

GENERAL NOTES:

- VISITS TO THE JOB SITE BY REPRESENTATIVES OF THE ENGINEER DO NOT SUBSTITUTE APPROVAL OF THE WORK PERFORMED BY THE CONTRACTOR OR HIS SUBCONTRACTORS AND ARE MERELY FOR THE PURPOSE OF OBSERVING THE WORK PERFORMED.
- CONTRACTOR SHALL NOTIFY ENGINEER/ARCHITECT OF ANY DISCREPANCIES, OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS BEFORE PROCEEDING WITH WORK INVOLVED IN ALL CASES, UNLESS OTHERWISE DIRECTED. THE MOST STRINGENT REQUIREMENTS SHALL GOVERN AND BE PERFORMED.
- CONTRACTOR SHALL VERIFY ALL CONDITIONS, DIMENSIONS AND ELEVATIONS, ETC., AT THE SITE AND SHALL CORROBORATE WORK PERFORMED BY ALL TRADES. **DO NOT SCALE DRAWINGS**
- SHOP DRAWINGS SHALL BE PREPARED BY THE ENGINEER/ARCHITECT PRIOR TO FABRICATION OR ERECTION FOR ANY PRE-ERECTED MEMBERS AND SHALL BE STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THIS STRUCTURE RESIDES.
- SIZES, LOCATIONS, LOADS, AND ANCHORAGE OF EQUIPMENT SHALL BE VERIFIED IN THE FIELD WITH EQUIPMENT MANUFACTURERS (SUPPLIERS) PRIOR TO FABRICATION OR INSTALLATION OF SUPPORTING STRUCTURES.
- TEMPORARY BRACING SHALL BE PROVIDED WHEREVER NECESSARY TO TAKE CARE OF ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED, INCLUDING WIND. SUCH BRACING SHALL BE LEFT IN PLACE AS LONG AS MAY BE REQUIRED FOR SAFETY, OR UNTIL ALL STRUCTURAL ELEMENTS ARE INSTALLED.
- URING AND AFTER CONSTRUCTION THE CONTRACTOR AND/OR OWNER SHALL KEEP LOADS ON THE STRUCTURE WITHIN THE LIMITS OF THE DESIGN LOAD.
- THE CONTRACTOR AND ALL SUBCONTRACTORS SHALL PERFORM THEIR TRADES AND DUTIES IN A MANNER CONFORMING TO THE PROCEDURES AND REQUIREMENTS AS STATED IN THE 2009 INTERNATIONAL BUILDING CODE (OR LATEST) ACCEPTED CODE ADOPTED BY THE LOCAL BUILDING OFFICIALS).
- ANY SPECIAL INSPECTIONS REQUIRED BY THE BUILDING OFFICIAL OR THE INTERNATIONAL BUILDING CODE ARE THE RESPONSIBILITY OF THE OWNER.
- CONTRACTOR SHALL BE RESPONSIBLE FOR SAFETY AND PROTECTION WITHIN AND ADJACENT TO THE JOB SITE.

CONCRETE MASONRY NOTES:

- CONCRETE MASONRY WALLS SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF $f'_m = 1500$ PSI.
- CONCRETE MASONRY UNITS SHALL BE LIGHT WEIGHT UNITS CONFORMING TO ASTM C90, GRADE N-1.
- MORTAR SHALL BE TYPE "M", BELOW GRADE, TYPE "S" ABOVE GRADE CONFORMING TO ASTM C270.
- WHERE NO FOOTING OR FOUNDATION WALL IS SHOWN UNDER A MASONRY WALL, FLOORS UNDER SUCH WALL SHALL INCLUDE A THICKENED SLAB.
- REINFORCED CONCRETE BLOCK WALLS
 - SEE PLANS FOR LOCATION OF REINFORCED WALLS.
 - WHEN ONE BAR IN A SINGLE CORE, PLACE IN CENTER UNLESS NOTED OTHERWISE.
 - WHEN TWO BARS IN A SINGLE CORE, PLACE 2" FROM EACH FACE UNLESS NOTED OTHERWISE.
 - FILL BLOCK CORE AT VERTICAL STEEL WITH 2000 PSI GROUT, FRODD OR VIBRATED IN PLACE. DO NOT USE PERMANENT REINFORCING STEEL TO CONSOLIDATE CONCRETE.
 - PLACEMENT OF CORE GROUT SHALL HAVE A MAXIMUM HEIGHT OF 4'-0" UNLESS CLEAR OUT HOLES ARE PROVIDED AT THE BOTTOM OF EACH GROUT LIFT, THEN A MAXIMUM HEIGHT OF 8'-0" BEFORE PLACEMENT OF CORE GROUT.
 - REINFORCING STEEL SHALL BE PLACED AT THE BOTTOM OF EACH GROUT LIFT, THEN A MAXIMUM HEIGHT OF 8'-0" BEFORE SCHEDULE FOR LAP LENGTH.
- CONCRETE BLOCK WALL UNITS: ALL OPENINGS IN CONCRETE BLOCK WALL NOT SPECIFICALLY INDICATED
 - HAVE OTHER TYPES OF UNITS SHALL HAVE REINFORCED BLOCK UNITS AS FOLLOWS:
 - SPANS 6'-1" TO 10'-0" HAVE 1# DEEP UNITS BLOCKS.
 - 4" & 6" WIDE UNITS TO HAVE (2) #5 B/M.
 - 8, 10, & 12" WIDE UNITS TO HAVE (2) #5 B/M.
 - WHERE UNITS IS CONTINUOUS OVER TWO OPENINGS WITH INTERMEDIATE SUPPORT, USE (2) #3 TOP AND (2) #5 BOTTOM, ALL BLOCK UNITS TO BE FILLED WITH 3,000 PSI CONCRETE.
 - EXTEND ALL UNITS A MINIMUM OF 8" BEYOND EACH EDGE OF OPENING.
 - WHERE UNITS BEARS ON CONCRETE BLOCK, FILL TWO COURSES OF BLOCK MINIMUM WITH CONCRETE.
 - HORIZONTAL REINFORCING SHALL BE CONTINUOUS AROUND ALL WALL INTERSECTIONS.
 - WRAP ALL OPENINGS W/(2) #3 REBAR EXTENDING PAST THE EDGE OF OPENING BY 24" MIN. EACH DIRECTION.
 - ALL OPENINGS GREATER THAN 6'-0" SHALL HAVE MASONRY COLUMN ON EACH SIDE TO BE 8"x16" W/(4) #5 BARS VERTICAL, TIED W/(2) #3 TIES @ 8" O.C. (U.N.O.).

