GENERAL STRUCTURAL REQUIREMENTS:

- I. NO CHANGES ARE TO BE MADE TO THESE STRUCTURAL PLANS WITHOUT THE KNOWLEDGE AND WRITTEN CONSENT OF THE ENGINEER OF RECORD.
- 2. ALL DIMENSIONS CONTROLLED BY EXISTING CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR AT THE SITE.
- 3. CONSTRUCTION AND MATERIALS SHALL BE INSTALLED IN ACCORDANCE WITH ALL THE REQUIREMENTS OF ALL LEGALLY CONSTITUTED PUBLIC AUTHORITIES HAVING
- JURISDICTION ON THE PROJECT, INCLUDING ALL COUNTY AND LOCAL ORDINANCES, AND THE SAFETY ORDERS OF THE STATE INDUSTRIAL ACCIDENT COMMISSION (OSHA).
- 4. THE GENERAL CONTRACTOR SHALL VISIT THE JOB SITE AND VERIFY ALL GRADES, DIMENSIONS, AND CONDITIONS PRIOR TO BIDDING AND COMMENCING CONSTRUCTION. 5. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE COORDINATION OF ALL THE WORK, INCLUDING THAT OF ALL SUB-TRADES
- 6. THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD IMMEDIATELY OF ANY DISCREPANCIES FOUND WITHIN THE STRUCTURAL CONTRACT DOCUMENTS. 7. ALL WORK PERFORMED SHALL CONFORM WITH THE REQUIREMENTS OF THE CURRENT ADOPTED EDITION OF THE INTERNATIONAL BUILDING CODE AND OTHER APPLICABLE GOVERNING CODES AND BUILDING ORDINANCES. REFER TO BUILDING DESIGN LOADS FOR ADDITIONAL INFORMATION.
- 8. ALL SUBCONTRACTORS SHALL BE RESPONSIBLE FOR REMOVAL OF ALL DEBRIS ACCUMULATED AS A RESULT OF THEIR OPERATION. ALL SCRAP, DEBRIS, AND OTHER EXCESS MATERIAL SHALL BE REMOVED FROM THE BUILDING SITE.
- 9. ALL STRUCTURAL MATERIALS SHALL BE FURNISHED AS SHOWN IN THESE STRUCTURAL PLANS UNLESS ALTERNATES ARE APPROVED IN WRITING BY THE ENGINEER
- 10. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR SHORING AND PROVIDING BRACING DURING CONSTRUCTION ERECTION TO SUPPORT ALL CONSTRUCTION LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED.
- II. SEE THE SPECIFICATIONS PACKAGE PRODUCED BY THE ARCHITECT FOR ADDITIONAL REQUIREMENTS, IF APPLICABLE.
- 12. THE STRUCTURAL DRAWINGS AND STRUCTURAL SPECIFICATIONS REPRESENT THE COMPLETED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO BRACING AND SHORING. OBSERVATION VISITS TO THE SITE BY FIELD REPRESENTATIVES OF THE ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ENGINEER ARE FOR THE PURPOSE OF QUALITY CONTROL AND IN THE INTEREST OF ACHIEVING COMPLIANCE WITH THE CONTRACT DOCUMENTS. THEY DO NOT GUARANTEE THE CONTRACTORS
- 13. THE SHOP DRAWING REVIEW PROCESS BY THE ENGINEER WILL ONLY COMMENCE AFTER THE SUBMITTED FABRICATION OF SHOP DRAWINGS HAVE BEEN AS FOLLOWS: A. INITIALLY REVIEWED AND ACCEPTED AS CONFORMING WITH THE STRUCTURAL CONSTRUCTION DRAWINGS BY THE RESPONSIBLE SUPERVISOR AND DRAWING

PERFORMANCE AND SHALL NOT BE CONSTRUED AS CONSTRUCTION SUPERVISION

- CHECKER WITH THEIR SIGNATURES. B. APPROVED AND ACCEPTED WITH A STAMP FROM THE GENERAL CONTRACTOR AS CONFORMING TO THE CONSTRUCTION DOCUMENTS.
- C. A MINIMUM OF 10 WORKING DAYS HAS BEEN ALLOCATED FOR THE REVIEW PROCESS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COORDINATION OF THE SHOP DRAWING REVIEW SCHEDULE.

BUILDING DESIGN LOADS:

I. GOVERNING CODE: IBC 2015, ASCE 7-10, AND LOCAL CODES WHERE APPLICABLE 2. APPLIED VERTICAL DESIGN LOADS TABLE:

A. METAL ROOF	15.8 PSF (1.)	20.0 PSF(2.)
3. FRAMED FLOOR	II.O PSF	40.0 PSF
5	- PSF	- PSF
D	- PSF	- PSF
<u>.</u> -	 - PSF	- PSF

(2.) 264 MSF SNOW LOAD = GROUND SNOW LOAD AT 8,600 ft MER MEBER CO., UTAF (3.) 444 PSF SNOW LOAD AT ROOF OVERHANGS

3. APPLIED WIND DESIGN BASIS TABLE: A. BASIC WIND SPEED (DESIGN 3-SECOND GUST)

	B. ENCLOSED BUILDING - CATEGORY	II
	C. WIND EXPOSURE	C
	D. IMPORTANCE FACTOR	I = 1.00
	E. DESIGN WIND PRESSURE - X DIRECTION	Px = 38.02 PSF
	F. DESIGN WIND PRESSURE - Y DIRECTION	Py = 38.02 PSF
4.	APPLIED SEISMIC DESIGN BASIS TABLE:	
	SEISMIC SOIL SITE CLASS TYPE	D
	A. BUILDING OCCUPANCY	<u> </u>
	B. SEISMIC IMPORTANCE	l = 1.00
	C. SEISMIC DESIGN CATEGORY	D
	D. MAPPED SHORT SPECTRAL RESPONSE	SS = 0.806
	E. MAPPED I SECOND SPECTRAL RESPONSE	SI = 0.267
	F. SHORT PERIOD SPECTRAL ACCELERATION	SDS = 0.633
	G. I SECOND SPECTRAL ACCELERATION	SDI = 0.332
	H. SITE LATITUDE & LONGITUDE	LAT = 41.36
		LNG = -111.74
	I. DESIGN PROCEDURE	EQUIVALENT LATERAL FORCE
	J. RESPONSE FACTOR - X DIRECTION	Rx = 6.5 PLY SHEAR WALLS
	K. BASE SHEAR FORCE - X DIRECTION	Vx = 0.0973 W
	L. RESPONSE FACTOR - Y DIRECTION	Ry = 6.5 PLY SHEAR WALLS
	M. BASE SHEAR FORCE - Y DIRECTION	√u = 0.0973 W
	N.	, <u> </u>

SPECIAL INSPECTIONS:

- I. REFER TO IBC SECTION IT04 FOR ADDITIONAL INFORMATION. 2. THE OWNER OR THE OWNER'S AGENT SHALL BE RESPONSIBLE FOR THE EMPLOYMENT OF THE REQUIRED SPECIAL INSPECTORS FOR THIS PROJECT. RICHMOND HOFFMAYER, INC. AND IT'S EMPLOYEES ARE NOT SPECIAL INSPECTORS NOR WILL BE RESPONSIBLE FOR EMPLOYING SPECIAL INSPECTORS FOR THE PROJECT
- 3. ALL OFFSITE FABRICATION OF STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES SHALL BE PERFORMED IN AN APPROVED FABRICATORS SHOP. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.
- 4. REQUIRED SPECIAL INSPECTIONS FOR EACH MATERIAL OR TYPE OF CONSTRUCTION ARE LISTED WITH THE INDIVIDUAL MATERIAL NOTES. 5. EXCEPTIONS TO REQUIRED SPECIAL INSPECTIONS:
- A. SPECIAL INSPECTIONS ARE NOT REQUIRED FOR WORK OF MINOR NATURE OR AS WARRANTED BY CONDITIONS IN THE JURISDICTION AS APPROVED BY THE BUILDING OFFICIAL B. SPECIAL INSPECTIONS ARE NOT REQUIRED FOR BUILDING COMPONENTS UNLESS
- THE DESIGN INVOLVES THE PRACTICE OF PROFESSIONAL ENGINEERING OR ARCHITECTURE AS DEFINED BY THE APPLICABLE STATE STATUTES. 6. THE PERMIT APPLICANT SHALL SUBMIT A STATEMENT OF SPECIAL INSPECTIONS
- PREPARED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. 7. SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. INSPECTORS SHALL REFER TO IBC SECTION

STATEMENT OF SPECIAL INSPECTIONS:

PROGRAM AND INSPECTOR INFORMATION, REFER TO SHEET SN.2. 2. THE FOLLOWING GENERAL NOTES FOR INDIVIDUAL MATERIALS LIST REQUIRED SPECIAL INSPECTIONS AS PERIODIC OR CONTINUOUS. UNLESS THERE ARE WRITTEN

EXCEPTIONS, ANY WORK FOR THIS PROJECT INVOLVING THE LISTED MATERIALS

- REQUIRES INSPECTIONS AS NOTED. 3. THE SEISMIC-FORCE-RESISTING SYSTEM AND SEISMIC SYSTEMS IN STRUCTURES SHALL BE INSPECTED, INCLUDING THE FOLLOWING:
- A. ANY PLYWOOD SHEAR WALLS WITH NAILING AT 4" ON CENTER OR LESS ... SEE THE SHEAR WALL SCHEDULE FOR TYPES AND THE PLANS FOR LOCATIONS.
- 4. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC-FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS AND HOLD-DOWNS - EXCEPT WHERE THE FASTENER SPACING IS MORE THAN 4" ON CENTER.

DEFERRED SUBMITTALS:

1704.1.2 FOR FURTHER INFORMATION.

I. DEFERRED SUBMITTALS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER-OF-RECORD, WHO SHALL REVIEW THEM AND FORWARD THEM TO THE BUILDING OFFICIAL WITH A NOTATION THAT THEY HAVE BEEN REVIEWED AND HAVE BEEN FOUND TO BE IN GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING OR STRUCTURE

2. THE DEFERRED ITEMS SHALL NOT BE INSTALLED UNTIL THEIR DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND APPROVED BY THE BUILDING OFFICIAL.

DEFERRED SUBMITTALS, CONTINUED:

3. ROOF FRAMING TRUSS SHOP DRAWINGS AND CALCULATIONS MUST BE SUBMITTED BY THE OWNER AFTER OBTAINING SHOP DRAWING APPROVAL IN WRITING FROM THE ENGINEER OF RECORD.

FOUNDATION NOTES:

THE SOILS REPORT IN ITS ENTIRETY SHALL BE INCLUDED AS PART OF THE CONTRACT DOCUMENTS. THE GENERAL CONTRACTOR AND CONCRETE SUB-CONTRACTOR SHALL REVIEW AND FAMILIARIZE THEMSELVES WITH THE SOILS REPORT. WHEN A SOILS REPORT IS NOT PROVIDED, MINIMUM IBC VALUES SHALL BE USED.

2. SOILS REPORT BY

IGES PROJECT NO. 01628-003 NOVEMBER 9, 2012 IGES PROJECT NO. 01628-01 DECEMBER I, 2016 SOIL DESIGN VALUES: FOOTINGS ARE TO BE FOUNDED 40" MIN. BELOW ADJACENT GRADE INTO

APPROVED COMPACTED FILL CONTINUOUS FOOTINGS 2,500 PSF. BEARING PAD FOOTINGS

4. LATERAL SOIL PRESSURE A. FLUID PRESSURE B. COEFFICIENT OF FRICTION C. PASSIVE PRESSURE

45 PCF ACTIVE 60 PCF AT-REST 3,600 PSF MAX.

- 5. EXCAVATION DEPTHS FOR THE FOUNDATIONS SHOWN ON THE DRAWINGS ARE TO BE MEASURED FROM THE LOWEST ADJACENT UNDISTURBED SOIL GRADE OR APPROVED COMPACTED EARTH GRADE WITH AT LEAST 5'-0" MINIMUM HORIZONTAL DISTANCE TO DAYLIGHT AT BOTTOM OF FOUNDATION EXCAVATION, UNLESS NOTED OTHERWISE.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING NECESSARY TO SUPPORT ANY CUT AND FILL BANKS DURING EXCAVATION, AND FOR FORMING AND PLACEMENT OF CONCRETE AND DRAINAGE.
- 7. FILLING AND BACK FILLING SHALL BE COMPACTED TO A MINIMUM OF 90% OR MORE IF SO NOTED, OF MAXIMUM DENSITY IN ACCORDANCE WITH THE SOILS REPORT AND ASTM 3. ALL FIELD WELDED REINFORCING STEEL SHALL BE GRADE 60 AND CONFORM TO TEST METHOD D-1557-78. FLOODING OF BACKFILL IS NOT PERMITTED.
- 8. ALL FILL AND BACKFILL MATERIAL SHALL BE APPROVED BY THE PROJECT SOILS ENGINEER WHERE APPLICABLE.
- 9. A PERMIT FROM THE STATE OF UTAH OSHA SHALL BE OBTAINED PRIOR TO THE ISSUANCE OF A BUILDING OR GRADING PERMIT WHEN A PERSON MUST DESCEND INTO AN EXCAVATION OR TRENCH WHICH IS GREATER THAN 5'-0" IN DEPTH. 10. WATER SHALL BE REMOVED FROM FOUNDATION EXCAVATIONS PRIOR TO PLACMENT

OF CONCRETE. CARE SHALL BE TAKEN SO AS NOT TO DRY OUT THE UNDERLYING

- NATURAL SOILS. II. A MINIMUM 15 MIL PLASTIC MEMBRANE SHALL BE PLACED ON THE PREPARED PAD SUB GRADE IN ALL AREAS WHERE FLOOR COVERING IS TO BE INSTALLED, UNLESS NOTED OTHERWISE ON THE PLANS. PLACE A MINIMUM OF 4 INCHES CLEAN GRAVEL UNDER
- 12. UNLESS NOTED OTHERWISE, CURBS, GUTTERS, AND SIDEWALK AREAS OF THE SLABS MAY BE PLACED DIRECTLY ON APPROVED 90% MIN. COMPACTED FILL.

FOUNDATION CONSTRUCTION SPECIAL INSPECTIONS:

I. REFER TO SHEET SN.2 FOR SPECIAL INSPECTION INFORMATION

THE MEMBRANE FOR PROTECTION PER THE GREEN BUILDING CODE.

CONCRETE NOTES:

- I. ALL CONCRETE CONSTRUCTION SHALL COMPLY WITH THE LATEST ADOPTED EDITION OF THE ACI CODE AND SPECIFICATIONS ACI 318-14/318R-14. THE FOLLOWING NOTES ARE PROVIDED FOR USE AS A GUIDE TO ALL REQUIREMENTS.
- 2. THE SPECIFIED COMPRESSIVE STRENGTH OF THE CONCRETE (F'C) FOR EACH PORTION OF THE STRUCTURE SHALL BE AS DESIGNATED BELOW, UNLESS NOTED OTHERWISE ON THE PLANS. STRENGTH REQUIREMENTS SHALL BE BASED ON A 28-DAY COMPRESSIVE STRENGTH TEST

SPECIAL INSPECTION: 3. 28 DAY CONCRETE DESIGN STRENGTH: 2,500 PSI (MIN.)

A. CONTINUOUS FOOTINGS f'c = 2,500 PSI (MIN.) f'c = 2,500 PSI (MIN.) B. PAD FOOTINGS SLAB-ON-GRADE D. RETAINING WALLS f'c = 2500 PSI (MIN.)

