

February 9, 2016

39 Summit LLCc/o Ms. Andrea Milnercc: Mrs. Cassandra Beresini314 Lytton Avenue, Suite 100Palo Alto, California 94301

IGES Project No. 02132-002

Subject: Response to Additional Review Comments - Engineering Geotechnical Investigation Report and Soil Nail Wall Lot 39R of Powder Mountain Resort 8365 East Summit Pass Weber County, Utah

Ms. Milner:

As requested, IGES has prepared the following response to additional review comments regarding the referenced geotechnical report and soil nail wall design package for Lot 39, part of the larger Powder Mountain Resort expansion project in Weber County, Utah. The review comments to be addressed were prepared by Taylor Geotechnical (TG); the comments were posted on Miradi (Weber County Website), and were provided to IGES on February 9, 2016 via email. The latest review comments are in regards to the review response by IGES (2016), which was prepared in response to TG's first engineering review comments posted on December 1, 2015. For convenience, the review comments will be presented first, followed by our response.

Comment No. 1

"Based on the slope analysis completed for the November 4, 2015 IGES document, the anticipated surcharge from the proposed home did not appear to be incorporated into the analysis. TG recommends IGES show the location of the home loads in the slope analysis.

Based on the slope analysis completed for the January 12, 2016 IGES document, the anticipated surcharge from the proposed home did not appear to be incorporated into the analysis. TG recommends IGES show the location of the home loads in the slope analysis."

Response to Comment No. 1

The slope stability analysis for both submittals (IGES, 2016 and 2015a) are the same. IGES has revised the slope stability analysis by adding a surcharge to simulate the load from a home. The surcharge assumes exterior continuous footings, 24" wide, with a 1,500 psf bearing stress. A floor surcharge of 200 psf has also been assumed. The reduction in driving force due to the removal of soil for the basement excavation has been conservatively neglected.

A summary of the revised slope stability analysis is attached.

Powder Mountain Resort, Weber County, Utah Lot 39R

Closure

We appreciate the opportunity to provide you with our services. If you have any questions please contact the undersigned at your convenience (801) 748-4044.

Respectfully Submitted, IGES, Inc.



David A. Glass, P.E. Senior Geotechnical Engineer

Attachments:

References

Revised Slope Stability Analysis

References

- IGES, Inc., 2016, Response to Review Comments Engineering, Geotechnical Investigation Report and Soil Nail Wall, Lot 39R of Powder Mountain Resort, 8365 East Summit Pass, Weber County, Utah, Project No. 02132-002, dated January 12, 2016.
- IGES, Inc., 2015a, Response to Additional Review Comments Geology, Geotechnical Investigation Report, Lot 39R of Powder Mountain Resort, 8365 East Summit Pass, Weber County, Utah, Project No. 02132-002, dated November 4, 2015.
- IGES, Inc., 2015b, Response to Review Comments Geology, Geotechnical Investigation Report, Lot 39R of Powder Mountain Resort, 8365 East Summit Pass, Weber County, Utah, Project No. 02132-002, dated September 23, 2015.
- IGES, Inc., 2015c, Design Package, Permanent Shoring System, Howery Residence, 8365 East Summit Pass (Lot 39R), Summit Eden Development, Weber County, Utah, Project No. 02132-001, dated July 6, 2015, latest revision August 27, 2015.
- IGES, Inc., 2015d, Geotechnical Investigation Report, Lot 39R of Powder Mountain Resort, 8365 East Summit Pass, Weber County, Utah Project No. 02052-001, dated June 3, 2015.
- IGES, Inc., 2012a, Design Geotechnical Investigation, Powder Mountain Resort, Weber County, Utah, Project No. 01628-003, dated November 9, 2012.
- IGES, Inc., 2012b, Preliminary Geotechnical Investigation, Powder Mountain Resort, Weber County, Utah, Project No. 01628-001, dated July 26, 2012.

