

COASTAL STEEL STRUCTURES

PROJECT NUMBER: U1408258A
 PROJECT NAME: Melissa Smith
 PROJECT LOCATION: Ogden, UT
 CUSTOMER: Smith Stone Supply, Inc.

Notes and Specifications

Building Erection Notes

- The general contractor and/or erector is responsible to safely and properly erect the metal building system in conformance with these drawings, OSHA requirements and metal building system in conformance with these drawings, OSHA requirements and either MBMA or CSA S16 standards pertaining to proper erection. This includes, but is not limited to, the correct use of temporary guys and bracing where needed for squaring, plumbing, and securing the structural and secondary framing. Secondary wall framing members (girts or bar joists) are not designed to function as a work platform or provide safety tie-off attachment in accordance with OSHA requirements. Secondary roof framing members (purlins or bar joists) are not designed to provide safety tie-off attachment in accordance with OSHA requirements.
- A325 & A490 Bolt Tightening requirements:**
It is the responsibility of the erector to ensure proper bolt tightness in accordance with applicable regulations. See the *RCSC Specification for Structural Joints Using A325 or A490 Bolts* or CAN/CSA S16 "Limit States Design of Steel Structures" for more information. The following criteria may be used to determine the bolt tightness (i.e., "snug-tight" or "fully-pretensioned"), unless required otherwise by local jurisdiction or contract requirements:
 B) All A325 bolts in primary framing (rigid frames and bracing) may be "snug-tight", except as follows:
 "Fully-pretensioned" A325 bolts if:
 a) Building supports a crane system with a capacity greater than 5 tons.
 b) Building supports machinery that creates vibration, impact or stress-reversals on the connections. The Engineer-of-Record for the project should be consulted to evaluate for this condition.
 c) The project site is located in a high seismic area. For IBC-based codes, "High Seismic Area" is defined as "Seismic Design Category" of "D", "E", or "F". See the "Building Loads" section of this page for the defined seismic design category for this project.
 d) Any connection designated in these drawings as "A325-SC". "Slip-Critical (SC)" connections must be free of paint, oil, or other materials that reduce friction at contact surfaces. Galvanized or lightly rusted surfaces are acceptable.
 In Canada, all A325 and A490 bolts shall be "fully pre-tensioned", except for secondary members (purlins, girts, opening framing, etc.) and flange braces.
 D) Secondary members (purlins, girts, opening framing, etc.) and flange brace connections may always be "snug-tight", unless indicated otherwise in these drawings.
- The metal building supplier shall be notified prior to any field modifications. Modifications shall be approved by the metal building supplier before work is undertaken.
- Common Abbreviations:
 a) TYP UNO - Typical Unless Noted Otherwise
 b) SLV - Short Leg Vertical
 c) LLV - Long Leg Vertical
 d) NS & FS - Near Side and Far Side
 e) O.A.L. - Overall Length
 f) SIM - Similar
 g) N/C - Not in Contract
 h) SL - Steel Line
 i) N/A - Not Applicable
 j) MBS - Metal Building Supplier
- Construction loads shall not be placed on any structural steel framework unless such framework is safely bolted, welded, or otherwise adequately secured.
- Purlins and girts shall not be used as an anchorage point for a fall arrest system unless written approval is obtained from the metal building supplier.
- Purlins may only be used as a walking/working surface when installing safety systems, after all permanent bridging has been installed and fall protection is provided.
- Construction loads may be placed only within a zone that is within 8 feet of the center line of the primary support member. CFR bundles should be placed directly over the rigid frames.
- All lifting devices must meet OSHA or MSHA standards and in no case is it acceptable to use structural members supplied by the MBS as a spreader bar or lifting device.

General Design Notes

- All structural steel sections and welded plate members are designed in accordance with ANSI/AISC 360 "Specifications for Structural Steel Buildings" or the CAN/CSA S16 "Limit States Design of Steel Structures", as required by the specified building code.
- All welding of structural steel is based on either AWS D1.1 "Structural Welding Code - Steel" or CAN/CSA W59 "Welded Steel Construction (Metal Arc Welding)", as required by the specified building code.
- All cold formed members are designed in accordance with ANSI/AISI S11 or CAN/CSA S136 "Specifications for the Design of Cold Formed Steel Structural Members", as required by the specified building code.
- All welding of cold formed steel is based on AWS D1.3 "Structural Welding Code - Sheet Steel" or CAN/CSA W59 "Welded Steel Construction (Metal Arc Welding)", as required by the specified building code.
- This Metal Building Supplier facility is IAS AC-472 Accredited and CAN/CSA A660 and W47.1 Certified (if applicable) for the design and manufacturing of Metal Building Systems.
- If joists are included with this project, they are supplied as a part of the systems engineered metal building and are fabricated in accordance with the requirements of Section 1926.758 of the OSHA safety standards for steel erection, dated January 18, 2001.

Material Specifications

Plate and Flange Material:
 5" - 12" Wide, to 1 1/4" Th. _____ A529 Grade 55
 Others _____ A572 Grade 50
 Built-Up Structural Web _____ A1011 SS (or HSLAS Class 1) Grade 55
 Hot-Rolled Structural _____ A36 or A572 Grade 50 or A992 Grade 50
 Structural Tube _____ A500 Grade B (46 KSI)
 Structural Pipe _____ A500 Grade B (42 KSI)
 Cold-Formed Structural _____ A1011 or A1039 SS (or HSLAS Class 1) Grade 55
 Classic Roof Panel _____ A792 Grade 80
 CFR / VR16 II Roof Panel _____ A792 Grade 50, Class 1
 All Wall Panel Profiles _____ A653 Grade 80, Class 1 or A792 Grade 80, Class 1
 Rod Bracing _____ A529 Grade 50
 Welds _____ AWS D1.1/D1.3 or CSA W59 per Building Code
 High-Strength Bolts _____ A325 Type 1 or A490 Type 1 Heavy Hex
 Machine Bolts _____ A307 Grade A Hex

PRIMARY AND SECONDARY STEEL PRIMER COLOR: RED
 ROOF SHEETING, TYPE: CR 26 GAGE, FINISH: Galvalume
 ROOF PANEL CLIP TYPE: N/A TALL SHORT UTILITY FIXED FLOATING
 THERMAL BLOCKS: YES NO EPS FOAM SPACER: YES NO
 SEAMING METHOD (FOR CFR ONLY): ROLL LOCK™ VISE LOCK™ VISE LOCK 360™
 COMPOSITE CFR DECK, TYPE: N/A GAGE, FINISH: _____
 ROOF LINE TRIM, PAINTED: Lightstone SP
 EXTERIOR WALL SHEETING, TYPE: CW 26 GAGE, FINISH: Brick Red SP
 EXTERIOR WALL CORNER TRIM FINISH: Lightstone SP
 EXTERIOR BASE TRIM, PAINTED: Brick Red SP
 FRAMED OPENING TRIM, PAINTED: Lightstone SP
 WALL FRAMED OPENING, SIZES: FSW (2) 14'-0" x 12'-0"
 BSW none
 LEW (1) 12'-0" x 12'-0"
 REW none
 INTERIOR WALL SHEETING, TYPE: none GAGE, FINISH: _____
 INTERIOR CEILING LINER, TYPE: none GAGE, FINISH: _____
 INTERIOR WALL TRIM, PAINTED: none

