NORTH 1120 WEST PROVO, UTAH 84604
PH: (801)356-1140 FAX: (801)356-0001

Structural Plans for:
POWDER MOUNTAIN CABIN 1500+

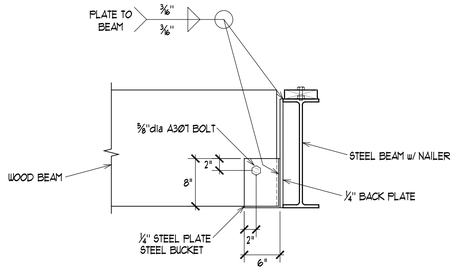
PROFESSIONAL STRUCTURAL ENGINEER
NO. 190917
JAY D. ADAMS
STATE OF UTAH
9/22/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	AS SHOWN
DATE:	JULY 28, 2017
JOB No.	17-089

CONSTRUCTION DETAILS

SHEET No.
S4.1

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017



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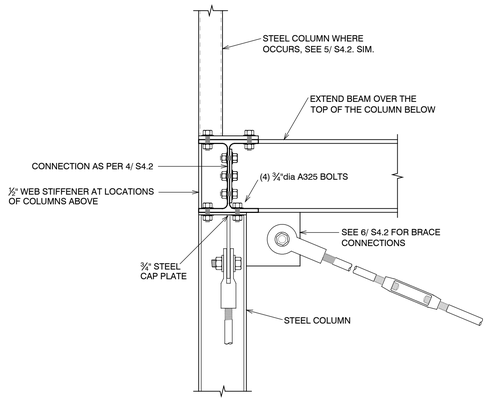

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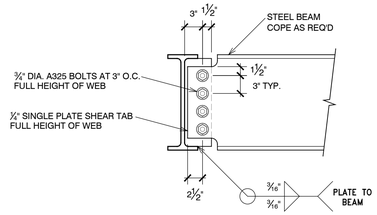

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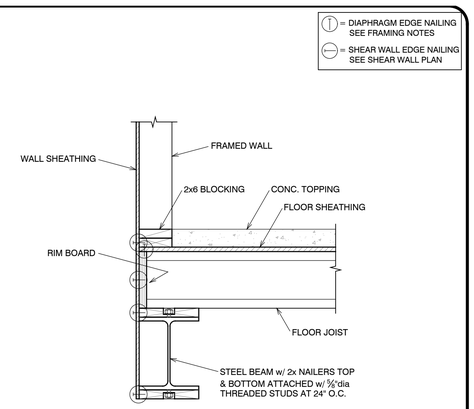


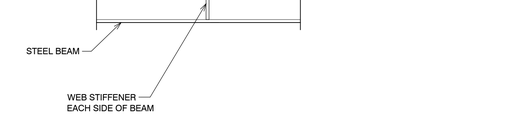
1 S4.2 CONSTRUCTION DETAIL
NO SCALE

NOT USED

11 S4.2 CONSTRUCTION DETAIL
NO SCALE

NOT USED

8 S4.2 CONSTRUCTION DETAIL
NO SCALE



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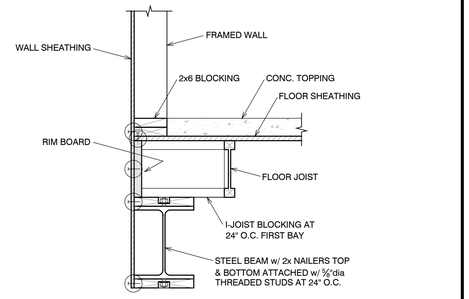
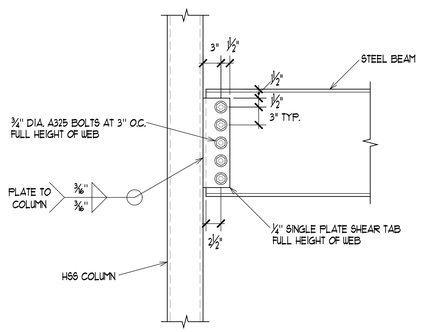