Lot 39; A-A'; 02132-002; Post-LS Failure; Setback; Static

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			** GSTABL/	by Garry н.	Gregory, P	.E. ^^		12	2	662.00	8616.10	6/5.00	8622.	00	2
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								1.6	2	0.00	8390 00	212 00	8466	00	2
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		S	LOPE STABIL	ITY ANALYSI	S SYSTEM			21	1	597.00	8578.00	662.00	8616.	10	2
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	(Inc Inc] Non] Anis	cludes Spen luding Pier linear Undr sotropic So	cer & Morge /Pile, Rein ained Shear il, Fiber-R	enstern-Pric forcement, Strength, einforced S	e Type Anal Soil Nail, Curved Phi oil, Bounda	ysis) Tieback, Envelope, ry Loads, Water	1	User	Specifi	ed Y-Origi	n = 8	350.00(ft)			
	Surf	faces, Pseu	do-Static E	arthquake,	and Applied	Force Options.		ISOTRO	OPIC SOI	L PARAMETE	RS				
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								2 1 2	(pe(s) o	5011					
	Analysis Ru	un Date:	2/9/20	16											
	Time of Rur	1:	2:09PM	I				Soil	Total	Saturated	Cohesion	Friction	Pore	Pressure	Piez.
	Run By:		DAG					Type	Unit Wt	. Unit Wt.	Intercept	Angle	Pressure	Constant	Surface
	Input Data	Filename:	C:al.					No.	(pcf)	(pcf)	(psf)	(deg)	Param.	(psf)	No.
	Output Filename: C:al.OUT														
	Unit System: English				1	130.0	136.0	0.0	39.0	0.00	0.0	0			
						2	120.0	126.0	250.0	26.0	0.00	0.0	0		
	Plotted Out	tput Filena	me: C:al.P	LT				3	125.0	130.0	100.0	34.0	0.00	0.0	0
							1								
								BOUNDA	ARY LOAD	(S)					
	PROBLEM DES	SCRIPTION:	Lot 39; A-	A'; 02132-0	02; Post-LS	Failure		DODIE							
			; Setback;	Static				3	3 Load(s) Specifie	d				
								Load		X-Left	X-Right	Intens	ity	Deflection	1
								No.		(ft)	(ft)	(psf)	(deg)	
	BOUNDARY CO	DORDINATES													
	16 Top	Boundarie	S					1		540.00	542.00	1500	.0	0.0	
	21 Total	L Boundarie	S					2		543.00	643.00	200	.0	0.0	
								3		644.00	646.00	1500	.0	0.0	
	Boundary	X-Loft	V-Loft	X_Pight	V_Pight	Soil Type									
	No	(f+)	(f+)	(f+)	(f+)	Below Brd		NOTE	- Inton	aity Ta Sn	acified As	A Uniform	ly Dietr	ibuted	
	NO.	(10)	(10)	(10)	(10)	Derow bild		NOIE	Force	Acting On	A Horizon	tally Proi	ected Su	rface	
	1	0.00	8410.00	45.00	8427.00	3	1		TOTCE	incering OII		carry rroj	could bu		
	2	45.00	8427.00	70.00	8427.50	3	-								
	3	70.00	8427.50	83.00	8426.00	3									
	4	83.00	8426.00	89.00	8427.70	1		A Cri	itical F	ailure Sur	face Seard	hing Metho	d, Usino	A Random	
	5	89.00	8427.70	91.10	8434.30	- 1		Techr	nique Fo	r Generati	ng Circula	r Surfaces	, Has Be	en Specif	led.
	6	91.10	8434.30	275.00	8508.00	- 1		5111	-1		5		, 20		
	7	275 00	8508 00	326 00	8524 30	1									
	8	326 00	8524 30	392 00	8540 50	1		2500	Trial S	urfaces Ha	ve Been Ge	nerated			
	9	392.00	8540.50	632.00	8610.00	1		2500	TTTAT D		, c Deen de				
	-					-									