- 4. WHERE IT IS DESIRED TO REMOVE SHORING, FORMS, OR LOAD THE CONCRETE BY OTHER MEANS, PRIOR TO THE SPECIFIED 28-DAY CURING PERIOD, TESTING MUST BE DONE TO SHOW THE REQUIRED STRENGTH HAS BEEN REACHED.
- 5. UNLESS NOTED OTHERWISE, CEMENT SHALL BE TYPE II (ASTM C-150) 6. PORTLAND CEMENT SHALL COMPLY WITH "SPECIFICATION FOR PORTLAND CEMENT" (ASTM C-595). PORTLAND BLAST-FURNACE SLAG CEMENT OR PORTLAND-POZZOLAN CEMENT SHALL CONFORM WITH "SPECIFICATION FOR BLENDED HYDRAULIC CEMENTS" (ASTM C-595)
- 7. CONCRETE USED FOR EXTERIOR APPLICATIONS SHALL BE AIR-ENTRAINED. 8. 25% FLY ASH AS SPECIFIED ON AI.2 SHALL BE USED.
- 9. ADMIXTURES USED SHALL REQUESTED IN WRITING TO THE ENGINEER FOR APPROVAL AND BE USED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS AND SUBJECT TO THE LIMITATIONS LISTED:
- A. THE AMOUNT OF CALCIUM CHLORIDE SHALL NOT EXCEED 2 PERCENT BY WEIGHT OF CEMENT. THE AMOUNT OF CALCIUM CHLORIDE SHALL BE DETERMINED BY THE METHOD DESCRIBED IN AASHTO T260-78. B. FOR PRE STRESSED CONCRETE AND FOR ALL CONCRETE WHICH WILL CONTAIN EMBEDMENTS OF, OR REMAIN IN CONTACT WITH ALUMINUM OR GALVANIZED METAL,
- IT SHALL BE DEMONSTRATED BY TESTS THAT THE MIXING WATER OF THE CONCRETE, INCLUDING THAT CONTRIBUTED BY THE AGGREGATES AND ANY ADMIXTURES USED, WILL NOT CONTAIN A DELETERIOUS AMOUNT OF CHLORIDE ION. PROTECTIVE MEASURES MUST BE SUBMITTED TO THE ENGINEER FOR APPROVAL
- 10. MIXING WATER FOR CONCRETE SHALL CONFORM TO THE REQUIREMENTS FOR WATER SPECIFIED IN ASTM C-94.
- II. AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO "SPECIFICATION FOR CONCRETE AGGREGATES" (ASTM C-33).
- 12. AGGREGATES FOR LIGHTWEIGHT CONCRETE SHALL CONFORM TO "SPECIFICATION FOR 9. STRUCTURAL MEMBERS AND STEEL CONNECTORS INCLUDING BOLTS, NAILS, ETC., LIGHTWEIGHT AGGREGATES FOR STRUCTURAL CONCRETE" (ASTM C-330). 13. FINE AND COARSE AGGREGATES SHALL BE REGARDED SEPARATELY. EACH SIZE OF
- COARSE AGGREGATE, AS WELL AS THE COMBINATION OF SIZES WHEN TWO OR MORE ARE USED, SHALL CONFORM TO THE APPROPRIATE GRADING REQUIREMENTS OF THE APPLICABLE ASTM SPECIFICATIONS. 14. THE NOMINAL MAXIMUM SIZE OF THE AGGREGATE SHALL NOT BE MORE THAN ONE-
- FIFTH OF THE NARROWEST DIMENSION BETWEEN THE SIDES OF THE FORMS, ONE-THIRD OF THE THICKNESS OF SLABS, NOR THREE-FOURTHS OF THE MINIMUM CLEAR SPACING BETWEEN THE REINFORCING BARS, UNLESS NOTED OTHERWISE. 15. CONCRETE FOR ALL PARTS OF THE WORK SHALL BE CAPABLE OF BEING PLACED MITHOUT EXCESSIVE SEGREGATION AND, WHEN HARDENED, SHALL BE CAPABLE OF
- DEVELOPING ALL THE CHARACTERISTICS REQUIRED BY THESE SPECIFICATIONS AND THE CONTRACT DOCUMENTS. 16. THE PROPORTIONS OF INGREDIENTS SHALL PRODUCE A CONCRETE MIXTURE THAT IS CAPABLE OF BEING WORKED INTO THE CORNERS AND ANGLES OF THE FORMS AND
- AROUND ALL REINFORCEMENT BY PLACING AND CONSOLIDATION WITHOUT PERMITTING THE MATERIALS TO SEGREGATE OR ALLOW EXCESSIVE WATER TO COLLECT ON THE SURFACE 17. ALL CONCRETE SHALL BE PROPORTIONED TO HAVE A SLUMP WITH A MAXIMUM OF 4
- INCHES IF CONSOLIDATION IS TO BE BY VIBRATION METHODS. WHERE VIBRATION IS NOT USED FOR CONSOLIDATION, THE SLUMP MAY BE A MAXIMUM OF 5 INCHES. 18. ALL REINFORCING STEEL, WIRE MESH, ANCHOR BOLTS, HOLD DOWN ANCHORS AND
- OTHER INSERTS SHALL BE SECURED IN POSITION AND INSPECTED BY THE BUILDING OFFICIAL PRIOR TO PLACMENT OF CONCRETE.
- I. FOR DETAILED INFORMATION REGARDING SPECIAL INSPECTIONS, SPECIAL INSPECTION 19. CONCRETE FLOOR SLABS ON GRADE SHALL BE A MINIMUM OF 4 INCHES THICK, UNLESS NOTED OTHERWISE. 20. SLAB CONTROL JOINTS SHALL NOT BE SPACED FARTHER APART THAN 36 TIMES THE
 - THICKNESS OF THE SLAB IN ANY DIRECTION. CONTROL JOINTS SHALL BE PROVIDED WHERE THE AREA OF CONCRETE PLACED EXCEEDS 400 SQUARE FEET. THE MAXIMUM RATIO OF LONG SIDES TO SHORT SIDES OF THE CONTROLLED AREA SHALL NOT EXCEED I-1/2 TO I.
 - 21. SLABS WHICH ARE TO RECEIVE CARPET OR RESILIENT FLOORING, OR DESIGNATED TO RECEIVE CARPET OR RESILIENT FLOORING, OR DESIGNATED TO REMAIN EXPOSED SHALL BE TROWELLED SMOOTH.
 - 22. CONCRETE FLOOR AND SLAB CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF ACI 302, "GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION". FLATNESS / LEVELNESS SHALL CONFORM TO THE REQUIREMENTS OF ASTM EII55 BASE ON THE
 - FOLLOWING VALUES: CATAGORY MIN. FLATNESS MIN. LEVELNESS A. SPECIAL USE FLOORS STUDIOS, RINKS B. COMMERCIAL/INDUSTRIAL OFFICE, AISLES COMMERCIAL/INDUSTRIAL MANUFACTURING F D. SUB-FLOORS w/ THIN COVER PAINT, CARPETING F. : E. SUB-FLOOR W/ THICK COVER TILE, STONE

CONCRETE CONSTRUCTION SPECIAL INSPECTIONS:

REFER TO SHEET SN.2 FOR SPECIAL INSPECTION INFORMATION

MASONRY NOTES NIT MASONRY CONSTRUCTION SHALL BE

METAL DECK NO I. ALL STEEL DECKING HOVE

FORMED STEEL.

VERCO MANUFACTURIN

2. ALL DECKING IS TO BE GALVANIZED

DECKING IS SHOWN ON THE DRAWINGS

DISCREPANCIES OR INCONSISTENCIES

UNITS MUST BE REMOVED.

DIAPHRAGM SCHEDULE.

INSTALLATION.

OTHERWISE

SHALL BE STAGGERED

FRAMING LUMBER NOTES:

TO AFFIXING THE PERMANENT ATTACHMENTS.

BUILDING OFFICIAL PRIOR TO COVERING.

ALL FLOOR AND ROOF SHEATHING SIZES.

THE ENGINEER OF RECORD FOR THE PROJECT

AND FULL DIAMETER FOR SMOOTH SHANK PORTION

A. STUDS 2" THICK x 4" WIDE, BY HT.= 8'-I" MAX.

D. JOISTS 2" TO 4" THICK \times 6" AND WIDER

TYPICAL TIMBER NAILING SCHEDULE:

C. LIGHT FRAMING 2" TO 4" THICK x 2" TO 4" WIDE

19. MAXIMUM LATERALLY UNSUPPORTED STUD HEIGHT TABLE:

C. 2x4 STUDS @ 24" o.c. SUPPORTING ONE ROOF & CEILING

2X4 STUDS @ 16" O.C. SUPPORTING ONE FLOOR & ROOF

3x4 STUDS @ 16" o.c. SUPPORTING TWO FLOORS & ROOF

F. 2x6 STUDS @ 16" o.c. SUPPORTING TWO FLOORS & ROOF

BEAMS AND STRINGERS 5" AND THICKER x 6" AND WIDER

18. TYPICAL MINIMUM LUMBER GRADE TABLE:

B. STUDS 2" THICK x 4" TO 8" WIDE

A. NONBEARING 2x4 STUDS @ 24" o.c

B. NONBEARING 2x6 STUDS @ 24" o.c

F. POSTS 5" x 5" AND LARGER

CONNECTION PER IBC TABLE 2304.9.1

" SUBFLOOR TO JOIST OR GIRDER

SOLE PLATE TO LIGIST OR BLOCKING

SOLE PLATE TO JOIST OR BLOCKING, AT

BLOCKING BETWEEN JOIST OR RAFTERS

TOP PLATES, LAPS AND INTERSECTIONS

CEILING JOISTS, LAPS OVER PARTITIONS

CEILING JOISTS TO PARALLEL RAFTERS

I"x8" SHEATHING OR LESS TO EACH BEARING

WIDER THAN I"x8" SHEATHING TO EACH BEARING

I" BRACE TO EACH STUD AND PLATE

CONTINUOUS HEADER, TWO PIECE

I"x6" SUBFLOOR OR LESS TO EACH JOIST

WIDER THAN I"x6" SUBFLOOR TO EACH JOIST

BRIDGING T

BRACED WALL PANEL

DOUBLED TOP PLATE

OUBLED TOP PLATE

RIM JOIST TO TOP PLAT

CONTINUOUS HEADER TO

BUILT-UP CORNER STUDS

BUILT-UP GIRDER AND BEAMS

BUILT-UP GIRDER AND BEAMS

OLLAR TIE TO RAFTER

ROOF RAFTER TO 2x RIDGE

ROOF RAFTER TO 2x RIDGE

JACK RAFTER TO HIP

LEDGER STRIF

TOP PLATE TO

BLOCK ALL EDGES EXCEPT WHERE NOTED ON THE PLANS.

REPORTS. SEE PLANS FOR REPORT NUMBERS.

APPROVAL PRIOR TO FABRICATION AND INSTALLATION.

CONFORMING WITH ASTM A 525.G60.

2. MASONRY DE A CONCRETE MASONRY UNI

2,000 PSI (MIN.) - AT 28 DAYS B. MORTAR - TYPE S ²51 (MIN.) - AT 28 DAYS C. GROUT D. MASONRY COMPRESSIVE STRENGTH 3. MORTAR SHALL BE FRESHLY PREPARED AND UNIFORMLY MIXED OF I PART PORTLAND CEMENT, I/2 MAXIMUM TO I/4 MINIMUM PARTS LIME PUTTY OR HYDRATED

- LIME AND 2-1/4 TO 3 PARTS CLEAN WASHED SAND. MORTAR IS TO BE TYPE "5". 4. GROUT SHALL BE PROPORTIONED BY VOLUME AND SHALL HAVE SUFFICIENT WATER ADDED TO PRODUCE CONSISTENCY FOR POURING WITHOUT SEGREGATION. FINE GROUT, FOR USE IN SPACES 4 INCHES OR LESS IN BOTH HORIZONTAL DIMENSIONS SHALL BE COMPOSED OF I PART PORTLAND CEMENT, TO WHICH MAY BE ADDED NOT MORE THAN I/IO PART HYDRATED LIME OR LIME PUTTY, AND 2-1/4 TO 3 PARTS SAND, AND NOT MORE THAN 2 PARTS PEA GRAVEL.
- 5. GROUT ALL CELLS SOLID, UNLESS OTHERWISE NOTED. ALL ISOLATED BOLTS EMBEDDED IN MASONRY SHALL BE SOLID GROUTED IN PLACE WITH A MINIMUM OF 2 INCHES OF GROUT SURROUNDING BOLT
- 5. PROVIDE **5/8"** DIAMETER ANCHOR BOLTS AT **2'-0"** ON CENTER VERTICALLY WHERE WOOD STUDS AND MASONRY WALLS ADJOIN, TYPICAL UNLESS NOTED OTHERWISE. 7. ALL REINFORCING STEEL, ANCHOR BOLTS, HOLD-DOWN ANCHORS, AND OTHER INSERTS 9. IF CONCRETE IS TO BE PLACED ON TOP OF DECKING, ALL OIL AND GREASE, OR OTHER

MASONRY CONSTRUCTION SPECIAL INSPECTIONS:

SHALL BE SECURED IN POSITION PRIOR TO GROUTING.

I. REFER TO SHEET SN.2 FOR SPECIAL INSPECTION INFORMATION REINFORCING STEEL NOTES:

- I. MAIN REINFORCING STEEL SHALL BE GRADE 60 AND CONFORM TO ASTM A-615 SPECIFICATIONS UNLESS NOTED OTHERWISE. ALL REINFORCING SHALL BE FROM
- IDENTIFIED STOCK WITH MILL ANALYSIS SUPPLIED. 2. ALL BARS AND DOWELS, #4 AND SMALLER, SHALL BE GRADE 40 MINIMUM AND SHALL CONFORM TO ASTM A-615 SPECIFICATIONS UNLESS NOTED OTHERWISE.
- ASTM A-706 SPECIFICATIONS. 4. ALL WELDING SHALL BE PERFORMED AS INDICATED BY THE GOVERNING BUILDING DEPARTMENT
- 5. WELDING SHALL BE PERFORMED USING ELECTRODES AS SPECIFIED IN THE STRUCTURAL WELDING CODE FOR REINFORCING STEEL, ANSI/AWS DI.4-92. THE REBAR MILL ANALYSIS SHALL INCLUDE THE CARBON EQUIVALENCE TO ESTABLISH WELDABILITY. NO WELDING SHALL BE DONE AT THE BEND IN A BAR, WELDING OF CROSS BARS (TACK WELDING) SHALL NOT BE PERMITTED UNLESS NOTED OTHERWISE.
- 6. ALL FIELD WELDING OF REINFORCING STEEL SHALL BE CONTINUOUSLY INSPECTED BY A REGISTERED DEPUTY INSPECTOR. 7. DOWELS SHALL BE PROVIDED AT CONSTRUCTION JOINTS AND SHALL BE THE SAME SIZE AND SPACING AS THE REINFORCING SHOWN FOR THE SUBSEQUENT CONCRETE
- CONSTRUCTION, UNLESS NOTED OTHERWISE. 8. REINFORCING STEEL SHALL HAVE A MINIMUM CONCRETE COVER AS LISTED IN SECTION 7 OF ACI 318-14, UNLESS NOTED OTHERWISE.
- 9. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR CONCRETE BONDING 10. ALL REINFORCING STEEL LAPS AND SPLICES SHALL BE AS INDICATED ON THE PLANS. 10. ALL BOLTS THROUGH WOOD SHALL HAVE PLATE WASHERS EXCEPT WHERE METAL
- LAPS OF MULTIPLE BARS IN ALL SPLICES SHALL BE STAGGERED II. CONTINUOUS INSPECTION OF CONCRETE SHALL BE SCHEDULED SUCH THAT REINFORCING STEEL, CONDUIT, SLEEVES, AND EMBEDDED ITEMS MAY BE CORRECTED II. PROVIDE BRIDGING AT FOR ALL JOISTS AND ROOF RAFTERS AT 10'-0" o.c. MAX., AND 5. PRIOR TO PLACEMENT OF OVERLYING GRIDS OF REINFORCING STEEL

STRUCTURAL STEEL NOTES:

- ALL STRUCTURAL STEEL SHALL CONFORM TO APPLICABLE ASTM REQUIREMENTS AND SHALL BE FABRICATED ACCORDING TO AISC PRACTICE AND SPECIFICATIONS FOR BUILDING. 2. ALL SHOP AND FIELD BOLTED CONNECTIONS SHALL BE IN ACCORDANCE WITH ASTM A-307 AMERICAN STANDARD REGULAR BOLTS, UNLESS NOTED OTHERWISE.
- 3. NO HOLES OTHER THAN THOSE SPECIFICALLY DETAILED SHALL BE ALLOWED THROUGH STRUCTURAL STEEL MEMBERS. NO CUTTING OR BURNING OF STRUCTURAL STEEL WILL BE PERMITTED WITHOUT PRIOR CONSENT OF THE ENGINEER OF RECORD.
- 4. STRUCTURAL STEEL SHOP DRAWINGS SHALL BE REVIEWED BY THE ENGINEER OF RECORD PRIOR TO FABRICATION. 5. COLUMN BASE PLATES ON LEVELING NUTS SHALL USE NON-SHRINK GROUT
- SPECIFICALLY DESIGNED FOR THE EXPOSURE CONDITIONS WHERE THE APPLICATION SHALL BE USED. NOTE CERTAIN MANUFACTURERS LIMITATIONS WITH REGARDS TO MOISTURE. THE MINIMUM COMPRESSIVE STRENGTH OF THE GROUT SHALL BE AS SPECIFIED IN THE CONCRETE SECTION OF THE GENERAL NOTES
- 6. ALL SURFACES TO BE GROUTED SHALL BE PROPERLY CLEANED OF ALL FOREIGN MATERIAL PRIOR TO THE GROUTING APPLICATION.
- ALL STRUCTURAL STEEL IS TO HAVE MINIMUM ALLOWABLE YIELD STRESS CAPACITIES EQUAL TO, OR GREATER THAN THE FOLLOWING