2 S4.2 CONSTRUCTION DETAIL
NO SCALE

NOT USED

12 S4.2 CONSTRUCTION DETAIL
NO SCALE



NOTE:
SKEW CONNECTION AS REQUIRED

9 S4.2 CONSTRUCTION DETAIL
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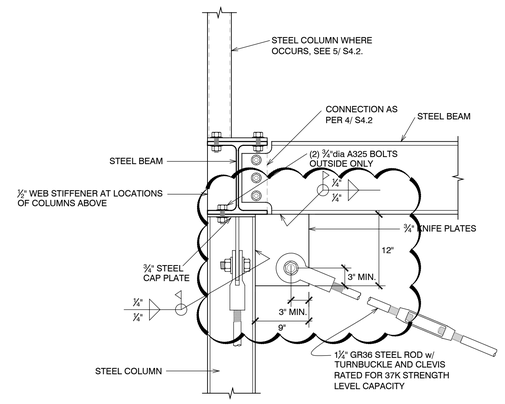

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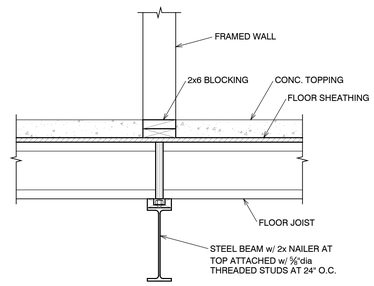

3 S4.2 CONSTRUCTION DETAIL
NO SCALE

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CHECKED BY:	J.D.A.
SCALE:	AS SHOWN
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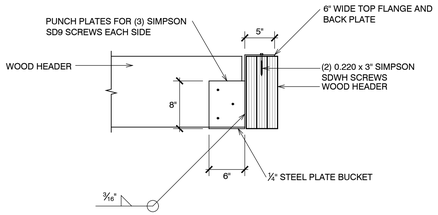
JOB No.	17-089

CONSTRUCTION DETAILS

SHEET No.
S4.2

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017



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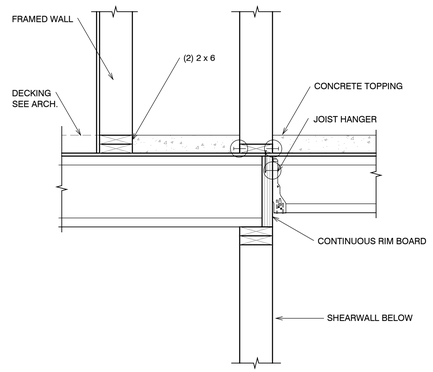

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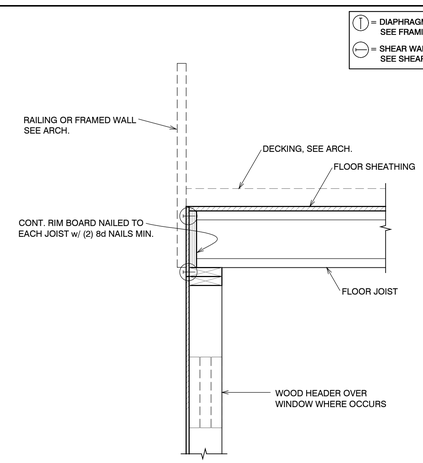

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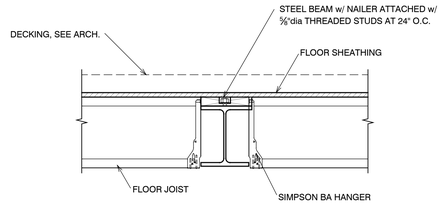

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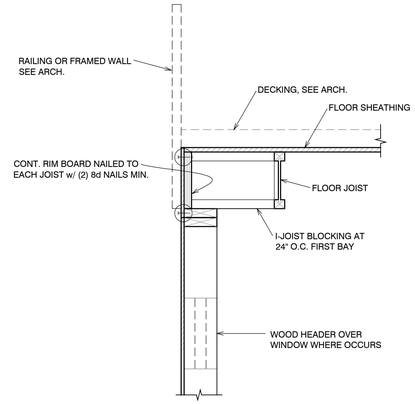