50 Surface(s) Initiate(s) From Each Of 50 Points Equally Spaced Along The Ground Surface Between X = 70.00(ft) and X = 100.00(ft)		Circ	le Center Factor	At X = of Safe	95.31 ty	; Y = 8	;770.59 ;	and Rad	ius =	343.33
Each Surface Terminates Between X = 200.00(ft) and X = 548.00(ft)			*** <u>1</u>	.628	* * *					
Unless Further Limitations Were Imposed, The Minimum Elevation At Which A Surface Extends Is $Y = 0.00(ft)$			Individua	al data	on the	19 sli	.ces			
25.00(ft) Line Segments Define Each Trial Failure Surface.				Water Force	Water Force	Tie Force	Tie Force	Earthq For	uake ce Sur	charge
	Slice No.	Width (ft)	Weight (lbs)	Top (lbs)	Bot (lbs)	Norm (lbs)	Tan (lbs)	Hor (lbs)	Ver (lbs)	Load (lbs)
Restrictions Have Been Imposed Upon The Angle Of Initiation.										
The Angle Has Been Restricted Between The Angles Of -30.0	1	1.2	27.0	0.0	0.0	0.	0.	0.0	0.0	0.0
And 10.0 deg.	2	2.1	988.0	0.0	0.0	0.	0.	0.0	0.0	0.0
	3	13.8	17117.7	0.0	0.0	0.	0.	0.0	0.0	0.0
	4	7.9	13982.6	0.0	0.0	0.	0.	0.0	0.0	0.0
	5	24.9	60552.2	0.0	0.0	0.	0.	0.0	0.0	0.0
	6	24.7	80562.8	0.0	0.0	0.	0.	0.0	0.0	0.0
Following Is Displayed The Most Critical Of The Trial	7	24.3	94078.3	0.0	0.0	0.	0.	0.0	0.0	0.0
Failure Surfaces Evaluated.	8	23.8	101041.2	0.0	0.0	0.	0.	0.0	0.0	0.0
	9	1.5	6532.4	0.0	0.0	0.	0.	0.0	0.0	0.0
t t Gefeter Destand Aug Gelevileted De Mbe Medified Disber Method t t	10	21.7	95144.5	0.0	0.0	0.	0.	0.0	0.0	0.0
* * Salety Factors are calculated by the Modified Bishop Method * *	10	10 0	74004 0	0.0	0.0	0.	0.	0.0	0.0	0.0
	12	2 0	10760 9	0.0	0.0	0.	0.	0.0	0.0	0.0
	14	2.9	67287 9	0.0	0.0	0.	0.	0.0	0.0	0.0
Total Number of Trial Surfaces Evaluated = 2500	15	1 4	3993 7	0.0	0.0	0.	0.	0.0	0.0	0.0
Total Number of fifth barraceb Evaluated - 2500	16	18 2	39338 8	0.0	0.0	0.	0.	0.0	0.0	0.0
Statistical Data On All Valid FS Values:	17	7 9	10244 1	0.0	0.0	0.	0.	0.0	0.0	0.0
FS Max = 2.581 FS Min = 1.628 FS Ave = 1.990	18	10.6	5664.5	0.0	0.0	0.	0.	0.0	0.0	0.0
Standard Deviation = 0.188 Coefficient of Variation = 9.42 %	19	0.7	26.4	0.0	0.0	0.	0.	0.0	0.0	0.0

Failure Surface Specified By 13 Coordinate Points

Point	X-Surf	Y-Surf
NO.	(10)	(10)
1	87.76	8427.35
2	112.75	8427.71
3	137.66	8429.89
4	162.34	8433.87
5	186.66	8439.64
б	210.50	8447.17
7	233.73	8456.41
8	256.23	8467.31
9	277.87	8479.83
10	298.55	8493.88
11	318.14	8509.41
12	336.56	8526.32
13	337.26	8527.06