	AOTHES EQUAL TO, OR OREATER T	TIMN THE FOLLOWING:					
	STRUCTURAL SHAPES	ASTM SPECIFICATION	MIN.	YIEL	D ST	RESS	
Α.	W SHAPES	A992		F, =	50	ksi	
В.	M SHAPES	A6/A6M		F, =	36	ksi	
C.	S SHAPES	A6/A6M		F, =	36	ksi	
D.	HP SHAPES	A6/A6M		F, =	36	ksi	
E.	AMERICAN STANDARD CHANNELS	A992		F, =	50	ksi	
F.	MISCELLANEOUS CHANNELS	A6/A6M		F, =	36	ksi	
G.	ANGLES	A6/A6M		F, =	_36	ksi	
Н.	PLATES AND BARS	A36		F, =	_36	ksi	
Ι.	ELECTRIC-RESISTANCE WELDED PIF	PES A53 TYPE E, GRADE	В	F _r =	_35	ksi	
J.	COLD FORMED STRUCTURAL TUBING	A500, GRADE B		F, =	46	ksi	
K.	CIOIO MECHANICAL WELDED TUBING	A29		F, =	28	ksi	
AL	L BOLTS ARE TO HAVE MINIMUM ALL	OWABLE YIELD STRESS (CAPA	ACITI	ES E	QUAL	T
OR	GREATER THAN THE FOLLOWING:						
	CONNECTOR	ACTM CRECIEIC ATION	MIN	YIFI	D ST	PEGG	

- ASTM SPECIFICATION MIN. YIELD STRESS CONNECTOR A. MACHINE BOLTS F. = 20 ksi F. = 44 <u>ksi</u> A325 3. HIGH STRENGTH BOLTS C. THREADED FASTENERS A36 F_r = <u>19.1 ksi</u> F, = <u>60 ksi</u> D. ANCHOR BOLTS
- EXPOSED TO THE COASTAL MARINE ENVIRONMENT SHALL BE HOT-DIPPED GALVANIZED AND HAVE A MINIMUM THICKNESS OF 3/8".
- 10. A <u>WEBER COUNTY, UTAH</u> BUILDING DEPARTMENT LICENSED FABRICATOR IS REQUIRED FOR STRUCTURAL STEEL FABRICATION.

WELDING NOTES:

- WELDING SHALL BE OF EITHER THE SHIELDED OR SUBMERGED ARC PROCESS AND PERFORMED BY CERTIFIED WELDERS. WELDERS SHALL BE QUALIFIED IN ACCORDANCE WITH THE "QUALIFICATIONS PROCEDURE" OF THE AMERICAN WELDING SOCIETY, LATEST EDITION. SHOP WELDING SHALL BE PERFORMED BY AN APPROVED, LICENSED FABRICATOR. ALL STRUCTURAL FIELD WELDING SHALL HAVE CONTINUOUS INSPECTION BY A REGISTERED DEPUTY INSPECTOR. REFER TO AWS DI.I FOR INFO. 2. ALL STRUCTURAL STEEL WELDING SHALL USE E70XX ELECTRODES.
- 3. ALL WELDING SHALL CONFORM WITH THE LATEST ADDITION OF THE WELDING CODE AWS DI.I. USE APPROVED ELECTRODES CONFORMING WITH ASTM A-233. WELDING SHALL BE PERFORMED BY BUILDING DEPARTMENT APPROVED CERTIFIED WELDERS. ALL FIELD WELDING SHALL BE CONTINUOUSLY INSPECTED BY A REGISTERED DEPUTY
- 4. ALL EXPOSED WELDS SHALL BE FILLED AND GROUND SMOOTH WHERE THE METAL WILL BE SUBJECT TO CONTACT WITH THE PUBLIC
- 5. SHOP WELDING SHALL BE DONE BY A CERTIFIED SHOP -APPROVED BY:_____
- 6. FIELD WELDING SHALL BE DONE BY CERTIFIED WELDERS -APPROVED BY:____

STEEL AND WELDING SPECIAL INSPECTIONS: REFER TO SHEET SN.2 FOR SPECIAL INSPECTION INFORMATION

- 2. EXCEPTIONS TO REQUIRED STEEL SPECIAL INSPECTIONS: A. SPECIAL INSPECTION OF THE STEEL FABRICATION PROCESS SHALL NOT BE REQUIRED WHERE THE FABRICATOR DOES NOT PERFORM ANY WELDING, THERMAL CUTTING OR HEATING OPERATION OF ANY KIND AS PART OF THE FABRICATION PROCESS. THE FABRICATOR SHALL BE REQUIRED TO SUBMIT A DETAILED
- PROCEDURE FOR MATERIAL CONTROL. REFER TO IBC 1704.3 FOR MORE INFO. 3. THE SPECIAL INSPECTOR NEED NOT BE CONTINUOUSLY PRESENT DURING WELDING OF THE FOLLOWING ITEMS, PROVIDED THE MATERIALS, WELDING PROCEDURES AND QUALIFICATIONS OF WELDERS ARE VERIFIED PRIOR TO THE START OF WORK; PERIODIC INSPECTIONS ARE MADE OF THE WORK IN PROGRESS; AND A VISUAL INSPECTION OF ALL WELDS IS MADE PRIOR TO COMPLETION OR PRIOR TO SHIPMENT OF SHOP WELDING.
- I. SINGLE-PASS FILLET WELDS NOT EXCEEDING 5/16".
- 2. FLOOR AND ROOF DECK WELDING.

STUDS AND JOISTS.

- 3. WELDED STUDS WHEN USED FOR STRUCTURAL DIAPHRAGM. 4. WELDED SHEET STEEL FOR COLD-FORMED STEEL FRAMING MEMBERS SUCH AS
- 5. WELDING OF STAIRS AND RAILING SYSTEMS.

MANUFACTURED WOOD BEAMS:

NY C ES-F 2078P & L.A.R.R. #23789

3. THE GALVANIZED DECK SHALL CONFORM WITH ASTM A-4-1. RADE E AND HAVE ZINC

5. ALL DECK WELDING SHALL BE IN CONFORMANCE WITH AWS DI.3. AND APPROVED ICC

6. SHOP DRAWINGS SHOWING SHEET LAYOUT, BLOCK OUTS, WELDING, STIFFENERS, AND ALL

CONNECTION SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND

7. PRIOR TO INSTALLATION, THE DECK INSTALLER SHALL CHECK THE SUPPORTING MEMBERS

8. PLACE THE DECK 50 AS TO ALIGN ALL RIBS, AND PLACE DECK UNITS END TO END PRIOR

MATERIAL WHICH MAY ADVERSELY AFFECT THE BONDING OF CONCRETE TO THE DECK

I. ALL FRAMING LUMBER SHALL CONFORM TO "STANDARD GRADING AND DRESSING

EXPOSED LUMBER SHALL BEAR NO MARKINGS WHICH WILL BE VISIBLE AFTER

3. ALL SHEATHING AND SHEAR PANELS SHALL BE IN PLACE AND INSPECTED BY THE

6. ALL NAILS USED FOR SHEATHING SHALL BE COMMON NAILS AS SPECIFIED ON THE

7. FRAMING HARDWARE SHALL BE SIMPSON "STRONG-TIE" CONNECTORS OR EQUAL. ALL

APA GRADE STAMPED, INCLUDING THE PANEL INDEX NUMBER. SEE THE SHEAR WALL

9. OSB SHALL CONFORM TO PRODUCT STANDARD PS-2-10. OSB SHALL BE APA GRADE

SIDE PLATES ARE SPECIFIED. BOLT HOLES SHALL BE PRE-DRILLED TO NOT MORE

8'-0" o.c. FOR FLOOR JOISTS. PROVIDE 2x MIN. SOLID BLOCKING AT ALL BEARING

STAMPED, INCLUDING THE PANEL INDEX NUMBER. STRUCT I IS REQUIRED FOR ALL

STRUCTURAL APPLICATIONS. DO NOT USE FOR EXTERIOR DECK SHEATHING.

THAN I/I6 INCH LARGER THAN THE BOLT DIAMETER, UNLESS NOTED OTHERWISE.

LOCATIONS WHERE THE LOAD ABOVE LIES ACROSS THE JOISTS, UNLESS NOTED

12. DIAPHRAGM NAILING NOTE: WHERE 8d NAILS ARE SPACED 2-1/2" o.c., OR 10d NAILS

ARE SPACED 3" o.c. AND HAVE A MINIMUM PENETRATION OF 1-5/8 INCHES INTO THE

SUPPORTING MEMBER, THE SUPPORTING MEMBER SHALL BE 3x NOMINAL AND NAILS

MAINS OR DRAINAGE PIPES PROPOSED TO BE SUPPORTED BY THE TIMBER ROOF

14. MINIMUM FIRE BLOCKING PER IBC AND LOCAL GOVERNING CODES SHALL BE

GALVANIZED OR STAINLESS STEEL, UNLESS IT CAN BE SHOWN THAT THE

PRESERVATIVE TREATMENT IS NON CORROSIVE TO STANDARD HARDWARE.

17. ALL HOLD DOWN BOLTS SHALL BE TIGHTENED JUST PRIOR TO COVERING THE WALL

15. ALL ANCHOR BOLTS, PLATE WASHERS, NAILS, HANGERS, & FASTENERS IN CONTACT

STRUCTURE SHALL BE SUBMITTED AS SHOP DRAWINGS AND SUBJECT TO REVIEW BY

PROVIDED AS NECESSARY IN ALL WALLS, FLOORS, AND ROOF. SEE ARCHITECTURAL

FASTENING

8d COMMONS

3) 8d COMMONS

(3) 16d @ 16"o.c.

2) 16d COMMONS

?) 16d COMMONS

2) 16d COMMONS

1) 8d COMMONS

3) 8d COMMONS

(2) 16d COMMONS

3) 8d COMMONS

1) 8d COMMONS

) 16d COMMONS

16d COMMONS

8d COMMONS

8d COMMONS

8d COMMONS

3) 8d COMMONS

(2) 20d COMMONS

IOD COMMONS

3) IOd COMMONS

2) 16d COMMONS

) 16d COMMONS

16d COMMONS

16d @ 16"o.c

LOCATION

TOENAIL EACH END

BLIND & FACE NAIL

BRACED WALL

FACE/FDGE NAIL

FACE NAIL TOP

AND BOTTOM

STAGGERED ON

ENDS AND EACH

AT EACH BEARING

FTG. FOOTING

GALV. GALVANIZED

HORIZ. HORIZONTAL

HEADER

GAGE OR GAUGE

HARDY FRAME

H.S.B. A325 HIGH STRENGTH BOLT

INSULATION

INSPECTION

INTERIOR

JOIST

GLUED-LAMINATED BEAM

MECHANICAL

ACCESSIBILITY FIRE

STATE, OR LOCAL REGULATIONS

EST COAST CODE CONSULTANTS, II

ELECTRICAL

G. GA.

GLB.

INSUL.

INSP

J. JST.

H. HDR.

ANFI C

YPICAL FACE NAIL

SCHEDULE FOR SHEAR PANEL SHEATHING SIZES. SEE THE DIAPHRAGM SCHEDULE FOR

SUBSTITUTES SHALL BEAR CURRENT ICBO. APPROVAL, AND IF REQUIRED BY THE

8. PLYWOOD SHALL CONFORM TO PRODUCT STANDARD PS-I-09. PLYWOOD SHALL BE

4. INSTALL ALL SHEATHING WITH THE FACE GRAIN ACROSS THE FRAMING SUPPORTS AND

FRAMING LUMBER SHALL BE DOUGLAS FIR, S4S, UNLESS NOTED OTHERWISE.

5. THE PERIMETER OF ALL OPENINGS THROUGH ROOFS AND FLOORS SHALL BE

SHEAR WALL SCHEDULE AND FLOOR AND ROOF DIAPHRAGM SCHEDULES.

BUILDING OFFICIAL, HAVE ADDITIONAL APPROVED RESEARCH REPORTS.

SHALL BEAR THE GRADE STAMP OF AN APPROVED GRADING AGENCY, EXCEPT

4. DESIGN SHALL BE IN CONFORMANCE WITH AISI SPECIFICATIONS FOR THE DESIGN OF COLD

COLLED STEEL. THE EXTENT OF THE STEEL

- PLANS SHALL BE MANUFACTURED BY THE 💹 . 🛮 ALL PARALLAM (PSL), TIMBERSTRAND (LSL), AND MICROLAM (LVL) BEAMS ARE TO BE FROM WEYERHAEUSER OR EQUIVALENT
 - 2. PARALLAMS (PSL) SHALL CONFORM TO ICC ESR-1387 AND HAVE AN ELASTIC
 - MODULUS E = 2,000,000 psi OR GREATER.
 - 3. TIMBERSTRAND (LSL) BEAMS SHALL CONFORM TO ICC ESR-1387 AND HAVE AN
 - ELASTIC MODULUS E = 1,500,000 psi FOR SIZES 4x14 AND LARGER, AND E = 1,300,000 psi FOR SIZES 4x12 AND SMALLER.
 - 4. MICROLAM (LVL) LAMINATED VENEER LUMBER SHALL CONFORM TO ICC ESR-1387 AND
 - HAVE AN ELASTIC MODULUS E = 1,900,000 psi OR GREATER. 5. ALL SIP PANEL BEAMS SHALL CONFORM TO ICC ESR-

MANUFACTURED WOOD JOIST NOTES:

- I. ALL PLYWOOD WEB JOISTS SHOWN ON THE STRUCTURAL PLANS SHALL BE AS SPECIFIED AND MANUFACTURED BY THE FOLLOWING: WEYERHAEUSER
- THE MANUFACTURED PLYWOOD WEB JOISTS SHALL HAVE A CURRENT ICC NUMBER AND FOR CORRECT ASSIGNMENT AND LAYOUT AND NOTIFY THE ENGINEER OF RECORD OF ANY OTHER APPROVED RESEARCH REPORTS AS FOLLOWS: ICC ESR-II53
 - 3. ALTERNATE MANUFACTURERS MAY BE USED SUBJECT TO THE APPROVAL OF THE ARCHITECT AND THE ENGINEER. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY
 - INFORMATION TO THE ARCHITECT OR ENGINEER FOR APPROVAL 4. CROSS BRIDGING AND/OR BRACING SHALL BE PROVIDED AND DETAILED AS REQUIRED TO ADEQUATELY BRACE ALL JOISTS. BRIDGING SHOULD BE INSTALLED AS

5. TEMPORARILY REMOVING WEB MEMBERS AND DRILLING OR CUTTING CHORDS IS NOT

- ERECTION PROCEEDS, AND TEMPORARY BRACING INSTALLED TO MAINTAIN ALIGNMENT AND PREVENT LATERAL MOVEMENT.
- PERMITTED. RULES NO. 16 OF THE WEST COAST LUMBER INSPECTION BUREAU (WCLIB). EACH PIECE 6. SHEATHING SHALL BE SECURELY FASTENED TO THE TOP CHORD. THE NAILING PATTERN SHALL BE STAGGERED TO AVOID SPLITTING AND TO ASSURE NAILING INTO

MANUFACTURED ... OP TRUSSES

EACH CHORD MEMBER.

- RILL USSES, AND DRAG TRUSSES
- 2. ALL BLOCKING, BRIDGIN ORING FOR TRUSSES SHALL BE BY SUPPORTED AND BE NAILED PER THE REQUIRED BOUNDARY NAILING AS LISTED IN THE THE TRUSS MANUFACTURER.
 - 3. LA CITY BUILDING DEPARTMENT LICENSED FAD.