DEFINITIONS:

- SPECIAL INSPECTION: INSPECTION AS HEREIN REQUIRED OF THE MATERIALS, WORKMANSHIP, AND CONSTRUCTION OF THE STRUCTURE. SPECIAL INSPECTIONS REQUIRE SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS (SEE SECTION 1709).
- SPECIAL INSPECTION, CONTINUOUS: THE FULL-TIME OBSERVATION OF WORK PERFORMED BY THE CONTRACTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
- SPECIAL INSPECTION, PERIODIC: THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK PERFORMED BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.
- STRUCTURAL OBSERVATION: THE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM AS CONSTRUCTED. THE SPECIAL INSPECTOR SHALL VERIFY THE WORKMANSHIP AND MATERIALS AS SHOWN ON THE APPROVED CONSTRUCTION DOCUMENTS. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTION REQUIRED BY SECTION 1709, SECTION 1704 OR OTHER SECTIONS OF THIS CODE.

CONTRACTOR RESPONSIBILITY:

- ACKNOWLEDGMENT OF AMENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE QUALITY ASSURANCE PLAN.
- ACKNOWLEDGMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIALS.
- PROCEEDURE FOR EXERCISE CONTROL WITHIN THE CONTRACTORS ORGANIZATION AND FREQUENCY OF REPORTING, AND THE DISTRIBUTION OF THE REPORTS.
- IDENTIFICATION AND QUALIFICATIONS OF THE PERSON(S) EXERCISING SUCH CONTROL AND THEIR POSITIONS IN THE ORGANIZATIONS.

FOOTINGS, FOUNDATIONS AND SLAB ON GRADE NOTES:

- ANY SOIL CONDITION ENCOUNTERED DURING EXCAVATION THAT IS CONTRARY TO THOSE USED FOR DESIGN OF FOOTINGS AS OUTLINED IN THE WORKING DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING.
- ALL FOOTINGS SHALL BEAR ON SOIL OF UNIFORMED GRANULAR FILL COMPACTED TO 95% OF MAX. DENSITY, BASED ON ASTM D 1587 METHOD OF COMPACTION. FILL SHALL BE PLACED IN LAYERS NOT TO EXCEED SIX INCHES IN DEPTH AFTER COMPACTION AND SHALL EXTEND DOWN TO IN-SITU SOILS. FILL SHALL BE COMPACTED UNDER ALL CONCRETE WORK ON THE SITE.
- NO FOOTINGS SHALL BE PLACED IN WATER, SNOW, FROZEN GROUND, OR UNSTABLE SOILS.
- ALL EXCAVATIONS ADJACENT TO AND BELOW FOOTING ELEVATION FOR OTHER TRADES SHALL BE ACCOMPLISHED PRIOR TO POURING ANY FOOTINGS.
- CONTRACTOR SHALL BE RESPONSIBLE FOR LATERALLY SUPPORTING ALL REMAINING THE FOUNDATION WALLS WHILE COMPACTION BEHIND WALLS AND UNTIL ALL SUPPORTING MEMBERS HAVE BEEN PLACED (SUCH AS FLOOR SLABS) WITH OPEN EXCAVATIONS AND FRENCHES SHALL BE SUPPORTED AND BARREADED BY CONTRACTOR TO CONFORM WITH OSHA SAFETY STANDARDS.
- ALL REINFORCEMENTS SHALL BE SECURELY TIED IN PLACE PRIOR TO POURING CONCRETE.
- PROVIDE DOWNERS IN FOOTING AND FOUNDATIONS TO MATCH ALL VERTICAL BARS IN WALLS AND COLUMNS ABOVE, UNLESS NOTED OTHERWISE.
- PROVIDE CONTROL JOINTS IN SLABS AT A MAX. OF 15 FT. O.C. EACH WAY AND AS SHOWN ON PLANS. POUR SLABS BETWEEN CONTROL JOINTS, SO THAT ADJACENT POURS ARE STAGGERED AT LEAST TWO DAYS APART.

CONCRETE NOTES:

- ALL COLUMNS, RETAINING WALLS AND ALL EXTERIOR FLATWORK, CURBS, GUTTERS, ETC., SHALL BE NORMAL WEIGHT CONCRETE WITH A COMPRESSIVE STRENGTH EQUAL TO AT LEAST 4,000 PSI. PER SQUARE INCH WITHIN 28 DAYS AFTER POURING. THE WATER/CEMENT RATIO SHALL BE NO GREATER THAN 0.45 AND SLUMP SHALL BE 4" OR LESS. MINIMUM CEMENT CONTENT SHALL BE 94# LBS. PER CUBIC YARD.
- ALL FOOTINGS, FOUNDATIONS, AND INTERIOR SLABS ON GRADE SHALL BE NORMAL WEIGHT CONCRETE WITH A COMPRESSIVE STRENGTH EQUAL TO A LEAST 3,000 PSI. PER SQUARE INCH WITHIN 28 DAYS AFTER POURING. THE WATER/CEMENT RATIO SHALL BE NO GREATER THAN 0.50 AND SLUMP SHALL BE 4" OR LESS. MINIMUM CEMENT CONTENT SHALL BE 97# LBS. PER CUBIC YARD.
- UNLESS OTHERWISE NOTED, ALL FOUNDATION WALL VERTICAL, COUD JOINTS SHALL BE KEPT WITH A KEY 1"-1/2" DEEP, A LENGTH 2" LESS THAN THE MEMBER, AND A WIDTH 1/2" OF THE MEMBER. REINFORCING SHALL BE CONTINUOUS THRU JOINT.
- ALL METAL BRIDGEWORK SHALL BE PREPARED TO BE REINFORCED WITH #2 BARS. AND SHALL CONFORM TO THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS ASTM A615 GRADE 60 BEAM AND COLUMN. THE REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF THE STANDARD SPECIFICATION A51M. A615 GRADE 60.
- ALL SLICES IN CONTINUOUS CONCRETE REINFORCING BARS SHALL LAP 18 BAR DIAMETERS. ALL SUCH SLICES SHALL BE MADE IN A REGION OF COMPRESSION UNLESS OTHERWISE SHOWN.
- ALL REINFORCEMENT BARS SHALL BE SECURELY ANCHORED AND SHALL BE SPACED FROM THE FORMS (UNLESS SHOWN OTHERWISE) AS FOLLOWS:
 - 2" IN BEAMS AND COLUMNS.
 - 2" IN PROTECTED WALLS AND SUSPENDED SLABS.
 - 3" ABOVE BOTTOM AND SIDES OF FOOTINGS.
- ALL OPENINGS IN CONCRETE WALLS SHALL BE REINFORCED WITH (2) #3 BARS EXTENDING 2'-0" MIN. BEYOND THE EDGE OF THE OPENING AT EACH FACE OF THE OPENING.
- ALL CONCRETE WORK SHALL BE PLACED, CURED, STRIPPED, AND PROTECTED AS DIRECTED BY THE SPECIFICATIONS AND ACI STANDARDS AND PRACTICES.
- BEFORE CONCRETE IS POURED CHECK WITH ALL TRADES TO INSURE PROPER PLACEMENT OF ALL OPENINGS, STEELERS, CURBS, CONDITIONS, BOTS, INSERS, ETC. RELATIVE TO WORK.
- CONTRACTOR IS RESPONSIBLE FOR ALL SHORING AND FORMWORK.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 AND SHALL HAVE A MINIMUM SIDE LAP OF 8" IN 315 AND ACI STANDARD 318-08.
- FOR STEPS IN FOUNDATION GREATER THAN 2 FEET, WIRE CORNER W/(2) #4 BARS EXTENDING 18" EACH DIRECTION.
- STRUCTURAL CONCRETE HAS BEEN DESIGNED AT 2,500 LBS. PER SQUARE INCH AND SPECIFIED AT A HIGHER STRENGTH CONCRETE AS STATED IN THE CONCRETE NOTES. **NO SPECIAL INSPECTIONS ARE REQUIRED** AS PER IRC SECTION 1704.4.

SPECIAL INSPECTION SCHEDULE

REQUIRED VERIFICATION AND INSPECTION OF MASONRY (LEVEL 1)		VERIFICATION & INSPECTION	FREQUENCY OF INSPECTION	IBC REFERENCE	REFERENCE FOR CRITERIA
AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:			CONTINUOUS	PERIODIC	ACI 530.1: 2.6A
PROPORTIONS OF SITE-PREPARED MORTAR			X	X	ACI 530.1: 3.3B
CONSTRUCTION OF MORTAR JOINTS			X	X	ACI 530.1: 3.4, 3.6A
LOCATION OF REINFORCEMENT AND CONNECTORS			X	X	
THE INSPECTION PROGRAM SHALL VERIFY:			X	X	ACI 530.1: 3.3G
SIZE AND LOCATION OF STRUCTURAL ELEMENTS			X	X	ACI 530.1: 1.13
TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING THE DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION			X	X	ACI 530.1: 1.22(G), 2.1.4, 3.1.6
SPECIFIED SIZE GRADE, AND TYPE OF REINFORCEMENT			X	X	ACI 530.1: 1.8C, 1.8D
PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40 F) OR HOT WEATHER (TEMPERATURE ABOVE 90 F)			X	X	2104.3, 2104.4
PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:			X	X	ACI 530.1: 3.2D
CLEANLINESS OF GROUT SPACE			X	X	ACI 530.1: 1.13, ACI 530.1: 3.4
PROPORTION OF MORTAR JOINTS			X	X	ACI 530.1: 2.6B
CONSTRUCTION OF MORTAR JOINTS			X	X	ACI 530.1: 3.3B
GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENT PROVISIONS			X	X	ACI 530.1: 3.5
PREPARATION OF ANY REQUIRED GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED			X	X	2105.2.2, 2105.3
COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SCHEDULES SHALL BE VERIFIED			X	X	ACI 530.1: 1.5

LUMBER NOTES:

- MEMBER GRADES SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:

GLU-LAM BEAMS	24F-14 DTR/DF
JOISTS	DDOLAS-FR/LARCH #2
COLUMNS	DDOLAS-FR/LARCH #2
STUDS	DDOLAS-FR/LARCH #2
STUDS NONBRANING WALLS	DDOLAS-FR/LARCH #2
PER-FAB JOISTS	AS PER MANUFACTURER
SILL PLATES IN CONTACT WITH CONCRETE	DDOLAS-FR/LARCH #2
TREATED FOR MOISTURE PROTECTION	
- WHERE NOT NOTED OTHERWISE, CONNECT ALL WOOD TO CONCRETE, WOOD TO STEEL, AND WOOD TO WOOD (EXCEPT STUD TO PLATE) WITH SURESH STRONG-TIE OR USE STRUCTURAL CONNECTORS. ANY OTHER SUBSTITUTION MUST BE APPROVED BY THE ENGINEER.
- WHERE MULTIPLE SILL PLATES ARE USED, ANCHOR BOLTS SHALL EXTEND THROUGH ALL SILL PLATES.
- BLOCK ALL HORIZONTAL EDGES OF PLYWOOD WALL SHEATHING WITH 2" NOMINAL BLOCKING. BLOCK EDGES OF PLYWOOD ON FLOORS AND ROOF AS DIRECTED ON DRAWINGS.
- SOLID 2" NOMINAL BLOCKING SHALL BE PROVIDED AT ENDS OR POINTS OF SUPPORT OF ALL WOOD JOISTS. GROSS BRIDGING OF NOT LESS THAN 1.33 MATERIAL SHALL BE PLACED IN ROWS BETWEEN SUPPORT POINTS NOT TO EXCEED 8'-0" APART, FOR SPANS OF 18'-0" AND GREATER.
- ALL LEADER BOLTS SHALL HAVE PLATE WASHERS WITH A MINIMUM DIA. EQUAL TO 3 TIMES THE BOLT DIA. UNLESS SHOWN OTHERWISE IN DETAILS.
- MINIMUM NAILING SHALL BE AS PER SECTION 2304.9.1 OF THE INTERNATIONAL BUILDING CODE.
- FASTENERS SUCH AS STAPLES, CAN ONLY BE SUBSTITUTED FOR NAILS AT A RATE EQUAL TO LOAD VALUES PROVIDED BY I.C.B.O. APPROVAL. SEE ATTACHED SCHEDULE.
- JOISTS SHALL HAVE BRIDGING, BLOCKING AND NOTCHED BEARING PLATES AS RECOMMENDED BY THE MANUFACTURER WITH A MINIMUM OF ONE ROW OF BRACING AT MID SPAN. MANUFACTURER SHALL SUPPLY AND INSTALL.
- ALL PRE-MANUFACTURED WOOD PRODUCTS SHALL BE PROVIDED BY THESE JOIST ROSE CASCADE CORP. OR LOUISIANA PACIFIC CORP. ANY OTHER SUBSTITUTION MUST BE APPROVED BY THE ENGINEER.
- FASTENERS FOR PRESSURE PRESERVATIVE WOOD SHALL BE HOT-DIPPED, GALVANIZED STEEL, OR STAINLESS STEEL.

ROOF NOTES:

- ROOF SHEATHING SHALL BE 7/16" APA RATED SHEATHING W/SPAN RATING OF 24/16 NAILED WITH 8d NAILS AT 6" O.C. AT ALL PANEL ENDS, SUPPORTED EDGES, TOP OF SHEAR WALLS AND ALL BLOCKING. 8d NAILS AT 12" O.C. ALONG INTERMEDIATE FRAMING MEMBERS. LAY SHEATHING WITH FACE GRAIN AT RIGHT ANGLES TO FRAMING WITH END JOINTS STAGGERED.
- BLOCK RATTERS AND/OR TRUSSES SOLID AT ALL BEARING POINTS.

DESIGN CRITERIA:

GOVERNING CODE	2009 IBC
OCCUPANCY CATEGORY	II
IMPORTANCE FACTOR	I = 1.00
EARTHQUAKE RESPONSE MODIFICATION COEFFICIENT	R = 5.0
SPECTRAL RESPONSE COEFFICIENTS	S ₁ = 119.44g S ₂ = 51.07g S ₃ = 81%g S ₄ = 51%g
SOIL CLASS	D (ASSUMED)
BASIC SEISMIC-FORCE-RESISTING SYSTEM	CMU SHEAR WALL
DESIGN BASE SHEAR	V = 6.7
SEISMIC RESPONSE COEFFICIENT	C _s = 1.2
ANALYSIS PROCEDURE	EQUILIBRIUM LATERAL FORCE PROCEDURE
WIND BASIC WIND SPEED (3 SECOND GUST)	90 MPH EXPOSURE C
SOIL BEARING PRESSURE	1500 PSF (ASSUMED)
PASSIVE SOIL PRESSURE	300 PCF
ACTIVE SOIL PRESSURE	35 PCF
SOIL REPORT BY	N/A
REPORT #	N/A
GROUND SNOW LOAD, S _s	43 PSF
FLAT ROOF SNOW LOAD, P _f	50 PSF
SNOW EXPOSURE FACTOR, C _e	1.0
SNOW LOAD IMPORTANCE FACTOR, I	1.0
SNOW LOAD IMPORTANCE FACTOR, I	1.0
ROOF DEAD LOAD	15 PSF
LIVE LOAD	20 PSF