2 S4.3 CONSTRUCTION DETAIL
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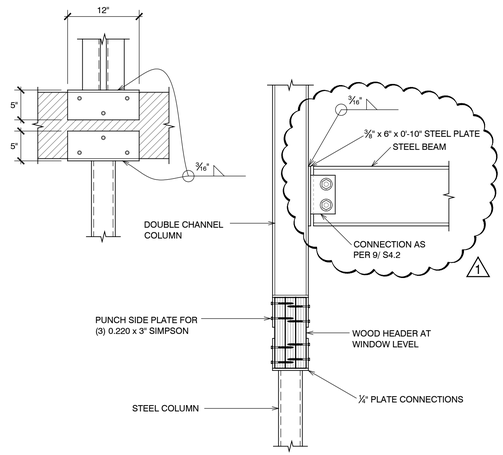

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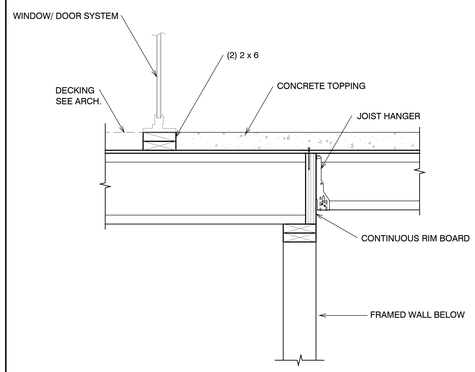

3 S4.3 CONSTRUCTION DETAIL
NO SCALE

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CONSTRUCTION DETAILS

SHEET No.
S4.3

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017

NOT USED

10 S5.1 CONSTRUCTION DETAIL
NO SCALE

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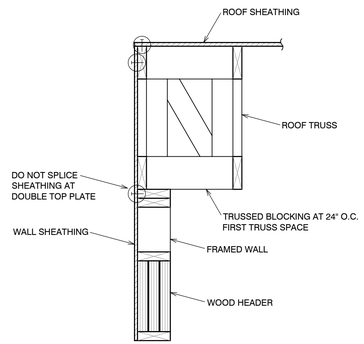
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



7 S5.1 CONSTRUCTION DETAIL
NO SCALE

NOT USED

4 S5.1 CONSTRUCTION DETAIL
NO SCALE

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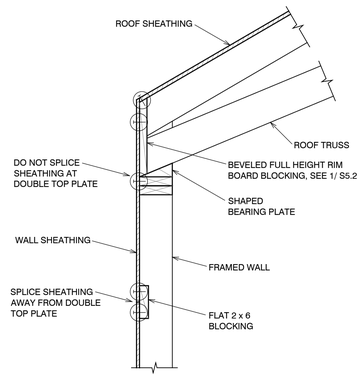
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



1 S5.1 CONSTRUCTION DETAIL
NO SCALE

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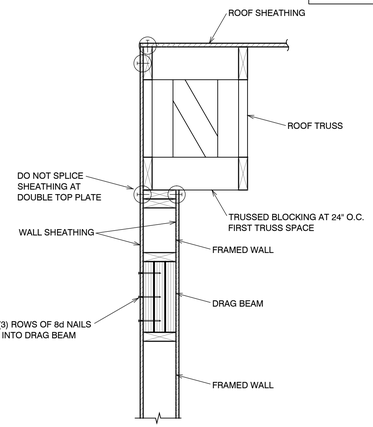
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



11 S5.1 CONSTRUCTION DETAIL
NO SCALE

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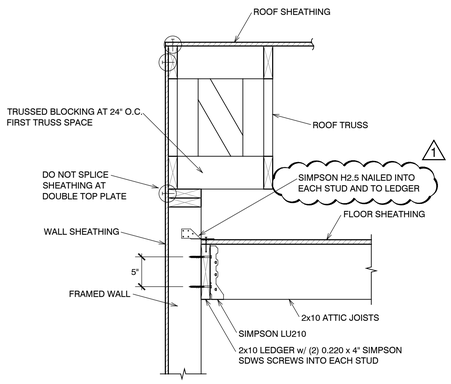
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