**** END OF GSTABL7 OUTPUT ****

Lot 39; A-A'; 02132-002; Post-LS Failure; Setback; P-Static

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				*** GSTABL	7 ***			10)	632.00	8610.00	650.00	8614.	40	1
			** GSTABL7	by Garry H.	Gregory, P	•.E. **		12	-	662.00	8616.10	675.00	8622.	00	2
								13	5	675.00	8622.00	700.00	8623.	60	2
	** Origin	al Version	1.0, Janua	ry 1996; Cu	rrent Versi	on 2.002,		14	Ł	700.00	8623.60	706.00	8629.	20	2
December	2001 **							15	5	706.00	8629.20	917.00	8680.	00	2
		(All R	ights Reser	ved-Unautho	rized Use F	rohibited)		16	5	917.00	8680.00	1000.00	8700.	00	2
			5					17	,	0 00	8410 00	70 00	8427	50	1
								1.8	1	0 00	8390 00	212 00	8466	00	2
								10	,	212 00	8466 00	406 00	8530	00	2
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	(Inc	ludes Spen	.cer & Morge	nstern-Pric	e Type Anal	ysis)		User	Specifie	ed Y-Origi	n = 83	350.00(it)		
	Incl	uding Pier	/Pile, Rein	forcement,	Soil Nail,	Tieback,	1								
	Nonl	inear Undr	ained Shear	Strength,	Curved Phi	Envelope,									
	Anis	otropic So	il, Fiber-R	einforced S	oil, Bounda	ry Loads, Water									
	Surf	aces, Pseu	do-Static E	arthquake,	and Applied	Force Options.		ISOTRO	PIC SOI	L PARAMETE	RS				
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								3 Ту	npe(s) o:	f Soil					
	Analysis Ru	n Date:	2/9/20	16											
	Time of Run	:	2:11PM					Soil	Total	Saturated	Cohesion	Friction	Pore	Pressure	Piez.
	Run By:		DAG					Type	Unit Wt	Unit Wt	Intercept	Angle	Pressure	Constant	Surface
	Input Data	Filename:	Cialp					No	(pcf)	(pcf)	(psf)	(deg)	Param	(psf)	No
	Output Filename: Cialp OUT						(POL)	(POT)	(PD1)	(acg)	1 al ani	(PD1)	1.0.		
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	UIIIC SYSCEM		FIGITS	11				2	120.0	126.0	250.0	39.0	0.00	0.0	0
	Plotted Output Filename: C:alp.PLT					3	120.0	130.0	100.0	34.0	0.00	0.0	0		
							1								
								POIND	סגסז עס	(9)					
	PROBLEM DES	CRIPTION:	Lot 39: A-	A': 02132-0	02: Post-LS	Failure		BOONDA	ICI LIOAD	(5)					
	FRODUBN DED	CRIFTION.	: Setback:	D_Static	027 FOSC LL	Parlare		-	Load(g) Specifie	d				
			/ DetDack/	r btatit				-	LOAU(S) ppecific					
								Load		X-I.eft	X-Right	Inten	aitv.	Deflection	1
								No		(f+)	(f+)	(ngt	=)	(deg)	-
	BOUNDARY CO	ORDINATES						110.		(10)	(10)	(59)	- /	(deg)	
	16 700	Boundaria						1		540 00	542 00	150		0 0	
	10 IOP 21 matel	Boundarie	-					1 1		540.00	542.00	100		0.0	
	ZI IOLAI	Boundarie	5					2		543.00	643.00	200	0.0	0.0	
								3		644.00	646.00	1900	J.U	0.0	
	Boundary	X-Left	Y-Left	X-Right	Y-Right	Soil Type									
	No.	(ft)	(ft)	(ft)	(ft)	Below Bnd		NOTE	- Inten	sity Is Sp	ecified As	A Uniform	nly Distr	ibuted	
									Force	Acting On	A Horizont	ally Pro	jected Su	rface.	
	1	0.00	8410.00	45.00	8427.00	3				-					
	2	45.00	8427.00	70.00	8427.50	3									
	3	70.00	8427.50	83.00	8426.00	3									
	4	83 00	8426 00	89 00	8427 70	1		A Hor	izontal	Earthousk	e Loading (oefficier	ht		
	5	89 00	8427 70	91 10	8434 30	- 1		0f0 1	70 Hag 1	Reen Accio	med				
	5	01.00	9/2/ 20	275 00	9509 00	± 1		010.1	., 5 110.5 1	Decir Abbig					
	0	91.1U	0434.30	2/3.00	0500.00	1		7 17-			Tandina Ci	eet at an t			
	/	2/5.00	8508.00	326.00	8524.30	1		A Ver	LICAL E	arcnquake	Loading Coe	erricient			
	8	326.00	8524.30	392.00	8540.50	1		U±U.C	UUU Has 1	вeen Assig	ned				
	9	392.00	8540.50	632.00	8610.00	1									