MINIMUM NAILING SCHEDULE

No.	CONNECTION	FASTENING			LOCATION
		NAILING	STAPLES	SPACING	
1	JOIST TO SILL OR GIRDER	3 8d	3 3"-14 GA.		TORNAIL EA END
2	BRODENS TO JOIST	2 8d	2 3"-14 GA.		TORNAIL EA END
3	BOTTOM PLATE TO JOIST OR BLOCKING	16d 16 o.c.	3"-14 GA. 12" o.c.		TOP FACE NAIL
4	AT BRACKED WALL PANEL	3 16d	16" o.c. 4 3"-14 GA. 12" o.c.		BRACKED WALL PANELS
5	TOP PLATE TO STUD	2 16d	3 3"-14 GA.		END NAIL
6	STUD TO BOTTOM PLATE (OPTIONAL)	4 8d	3 3"-14 GA.		TORNAIL
7	DOUBLE STUDS	2 16d	3 3"-14 GA.		END NAIL
8	DOUBLE TOP PLATES	16d 16 o.c.	3"-14 GA. 8" o.c.		FACE NAIL
9	BECKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	8 16d	12 3"-14 GA. 12" o.c.		TOP FACE NAIL
10	RAFTERS TO TOP PLATE	3 8d	3 3"-14 GA.		TORNAIL
11	RM JOIST TO TOP PLATE	8d	16" o.c. 3 3"-14 GA. 16" o.c.		TORNAIL
12	TOP PLATES, LAPS & INTERSECTIONS	2 16d	3 3"-14 GA.		FACE NAIL
13	CONTINUOUS HEADER, TWO PIECES	3 8d	5 3"-14 GA.		ALONG EDGE
14	CONTINUOUS HEADER TO STUD	4 16d	4 3"-14 GA.		TORNAIL
15	CEILING JOISTS OVER PARTITIONS	3 16d	4 3"-14 GA.		FACE NAIL
16	CEILING JOISTS OVER PARTITIONS	3 16d	4 3"-14 GA.		FACE NAIL
17	CEILING JOISTS TO PARALLEL RAFTERS	3 8d	3 3"-14 GA.		TORNAIL OR FACE NAIL
18	RAFTER TO PLATE	16d 24" o.c.	3"-14 GA. 16" o.c.		TORNAIL
19	BUILT-UP CONCRETE STUDS	16d 24" o.c.	3"-14 GA. 16" o.c.		FACE NAIL
20	BUILT-UP GIRDER AND BEAMS	20d 32" o.c.	3"-14 GA. 24" o.c.		FACE NAIL @ TOP & BOTTOM FACE NAIL AT ENDS AND AT EACH SPlice
20a	BUILT-UP GIRDER AND BEAMS (OPTIONAL)	2 20d	3 3"-14 GA.		FACE NAIL AT ENDS AND AT EACH SPlice
21	COLLAR TIE TO RAFTER	3 10d	4 3"-14 GA.		FACE NAIL
22	JACK NAILER TO HIF (OPTIONAL)	2 16d	3 3"-14 GA.		TORNAIL
22a	JACK NAILER TO HIF (OPTIONAL)	2 16d	3 3"-14 GA.		FACE NAIL
23	ROOF RAFTER TO 2x RIDGE BEAM	2 16d	3 3"-14 GA.		TORNAIL OR FACE NAIL
24	JOIST TO RM JOIST	3 16d	3 3"-14 GA.		FACE NAIL
25	LEADER STRIP	3 16d	4 3"-14 GA.		FACE NAIL

- COMMON OR BOX NAILS ARE PERMITTED TO BE USED, EXCEPT WHERE OTHERWISE NOTED.
- STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16" INCH.
- SEE IBC TABLE 2304.9.1 FOR ADDITIONAL NAILING REQUIREMENTS.

TABLE OF EQUIVALENT FASTENERS STAPLES, NAILS AND T-NAILS

COMMON NAIL	EQUIV. SPACING OF BAR FASTENERS (VALID FOR LATERAL LOAD ONLY)			
	GAUGE	STAPLES	NAILS/T-NAILS	1/2"
SPACING	16	15	14	11.3
PENETRATION	1"	1"	1"	1-1/4"
4"	3-1/2"	4"	5"	4"
6"	5"	6"	7"	6"
8"	6-1/2"	8"	9-1/2"	8"
10"	8-1/2"	10"	12"	10"
12"	10"	12"	14-1/2"	14-1/2"
3"	2"	3-1/2"	4"	3"
4"	2-1/2"	3"	3-1/2"	4"
6"	4"	5"	5"	6"
8"	5-1/2"	6-1/2"	8"	6-1/2"
10"	6-1/2"	8"	10"	8"
12"	8"	10"	12"	10"
4"	2"	2-1/2"	3"	2-1/2"
6"	3-1/2"	4"	5"	3-1/2"
8"	4-1/2"	5-1/2"	6-1/2"	4-1/2"
10d AT	5-1/2"	7"	8"	6-1/2"
12"	6-1/2"	8"	9-1/2"	8-1/2"

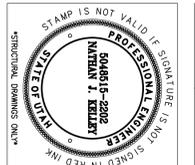
NOTE: PENETRATION IS THE DEPTH OF EMBEDMENT OF THE STAPLE OR NAIL INTO THE MAIN MEMBER REQUIRED TO ATTAIN ITS FULL CAPACITY (SHEAR VALUE) FOR LATERAL LOADING.

IT IS THE RESPONSIBILITY OF THE PERSON(S) OR ORGANIZATION REQUESTING ENGINEERING TO ENSURE THAT ENGINEERING SPECIFICATIONS ARE ADHERED TO.

Hancock and Associates Equipment Shed
950 NORTH 4500 WEST -- WEST WEBER, UT
GENERAL STRUCTURAL NOTES

REVISIONS	DATE	DESCRIPTION

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Project Info.
Engineer: N.K.K.
Designer: N.K.K.
Begin Date: 02 JUNE 2011
Name: _____

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FOUNDATION WALL SCHEDULE

(SEE FOOTING & FOUNDATION WALL DETAIL)

NAME OR SYM.	SIZE / REBAR / DESCRIPTION
FW-1 (9' MAX HEIGHT)	8" CONCRETE
HORIZ. STEEL	#4 REBAR @ 18" O.C.
VERT. STEEL	#4 REBAR @ 24" O.C.
FW-2 (9' MAX HEIGHT)	8" CONCRETE
HORIZ. STEEL	#4 REBAR @ 18" O.C.
VERT. STEEL	#4 REBAR @ 18" O.C.

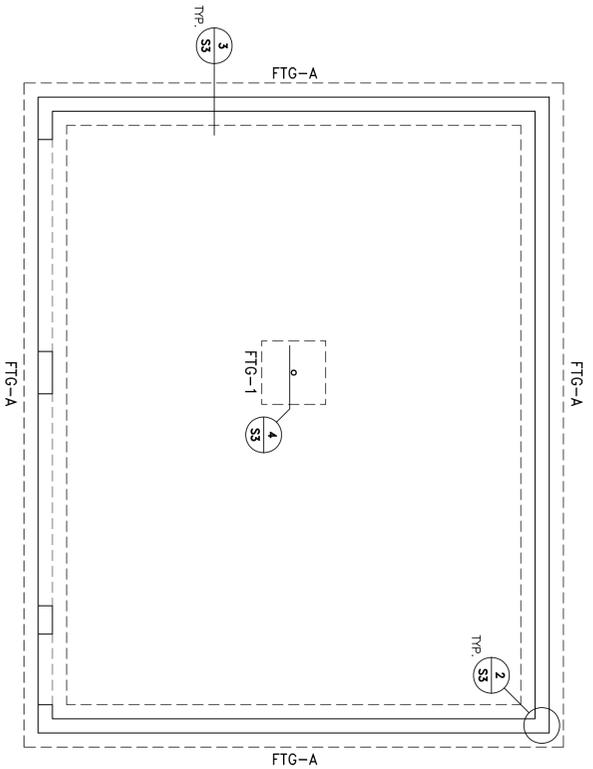
FOUNDATION BOLTS
USE 5/8"x10" 3" BOLTS, W/ 1/4" 3"x3"x1/4" PLATE WASHERS ON EACH BOLT. SEE SHEAR WALL SCHEDULE FOR REQUIRED ANCHOR BOLT SPACING.