YES NO
 DOWNSPOUTS PAINTED: _____ GUTTERS PAINTED: _____
 WALKDOORS, QUANTITY: (2) 3070 PAINTED: White
 WINDOWS: _____ PAINTED: _____
 INSULATION (NOT BY MBS), ROOF: 3 INCH WALLS: 3 INCH
 CRANES (SEE CRANE PLAN FOR ADDITIONAL CRANE INFORMATION)
 MEZZANINE (SEE MEZZANINE PLAN FOR ADDITIONAL MEZZANINE INFO)
 WALL TRANSLUCENT PANELS: _____
 ROOF TRANSLUCENT PANELS: _____
 INSULATED PANELS YES NO
 PIPE JACKS, SIZE: _____ QUANTITY: _____
 ROOF FRAMED OPENINGS, SEE ROOF FRAMING PLAN FOR SIZES
 RIDGE VENTS, 10'-0" LONG X 9" THROAT. QUANTITY: _____

YES NO
 FASCIA, PROJECTION: _____ TOP OF FASCIA HEIGHT: _____
 FACE PANEL, TYPE: _____ GAGE, FINISH: _____
 BACK PANEL, TYPE: _____ GAGE, FINISH: _____
 CAP TRIM PAINTED: _____ BASE TRIM PAINTED: _____
 CLOSED SYSTEM, CLEAR UNDER SOFFIT TRIM:
 SOFFIT PANEL, TYPE: _____ GAGE, FINISH: _____
 SOFFIT TRIM AT BUILDING LINE PAINTED: _____
 OPEN SYSTEM, (NO SOFFIT PANEL PROVIDED)
 CLEAR UNDER FASCIA: _____
 PARAPET SYSTEM
 STRUCTURAL PARAPET NON-STRUCTURAL PARAPET
 TOP OF PARAPET HEIGHT: _____
 BACKER PANEL, TYPE: _____ GAGE, FINISH: _____
 CANOPY (EXPOSED BEAM), PROJECTION: _____
 AT EAVE LINE BELOW EAVE
 ROOF PANEL, TYPE: _____ GAGE, FINISH: _____
 SOFFIT PANEL, TYPE: _____ GAGE, FINISH: _____
 SOFFIT TRIM AT BUILDING LINE PAINTED: _____
 CLEAR UNDER CANOPY BEAM: _____
 EAVE EXTENSION (CONCEALED BEAM), PROJECTION: _____
 SOFFIT PANEL, TYPE: _____ GAGE, FINISH: _____
 SOFFIT TRIM AT BUILDING LINE PAINTED: _____
 RAKE EXTENSION, PROJECTION: _____
 SOFFIT PANEL, TYPE: _____ GAGE, FINISH: _____
 SOFFIT TRIM AT BUILDING LINE PAINTED: _____
 PARTITION WALL SHEETING
 PANEL TYPE: _____ GAGE, FINISH: _____
 PARTITION WALL TRIM COLOR: _____

ERECTOR NOTE:

ALTERNATE FASTENERS HAVE BEEN SUBSTITUTED ON THIS BUILDING. WHERE THE DRAWINGS INDICATE AN H1040 STRUCTURAL FASTENER, H1041 FASTENERS WITH WASHERS HAVE BEEN SUPPLIED. WHERE THE DRAWINGS INDICATE AN H1060 TRIM FASTENER, H1061 FASTENERS WITH WASHERS HAVE BEEN SUPPLIED.

FOR OCCUPANCY CATEGORY I OR II BUILDINGS, IBC ALLOWS FOR SINGLE STORY BUILDINGS TO HAVE NO LIMIT FOR SEISMIC STORY DRIFT. PLEASE NOTE THAT ANY INTERIOR WALLS, PARTITIONS, CEILINGS, AND EXTERIOR WALLS SHOULD BE DETAILED (BY OTHERS) TO ACCOMMODATE THIS STORY DRIFT.

BUILDING LOADS

DESIGN CODE: IBC 12
 ROOF LIVE LOAD: 20.00 PSF MBMA OCC. CLASS: II
 LIVE LOAD REDUCIBLE Yes
 GROUND SNOW LOAD: 43.0 PSF SNOW EXP. FACTOR, Ce: 1.00
 SNOW IMPORTANCE FACTOR, Is: 1.00
 WIND: 115 mph WIND IMPORTANCE FACTOR, Iw: _____
 EXPOSURE: C
 UL 90 No
 Classic Roof-Const. No. 161; Classic Roof w/ Translucent Panel-Const. No. 167
 CFR Roof-Const. No. 552; CFR Roof w/ Translucent Panel-Const. No. 590;
 Composite CFR Roof-Const. No. 552A; VR16 II Roof-Const. No. 332.
 SEISMIC INFORMATION Ss: 1.368 S1: 0.502
 Design Sds/Sd1: 0.912 / 0.502 Site Class: D
 Seismic Imp. Factor: 1.00 Seismic Design Category: D
 Analysis Procedure: Equivalent Lateral Force Method
 Basic SFRS: Ordinary Steel Moment Frames and Centrally-Braced Frames

NOTES:

1) COLLATERAL DEAD LOADS, UNLESS OTHERWISE NOTED, ARE ASSUMED TO BE UNIFORMLY DISTRIBUTED. WHEN SUSPENDED SPRINKLER SYSTEMS, LIGHTING, HVAC EQUIPMENT, CEILINGS, ETC., ARE SUSPENDED FROM ROOF MEMBERS, CONSULT THE M.B.S. IF THESE CONCENTRATED LOADS EXCEED 200 POUNDS, OR IF INDIVIDUAL MEMBERS ARE LOADED SIGNIFICANTLY MORE THAN OTHERS.

2) THE DESIGN OF STRUCTURAL MEMBERS SUPPORTING GRAVITY LOADS IS CONTROLLED BY THE MORE CRITICAL EFFECT OF ROOF LIVE LOAD OR ROOF SNOW LOAD, AS DETERMINED BY THE APPLICABLE CODE.

BUILDING	
ROOF DEAD (PSF):	3.00
PRI. COL. (PSF):	1
SEC. COL. (PSF):	1
SNOW Ct:	1.00
SNOW Cs:	1.00
ROOF SNOW (PSF):	30.1
WIND ENCLOSURE:	Closed
GCP:	± 0.18
SEISMIC R:	3.25
SEISMIC Cs:	0.281
BASE SHEAR (KIPS):	20.1

ERECTION MANUALS REQUIRED

(ERECTION MANUALS ARE SHIPPED WITH THE BUILDING IN A WAREHOUSE PACKING CRATE)

CFR ROOF H9700 OR H8260 SINGLE CURB (H9850)
 CLASSIC ROOF H9420 OR H8201 DOUBLE CURB (H9800)
 WALL SHEETING H9430 OR H8300 VR16 II (H9925)

DRAWING INDEX

COVERSHEET C1, C2, C3
 ANCHOR BOLT DRAWINGS F1, F2
 COLUMN BASE REACTIONS F2
 STRUCTURAL/SHEETING DRAWINGS _____
 DETAILS _____

NOV 19 2014

FOR CONSTRUCTION

PROJECT NAME: MELISSA SMITH
 OGDEN, UT
 CUSTOMER NAME: SMITH STONE SUPPLY, INC.
 OGDEN, UT
 JOB NUMBER: U1408258A
 SHEET TITLE: COASTAL STEEL STRUCTURES
 SHEET: C1 of 3

REG. NO. 19192014
 COLTON DAVIS
 SIMMONS
 STATE OF UTAH

DESIGNER: COLTON DAVIS
 CHECKED: COLTON DAVIS
 DATE: 11/19/14

COASTAL STEEL STRUCTURES

PROJECT NUMBER:	U1408258A
PROJECT NAME:	Melissa Smith
PROJECT LOCATION:	Ogden, UT
CUSTOMER:	Smith Stone Supply, Inc.