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NO SCALE

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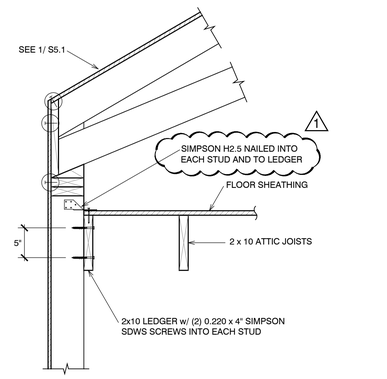
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



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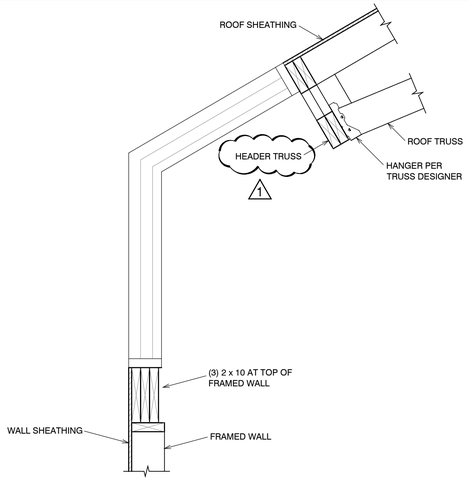
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



2 S5.1 CONSTRUCTION DETAIL
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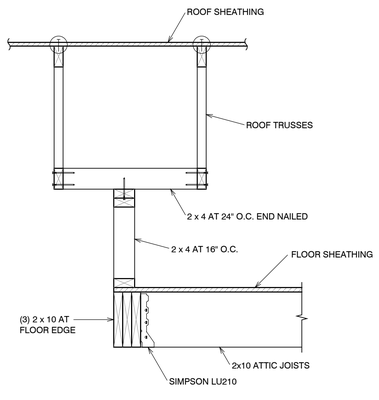
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



9 S5.1 CONSTRUCTION DETAIL
NO SCALE

NOT USED

6 S5.1 CONSTRUCTION DETAIL
NO SCALE

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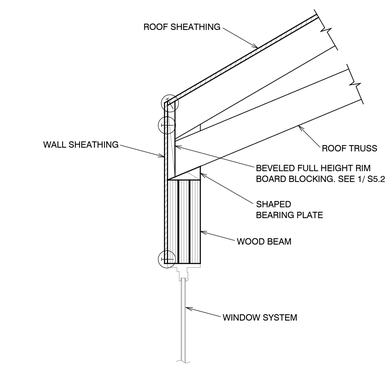
= SHEAR WALL EDGE NAILING SEE SHEAR WALL PLAN



3 S5.1 CONSTRUCTION DETAIL
NO SCALE

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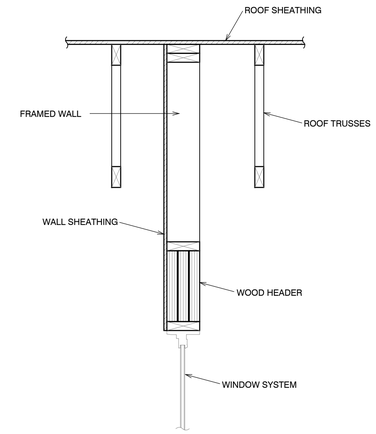
NO. 190917
JAY D. ADAMS
STATE OF UTAH
9/22/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	AS SHOWN
DATE:	JULY 28, 2017
JOB No.	17-089

CONSTRUCTION DETAILS

SHEET No.
S5.1

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017



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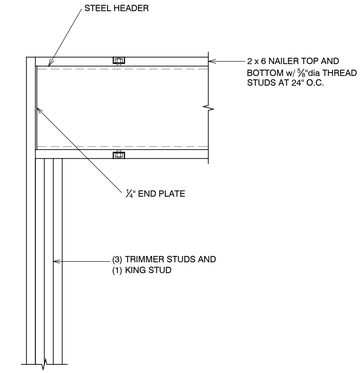