- NOTES:
- FOUNDATION WALLS ARE FW-1 UNLESS WALL HEIGHT IS GREATER THAN 8'-0" AND LESS THAN OR EQUAL TO 9'-0". THEN FOUNDATION WALLS ARE FW-2.
 - FOUNDATION WALLS GREATER THAN 9'-0" SHALL BE DESIGNED BY THE ENGINEER AND WILL BE NOTED ON THE PLANS.

FOOTING SCHEDULE

SYM.	DEPTH	WIDTH	REBAR
FTG-A	10"	1'-8"	(2) #4 REBAR CONTINUOUS
FTG-1	10"	3'-0" SQ.	(4) #4 REBAR E.W.

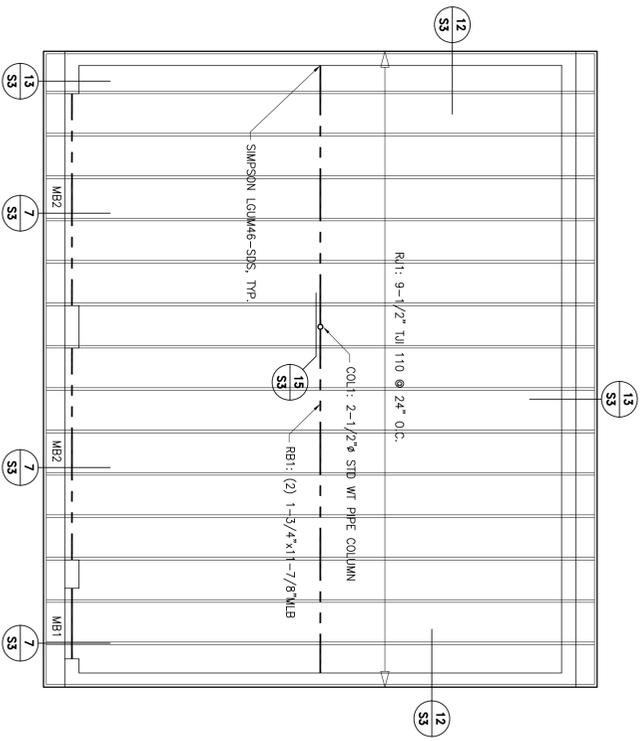
NOTE:
CONTRACTOR TO FIND DIMENSIONS FOR THE FOUNDATION OFF OF ARCHITECTURAL DRAWINGS.



FOOTING & FOUNDATION PLAN

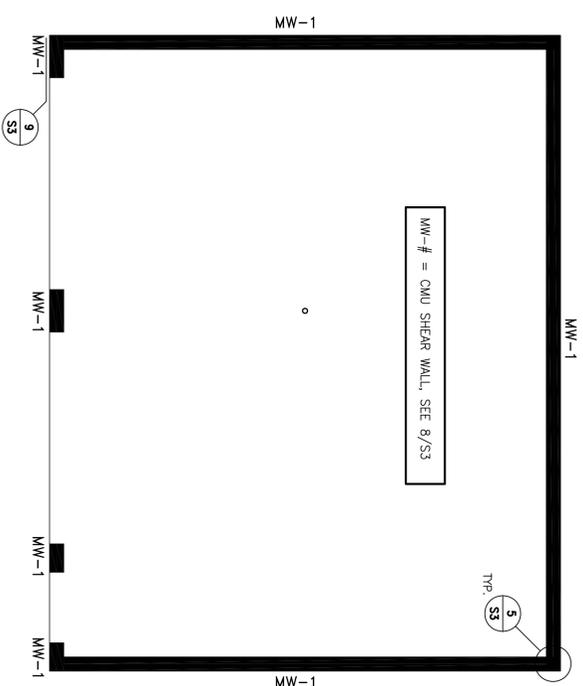
SCALE: 1/4" = 1'-0"

- LUMBER NOTE:**
- BEAM SIZES ARE BASED ON A MINIMUM STRENGTH REQUIREMENTS. SIZES MAY BE INCREASED FOR ARCHITECTURAL OR CONSTRUCTION PURPOSES.
 - ALL LUMBER IS #2 0F/L UNLESS NOTED OTHERWISE.
0F/L = DOUGLAS FIR-LARCH
MFB = 1 1/2" MICRO LAM
MFB2 = 1 1/2" MICRO LAM
VISUALLY GRADED WESTERN SPS
UNLESS NOTED OTHERWISE.
 - 2-PLY AND 3-PLY PRE-ENGINEERED WOOD BEAMS SHALL BE NAILD TOGETHER AS PER MANUFACTURER'S SPECIFICATIONS. 4-PLY AND GREATER PRE-ENGINEERED WOOD BEAMS SHALL BE ATTACHED W/ (2) ROWS 1/2" thru-bolts @ 12" S.C. SPACED 2" FROM TOP AND BOTTOM SURFACE. SEE MANUFACTURER'S SPECIFICATIONS FOR ALL OTHER CONNECTION CONDITIONS.
 - SOLID BLOCKING OR GUSSET BLOCKS REQUIRED IN ALL SPANS BETWEEN COLUMNS. VERIFY ALL COLUMN LOADS DOWN TO FTG. OR FND.