STRUCTURAL TESTS AND INSPECTION:

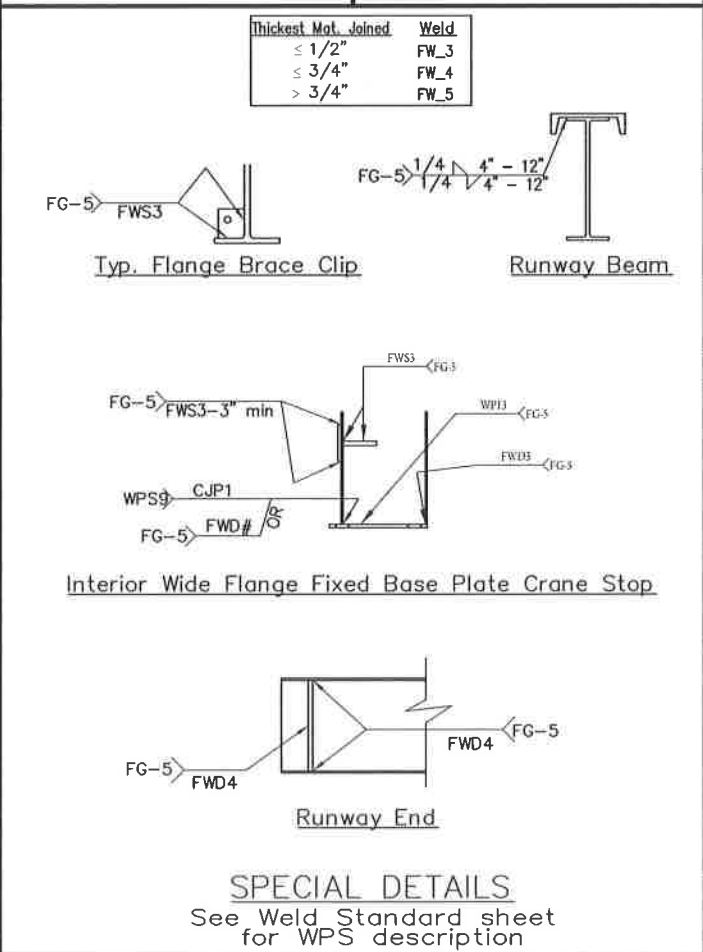
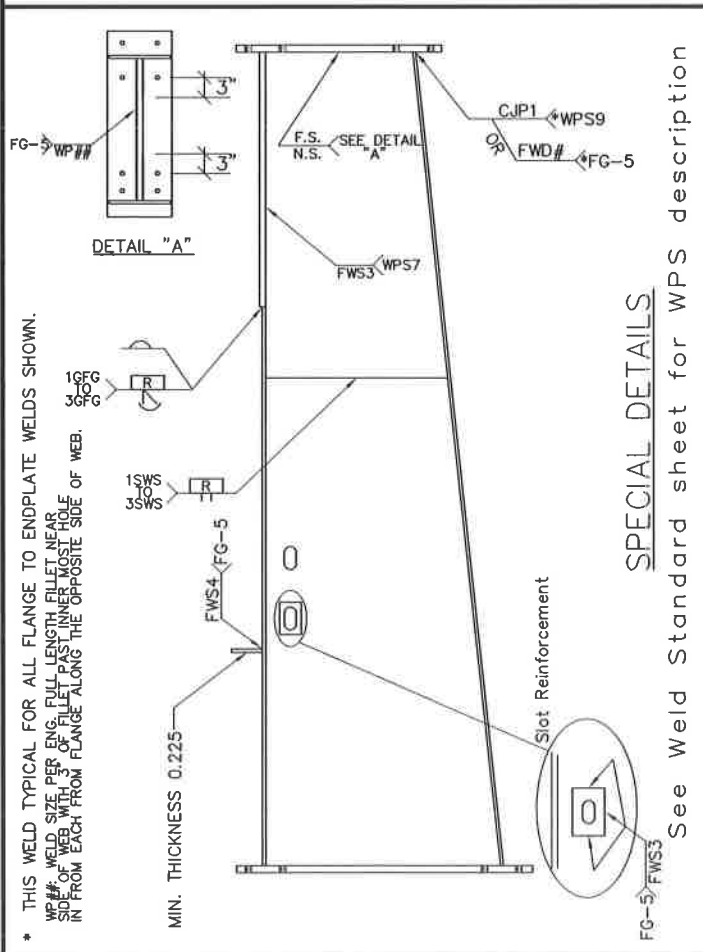
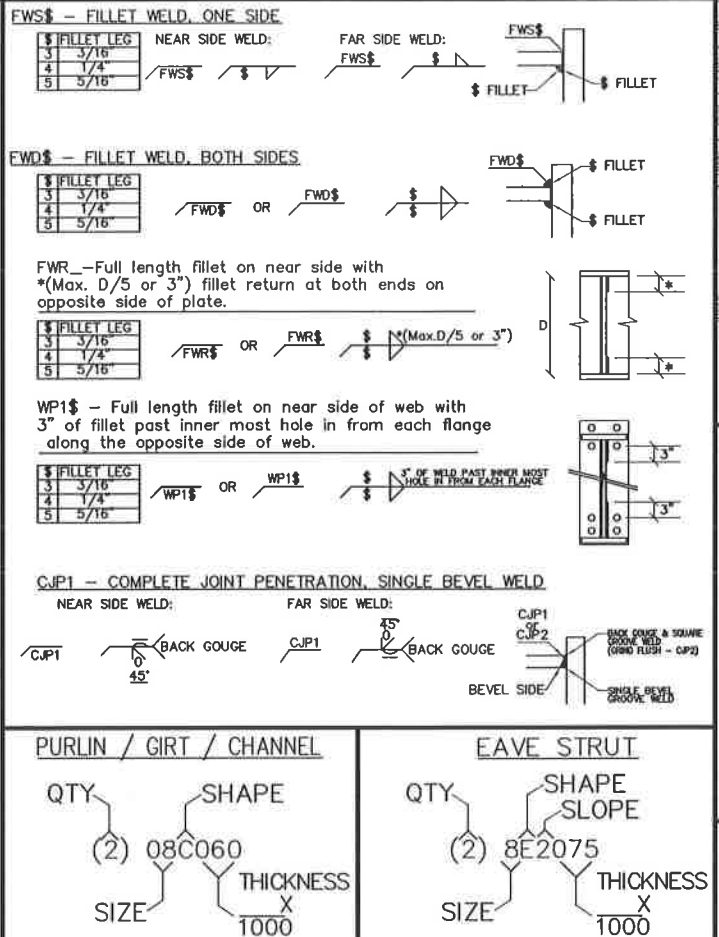
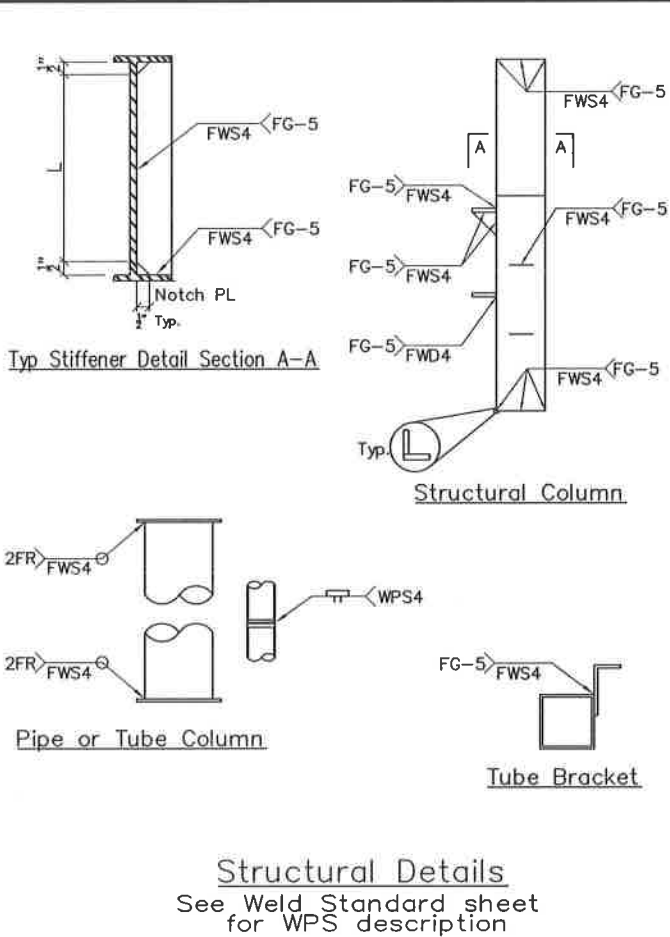
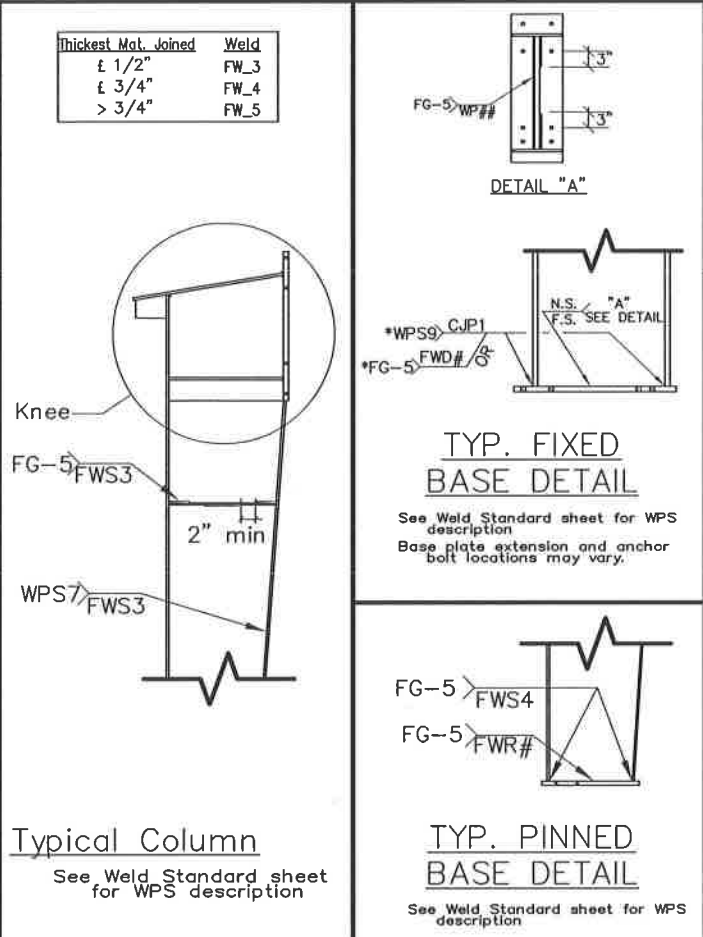
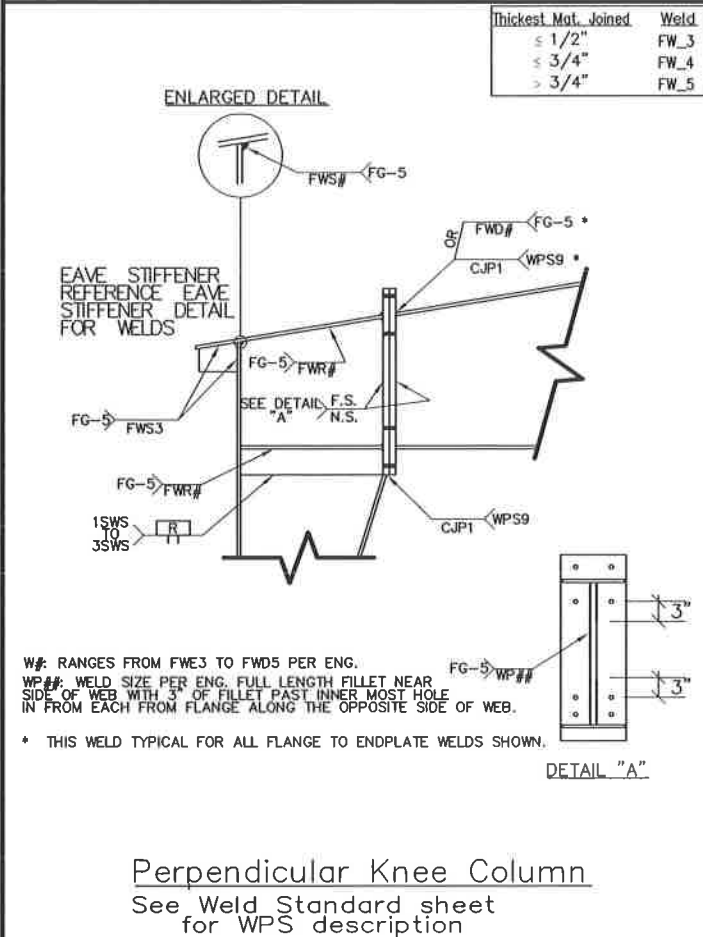
- 1) THE SPECIAL INSPECTOR'S DUTIES ARE AS DESCRIBED IN SPECIAL INSPECTION. THE SPECIAL INSPECTOR'S DUTIES ARE AS DESCRIBED IN IBC 1704.3 AND IBC 1705
- 2) ALL TESTS AND INSPECTIONS SHALL BE PERFORMED BY AN INDEPENDENT TESTING AND INSPECTION AGENCY EMPLOYED BY THE OWNER OR ARCHITECT.
- 3) THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING THE TEST AND INSPECTION FIRM WITH A SCHEDULE TO FACILITATE THE PROPER COORDINATION OF WORK.
- 4) PORTIONS OF WORK REQUIRING SPECIAL INSPECTION:

AGENCY RESPONSIBLE FOR INSPECTION AND TESTING TO BE NAMED BY OWNER LATER.