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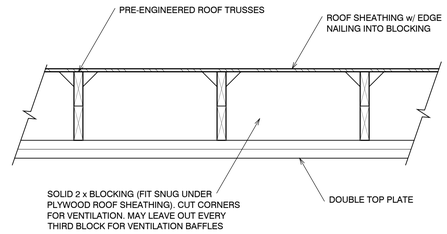

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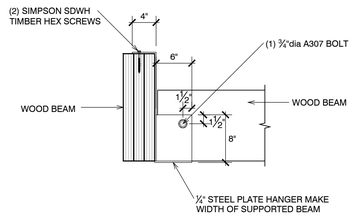

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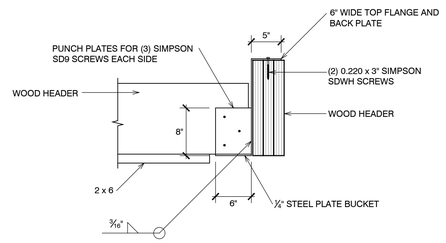

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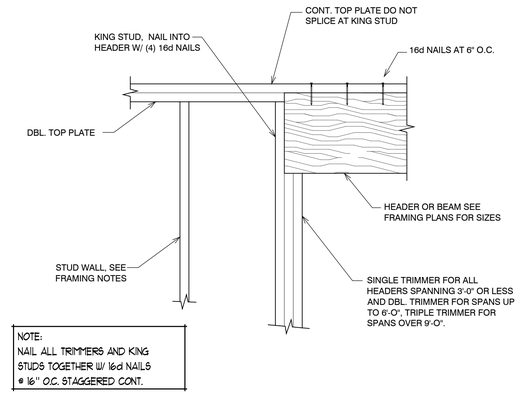

2 S5.2 CONSTRUCTION DETAIL
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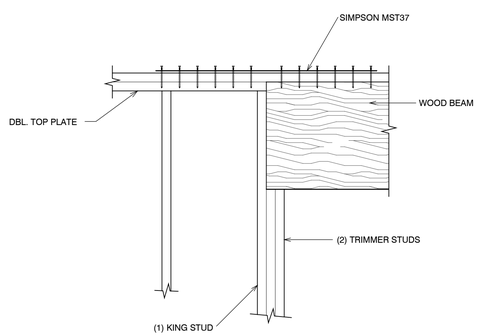

9 S5.2 CONSTRUCTION DETAIL
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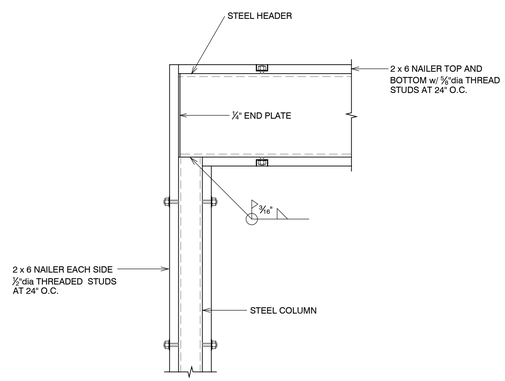

6 S5.2 CONSTRUCTION DETAIL
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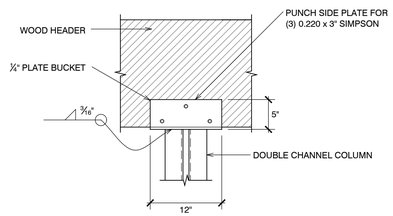

3 S5.2 CONSTRUCTION DETAIL
NO SCALE

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DESIGNED BY:	J.D.A.
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SCALE:	AS SHOWN
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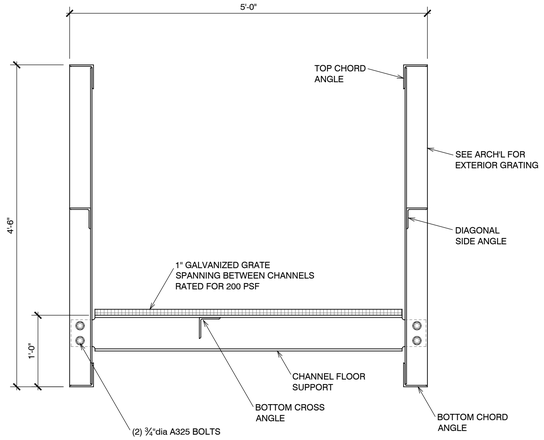
JOB No.	17-089