Cavitation Pressure = 0.0(psf)

Trial Failure Surface Specified By 13 Coordinate Points

Point	X-Surf	Y-Surf
No.	(ft)	(ft)
1	87.76	8427.35
2	112.75	8427.71
3	137.66	8429.89
4	162.34	8433.87
5	186.66	8439.64
6	210.50	8447.17
7	233.73	8456.41
8	256.23	8467.31
9	277.87	8479.83
10	298.55	8493.88
11	318.14	8509.41
12	336.56	8526.32
13	337.26	8527.06

Circle Center At X = ~95.30 ; Y = ~8770.61; and Radius = ~343.35

* * Factor Of Safety Is Calculated By The Modified Bishop Method * *

Factor Of Safety For The Preceding Specified Surface = 1.077

Table 1 - Individual Data on the 19 Slices

			Water	Water	Tie	Tie	Earthqu	ıake	
			Force	Force	Force	Force	Ford	ce Sur	charge
Slice	Width	Weight	Top	Bot	Norm	Tan	Hor	Ver	Load
No.	(ft)	(lbs)	(lbs)	(lbs)	(lbs)	(lbs)	(lbs)	(lbs)	(lbs)
1	1.2	26.9	0.0	0.0	0.0	0.0	4.6	0.0	0.0
2	2.1	987.8	0.0	0.0	0.0	0.0	167.9	0.0	0.0
3	13.8	17123.6	0.0	0.0	0.0	0.0	2911.0	0.0	0.0
4	7.9	13968.0	0.0	0.0	0.0	0.0	2374.6	0.0	0.0
5	24.9	60555.9	0.0	0.0	0.0	0.0	10294.5	0.0	0.0
6	24.7	80561.4	0.0	0.0	0.0	0.0	13695.4	0.0	0.0
7	24.3	94067.8	0.0	0.0	0.0	0.0	15991.5	0.0	0.0
8	23.8	101039.0	0.0	0.0	0.0	0.0	17176.6	0.0	0.0
9	1.5	6547.8	0.0	0.0	0.0	0.0	1113.1	0.0	0.0
10	21.7	95122.0	0.0	0.0	0.0	0.0	16170.7	0.0	0.0
11	22.5	96387.7	0.0	0.0	0.0	0.0	16385.9	0.0	0.0
12	18.8	74808.1	0.0	0.0	0.0	0.0	12717.4	0.0	0.0
13	2.9	10754.2	0.0	0.0	0.0	0.0	1828.2	0.0	0.0
14	20.7	67308.1	0.0	0.0	0.0	0.0	11442.4	0.0	0.0

15	1.5	4019.8	0.0	0.0	0.0	0.0	683.4	0.0	0.0
16	18.1	39301.6	0.0	0.0	0.0	0.0	6681.3	0.0	0.0
17	7.9	10244.9	0.0	0.0	0.0	0.0	1741.6	0.0	0.0
18	10.6	5660.1	0.0	0.0	0.0	0.0	962.2	0.0	0.0
19	0.7	26.0	0.0	0.0	0.0	0.0	4.4	0.0	0.0
		Table 2	- Base	Stress	Data on the	19	Slices		