FIRST FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"



FIRST FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"

Hancock and Associates
Equipment Shed
950 NORTH 4500 WEST -- WEST WEBER, UT

FOOTING & FOUNDATION/SHEAR WALL AND ROOF FRAMING PLAN

REVISIONS	
DATE	DESCRIPTION

Reeve & Associates, Inc.
4155 S. HARRISON BLVD., SUITE 310, OGDEN, UTAH 84403
TEL: (801) 621-3100 FAX: (801) 621-2666 www.reeve-assoc.com
LAND PLANNERS • CIVIL ENGINEERS • LAND SURVEYORS
TRAFFIC ENGINEERS • STRUCTURAL ENGINEERS • LANDSCAPE ARCHITECTS

STAMP IS NOT VALID IF SIGNATURE IS NOT SIGNED IN RED INK

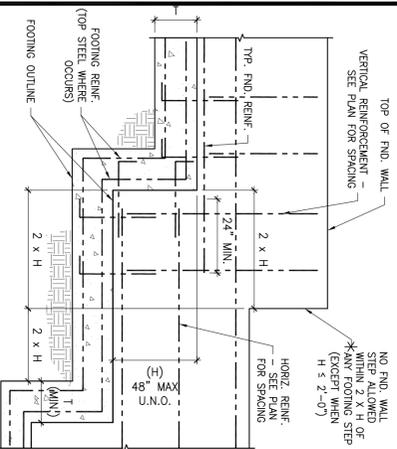
PROFESSIONAL ENGINEER
NATHAN J. KELLEY
5048815-2802
STATE OF UTAH
STRUCTURAL DIVISION (ENR)

Project Info.

Engineer: N.J.K.
Designer: N.J.K.
Begin Date: 02 JUNE 2011
Name: _____
Number: 5921-03

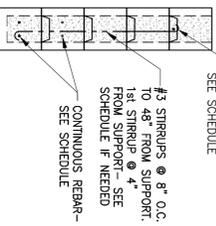
Sheet **S2** of **S3** Sheets

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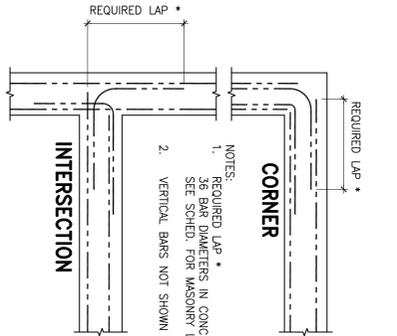


1 STEP FOOTING (TYP.)
SCALE: NONE

MARK	NOMINAL THICKNESS	BOTTOM REIN.	TOP REIN.	VERTICAL REIN. (SEE DET. BELOW)	MIN. GROUT DEPTH
MB-1	8"	(2) #4	(2) #4	(2) #4	16"
MB-2	8"	(2) #4	(2) #4	(2) #4	32"
MB-3	8"	(2) #5	(2) #5	(2) #3 @ 8" O.C.	56"



7 MASONRY BEAM SCHEDULE
SCALE: NONE

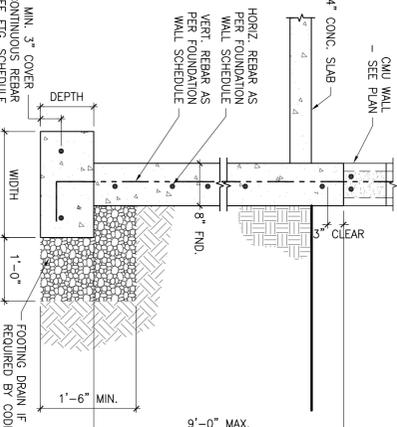


2 HORIZONTAL REINFORCEMENT (TYP.)
SCALE: NONE

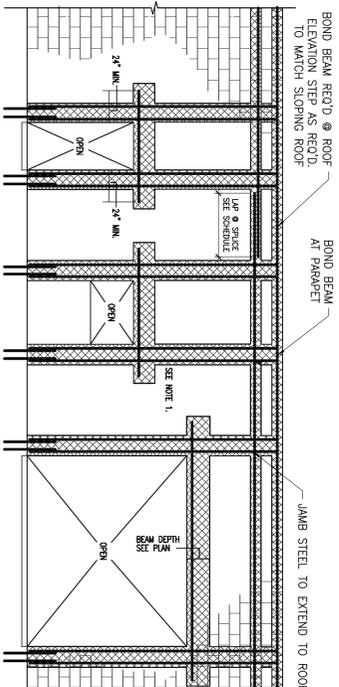
MARK	THICK	VERT. REIN. SIZE	HORIZ. REIN. SPACE	BOND BEAM REIN. @ ROOF @ FLOOR @ PARAPET
MW-1	8"	#5	32" O.C. (2)#4	48" O.C. (2)#4 (2)#4 (2)#4

- NOTES:
1. FOR ANY CMU WALLS NOT SPECIFICALLY CALLED OUT IN PLANS SHALL BE MW-1
2. VERTICAL REINFORCEMENT TO BE @ CENTER
3. SEE STRUCTURAL NOTES FOR ADDITIONAL INFORMATION.

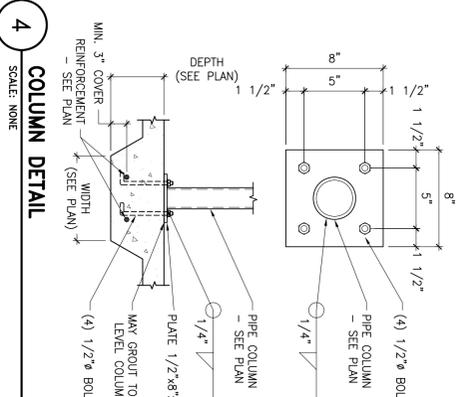
8 MASONRY WALL SCHEDULE
SCALE: NONE



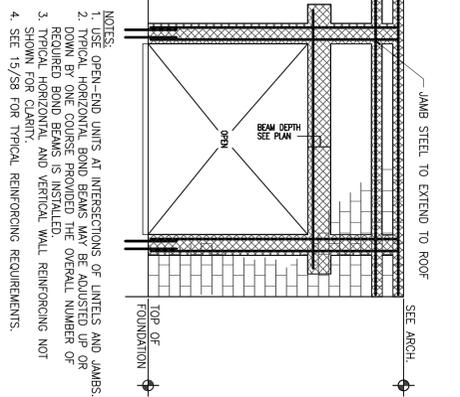
3 FOOTING & FND. WALL DETAIL
SCALE: NONE