CONSTRUCTION DETAILS

SHEET No.
S5.2

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017



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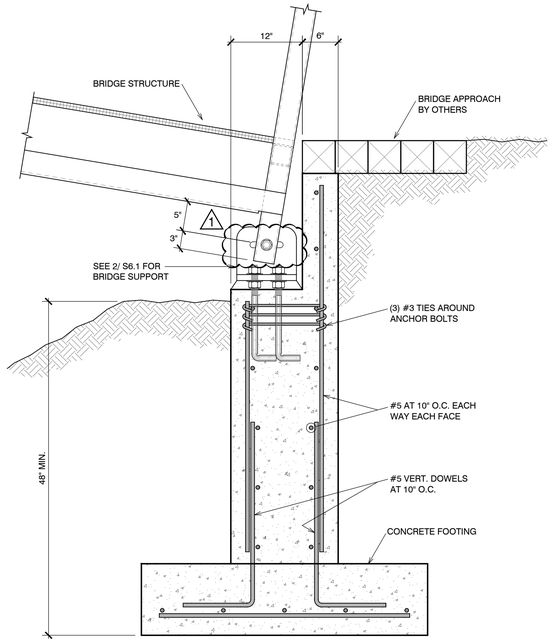
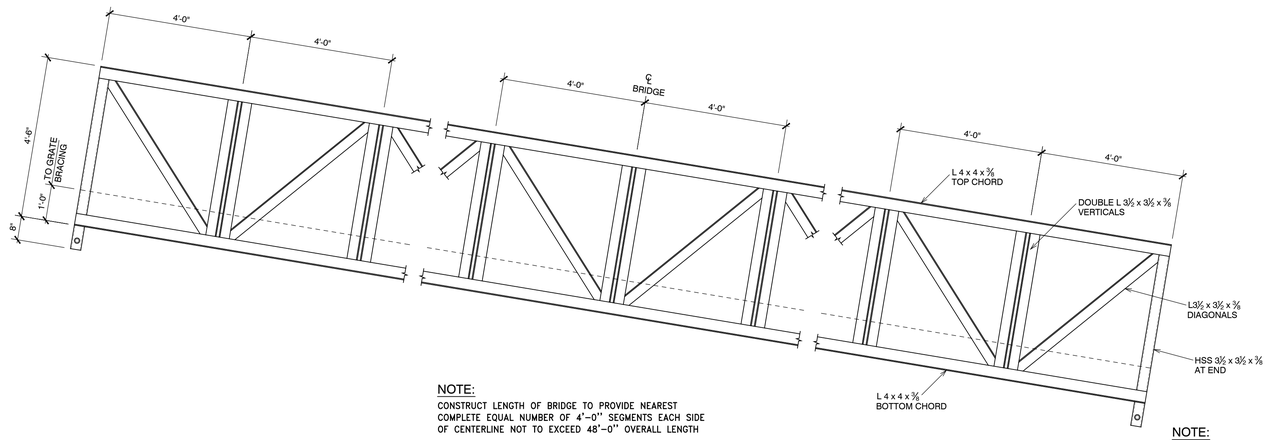