	YES	NO	N/A
A. STRUCTURAL STEEL:			
1. MILL REPORTS AND IDENTIFICATION OF STEEL (AFFIDAVIT OF COMPLIANCE)	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
2. SAMPLING AND TESTING OF SPECIMENS	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
B. WELDING:			
1. ALL STRUCTURAL WELDING (INCLUDES DECKING AND WELDED STUDS), EXCEPT WELDING IN APPROVED SHOPS PER IBC 1704.2.2	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
2. ULTRASONIC TESTING OF FULL PENETRATION WELD CONNECTIONS AT MOMENT FRAMES, BRACED FRAMES, BEAM SPLICES, AND FIELD WELDS.	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
3. STRUCTURAL LIGHT GAGE METAL FRAME WELDING	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
C. BOLTING:			
1. HIGH STRENGTH BOLT A325SC AND A490SC (PRETENSION VERIFICATION)	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
2. HIGH STRENGTH BOLT A325N AND A490X (PER COVER SHEET NOTES)	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
3. EXPANSION/ADHESIVE ANCHORS IN CONCRETE OR MASONRY	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>

<small>The steel shown on this drawing is the material designated by the manufacturer and is the property of the manufacturer. The drawings and the metal buildings which they represent are the product of the Metal Building Manufacturer. The registered professional engineer whose name appears on these drawings is licensed by the State of Utah and his/her signature and seal represent the project engineer of record and shall not be construed as such.</small>	PROJECT NAME MELISSA SMITH OGDEN, UT	COASTAL STEEL STRUCTURES 4800 NW 2ND AVE, SUITE 5 BOCA RATON, FL 33431 PHONE: (561) 221-6600 TOLL FREE: (888) 783-3535 FAX: (888) 783-3535	DATE 11/19/14
	CUSTOMER NAME SMITH STONE SUPPLY, INC. OGDEN, UT	SHEET TITLE U1408258A	SHEET C2 of 3

STANDARD WELDS AND LEGEND



WPS #	Description	Weld Code	Process	Position	Limitation	Plant Location	
						UT	SC/TX
FG-4	3" & 1/2" multi-pass fillet weld	AWSD1.1-06	GAW (3" multi-pass fillet weld)	1G	1/8" - 1 1/2"	X	X
FG-5	1/2" & 3/4" single-pass fillet weld	AWSD1.1-06	GAW (1/2" single-pass fillet weld)	2F	1/8" - 1 1/2"	X	X
1GFG	3" through 1" flange splice	AWSD1.1-06	GAW (3" through 1" flange splice)	1G	1/8" - 1 1/2"	X	X
2GFG	1/2" flange splice	AWSD1.1-06	GAW (1/2" flange splice)	1G	1/8" - 1 1/2"	X	X
3GFG	1/2" to 1/4" flange splice	AWSD1.1-06	GAW (1/2" to 1/4" flange splice)	1G-2F	1/8" - 1 1/2"	X	X
2036	1/2" to 1/4" flange splice	AWSD1.1-06	GAW (1/2" to 1/4" flange splice)	1G-2F	1/8" - 1 1/2"	X	X
5GFG	1" flange splice	AWSD1.1-06	GAW (1" flange splice)	1G-2F	1/8" - 1 1/2"	X	X
1SWS	web splice for 0.275" thick through 0.313" thick	AWSD1.1-06	SAW (0.275-0.313" web splice)	1G	0.375 - 0.313	X	X
2SWS	web splice for 0.125" thick through 0.150" thick	AWSD1.1-06	SAW (0.125-0.150" web splice)	1G	0.125 - 0.150	X	X
3SWS	web splice for 0.175" thick through 0.250" thick	AWSD1.1-06	SAW (0.175-0.250" web splice)	1G	0.175 - 0.250	X	X
2038	web splice for 0.375" thick through 0.500" thick	AWSD1.1-06	SAW (0.375-0.500" web splice)	1G	0.375 - 0.500	X	X
2FR	pipe to endplate weld	AWSD1.1-06	GAW (Pipe-to-Endplate Weld)	2F ROTATED	6" thru 10"	X	X
FBC-1	Rod to plate/angle weld	AWSD1.1-06	GAW (Rod-to-Plate Weld)	1G	1/8" thru 1 1/2"	X	X
WPS4	pipe splice for 0.134" thick through 0.375" thick	AWSD1.1-06	GAW (Pipe splice for 0.134" thru 0.375")	1G	0.375 - 0.10"	X	X
WPS5	pipe splice for 0.375" thick through 0.500" thick	AWSD1.1-06	GAW (Pipe splice for 0.375" thru 0.500")	1G ROTATED	0.375 - 0.50" Diameter	X	X
WPS6	wide-flange beam splice, all sizes	AWSD1.1-06	GAW (Hot-Rolled Splice Weld)	1G	0.313 - 1.50	X	X
WPS7	flange to web weld made by autowelder using 0.062" electrode	AWSD1.1-06	SAW (Autowelder Weld)	2F	0.125 - 1.00	X	X
WPS8	flange to web weld made by autowelder using 0.052" electrode	AWSD1.1-06	SAW (Small Autowelder Welds)	2F	0.125 - 1.00	X	X
WPS9	complete penetration groove weld for tee connection, 1/2" thick	AWSD1.1-06	GAW (Flange to endplate weld)	1G	0.375 - 1.00	X	X
WPS11	vertical tack-fitters	AWSD1.1-06	GAW (Vertical tack-fitters)	3F	0.125 - Unlimited	X	X
WPS-1	Cold-Form seam stitch weld	AWSD1.3-98	GAW (CF seam stitch weld)	FLAT	0.0525" - 0.210"	X	X
WPS-1a	Cold-Form seam stitch weld (galvanized)	AWSD1.3-98	GAW (CF seam stitch weld galvanized)	FLAT	0.0525" - 0.210"	X	X
WPS-2	Cold-Form seam weld	AWSD1.3-98	GAW (CF seam weld)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-2a	Cold-Form seam weld (galvanized)	AWSD1.3-98	GAW (CF seam weld galvanized)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-3	cold-form seam weld to support steel ≥ 0.3125" thick	AWSD1.3-98	GAW (CF seam weld to support steel)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-3a	cold-form seam weld to support steel ≥ 0.3125" thick (galvanized)	AWSD1.3-98	GAW (CF seam weld to support steel galvanized)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-4	cold-form tee connection fillet weld	AWSD1.3-98	GAW (CF tee fillet weld)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-4a	cold-form tee connection fillet weld (galvanized)	AWSD1.3-98	GAW (CF tee fillet weld galvanized)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-5	cold-form tee connection fillet weld to support steel ≥ 0.3125" thick	AWSD1.3-98	GAW (CF tee fillet to support steel)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-5a	cold-form tee connection fillet weld to support steel ≥ 0.3125" thick (galvanized)	AWSD1.3-98	GAW (CF tee fillet to support steel galvanized)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-6	cold-form lap fillet weld	AWSD1.3-98	GAW (CF lap fillet weld)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-6a	cold-form lap fillet weld (galvanized)	AWSD1.3-98	GAW (CF lap fillet weld galvanized)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-7	cold-form lap fillet weld to support steel ≥ 0.3125" thick	AWSD1.3-98	GAW (CF lap fillet to support steel)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X
WPS-7a	cold-form lap fillet weld to support steel ≥ 0.3125" thick (galvanized)	AWSD1.3-98	GAW (CF lap fillet to support steel galvanized)	HORIZ.	t1=0.0525" - t2 ≥ 0.3125"	X	X