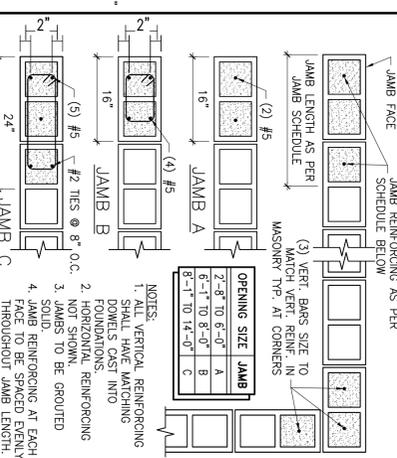
9 MASONRY WALL DETAIL
SCALE: NONE



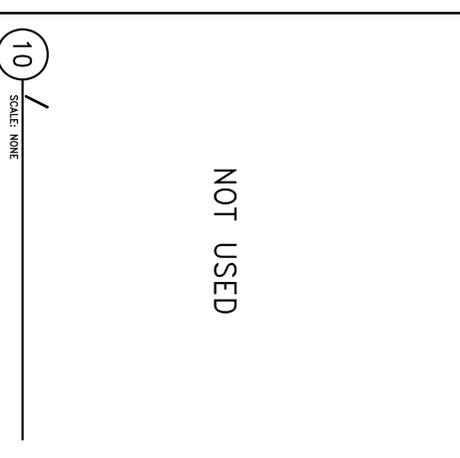
4 COLUMN DETAIL
SCALE: NONE



15 BEAM-TO-COLUMN CONNECTION
SCALE: NONE



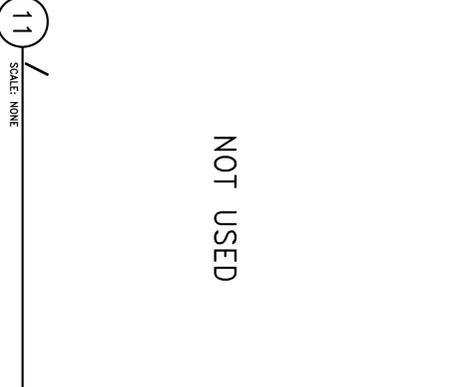
5 JAMB DETAIL (TYP.)
SCALE: NONE



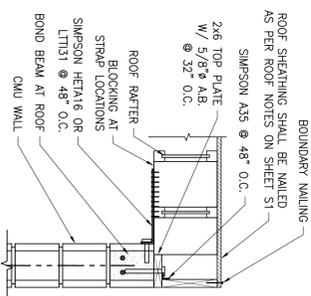
10 JAMB DETAIL (TYP.)
SCALE: NONE

BAR SIZE	NO.	DIA.	LAP LENGTH
#3	0.375	19"	
#4	0.500	25"	
#5	0.625	31"	
#6	0.750	60"	
#7	0.875	81"	
#8	1.000	113"	
#9	1.128	143"	
#10	1.270		MECHANICAL FASTENERS
#11	1.410		MECHANICAL FASTENERS

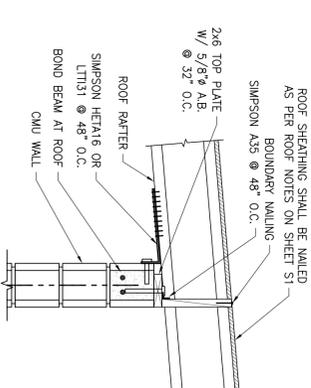
6 MASONRY REBAR LAP SCHEDULE
SCALE: NONE



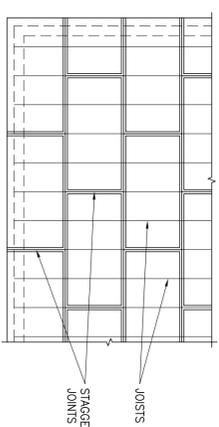
11 MASONRY REBAR LAP SCHEDULE
SCALE: NONE



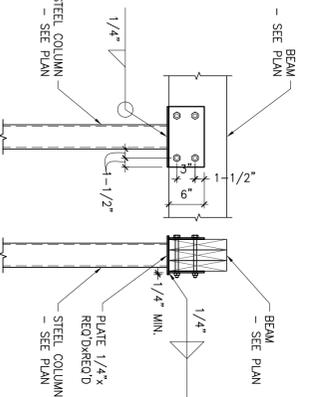
12 ROOF CONNECTION DETAIL
SCALE: NONE



13 ROOF CONNECTION DETAIL
SCALE: NONE



14 HORIZONTAL SHEATHING LAYOUT (TYP.)
SCALE: NONE



15 BEAM-TO-COLUMN CONNECTION
SCALE: NONE

NOT USED

NOT USED

NOT USED

NOT USED

NOT USED

NOT USED

18 SCALE: NONE

19 SCALE: NONE

20 SCALE: NONE

21 SCALE: NONE

22 SCALE: NONE

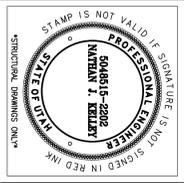
23 SCALE: NONE

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REVISIONS	DATE	DESCRIPTION
1		

Hancock and Associates
Equipment Shed
950 NORTH 4500 WEST -- WEST WEBER, UT
STRUCTURAL DETAILS



Project Info.
Engineer: N.L.K.
Designer: N.L.K.
Begin Date: 02 JUNEF. 2011
Name:
Number: 5921-03
Sheet **S3** of **S3** Sheets