Slice	Alpha	X-Coord.	Base	Available	Mobilized
No.	(deg)	Slice Cntr	Leng.	Shear Strength	Shear Stress
*		(ft)	(ft)	(psf)	(psf)
1	0 83	88 38	1 24	17 39	2 76
2	0.83	90.05	2 10	376 79	8 24
2 2	0.83	97 99	13 77	995 93	18 19
4	0.83	108 81	7 88	1107 65	26.02
5	5 00	125 21	25 01	1380 93	211 22
6	9 16	150 00	25 00	1716 64	513 24
7	13.35	174.50	25.00	1929.17	868.83
8	17.53	198.58	25.00	2027.04	1217.38
9	21.69	211.25	1.61	2015.79	1501.03
10	21.69	222.86	23.39	2020.85	1503.49
11	25.85	244.98	25.00	1918.38	1680.90
12	30.05	265.61	21.69	1738.24	1727.76
13	30.05	276.43	3.32	1646.10	1625.18
14	34.19	288.21	25.00	1405.00	1513.04
15	38.41	299.28	1.85	1176.51	1348.16
16	38.41	309.07	23.14	1099.27	1055.04
17	42.55	322.07	10.67	624.37	649.62
18	42.55	331.28	14.33	256.75	267.23
19	46.75	336.91	1.02	16.74	21.53

Sum of the Resisting Forces (including Pier/Pile, Tieback, Reinforcing Soil Nail, and Applied Forces if applicable) = 414392.16 (lbs)

Average Available Shear Strength (including Tieback, Pier/Pile, Reinforcing, Soil Nail, and Applied Forces if applicable) = 1501.30(psf)

Sum of the Driving Forces = 384919.50 (lbs)

Average Mobilized Shear Stress = 1394.53(psf)

Total length of the failure surface = 276.02(ft)

CAUTION - Factor Of Safety Is Calculated By The Modified Bishop Method. This Method Is Valid Only If The Failure Surface Approximates A Circular Arc.