Anchor Bolts for Construction

COASTAL STEEL STRUCTURES

4600 NW 2ND AVE, SUITE 5
 BOCA RATON, FL 33431
 PHONE: (561) 221-6600
 FAX: (561) 221-6600
 (888) 785-3535 (888) 783-3535

PROJECT NAME: MELISSA SMITH OGDEN, UT
 CUSTOMER NAME: SMITH STONE SUPPLY, INC.
 OGDEN, UT

JOB NUMBER: U1408258A
 SHEET TITLE: SHEET

C3 of 3

ANCHOR BOLT SUMMARY

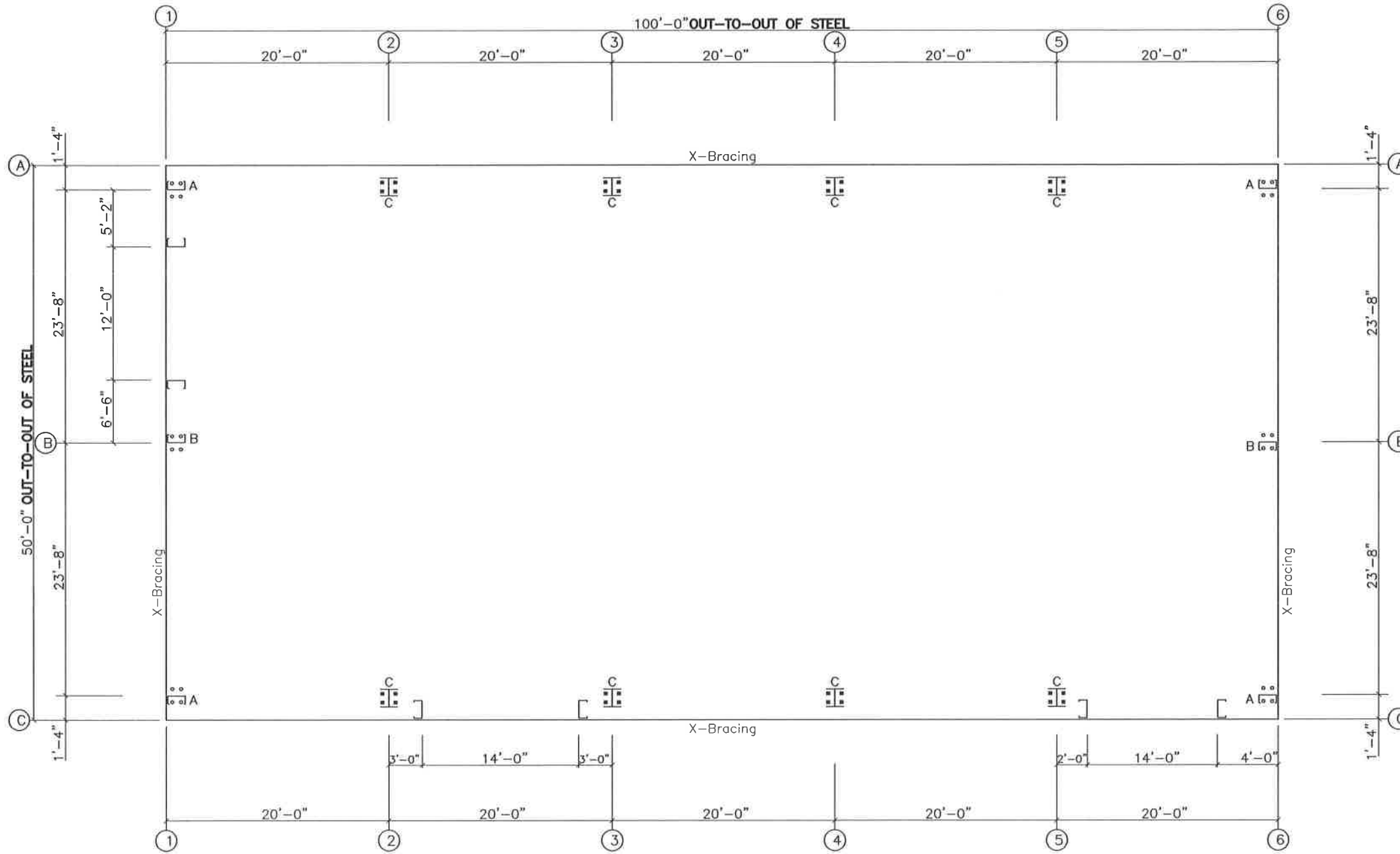
Qty	Locate	Dia (in)	Type	Proj (in)
24	Endwall	3/4"	F1554	3.00
32	Frame	1"	F1554	3.00