ALL CONNECTORS IN CONTACT WITH THE PRESSURE TREADED LUMBER MAY BE STAINLESS STEEL

HARDWARE CORROSION PROTECTION W/ PRESSURE TREATED LUMBER:

- ANY CONNECTORS THAT ARE NOT STAINLESS STEEL SHALL BE HOT-DIP GALVANIZED PER ASTM 123 WITH 2.0 oz./ft^2 MINIMUM COATING WEIGHT ANY FASTENERS THAT ARE NOT STAINLESS STEEL SHALL BE HOT-DIP GALVANIZED
- PER ASTM 153 WITH 2.0 oz./ft^2 MINIMUM COATING WEIGHT. CONSULT WITH YOUR PRESSURE TREATED LUMBER SUPPLIER FOR OTHER CORROSION RESISTANT COATINGS AND FOR FASTENERS OR CONNECTORS THAT MAY BE USED

WITH THEIR PRODUCTS. SUBMIT SPECIFICATIONS TO ENGINEER FOR REVIEW AND

APPROVAL PRIOR TO USING. CONSULT WITH YOUR CONNECTION MANUFACTURER FOR SPECIFICS ON THEIR CORROSION-RESISTANT PRODUCTS FOR USE WITH PRESSURE-TREATED WOOD AND SUBMIT TO ENGINEER FOR REVIEW AND APPROVAL PRIOR TO USING.

GLUED-LAMINAT TIMBER NOTES

- ALL SINGLE SPAN & JET LA A D BERS SHALL BE COMBINATION 24F-V4,
- ALL CANTILEVEN SHALL BE 24F-V8, UNLESS NOTED 13. THE ROOF DESIGN LOADS GIVEN IN THE GENERAL NOTES ARE MINIMUM DESIGN LOADS. OTHERWISE MECHANICAL EQUIPMENT NOT SHOWN ON THESE DRAWINGS AND ANY FIRE SPRINKLER 3. ALL GLUED LAMINATED MEMBERS SH MANUFACTURED BY A FABRICATOR
 - LICENSED BY THE LOCAL GOVERNING BUILD 4. ALL FABRICATION AND WORKMANSHIP SHALL BE IN CONFORMANCE WITH THE CURRENT EDITION OF THE STANDARD SPECIFICATIONS FOR STRUCTURAL GLUED- LAMINATED DOUGLAS FIR (COAST REGION) LUMBER BY THE AMERICAN INSTITUTE OF TIMBER
 - CONSTRUCTION. 5. FINISH OF THE MEMBERS SHALL BE INDUSTRIAL APPEARANCE GRADE IN
- WITH PRESSURE TREATED WOOD (SILL PLATES OR LEDGERS) SHALL BE HOT DIPPED CONFORMANCE WITH THE STANDARD APPEARANCE GRADES OF THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION, UNLESS NOTED OTHERWISE. 16. PROVIDE LEAD HOLE FOR LAG SCREWS 40% TO 70% OF THREADED SHANK DIAMETER 6. ALL MEMBERS SHOWN ARE FOR DRY USE ONLY.

CTDICTIDAL DI ANI ARROEVIATIONIC

S 1	ructi	IRAL PLAN ABBREVIA	∖ Tk	ONS:	
A.	@ A.B.	AT ANCHOR BOLT	Κ.	kips	x 1000 POUNDS
	ABV. A/C A.F.F.	ABOVE AIR CONDITIONING ABOVE FINISHED FLOOR ALTERNATE	L.		POUND FORCE LIGHT WEIGHT LAMINATED STRAND LUMBER LAMINATED VENEER LUMBER
	B.G. BLK. BLKG. BM. B.N.	BELOW GRADE BLOCK BLOCKING BEAM BOUNDARY NAILING	M.	MAX. MECH. MIN.	A307 MACHINE BOLT MAXIMUM MECHANICAL MINIMUM MISCELLANEOUS MICRO=LAM METAL
С.	€ CJ. C.J. CMB. CMU.	CAMBER	N.	N.G. N.I.C. N.T.S.	NATURAL GRADE NOT IN CONTRACT NOT TO SCALE
	COL. CONC.	COLUMN CONCRETE CONTINUOUS	0.	o/ O.C. OPNG.	OVER ON CENTER OPENING
D.	φ d D DBL D.F. do	DIAMETER NAIL PENNY DEPTH DOUBLE DOUGLAS FIR REPEAT FRAMING	P.	PRLLM. PSF. PSL	PARALLAM BEAM
E.	ELEV. E.N. EQ. EQUIP.	EXISTING EACH FACE EACH WAY ELEVATION EDGE NAIL EQUAL EQUIPMENT EXTERIOR	R.	REINF. RET.	DOUGLAS FIR LUMBER REINFORCEMENT RETAINING REQUIRED ROUGH OPENING
F.	F.F. F.G. F.J. FLR. F.O.B. F.O.S. FRMG.	FINISH FLOOR FINISH GRADE FLOOR JOIST FLOOR FACE OF BUILDING FACE OF STUD FRAMING	S.	SPECS. SQ. S.S.	SIMILAR SHEET METAL SCREW SPECIFICATIONS

M. M **PLAN REVIEW ACCEPTANCE** W.W.F. WELDED WIRE FABRIC STRUCTURA PLUMBING **TENERGY**

T. T&B

T&G

TOP

T.O.S.

TSTR

TYP.

V. VERT. VERTICAL

T.O.M.

SIMPSON STRONG WALL

TOP AND BOTTOM

TOP OF PARAPET

TOP OF WALL

TIMBERSTRAND

U.N.O. UNLESS NOTED OTHERWISE

MIDE OR MIDTH

TUBE STEEL

TYPICAL

U. U.B.C. UNIFORM BUILDING CODE

TOP OF SHEATHING

TONGUE AND GROOVE

TRUS JOIST TJI JOIST

STATUS DESCRIPTION BUILDING DEPARTMENT 08/27/201 SUBMITTAL #1

VOID WITHOUT SIGNATURE & SEAL CHECKED BY:

JOB NUMBER 2017-0610

	HEDULE OF SPECIAL INSPECTIONS:				RY SPECIAL INSPECTIONS:		
ŧ	NOTE: "X" INDICATES THAT AN INSPECTION IS REQUIRE PERIODIC AS NOTED IN THE APPROPRIATE COLUMN.				CIAL INSPECTIONS AND VERIFICATIONS FOR I EQUIRED IN THE FOLLOWING TABLE: (REFERE)		
	NOTE: "" INDICATES THAT AN ACTIVITY IS EITHER A WHOSE FREQUENCY IS DEFINED IN SOME OTHER MANNE		TIVITY OR ONE		SPECIAL INSPECTION FOR MASONRY HE START OF MASONRY CONSTRUCTION	CONTINUOUS	PERIODI
	EEL AND WELDING SPECIAL INSPECT				FY THE FOLLOWING TO ENSURE COMPLIANCE:		
1.	STEEL SHOP QUALITY CONTROL PROCEDURES: (REFER	ENCE IBC 1704.	2.1)	<u>а.</u> b.	PROPORTIONS OF SITE-PREPARED MORTAR CONSTRUCTION OF MORTAR JOINTS		X
_	VERIFICATION AND INSPECTION OF STEEL CONSTRUCITON INSPECT FABRICATOR'S FABRICATION AND	CONTINUOUS	PERIODIC	C.	LOCATION OF REINFORCEMENT, CONNECTORS, PRESTRESSING TENDONS, AND ANCHORAGES		×
	QUALITY CONTROL PROCEDURES			d.	PRESTRESSING TECHNIQUE		×
	THE SPECIAL INSPECTIONS AND VERIFICATIONS FOR S BE AS REQUIRED IN THE FOLLOWING TABLE: (REFEREN			е.	GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES		×
	DE AS REQUIRED IN THE FOLLOWING TABLE: (REFEREN VERIFICATION AND INSPECTION OF STEEL CONSTRUCITON			2. VERI	FY: SIZE AND LOCATION OF STRUCTURAL ELEMENTS		×
Ī	I. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:		_	<u> </u>	TYPE, SIZE, AND LOCATION OF ANCHORS,		×
	a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE		×		INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES		
_	APPROVED CONSTRUCTION DOCUMENTS			——————————————————————————————————————	OR OTHER CONSTRUCTION SPECIFIED SIZE, GRADE, AND TYPE OF		×
	 b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED 		×	d.	REINFORCEMENT WELDING OF REINFORCING BARS	X	
=	2. INSPECTION OF HIGH-STRENGTH BOLTING: a. BEARING TYPE CONNECTIONS		×	<u> </u>	PROTECTION OF MASONRY DURING COLD	~	×
-	b. SLIP-CRITICAL CONNECTIONS	X	×		WEATHER (TEMPERATURE BELOW 40 DEGREES F) OR HOT WEATHER		
	3. MATERIAL VERIFICATION OF STRUCTURAL STEEL: a. IDENTIFICATION MARKINGS TO CONFORM TO				(TEMPERATURE ABOVE 90 DEGREES F) APPLICATION AND MEASUREMENT OF		X
	ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS				PRESTRESSING FORCE		
-	b. MANUFACTURER'S MILL TEST REPORTS				R TO GROUTING VERIFY THE FOLLOWING TO FY COMPLIANCE:		
_	4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS: a. IDENTIFICATION MARKINGS TO CONFORM TO			<u>a.</u> b.	GROUT SPACE IS CLEAN PLACEMENT OF REINFORCEMENT AND		×
_	AWS DESIGNATION LISTED IN THE WPS b. MANUFACTURER'S CERTIFICATE OF				CONNECTORS AND PRESTRESSING TENDONS AND ANCHORAGE		
-	COMPLIANCE REQUIRED 5. INSPECTION OF WELDING			C .	PROPORTIONS OF SITE-PREPARED GROUT AND		×
-	a. STRUCTURAL STEEL			d.	PRESTRESSING GROUT FOR BONDED TENDONS CONSTRUCTION OF MORTAR JOINTS		×
	I) COMPLETE AND PARTIAL PENETRATION GROOVE WELDS	X			TY GROUT PLACEMENT TO ENSURE COMPLIANCE CODE AND CONSTRUCTION DOCUMENT PROVISIONS	×	
-	2) MULTIPASS FILLET WELDS 3) SINGLE PASS FILLET WELDS > 5/16"	X		<u>a.</u>	OBSERVE GROUTING OF PRESTRESSING	×	
-	4) SINGLE PASS FILLET WELDS ≤ 5/16" IN SIZE		X		BONDED TENDONS ERVE PREPARATION OF REQUIRED GROUT	×	
-	5) FLOOR AND ROOF DECK WELDS b. REINFORCING STEEL		×		IMENS, MORTAR SPECIMENS, AND/OR PRISMS FY COMPLIANCE WITH REQUIRED INSPECTION		×
-	I) VERIFICATION OF WELDABILITY		×	_	VISIONS OF THE CONSTRUCTION DOCUMENTS APPROVED SUBMITTALS		
_	OF REINFORCING STEEL OTHER THAN ASTM A706				ONS TO REQUIRED MASONRY SPECIAL INSPEC	CTIONS:	
	 REINFORCING STEEL-RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL 	X			IAL INSPECTION OF THE MASONRY SHALL NO PLACES, MASONRY HEATERS OR MASONRY C		
	MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR				STRUCTED IN ACCORDANCE WITH I BC SECTION 2 SIAL INSPECTION SHALL NOT BE REQUIRED FO	·	
-	WALLS, AND SHEAR REINFORCEMENT 3) SHEAR REINFORCEMENT	X		MAS	ONRY, GLASS UNIT MASONRY OR MASONRY V		
-	4) OTHER REINFORCING STEEL		X		<mark>TION 2109, 2110 OR CHAPTER 14.</mark> CIAL INSPECTIONS AND VERIFICATIONS FOR 1	MASONRY CONS	TRUCTION
:	INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION		X		EQUIRED IN THE FOLLOWING TABLE: (REFERE) SPECIAL INSPECTION FOR MASONRY	NCE IBC TABLE CONTINUOUS	1704.5.3) PERIOD
	DOCUMENTS: a. DETAILS SUCH AS BRACING AND STIFFENING			I. FRON	1 THE BEGINNING OF MASONRY CONSTRUCTION FOLLOWING SHALL BE VERIFIED TO ENSURE		
	b. MEMBER LOCATIONSc. APPLICATION OF JOINT DETAILS AT EACH				PLIANCE:		
- 2 t	CONNECTION EXCEPTIONS TO REQUIRED STEEL SPECIAL INSPECTION	IC		a.	PROPORTIONS OF SITE-PREPARED MORTAR, GROUT, AND PRESTRESSING GROUT FOR BONDED TENDONS		×
	A. SPECIAL INSPECTION OF THE STEEL FABRICATION	PROCESS SHAL		<u></u> b.	PLACEMENT OF MASONRY UNITS AND CONSTRUCTION OF MORTAR JOINTS		×
	REQUIRED WHERE THE FABRICATOR DOES NOT PER CUTTING OR HEATING OPERATION OF ANY KIND AS	PART OF THE	FABRICATION	C .	PLACEMENT OF REINFORCEMENT, CONNECTORS		×
	PROCESS. THE FABRICATOR SHALL BE REQUIRED PROCEDURE FOR MATERIAL CONTROL. REFER TO			d.	AND PRESSTRESSING TENDONS AND ANCHORAGES GROUT SPACE PRIOR TO GROUTING	×	
i	B. THE SPECIAL INSPECTOR NEED NOT BE CONTINUOUS OF THE FOLLOWING ITEMS, PROVIDED THE MATERIA			<u>e.</u> f.	PLACEMENT OF GROUT PLACEMENT OF PRESTRESSING GROUT	X	
	QUALIFICATIONS OF WELDERS ARE VERIFIED PRIO	R TO THE STAF	RT OF WORK;	2. VERI	FY:		
	PERIODIC INSPECTIONS ARE MADE OF THE WORK I INSPECTION OF ALL WELDS IS MADE PRIOR TO CO			<u>a.</u> b.	SIZE AND LOCATION OF STRUCTURAL ELEMENTS TYPE, SIZE, AND LOCATION OF ANCHORS.		×
				D.	111 2, 3122, 7110 2007 111011 01 7 11011010,	X	
	SHIPMENT OF SHOP WELDING. I. SINGLE-PASS FILLET WELDS NOT EXCEEDING 5,	/16".		D.	INCLUDING OTHER DETAILS OF ANCHORAGE OF		
	 SINGLE-PASS FILLET WELDS NOT EXCEEDING 5, FLOOR AND ROOF DECK WELDING. 				INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION		<u> </u>
	 SINGLE-PASS FILLET WELDS NOT EXCEEDING 5, FLOOR AND ROOF DECK WELDING. WELDED STUDS WHEN USED FOR STRUCTURAL D WELDED SHEET STEEL FOR COLD-FORMED STE 	PIAPHRAGM.	EMBERS SUCH AS	- C.	INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION SPECIFIED SIZE, GRADE, AND TYPE OF REINFORCEMENT		X
	 SINGLE-PASS FILLET WELDS NOT EXCEEDING 5, FLOOR AND ROOF DECK WELDING. WELDED STUDS WHEN USED FOR STRUCTURAL D 	PIAPHRAGM.	EMBERS SUCH AS		INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION SPECIFIED SIZE, GRADE, AND TYPE OF REINFORCEMENT WELDING OF REINFORCING BARS PROTECTION OF MASONRY DURING COLD		×
CC	 SINGLE-PASS FILLET WELDS NOT EXCEEDING 5, FLOOR AND ROOF DECK WELDING. WELDED STUDS WHEN USED FOR STRUCTURAL D WELDED SHEET STEEL FOR COLD-FORMED STE STUDS AND JOISTS. 	PIAPHRAGM. EL FRAMING M		C.	INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION SPECIFIED SIZE, GRADE, AND TYPE OF REINFORCEMENT WELDING OF REINFORCING BARS		
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C. SPECIAL INSPECTIONS SHALL NOT BE REQUIRED FOR NONSTRUCTURAL CONCRETE SLABS SUPPORTED DIRECTLY ON THE GROUND, NOR PATIOS, DRIVEWAYS AND SIDEWALKS ON GRADE.