**** END OF GSTABL7 OUTPUT ****

1

Lot 39; A-A'; 02132-002; Post-LS Failure; Setback; Sat. Unit Weight; Static

C:\DOCUME~1\DAVIDG\DESKTOP\LOT39~1\A2.PL2 Run By: DAG 2/9/2016 2:12PM





				*** GSTABL	7 ***			1	0	632.00	8610.00	650.00	8614.	40	1
								1	1	650.00	8614.40	662.00	8616.	10	1
			** GSTABL7	by Garry H.	Gregory, F	P.E. **		1	2	662.00	8616.10	675.00	8622.	00	2
								1	3	675.00	8622.00	700.00	8623.	60	2
	** Origin	nal Version	1.0, Janua	ry 1996; Cu	rrent Versi	on 2.002,		1	4	700.00	8623.60	706.00	8629.	20	2
December	2001 **							1	5	706.00	8629.20	917.00	8680.	00	2
		(All R	ights Reser	ved-Unautho	rized Use F	rohibited)		1	б	917.00	8680.00	1000.00	8700.	00	2
								1	7	0.00	8410.00	70.00	8427.	50	1
								1	8	0.00	8390.00	212.00	8466.	00	2
								1	9	212.00	8466.00	406.00	8530.	00	2
* * * * * * * * *	* * * * * * * * * * * * *	* * * * * * * * * * *	* * * * * * * * * * *	* * * * * * * * * * *	* * * * * * * * * * *	* * * * * * * * * * * * * * * *		2	0	406.00	8530.00	597.00	8578.	00	2
		S	LOPE STABIL	ITY ANALYSI	S SYSTEM			2	1	597.00	8578.00	662.00	8616.	10	2
	Modi	ified Bisho	p, Simplifi	.ed Janbu, o	r GLE Metho	d of Slices.									
	(Inc	ludes Spen	cer & Morge	nstern-Pric	e Type Anal	ysis)		User	Specifi	ed Y-Origi	n = 83	350.00(ft))		
	Incl	luding Pier	/Pile, Rein	forcement,	Soil Nail,	Tieback,	1		-	5					
	Nonl	linear Undr	ained Shear	Strength,	Curved Phi	Envelope,									
	Anis	sotropic So	il, Fiber-R	einforced S	oil, Bounda	ry Loads, Water									
	Surfaces, Pseudo-Static Earthquake, and Applied Force Options.							ISOTR	OPIC SOI	L PARAMETE	RS				
				1	11										
*******	* * * * * * * * * * * * *	* * * * * * * * * * *	******	* * * * * * * * * * *	******	* * * * * * * * * * * * * * * * * *									
								3 Т	vpe(s) o	of Soil					
								5 1	100(0) 0						
	Analysis Ru	ın Date:	2/9/20	16											
	Time of Rur	n:	2:120	1				Soil	Total	Saturated	Cohesion	Friction	Pore	Dreggure	Diez
	Pun By:		DAG					Type	IIni+ W+	Unit Wt	Intercent	Angle	Dregure	Constant	Surface
	Run By: DAG							No	(pcf)	(pcf)	(nef)	(deg)	Daram	(pef)	No
	Output Filename: C:a2 OUT						NO.	(per)	(per)	(psr)	(deg)	Falam.	(psr)	NO.	
	Unit Custon		C.az.C	b				1	126 0	126 0	0 0	20 0	0 00	0 0	0
	Unit System. English						1	120.0	126.0	250.0	39.0	0.00	0.0	0	
	District Output Filonome. Cool DIF						2	120.0	120.0	250.0	26.0	0.00	0.0	0	
	Piotted Out	put filena	me. C.az.F	11			1	3	125.0	130.0	100.0	34.0	0.00	0.0	0
							T								
										(C)					
	DDODIEM DEC		T of 20 . A	NI · 02122 0	02 · Dogt IC	Failure		BOUND	ARI LOAD	(5)					
	PROBLEM DES	SCRIPTION.	LOL 397 A-	Cot UZISZ-U	Woight: Sta	failure			2 Tood/a) Crocific	4				
			, SetDack,	Sat. UIIIt	weight, sta				5 LUAU(S) specifie	a				
								Taad		V Toft	V. Dánht	Techono		Deflection	
								Load		X-Lell	X-RIGHL	incens	31LY = \	Dellection	1
								NO.		(IC)	(IC)	(psi	=)	(aeg)	
	BOUNDARY CC	JORDINATES													
	16	Develop	_					-		F 4 0 0 0	F40.00	1500		0 0	
	16 Top	Boundarie	S					1		540.00	542.00	1500	0.0	0.0	
	21 Total	L Boundarie	S					2		543.00	643.00	200	0.0	0.0	
								3		644.00	646.00	1500	0.0	0.0	
	De auleur	W. T 54	TT T - C -	Tr. D. L. L.	TT Disk:										
	Boundary	X-Leit	Y-Leit	X-Right	Y-Right	Soil Type									
	No.	(1t)	(1t)	(1t)	(1t)	Below Bnd		NOTE	- Inten	sity Is Sp	ecified As	A Unitorn	n⊥y Distr	ibuted	
							-		Force	Acting On	A Horizont	tally Pro	jected Su	rtace.	
	1	0.00	8410.00	45.00	8427.00	3	1								
	2	45.00	8427.00	70.00	8427.50	3									
	3	70.00	8427.50	83.00	8426.00	3									
	4	83.00	8426.00	89.00	8427.70	1		A Cr	itical F	'ailure Sur	face Search	hing Metho	od, Using	A Random	
5 89.00 8427.70 91.10 8434.30 1				Tech	nique Fo	or Generati	ng Circula	r Surfaces	s, Has Be	en Specifi	.ed.				
	б	91.10	8434.30	275.00	8508.00	1									
	7	275.00	8508.00	326.00	8524.30	1									
	8	326.00	8524.30	392.00	8540.50	1		2500	Trial S	urfaces Ha	ve Been Ger	nerated.			
	9	392.00	8540.50	632.00	8610.00	1									