ANCHOR BOLT PLAN

GENERAL NOTES

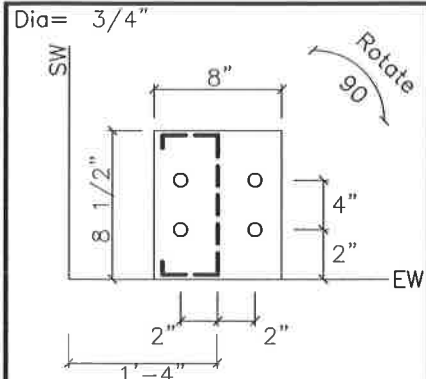
1. THE SPECIFIED ANCHOR ROD DIAMETER ASSUMES F1554 GRADE 36 UNLESS NOTED OTHERWISE. ANCHOR ROD MATERIAL OF EQUAL DIAMETER MEETING OR EXCEEDING THE STRENGTH REQUIREMENTS SET FORTH ON THESE DRAWINGS MAY BE UTILIZED AT THE DISCRETION OF THE FOUNDATION DESIGN ENGINEER. ANCHOR ROD EMBEDMENT LENGTH SHALL BE DETERMINED BY THE FOUNDATION DESIGN ENGINEER.
2. METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR PROJECT FOUNDATION DESIGN. THE FOUNDATION DESIGN IS THE RESPONSIBILITY OF A REGISTERED PROFESSIONAL ENGINEER, FAMILIAR WITH LOCAL SITE CONDITIONS.
3. ALL ANCHOR RODS, FLAT WASHERS FOR ANCHOR RODS, EXPANSION BOLTS, AS WELL AS ALL CONCRETE/MASONRY EMBEDMENT PLATES ARE NOT BY METAL BUILDING MANUFACTURER.
4. THIS DRAWING IS NOT TO SCALE.
5. FINISHED FLOOR ELEVATION = 100'-0" UNLESS NOTED OTHERWISE.
6. "SINGLE" CEE COLUMNS SHALL BE ORIENTED WITH THE "TOES" TOWARD THE LOW EAVE UNLESS NOTED OTHERWISE.
7. ANCHOR RODS ARE REQUIRED ONLY IN THE QUANTITIES SPECIFIED. BASEPLATES MAY BE FABRICATED WITH MORE HOLES THAN NEEDED FOR THIS PROJECT.
8. THE ANCHOR BOLT LOCATIONS PROVIDED BY METAL BUILDING MANUFACTURER SATISFY PERTINENT REQUIREMENTS FOR THE DESIGN OF THE MATERIALS SUPPLIED BY THE METAL BUILDING MANUFACTURER. PLEASE NOTE THAT THESE REQUIREMENTS MAY NOT SATISFY ALL ANCHOR BOLT CONCRETE EDGE DISTANCE REQUIREMENTS DEPENDING ON THE DETAILS OF THE FOUNDATION DESIGN. BECAUSE FOUNDATION DESIGN IS NOT WITHIN THE METAL BUILDING MANUFACTURER'S SCOPE OF WORK, IT IS THE RESPONSIBILITY OF THE QUALIFIED PROFESSIONAL DESIGNING THE FOUNDATION TO MAKE CERTAIN THAT SUFFICIENT CONCRETE EDGE DISTANCE IS PROVIDED FOR THE ANCHOR BOLTS IN THE DETAILS OF THE FOUNDATION DESIGN.

ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

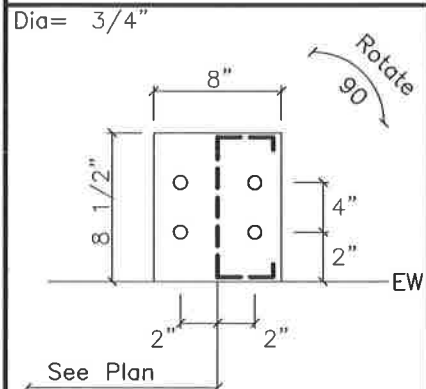


○ Dia = 3/4"
⊗ Dia = 1"

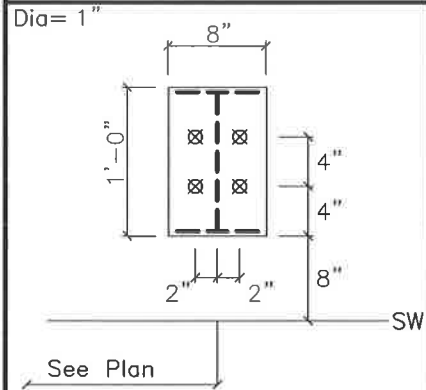
<p>PROJECT NAME MELISSA SMITH OGDEN, UT</p>	<p>CUSTOMER NAME SMITH STONE SUPPLY, INC. OGDEN, UT</p>	<p>JOB NUMBER U1408258A</p>	<p>SHEET TITLE</p>	<p>DATE 11/19/14</p>
<p>COASTAL STEEL STRUCTURES</p> <p>4800 NW 2ND AVE, SUITE 5 BOCA RATON, FL 33431 PHONE: (561) 221-6600 TOLL FREE: (888) 783-3535 FAX: (888) 783-3535</p>				<p>ANCHOR BOLTS FOR CONSTRUCTION</p> <p>MBS JBU RHW CDS</p>



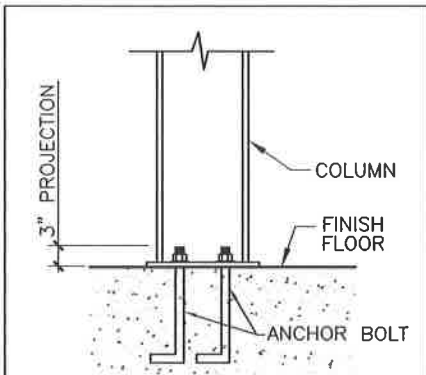
DETAIL A



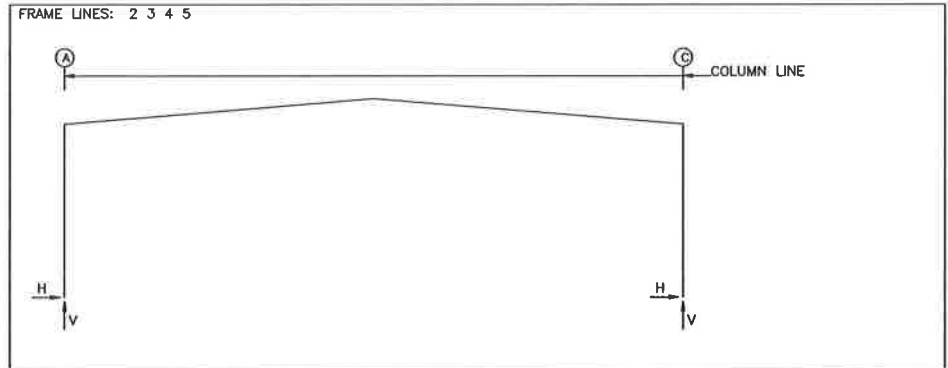
DETAIL B



DETAIL C



TYPICAL COLUMN BASE PLATE DETAIL



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Dia	Base Plate (in)			Elev. (in)
				Width	Length	Thick	
2*	A	4	1.000	8.000	12.00	0.375	0.0
2*	C	4	1.000	8.000	12.00	0.375	0.0

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Dia	Base Plate (in)			Elev. (in)
				Width	Length	Thick	
1	A	4	0.750	8.000	8.500	0.375	0.0
1	B	4	0.750	8.000	8.500	0.375	0.0
1	C	4	0.750	8.000	8.500	0.375	0.0
6	B	4	0.750	8.000	8.500	0.375	0.0
6	A	4	0.750	8.000	8.500	0.375	0.0

BUILDING BRACING REACTIONS

Loc	Line	Col Line	± Reactions (k)		Panel Shear (lb/ft)	
			Wind	Seismic	Wind	Seis
L-EW	1	B,C	1.5	0.9	2.4	1.5
F-SW	6	C	3.4	4.0	2.4	10.5
R-EW	6	C,B	1.5	0.9	2.4	1.5
B-SW	A	4,3	4.0	2.4	10.5	6.3

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	A	1.1	2.2	0.3	0.5	3.6	6.3	9.1	15.8	-7.3	-10.9	-2.2	-7.3
2*	C	-1.1	2.2	-0.3	0.5	-3.6	6.3	-9.1	15.8	7.3	-10.9	2.2	-7.3

Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	A	-6.0	-6.3	-0.9	-2.7	-2.9	-10.1	-3.2	-8.1	-2.5	-1.3	2.5	1.3
2*	C	0.9	-2.7	6.0	-6.3	3.2	-8.1	2.9	-10.1	-2.5	1.3	2.5	-1.3