THE REQUIRED VERIFICATION AND SPECIAL INSPECTION FOR SOILS	SHALL CONFORM	RESPONSIBILITY	FIRM	ADDRESS, TELEPHONE, E-MAIL
TO THE FOLLOWING TABLE: (REFERENCE TABLE IBC 1704.7) VERIFICATION AND INSPECTION TASK OF SOIL CONTINUOUS I. VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE	PERIODIC X	I. SPECIAL INSPECTION (EXCEPT FOR GEOTECHNICAL)	T.B.D.	-
TO ACHIEVE DESIGN BEARING CAPACITY 2. VERIFY EXCAVATIONS EXTEND TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL		2. MATERIAL TESTING	T.B.D.	-
3. PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS 4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND X	×	3. GEOTECHNICAL	T.B.D.	-
LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL 5. PRIOR TO PLACEMENT OF CONTROLLED FILL.	_ x	INSPECTIONS		
OBSERVE SUBGRADE AND VERIFY THE SITE HAS BEEN PREPARED PROPERLY		4	-	-
THE REQUIRED VERIFICATION AND SPECIAL INSPECTION OF PILE FOR CONFORM TO THE FOLLOWING TABLE: (REFERENCE TABLE IBC 1704	1.8)	SEISMIC REQUIRE	MENTS	1
VERIFICATION AND INSPECTION TASK OF PILES CONTINUOUS I. VERIFY PILE MATERIALS, SIZES AND LENGTHS COMPLY WITH THE REQUIREMENTS 2. DETERMINE CAPACITIES OF TEST PILES AND CONDUCT X	PERIODIC	DESCRIPTION OF SEISMIC SYSTEMS SUBJECT TO SP SYSTEM		
ADDITIONAL LOAD TESTS, AS REQUIRED 3. OBSERVE DRIVING OPERATIONS AND MAINTAIN X COMPLETE AND ACCURATE RECORDS FOR EACH PILE		PLYWOOD SHEAR WALLS	ALL SHEAR WALL CENTER OR LESS	S WITH EDGE NAIL SPACING OF 4" ON - SEE PLANS
 VERIFY LOCATIONS OF PILES AND THEIR PLUMBNESS: X a. CONFIRM TYPE AND SIZE OF HAMMER b. RECORD NUMBER OF BLOWS PER FOOT OF PENETRATION 		2	-	
c. DETERMINE REQUIRED PENETRATIONS TO ACHIEVE DESIGN CAPACITY d. RECORD TIP AND BUTT ELEVATIONS AND		3	-	
RECORD ANY PILE DAMAGE 5. FOR STEEL PILES, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH SECTION 1704.3		4	-	
6. FOR SPECIALTY PILES, PERFORM ADDITIONAL INSPECTIONS AS DETERMINED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE		THE EVIENT OF THE CEICA	ALC FORCE PECICINA	NC CYCTEM IC DEFINED IN MODE DETA
 FOR AUGERED UNCASED PILES AND CAISSON PILES, PERFORM INSPECTIONS IN ACCORDANCE WITH SECTION 1704.9 		THE EXTENT OF THE SEISM IN THE CONSTRUCTION DO WIND REQUIREME	CUMENTS.	NG SYSTEM IS DEFINED IN MORE DETA
THE REQUIRED VERIFICATION AND SPECIAL INSPECTION OF PIER FOR SHALL CONFORM TO THE FOLLOWING TABLE: (REFERENCE TABLE IS VERIFICATION AND INSPECTION TASK OF PIERS CONTINUOUS	3C 1704.9)	DESCRIPTION OF WIND-FO	ORCE-RESISTING SYS	STEM AND DESIGNATED SEISMIC AS PER SECTION 1705.4.2:
. OBSERVE DRILLING OPERATIONS AND MAINTAIN X COMPLETE AND ACCURATE RECORDS FOR EACH PILE 2. VERIFY LOCATIONS OF PIERS AND THEIR PLUMBNESS: X a. CONFIRM PIER DIAMETERS		I. PLYWOOD SHEAR WALLS		S WITH EDGE NAIL SPACING OF 4" ON
 a. CONFIRM PIER DIAMETERS b. CONFIRM BELL DIAMETERS IF APPLICABLE c. CONFIRM LENGTHS, EMBEDMENT INTO BEDROCK IF APPLICABLE 		2	-	
 d. CONFIRM ADEQUATE END STRATA BEARING CAPACITY 		3	-	
EXCEPTIONS TO REQUIRED FOUNDATION SPECIAL INSPECTIONS: A. SPECIAL INSPECTION IS NOT REQUIRED DURING PLACEMENT OF HAVING A TOTAL DEPTH OF 12" OR LESS.	CONTROLLED FILL			
DDITIONAL SPECIAL INSPECTIONS: SEISMIC RESISTANCE: (REFERENCE IBC 1705.3)		4		
VERIFICATION AND INSPECTION TASK CONTINUOUS SUSPENDED CEILING SYSTEMS AND THEIR ANCHORAGE	PERIODIC	THE EXTENT OF THE WIND THE CONSTRUCTION DOCL		SYSTEM IS DEFINED IN MORE DETAIL
WIND RESISTANCE: (REFERENCE IBC 1705.4) VERIFICATION AND INSPECTION TASK CONTINUOUS ROOF CLADDING AND ROOF FRAMING CONNECTIONS	PERIODIC	STATEMENT OF S		
2. WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING 3. ROOF AND FLOOR DIAPHRAGM SYSTEMS, INCLUDING			TYPE V TWO STORY	K RIDGE, EDEN, UTAH Y RESIDENCES S IS SUBMITTED IN FULFILLMENT OF TH
COLLECTORS, DRAG STRUTS AND BOUNDARY ELEMENTS 4. VERTICAL WIND-FORCE-RESISTING SYSTEMS, INCLUDING		A. SCHEDULE OF SPE		ID 1705. INCLUDED ARE: AND TEST APPLICABLE TO THIS PROJ ONS 1704 AND 1705
BRACED FRAMES, MOMENT FRAMES, AND SHEAR WALLS 5. WIND-FORCE-RESISTING SYSTEM CONNECTIONS TO THE		3. SPECIAL INSPE	ECTIONS FOR SEISMI ECTIONS FOR WIND F ING AGENCIES AND	
FOUNDATION 6. FABRICATION AND INSTALLATION OF SYSTEMS OR COMPONENTS REQUIRED TO MEET THE IMPACT RESISTANCE REQUIREMENTS OF SECTION 1609.1.2		2. SPECIAL INSPECTIONS		ND INSPECTIONS. BE PERFORMED IN ACCORDANCE WI THIS STATEMENT AND SECTIONS 1704
PECIAL INSPECTIONS FOR SEISMIC RESISTANC		3. THE SCHEDULE OF SPI TESTS REQUIRED. SP	ECIAL INSPECTORS	SUMMARIZES THE SPECIAL INSPECTION WILL REFER TO THE APPROVED PLAN INSPECTION REQUIREMENTS. ANY
THE REQUIRED VERIFICATION AND SPECIAL INSPECTION FOR SEISM SHALL CONFORM TO THE FOLLOWING SECTIONS: VERIFICATION AND INSPECTION TASK CONTINUOUS I. SPECIAL INSPECTION FOR WELDING IN ACCORDANCE X		ADDITIONAL TESTS A SPECIFICATIONS WILL 4. INTERIM REPORTS WIL	ND INSPECTIONS RE . ALSO BE PERFORM .L BE SUBMITTED TO	QUIRED BY THE APPROVED PLANS AN 1ED. THE BUILDING OFFICIAL AND THE
WITH AISC 341 (IBC SECTION 1707.2) THE REQUIRED VERIFICATION AND SPECIAL INSPECTION FOR SEISM	IIC RESISTANCE	SECTION 1704.1.2. 5. A FINAL REPORT OF	SPECIAL INSPECTION	ESPONSIBLE CHARGE IN ACCORDANC NS DOCUMENTING REQUIRED SPECIAL
SHALL CONFORM TO THE FOLLOWING: (REFERENCE IBC 1707.3) VERIFICATION AND INSPECTION TASK CONTINUOUS I. INSPECT FIELD GLUING OPERATIONS OF ELEMENTS OF THE SEISMIC FORCE-RESISTING-SYSTEM	PERIODIC	INSPECTIONS SHALL E	BE SUBMITTED PRIOR (SECTION 1704.1.2).	OF ANY DISCREPANCIES NOTED IN TH R TO THE ISSUANCE OF A CERTIFICATE THE FINAL REPORT WILL DOCUMENT:
2. INSPECT NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC-FORCE-RESISTING SYSTEM, INCLUDING:	×	B. CORRECTION OF I	DISCREPANCIES NOT ZES HIS/HER OBLIGA	ED IN INSPECTIONS TION TO ENSURE THAT THE CONSTRUC DOCUMENTS AND TO IMPLEMENT THIS
a. WOOD SHEAR WALLSb. WOOD DIAPHRAGMSc. DRAG STRUTS, BRACES		PROGRAM OF SPECIA	AL INSPECTIONS. IN INER WILL RETAIN A	PARTIAL FULFILLMENT OF THESE ND DIRECTLY PAY FOR THE SPECIAL
d. SHEAR PANELS e. HOLD-DOWNS THE REQUIRED VERIFICATION AND SPECIAL INSPECTION FOR SEISM	IIC RESISTANCE	OFFICIAL WILL:		HE UNDERSTANDING THAT THE BUILDIN ATIONS OF THE SPECIAL INSPECTORS
SHALL CONFORM TO THE FOLLOWING: (REFERENCE IBC 1707.4) INSPECTION TASK OF COLD-FORMED STEEL FRAMING CONTINUOUS I. WELDING OF ELEMENTS OF THE SEISMIC-FORCE- X		WILL PERFORM TH B. MONITOR SPECIAL THE SPECIAL INSP	IE INSPECTIONS _ INSPECTION ACTIV PECTORS ARE QUALI	ITIES ON THE JOB SITE TO ASSURE TH FIED AND ARE PERFORMING THEIR
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2. INSPECT ERECTION AND FASTENING OF INTERIOR AND EXTERIOR NON-BEARING WALLS

3. INSPECT ERECTION AND FASTENING OF INTERIOR VENEER WEIGHING MORE THAN 5 psf

WEIGHING MORE THAN 15 psf

RESPONSIBILITY	FIRM	ADDRESS, TELEPHONE, E-MAIL
I. SPECIAL INSPECTION (EXCEPT FOR GEOTECHNICAL)	T.B.D.	-
2. MATERIAL TESTING	T.B.D.	-
3. GEOTECHNICAL	T.B.D.	-
INSPECTIONS		
4	-	-
	MENTO	
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RICHMOND HOFFMAYER	STRUCTURAL ENGINEERING 4425 CANOGA AVE WOODLAND HILLS CA 91364 (PHONE (818) 347-7008 FAX (818) 883-8869
VILLLAGE NESTS AT SUMMIT	STRUCTURAL SPECIAL INSPECTION NOTES
STATUS 100%	DESCR BUILDING D SUBMI
& d	64 5510M

MECHANICAL PLUMBING □ ELECTRICAL □ ENERGY □ ACCESSIBILITY □ FIRE PLAN REVIEW ACCEPTANCE OF DOCUMENT: DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS.

WEST COAST CODE CONSULTANTS, INC

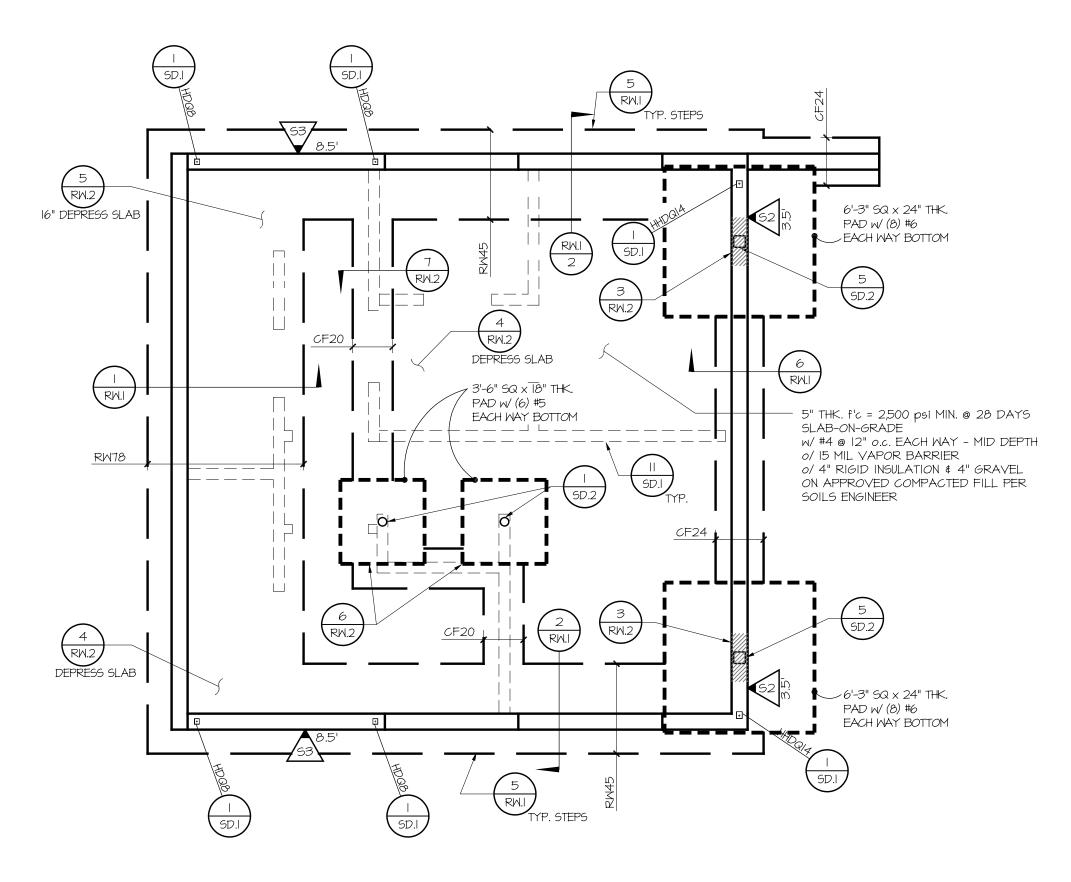
DEPARTMENT 08/27/2017 MITTAL #I

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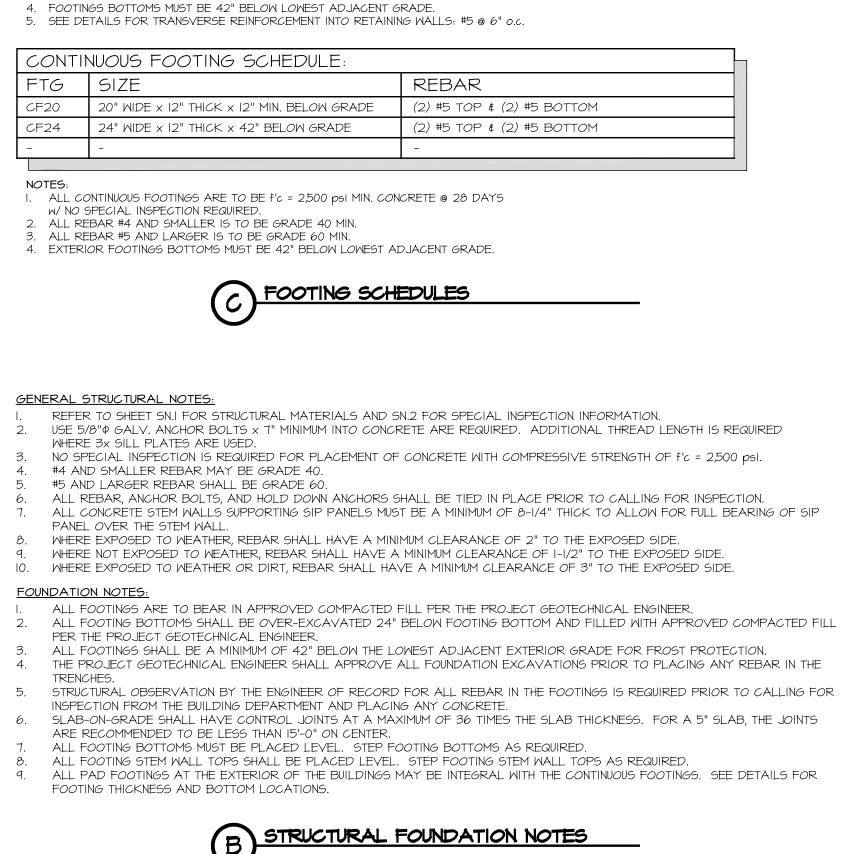
2017-0610

CHECKED BY:



FELDSPAR LVN - UNIT 18





CONTINUOUS FOOTING

- INDICATES CONCRETE

CONTINUOUS FOOTING

- INDICATES CONCRETE

STEM WALL ABOVE

FLOOR

STEM WALL BELOW

1 - - - 1 FLOOR

WALL ABOVE

REBAR

(3) #5 BOTTOM - CONTINUOUS - LONGITUDINAL

(5) #5 BOTTOM - CONTINUOUS - LONGITUDINAL

(6) #5 BOTTOM - CONTINUOUS - LONGITUDINAL

RETAINING WALL FOOTING SCHEDULE:

45" WIDE x I2" THICK x 42" BELOW GRADE

60" WIDE x 12" THICK x 42" BELOW GRADE

I. ALL CONTINUOUS FOOTINGS ARE TO BE f'c = 2,500 psi MIN. CONCRETE @ 28 DAYS w/

RW78 78" WIDE x 12" THICK x 42" BELOW GRADE

2. ALL REBAR #4 AND SMALLER IS TO BE GRADE 40 MIN. 3. ALL REBAR #5 AND LARGER IS TO BE GRADE 60 MIN.

NO SPECIAL INSPECTION REQUIRED.