50 Surface(s) Initiate(s) From Each Of 50 Points Equally Spaced Along The Ground Surface Between $X = 70.00(ft)$ and $X = 100.00(ft)$		Circ	le Center	At X =	99.36	; Y = 8	3710.44 ;	and Rad	ius =	284.13
			Factor	of Safe L.621	ty ***					
Each Surface Terminates Between X = 200.00(ft) and X = 548.00(ft)										
Unless Further Limitations Were Imposed, The Minimum Elevation At Which A Surface Extends Is $Y = 0.00(ft)$			Individua	al data	on the	17 sli	ces			
				Water	Water	Tie	Tie	Earthq	uake	
25.00(ft) Line Segments Define Each Trial Failure Surface.				Force	Force	Force	Force	For	ce Sur	charge
	Slice	Width	Weight	Top	Bot	Norm	Tan	Hor	Ver	Load
	No.	(ft)	(lbs)	(lbs)	(lbs)	(lbs)	(lbs)	(lbs)	(lbs)	(lbs)
Restrictions Have Been Imposed Upon The Angle Of Initiation.										
The Angle Has Been Restricted Between The Angles Of -30.0	1	3.7	267.8	0.0	0.0	0.	0.	0.0	0.0	0.0
And 10.0 deg.	2	2.1	1248.7	0.0	0.0	0.	0.	0.0	0.0	0.0
	3	10.9	14648.0	0.0	0.0	0.	0.	0.0	0.0	0.0
	4	8.3	15394.9	0.0	0.0	0.	0.	0.0	0.0	0.0
	5	24.9	63299.7	0.0	0.0	0.	0.	0.0	0.0	0.0
	6	24.6	83044.4	0.0	0.0	0.	0.	0.0	0.0	0.0
Following Is Displayed The Most Critical Of The Trial	7	24.2	94802.6	0.0	0.0	0.	0.	0.0	0.0	0.0
Failure Surfaces Evaluated.	8	23.5	98571.2	0.0	0.0	0.	0.	0.0	0.0	0.0
	9	4.5	18909.1	0.0	0.0	0.	0.	0.0	0.0	0.0
	10	18.2	75893.0	0.0	0.0	0.	0.	0.0	0.0	0.0
	11	21.7	84304.9	0.0	0.0	0.	0.	0.0	0.0	0.0
\ast \ast Safety Factors Are Calculated By The Modified Bishop Method \ast \ast	12	20.5	68008.9	0.0	0.0	0.	0.	0.0	0.0	0.0
	13	1.7	4980.5	0.0	0.0	0.	0.	0.0	0.0	0.0
	14	0.9	2653.5	0.0	0.0	0.	0.	0.0	0.0	0.0
	15	16.5	36923.2	0.0	0.0	0.	0.	0.0	0.0	0.0
Total Number of Trial Surfaces Evaluated = 2500	16	17.7	14710.2	0.0	0.0	0.	0.	0.0	0.0	0.0
	17	0.1	0.7	0.0	0.0	0.	0.	0.0	0.0	0.0
<pre>Statistical Data On All Valid FS Values: FS Max = 2.582 FS Min = 1.621 FS Ave = 1.987 Standard Deviation = 0.189 Coefficient of Variation = 9.52 %</pre>										
					_					

Failure Surface Specified By 12 Coordinate Points

Point	X-Surf	Y-Surf
110.	(10)	(10)
1	85.31	8426.65
2	110.31	8426.52
3	135.22	8428.58
4	159.86	8432.82
5	184.03	8439.21
6	207.54	8447.71
7	230.22	8458.23
8	251.88	8470.71
9	272.36	8485.04
10	291.51	8501.12
11	309.16	8518.82
12	309.27	8518.95

**** END OF GSTABL7 OUTPUT ****