Frame Line	Column Line	F1UNB_SL_L		F1UNB_SL_R	
		Horiz	Vert	Horiz	Vert
2*	A	7.4	15.8	7.4	8.7
2*	C	-7.5	8.7	-7.4	15.8

2* Frame lines: 2 3 4 5

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead		Collat		Live		Snow		Wind_Left1		Wind_Right1		Wind_Left2		Wind_Right2		Wind Press		Wind Suct		Wind Long1		Wind Long2	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	A	0.7	0.1	2.6	4.1	-3.8	-2.3	-3.8	-2.3	-1.9	2.1	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	
1	B	0.7	0.1	2.6	4.1	-3.8	-2.3	-3.8	-2.3	-1.9	2.1	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	
1	C	0.7	0.1	2.6	4.1	-3.8	-2.3	-3.8	-2.3	-1.9	2.1	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	

Frm Line	Col Line	Seis Left		Seis Right		E1UNB_SL_L		E1UNB_SL_R		E1PAT_LL_1		E1PAT_LL_2	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	A	0.1	-0.1	0.0	4.8	0.0	1.2	0.0	2.6	0.0	2.5	0.0	
1	B	-0.3	0.3	0.0	6.4	0.0	6.4	0.0	2.5	0.0	0.0	2.6	
1	C	0.2	0.1	0.0	1.2	0.0	4.8	0.0	0.0	0.0	2.6	0.0	

Frm Line	Col Line	Dead		Collat		Live		Snow		Wind_Left1		Wind_Right1		Wind_Left2		Wind_Right2		Wind Press		Wind Suct		Wind Long1		Wind Long2	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
6	A	0.7	0.1	2.6	4.1	-3.8	-2.3	-3.8	-2.3	-1.9	2.1	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	
6	B	0.7	0.1	2.6	4.1	-3.8	-2.3	-3.8	-2.3	-1.9	2.1	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	
6	C	0.7	0.1	2.6	4.1	-3.8	-2.3	-3.8	-2.3	-1.9	2.1	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	-2.4	-4.2	

Frm Line	Col Line	Seis Left		Seis Right		E2UNB_SL_L		E2UNB_SL_R		E2PAT_LL_1		E2PAT_LL_2	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
6	A	0.1	0.2	0.0	4.8	0.0	1.2	0.0	2.6	0.0	2.5	0.0	
6	B	-0.3	0.3	0.0	6.4	0.0	6.4	0.0	2.5	0.0	0.0	2.6	
6	C	0.2	0.1	0.0	1.2	0.0	4.8	0.0	0.0	0.0	2.6	0.0	

GENERAL NOTES

- ALL LOADING CONDITIONS ARE EXAMINED. THE MAXIMUM AND MINIMUM HORIZONTAL (H) AND VERTICAL (V) REACTIONS AND THE CORRESPONDING VERTICAL (V) OR HORIZONTAL (H) REACTIONS ARE REPORTED.
- REACTIONS ARE PROVIDED BY LOAD CASE IN ORDER TO AID THE FOUNDATION ENGINEER IN DETERMINING THE APPROPRIATE LOAD FACTORS AND COMBINATION TO BE USED WITH EITHER WORKING STRESS OR ULTIMATE STRENGTH DESIGN METHODS. WIND LOAD CASES ARE GIVEN FOR EACH PRIMARY WIND DIRECTION.
- FOR ASCE7-10 BASED BUILDING CODES THE UNFACTORED LOAD CASE REACTIONS DUE TO WIND ARE GENERATED USING ULTIMATE DESIGN WIND SPEEDS (V_{ult}).
- POSITIVE (+) REACTIONS ARE AS SHOWN ABOVE. FOUNDATION LOADS ARE IN OPPOSITE DIRECTIONS.
- BRACING REACTIONS ARE IN THE PLANE OF THE BRACE WITH THE HORIZONTAL REACTION (H) ACTING AWAY FROM THE BRACED BAY AND THE VERTICAL REACTION (V) ACTING DOWNWARD.

***** RIGID FRAME LOAD CASE ABBREVIATIONS: *****
 Wind_L1/Wind_R1: LATERAL WIND FROM THE LEFT/RIGHT, CASE 1
 Wind_L2/Wind_R2: LATERAL WIND FROM THE LEFT/RIGHT, CASE 2
 Wind_Ln1/Wind_Ln2: LONGITUDINAL WIND, CASE 1/2
 Seismic_L/Seismic_R: LATERAL SEISMIC LOAD FROM LEFT/RIGHT
 LWIND#_L/E#_R: LONGITUDINAL WIND EDGE ZONES
 F#UNB_SL_L/F#UNB_SL_R: UNBALANCED ROOF SNOW WITH WIND FROM LEFT/RIGHT
 F#PAT_LL_#/F#PAT_SL_#: PARTIAL LIVE/SNOW LOADING FOR CONTINUOUS BEAM SYSTEMS

***** ENDWALL COLUMN LOAD CASE ABBREVIATIONS: *****
 Collat: COLLATERAL LOAD
 Rafter Wind_L/Rafter Wind_R: LATERAL WIND FROM THE LEFT/RIGHT
 Brace Wind_L/Brace Wind_R: LATERAL WIND FROM THE LEFT/RIGHT
 Wind_P/Wind_S: LONGITUDINAL WIND PRESSURE/SUCTION ON COLUMNS
 Wind_Ln: LONGITUDINAL WIND SUCTION ON ROOF
 Seis_L/Seis_R: LATERAL SEISMIC LOAD FROM LEFT/RIGHT
 E#UNB_SL_L/E#UNB_SL_R: UNBALANCED ROOF SNOW WITH WIND FROM LEFT/RIGHT
 E#PAT_LL_#/E#PAT_SL_#: PARTIAL LIVE/SNOW LOADING FOR CONTINUOUS BEAM SYSTEMS

FOUNDATION DESIGN NOTE:
 THE ORIENTATION OF THE ANCHOR BOLT DETAILS SHOWN ON THIS PAGE MAY NOT COINCIDE WITH THE ACTUAL COLUMN ORIENTATION SHOWN ON PAGE F1. PLEASE REFERENCE THE SIDEWALL (SW) AND ENDWALL (EW) STEEL LINES SHOWN ON THE ANCHOR BOLT DETAILS WITH THE ANCHOR BOLT PLAN ON PAGE F1 DURING LAYOUT OF COLUMN AND ANCHOR BOLT LOCATIONS.

PROJECT NAME MELISSA SMITH OGDEN, UT CUSTOMER NAME SMITH STONE SUPPLY, INC. OGDEN, UT JOB NUMBER U1408258A	PROJECT DATE 11/19/14 RHW JBU MBS Anchor Bolts for Construction	COASTAL STEEL STRUCTURES 4800 NW 2ND AVE, SUITE 5 BOCA RATON, FL 33431 PHONE: (561) 221-6600 FAX: (561) 221-6600 (888) 785-3535 (888) 783-3535
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SHEET TITLE
 SHEET NUMBER
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