FTG SIZE

NO NO NO

MECHANICAL

STATE, OR LOCAL REGULATIONS.

VEST COAST CODE CONSULTANTS, INC

ELECTRICAL

PLUMBING

ENERGY

VOID WITHOUT SIGNATURE & SEAL CHECKED BY: JOB NUMBER:

DESCRIPTION

BUILDING DEPARTMENT 08/27/2017 SUBMITTAL #1

SEE SCHEDULE THIS SHEET FOR INFO 2017-0610 - INDICATES DETAIL REFERENCE

— INDICATES MINIMUM

LENGTH OF PLY

- INDICATES SHEAR

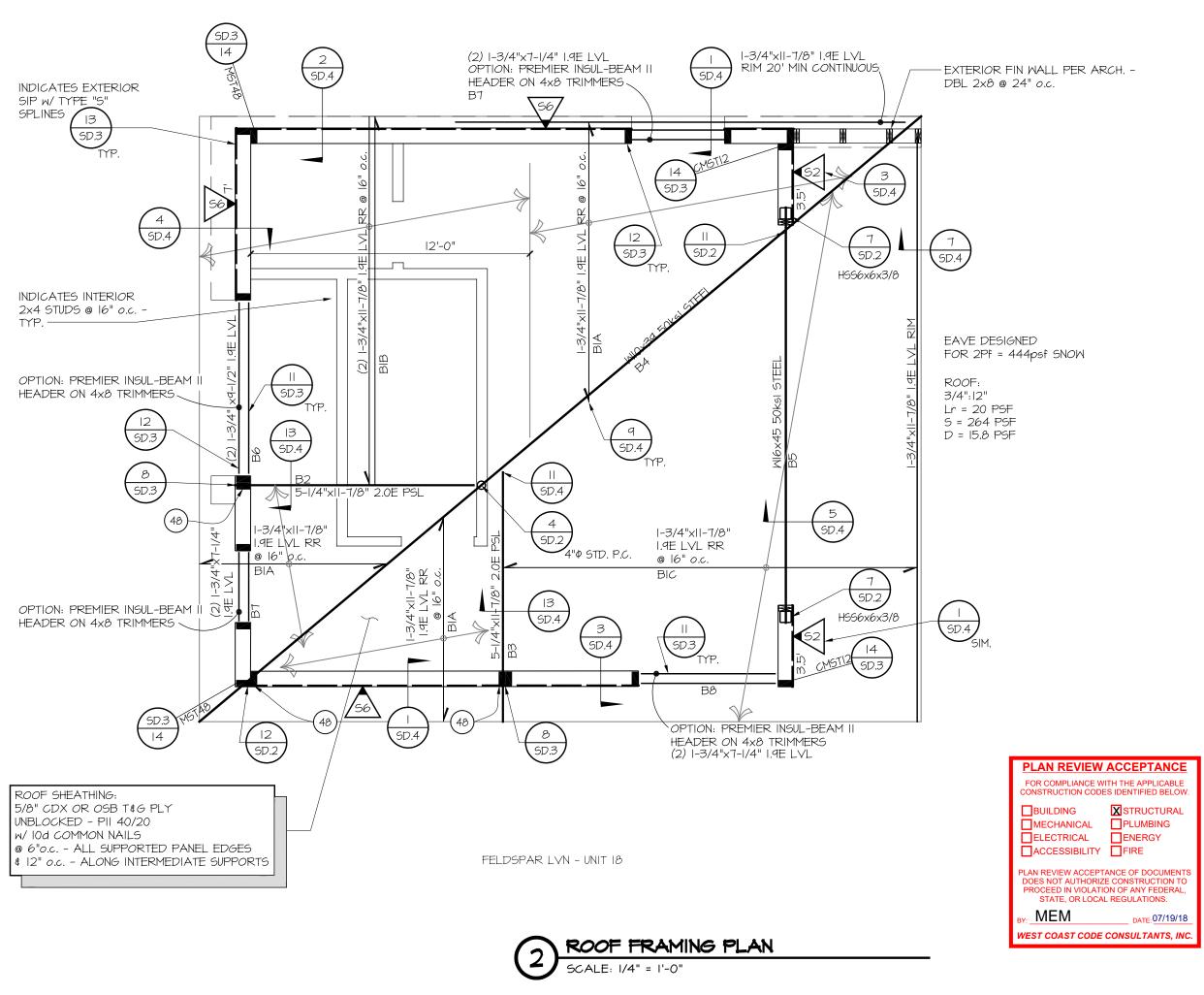
WALL REQUIRED -

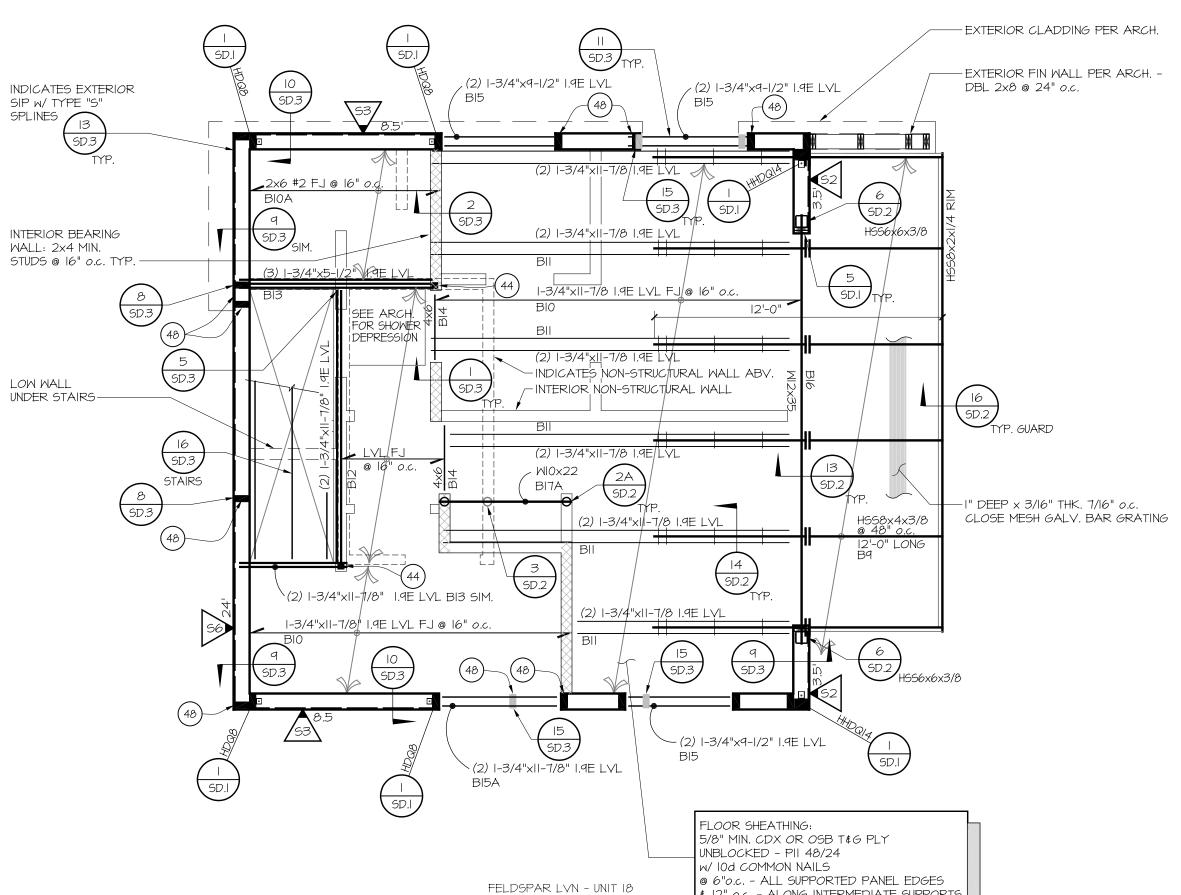
- INDICATES DETAIL

REFERENCE SHEET

SHEAR WALL

XX ft







\$ 12" o.c. - ALONG INTERMEDIATE SUPPORTS

MARK	MATERIAL THICKNESS STRUCT I PLY OR OSB	INTERIOR SPLINE TYPE	CHORD FRAMING AT PANEL ENDS	CHORD NAILING 0.113 x 2-3/8"	PLATE NAILING 0.113 x 2-3/8" ROUND HEAD	ANCHOR BOLT SPACING (3.) \$ (4.)	SPLINE FRAMING AT PANEL JOINTS	SPLINE NAILING 0.113 x 2-3/8"	SHEARWALL CAPACITY	SPECIAL INSPECTIC REQUIRED (9.)
SINGLE SIDED SHEAR WALLS										
56	1/2" EACH SIDE	TYPE "5"	4× MIN.	6" o.c.	6" o.c.	5/8"¢ @ 48 o.c.	3/4" THK. 3" WIDE	6" o.c.	360 plf	NO
53	I/2" EACH SIDE	TYPE "S"	4× MIN.	3" o.c. STAGGERED	3" o.c. STAGGERED	5/8"¢ @ 16" o.c.	23/32" THK. 3" WIDE	3" o.c. STAGGERED	720 plf	YES
52	I/2" EACH SIDE	TYPE "5"	4× MIN.	2" o.c. STAGGERED	2" o.c. STAGGERED	5/8"¢ @ 8" o.c.	23/32" THK. 3" WIDE	2" o.c. STAGGERED	920 plf	YES

MARK	MATERIAL THICKNESS STRUCT I PLY OR OSB	NAILING AT ALL PANEL EDGES (1.) \$ (2.)	SILL PLATE & FRAMING @ ADJOINING PANEL EDGES	ANCHOR BOLT SPACING (3.) \$ (4.)	SILL NAIL SPACING FOR PLY TO 3/4" THK (5.)	SIMPSON SDS 1/4 × 6 SCREIVE TO SILL PLATE (6.)	A35 CLIP SPACING-TOP PLATE TO FRAM'G ABV.	SEISMIC SHEARWALL CAPACITY (8.)	WIND SHEARWALL CAPACITY	SPECIAL INSPECTION REQUIRED (9.)
SINGLE	SIDED SHEAR	WALLS	•							
6	15/32"	10d @ 6" o.c.	2x MIN.	5/8"¢ @ 48 o.c.	16d @ 5" o.c.	18" o.c.	18" o.c.	340 plf	349 plf	NO
$\overline{\wedge}$	15/32"	10d @ 4" o.c.	3x MIN.	5/8"Φ @ 24" <i>o.c.</i>	-	9" o.c.	8" o.c. (8.)	510 plf	714 plf	YES

SHEAR WALL SCHEDULE FOOTNOTES:

- ALL PLYWOOD PANEL EDGE NAILING IS TO BE COMMON NAILS WITH IOD HAVING I-5/8" MINIMUM PENETRATION INTO FRAMING. ALL NAILS ARE TO HAVE I/2" MINIMUM EDGE DISTANCE FROM PANEL ENDS AND EDGES. DO NOT BREAK SURFACE LAM OF PLY
- WITH NAIL HEAD. 3. 5/8" A307 ANCHOR "J" BOLTS x 7" MIN. INTO CONCRETE FOOTINGS. NOTE: ADDITIONAL THREAD LENGTH IS REQUIRED FOR 3X
- ALL ANCHOR BOLTS SHALL USE 3"x3"x5/16" PLATE WASHERS. DIAGONALLY SLOTTED PLATE WASHERS MAY BE USED W/ ADDITIONAL CUT WASHER PLACED BETWEEN THE NUT AND THE PLATE WASHER. THE DIAGONAL SLOT MAY BE L3/4" LONG x 13/16"
- 5. USE COMMON NAILS FOR CONNECTING PLATES TO JOISTS AND BLOCKING 16d FOR 2x AND 30d FOR 3x. USE 3x NOMINAL
- BLOCKING OR RIM JOIST FOR ALL SILL NAILING. NAILS SHALL BE AT LEAST 1/2" FROM ALL EDGES OF SILL AND BLOCKING. WHERE MULTIPLE ROWS ARE REQUIRED, SPACE ROWS 1/2" MIN. BUT TAKE CARE NOT TO SPLIT THE WOOD.
- 6. SDS 1/4"x6" WOOD SCREWS BY SIMPSON SHALL BE INSTALLED PER ICC-ES ESR-2236 SHALL HAVE 2-3/4" MIN. EMBED INTO FRAMING BELOW.
- LOAD VALUES ARE BASED ON THE MINIMUM CAPACITIES FROM THE IBC TABLE 2306.4.I. WHERE CLIP SPACING PREVENTS CLIPS ON A SINGLE SIDE OF THE WALL FROM FITTING, ALTERNATE THE CLIPS ON EACH SIDE OF THE TOP PLATE TO THE RIM/BLOCKING ABOVE AND SPACE EACH LINE OF CLIPS 2 TIMES THE VALUE SHOWN IN THE SCHEDULE
- 9. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC-FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR
- PANELS AND HOLD DOWNS. 10. SHEAR WALL INFORMATION FOR SEISMIC DESIGN CATEGORIES A, B, C, D, E & F, PER NTA REPORT PRS032808-3, TABLE 10.

NG HEADERS
4x4 #2
4x6 #2
4x8 #2
4xI0 #2
4xI2 #2

- I. USE HEADERS LISTED IN TABLE FOR ALL "HDR" ON PLANS UNLESS NOTED OTHERWISE
- REFER TO SHEET SN.I FOR NAILING INFORMATION 3. MATCH "HDR" WIDTH TO STUD WIDTH ON PLANS WHERE LARGER
- ALLOWABLE SPANS JOIST SPACING @ 16" o.c. 2×12 ALL JOISTS ARE TO SPRUCE/PINE/FIR #2 MIN.