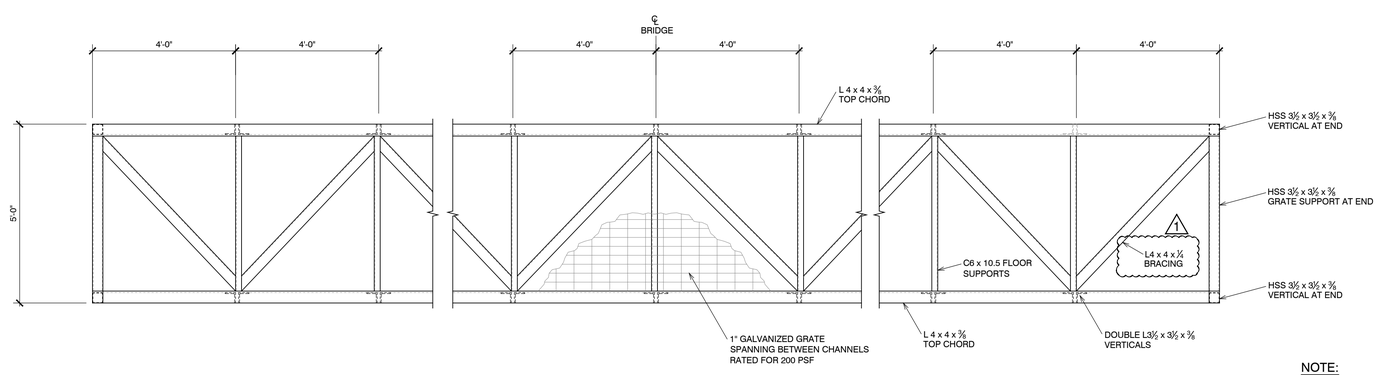
1 S6.1 CONSTRUCTION DETAIL
NO SCALE



NOTE:
CONSTRUCT LENGTH OF BRIDGE TO PROVIDE NEAREST COMPLETE EQUAL NUMBER OF 4'-0" SEGMENTS EACH SIDE OF CENTERLINE NOT TO EXCEED 48'-0" OVERALL LENGTH

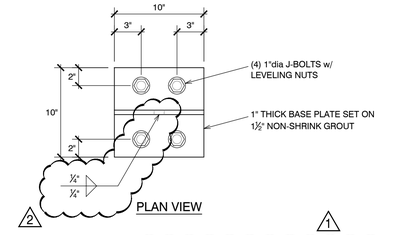
NOTE:
ALL CONTACT POINTS TO BE WELDED ALL AROUND WITH 5/16 FILLET WELDS. PROVIDE SHOP DRAWINGS OF ASSEMBLIES FOR REVIEW AND APPROVAL

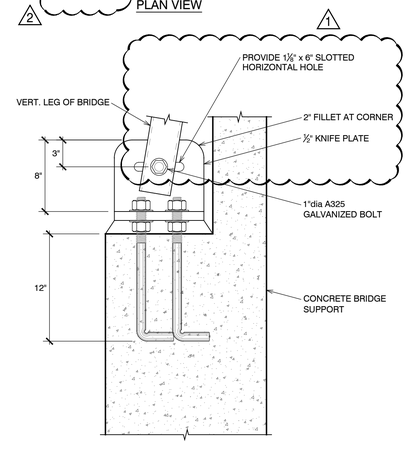
4 S6.1 SIDE ELEVATION
NO SCALE



NOTE:
ALL CONTACT POINTS TO BE WELDED ALL AROUND WITH 5/16 FILLET WELDS. PROVIDE SHOP DRAWINGS OF ASSEMBLIES FOR REVIEW AND APPROVAL

5 S6.1 PLAN VIEW
NO SCALE





2 S6.1 CONSTRUCTION DETAIL
NO SCALE

DYNAMIC STRUCTURES
1887 NORTH 1120 WEST PROVO, UTAH 84604
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Structural Plans for:
POWDER MOUNTAIN CABIN 1500+

PROFESSIONAL STRUCTURAL ENGINEER
NO. 190917
JAY D. ADAMS
STATE OF UTAH
9/22/17

DESIGNED BY:	J.D.A.
CHECKED BY:	J.D.A.
SCALE:	AS SHOWN
DATE:	JULY 28, 2017
JOB No.	17-089

CONSTRUCTION DETAILS

SHEET No.
S6.1

2 PLAN REVIEW-09/22/2017
1 PLAN REVIEW-09/05/2017