CEILING JOIST SCHEDULE

- SPANS ARE BASED ON 20 psf LIVE LOAD \$ II psf DEAD LOAD SPANS ARE LIMITED TO L/240 LIVE LOAD
- DEFLECTION 4. REFER TO SHEET SN.I FOR NAILING INFORMATION

(C) TYPICAL FRAMING SCHEDULES

- REFER TO SHEET SN.I FOR STRUCTURAL MATERIALS AND SN.2 FOR SPECIAL INSPECTION INFORMATION.
- ALL BOLT HOLES SHALL BE DRILLED 1/32" TO 1/16" OVERSIZED. FOR LAG SCREWS, PROVIDE A LEAD HOLE 40% TO 70% OF THE THREADED SHANK DIAMETER AND A FULL DIAMETER LOAD HOLE FOR THE SMOOTH PORTION OF THE SHANK.
- 4. ALL HOLD DOWN BOLTS MUST BE TIGHTENED JUST PRIOR TO INSPECTION AND AGAIN JUST PRIOR TO COVERING THE WALL ALL NAILS ARE TO BE COMMON NAILS.
- ALL FRAMING SHALL BE SPRUCE/PINE/FIR #2 OR BETTER, U.N.O.
- ALL NAILS THROUGH SHEATHING ARE TO HAVE I/2" MINIMUM EDGE DISTANCE FROM PANEL ENDS AND EDGES. ALL EXTERIOR WALLS ARE TO BE 2x6 #2 STUDS MIN. @ 16" o.c., UNLESS NOTED AS SIP.
- ALL INTERIOR BEARING AND SHEAR WALLS ARE TO BE 2x4 #2 STUDS MIN., U.N.O. @ 16" o.c IO. ALIGN ALL JOISTS, RAFTERS, AND/OR TRUSSES OVER STUDS BELOW, U.N.O. ADD STUDS AS NECESSARY.
- HEADERS, CEILING JOISTS, FLOOR JOISTS, AND ROOF RAFTERS NOT SPECIFICALLY SIZED ON THE PLANS SHALL BE PER THE "CONVENTIONAL LIGHT-FRAME CONSTRUCTION" SECTION OF THE CURRENT CODE - SCHEDULES THIS SHEET.
- FOR ALLOWABLE HOLES IN PLYWOOD-WEB JOISTS, REFER TO MANUFACTURERS INFO. 13. ROOF AND FLOOR SHEATHING BLOCKING MAY BE FLAT 2x AT ALL PANELS EDGES UNLESS NOTED AS FULL DEPTH BLOCKING. SHEAR WALL BLOCKING MUST BE FULL DEPTH OF WALL STUDS. SEE THE SHEAR WALL SCHEDULE FOR MINIMUM THICKNESS
- 14. REFER TO ARCH. PLANS FOR FIRE BLOCKING REQUIREMENTS. 15. ROOF AND FLOOR SHEATHING SHALL BE PLACED WITH FACE GRAIN PERPENDICULAR TO ALL SUPPORTS, FASTENED WITH COMMON NAILS PER PLAN, AND INSPECTED PRIOR TO COVERING.
- SHEATHING OVER OR UNDER ALL EAVES SHALL BE CCX, EXPOSURE I PLYMOOD OR T&G., U.N.O. 17. BOUNDARY NAIL PLYWOOD SHEATHING ALONG ALL DRAG BEAMS W/ COMMON NAILS @ 6" o.c. TO MATCH REQUIRED DIAPHRAGM NAIL SIZES ON PLAN, U.N.O.
- 18. FRAMING CLIPS ARE TO BE SIMPSON LTP4 OR A35 PER SHEAR WALL SCHEDULE, U.N.O. 8d x 1-1/2" COMMON NAILS MAY BE USED WHERE NOT APPLIED OVER PLYWOOD. 8d COMMON NAILS WILL ACHIEVE 100% LOAD CAPACITY THROUGH PLYWOOD. 20. SPECIAL INSPECTION IS REQUIRED FOR ALL SHEAR WALLS WITH NAILING 4" ON CENTER AND CLOSER. REFER TO THE SHEAR WALL SCHEDULE FOR INFORMATION.

GENERAL SHEAR WALL SCHEDULE NOTES:

- A. ALL PLYWOOD IS TO BE STRUCTURAL I GRADE w/ (4) PLIES MINIMUM, AND SHALL BE APPLIED DIRECTLY TO FRAMING MEMBERS. PLY SHEETS MAY BE APPLIED EITHER VERTICALLY OR HORIZONTALLY ACROSS THE STUDS. WHERE STUDS ARE SPACED AT 16" o.c., PLY IS TO BE NAILED TO ALL INTERMEDIATE STUDS AT 12" o.c. WHERE STUDS ARE SPACED FARTHER THAN 16" o.c., PLY 15 TO BE NAILED TO ALL INTERMEDIATE STUDS @ 6" o.c.
- ALL PLYWOOD JOINT NAILING AND SILL NAILING IS TO BE STAGGERED. ALL ANCHOR BOLTS MUST USE 3"x3"x5/16" PLATE WASHERS W/ DIAGONAL SLOTS ALLOWED. NO CUT WASHERS ARE ALLOWED. SEE STRUCTURAL FRAMING NOTES FOR ADDITIONAL INFORMATION. ALL ANCHOR BOLTS ARE TO BE INSTALLED INTO 2,500 psi MINIMUM CONCRETE @ 28 DAYS OR SOLID GROUTED MASONRY PER
- PROVIDE PRE-DRILLED HOLES 65% TO 75% OF THE NAIL DIAMETER FOR NAILS LARGER THAN 20d. PRE-DRILL ALL PILOT HOLES FOR LAG SCREWS. REFER TO STRUCTURAL FRAMING NOTES FOR ADDITIONAL INFORMATION. ALL LAGS SHALL BE FASTENED INTO THE CENTERLINE OF THE RIMS OR BLOCKING BELOW THE PLY DIAPHRAGM WHERE OCCUR. J. STRUCTURAL OBSERVATION IS REQUIRED FOR ALL SHEAR WALL PANELS.

- INDICATES MINIMUM

POST SIZE $48 = 4 \times 8$

— INDICATES MINIMUM

LENGTH OF PLY

- INDICATES SHEAR

WALL REQUIRED -

SHEET FOR INFO

- INDICATES MAXIMUM

DESIGN FORCE ON

- INDICATES DETAIL

- INDICATES DETAIL

REFERENCE SHEET

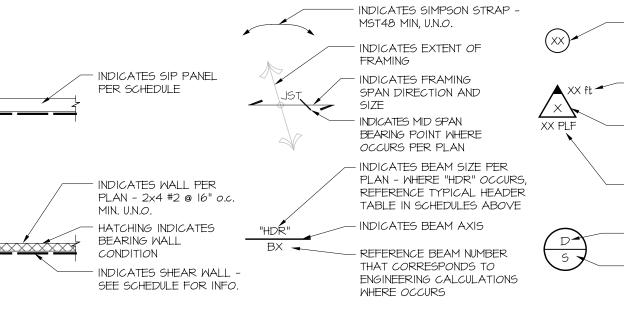
SHEAR WALL

REFERENCE

SEE SCHEDULE THIS

SHEAR WALL





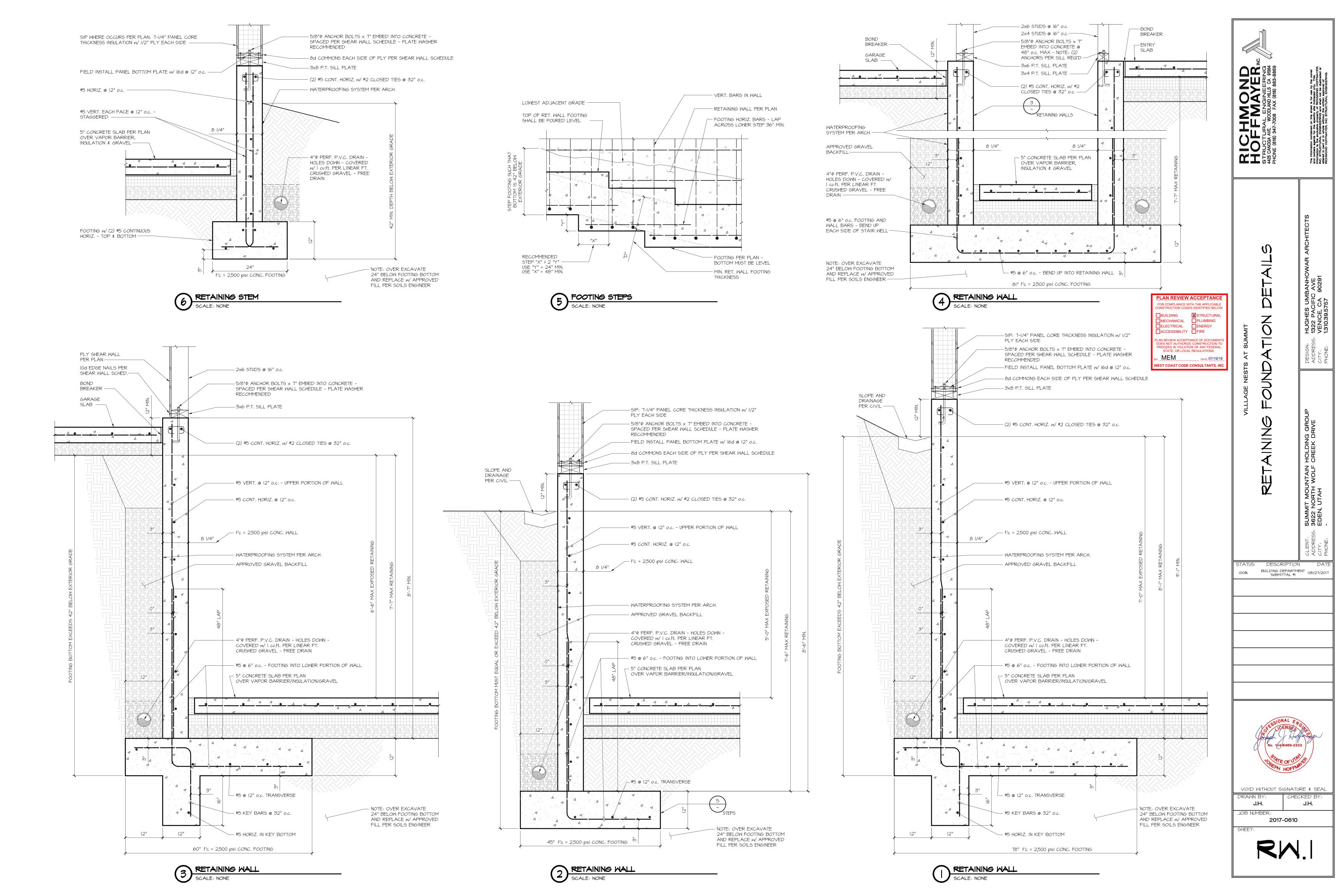
PLAN, U.N.O. SEE GENERAL NOTES FOR ADDITIONAL INFORMATION.

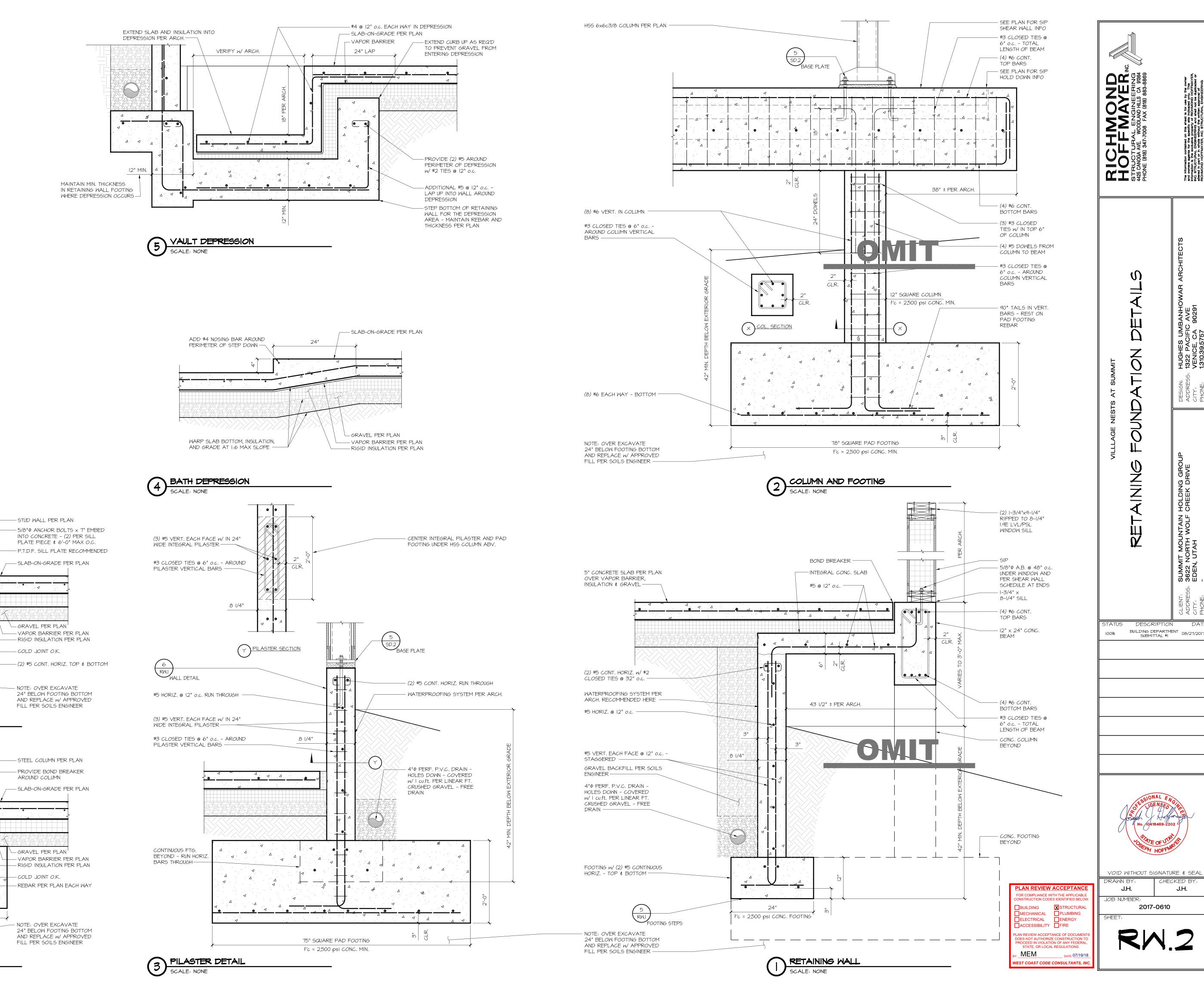
STATUS DESCRIPTION BUILDING DEPARTMENT 08/27/2017 SUBMITTAL #1

No. 10418469-2202

VOID WITHOUT SIGNATURE & SEAL CHECKED BY: JOB NUMBER:

2017-0610





20" MIN.

INTERIOR CONTINUOUS FOOTING

SCALE: NONE

SD.2 BASE PLATE

PAD FOOTING

f'c = 2,500 psi CONC.

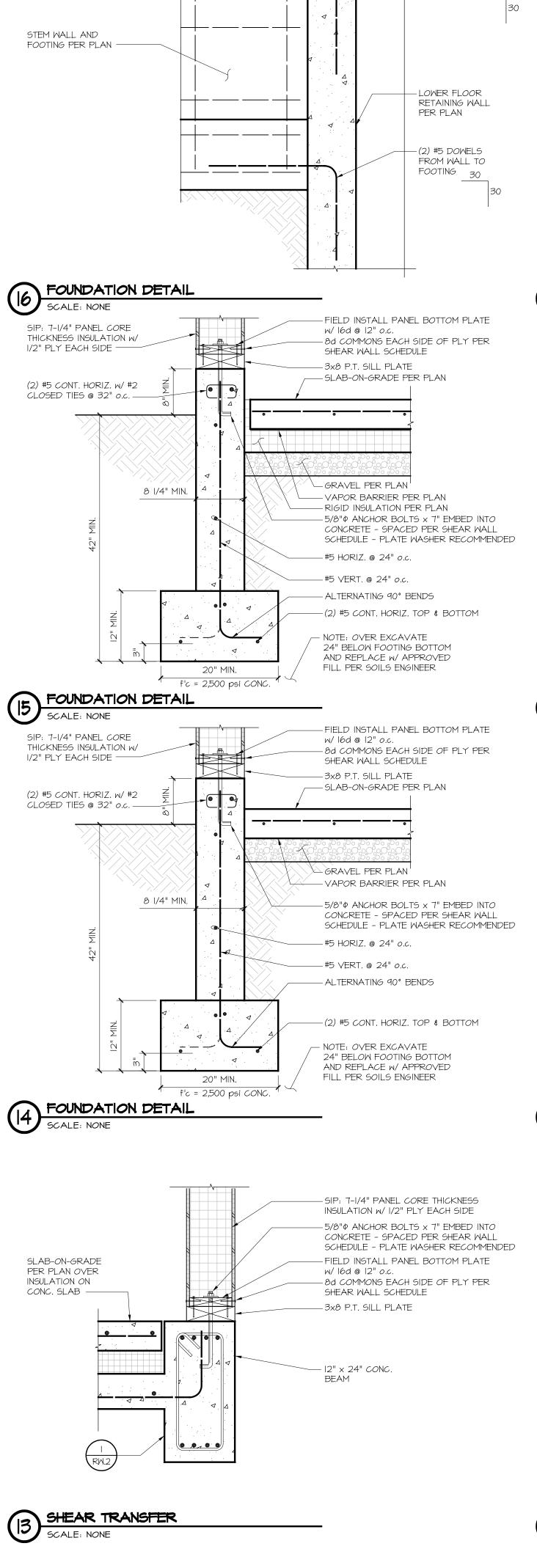
SQUARE PAD PER PLAN

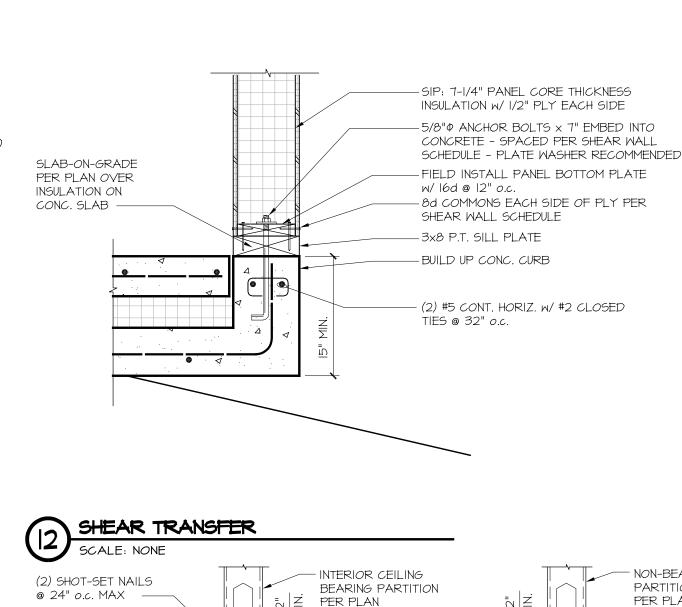
f'c = 2,500 psi CONC.

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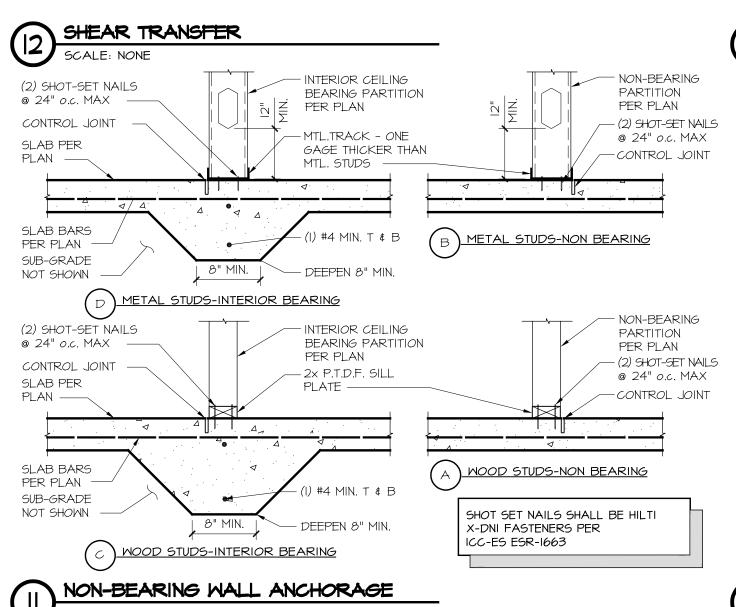


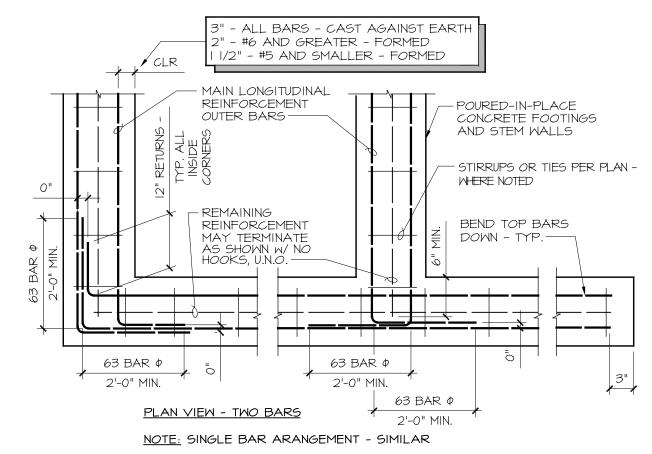


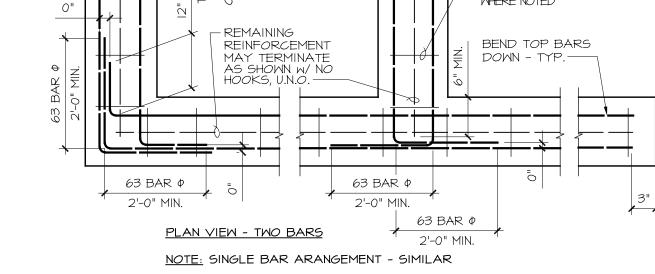
(2) #5 DOWELS FROM

WALL TO TOP OF

STEM WALL



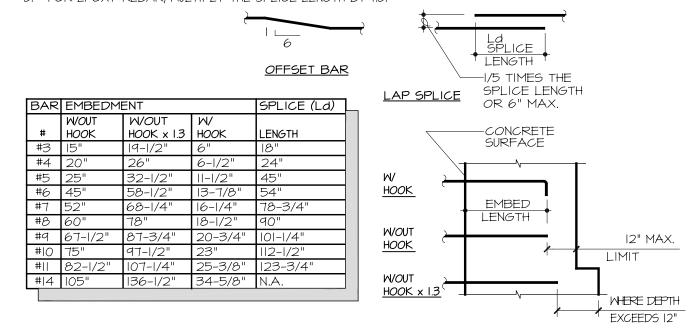


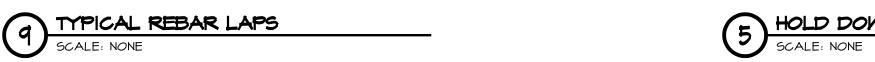


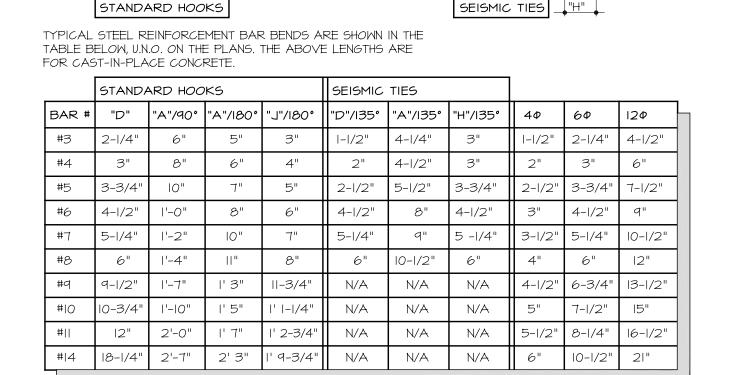


TYPICAL SPLICE LENGTHS ARE SHOWN IN THE TABLE BELOW, U.N.O. ON THE PLANS. THESE SPLICES ARE FOR CLASS A SPLICES. WHERE CLASS B SPLICES OCCUR, SPECIFIC DETAILS WILL BE PROVIDED. CLASS B SPLICES ARE 1.3 x Ld.

- THE TABLE LENGTHS ARE FOR CAST-IN-PLACE CONCRETE. 2. ALL REINFORCING STEEL LAPS OR SPLICES SHALL BE AS INDICATED ON THE PLANS, WHERE LAP OR SPLICE LOCATIONS ARE NOT SPECIFICALLY INDICATED. LAPS OR SPLICES SHALL BE STAGGERED
- AT LEAST 2'-0" LONG 3. SPLICES ARE BASED ON F'C = 2,500 psi MIN. HIGHER STRENGTH CONCRETE DOES NOT REQUIRE
- LONGER SPLICES. 4. WHERE MORE THAN 12" OF CONCRETE EXISTS BELOW THE REBAR, THE EMBED IS 1.3 X DEEPER THAN
- THE STANDARD WOUT HOOK EMBED. 5. FOR EPOXY REBAR, MULTIPLY THE SPLICE LENGTH BY 1.5.







DIMENSION

DIMENSION

TYPICAL REBAR BENDS

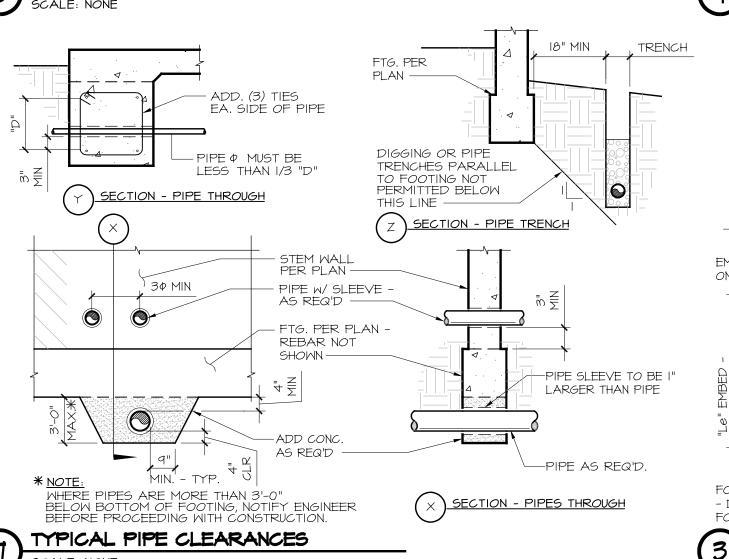
<u>90° BEND</u>

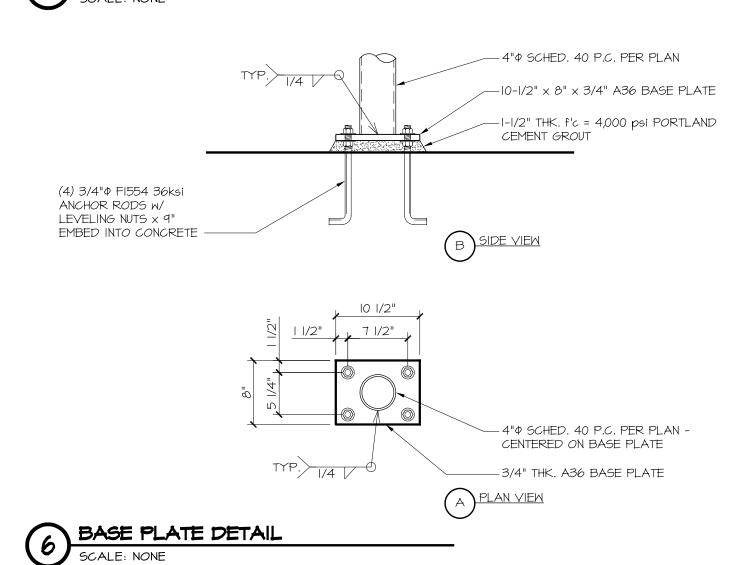
DETAILING '

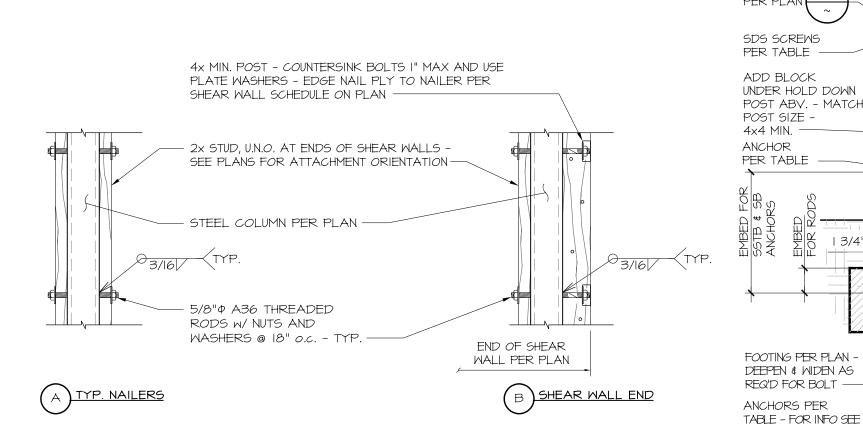
DIMENSION

"D"—

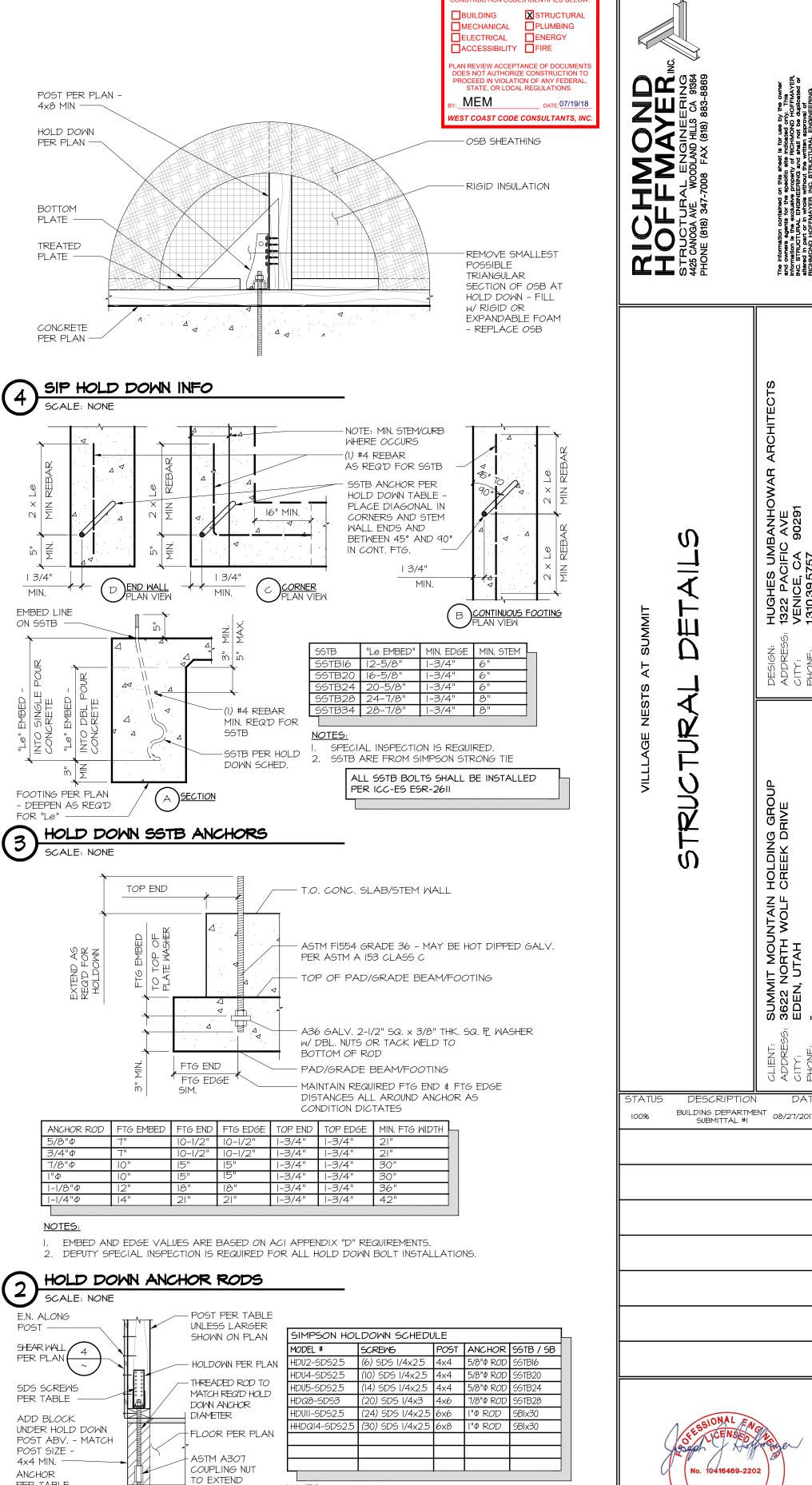
<u>135° HOOK</u>











I. THREADED ROD TO BE GALV. ASTM FI554 GRADE

2. TIGHTEN ALL BOLTS PRIOR TO COVERING.

SDS SCREW AXIS - SEE BELOW ...

AND FOOTING END/CORNER - PLAN VIEW.

STRONG TIE.

36. ALTERNATIVE: SSTB BOLTS PER SIMPSON

3. MAINTAIN A MINIMUM OF 5" BETWEEN ANCHOR BOLT

4. ORIENT POST SO LONG DIMENSION IS PARALLEL TO

ALL HDU SHALL BE INSTALLED PER ICC-ES ESR-2330

ALL HDQ SHALL BE INSTALLED PER ICC-ES ESR-2330

ALL HHDQ SHALL BE INSTALLED PER ICC-ES ESR-2330

-HOLD DOWN

VOID WITHOUT SIGNATURE & SEAL

2017-0610

CHECKED BY:

DRAWN BY

J.H.

JOB NUMBER:

SHEET:

ANCHOR

STEM.

1 3/4"

HOLD DOWN DETAIL

FOR COMPLIANCE WITH THE APPLICABLE

