UNLESS NOTED OTHERWISE, ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST BUILDING CODE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL EXISTING CONDITIONS AT THE JOB SITE, AND TO FULLY COORDINATE ALL DIMENSIONS AND CONDITIONS OF DETAILS WITH OTHER DISCIPLINES. ANY FIELD CONDITIONS REQUIRING CONSTRUCTION THAT IS DIFFERENT FROM THAT SHOWN ON THE PLANS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT. ANY CONFLICTING DETAILS SHOWN IN THE DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO THE CONSTRUCTION OF SAID DETAIL. DO NOT SCALE DRAWINGS. ANY QUESTIONS REGARDING THE CONSTRUCTION DOCUMENTS SHALL BE SUBMITTED TO THE ARCHITECT IN THE FORM OF A WRITTEN REQUEST FOR INFORMATION (RFI).

ALL SUPPORT OF CONSTRUCTION LOADS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ALL SHORING AND BRACING REQUIRED FOR THE PROTECTION OF LIFE AND PROPERTY DURING THE CONSTRUCTION PROCESS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ALL WORK SHALL BE DONE IN ACCORDANCE WITH OSHA REQUIREMENTS POTENTIAL CONFLICTS BETWEEN THESE DOCUMENTS AND OSHA REQUIREMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE WORK. ALL PROCEDURES OF SOIL EXCAVATION, BACK FILL, AND SUPPORT OF ADJACENT PROPERTY DURING EARTHWORK SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. ALL DIMENSIONS INDICATED ON PLANS SHALL BE TO FACE OF STUDS, FACE OF CONCRETE BLOCK, FACE OF ROUGH CONCRETE, CENTERLINE OF COLUMNS, BOTTOM OF METAL DECK, AND TOP OF SLAB, UNLESS NOTED OTHERWISE. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS NOT INDICATED ON STRUCTURAL DRAWINGS. THE FOLLOWING DESIGN CRITERIA SHALL BE ENFORCED.

GOVERNING BUILDING CODE: IBC 2018 RISK CATEGORY: II (IBC TABLE 1604.5)

- ROOF DEAD LOAD: 20 PSF (15 PSF + 5 PSF FOR SOLAR PANELS) ROOF LIVE LOAD: 20 PSF (NON-CONCURRENT WITH ROOF SNOW LOAD)
- ROOF SNOW LOAD: A. GROUND SNOW LOAD Pg = 60 PSF
- B. FLAT ROOF SNOW LOAD Pf = 50 PSF (SNOW DRIFT PER ASCE 7)
- C. SLOPED ROOF SNOW LOAD Ps = 25 PSF
- D. SNOW EXPOSURE FACTOR Ce = 1.2
- E. SNOW LOAD IMPORTANCE FACTOR Is = 1.0

F. THERMAL FACTOR Ct = 1.0

4. LIVE: 40 PSF (RESIDENTIAL)

- BASIC WIND SPEED: 105 MPH USED IN CALCS
- WIND EXPOSURE TYPE: C
- WIND IMPORTANCE FACTOR, Iw= 1.0 4. INTERNAL PRESSURE COEFFICIENT= ±0.18

SEISMIC IMPORTANCE FACTOR Ie= 1.0 SITE COEFFICIENTS

- A. SDS = 0.767g
- B. Ct = 0.02C. SOIL SITE CLASS= D
- D. SEISMIC DESIGN CATEGORY= D
- 4. BASIC LFRS = LIGHT FRAMED WALL WITH SHEAR WALLS (LONGITUDINAL DIR.) A. RESPONSE MODIFICATION COEFFICIENT R=6.5
- B. W WEIGHT OF STRUCTURE:
- C. DESIGN BASE SHEAR = 0.118W (ULTIMATE), 0.084W (SERVICE) D. DESIGN PROCEDURE: EQUIVALENT LATERAL FORCE
- . BASIC LFRS = LIGHT FRAMED WALL WITH SHEAR WALLS (TRANSVERSE DIR.) A. RESPONSE MODIFICATION COEFFICIENT R=2 B. W WEIGHT OF STRUCTURE:
- C. DESIGN BASE SHEAR = 0.384W (ULTIMATE), 0.274W (SERVICE)
- D. DESIGN PROCEDURE: EQUIVALENT LATERAL FORCE

ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE ARCHITECT/ENGINEER FOR REVIEW ALTERNATE MATERIALS THAT ARE SURMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION OR THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW. ALTERNATES THAT REQUIRE SUBSTANTIAL EFFORT TO REVIEW WILL NOT BE REVIEWED UNLESS AUTHORIZED BY THE OWNER.

IN CASE OF DISCREPANCIES BETWEEN THE GENERAL NOTES, SPECIFICATIONS PLAN/DETAILS OR REFERENCE STANDARDS. THE ARCHITECT/ENGINEER SHALL DETERMINE WHICH SHALL GOVERN. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK. SHOULD ANY DISCREPANCY BE FOUND IN THE CONTRACT DOCUMENTS, THE CONTRACTOR WILL BE DEEMED TO HAVE INCLUDED IN THE PRICE THE MOST EXPENSIVE WAY OF COMPLETING THE WORK, UNLESS PRIOR TO THE SUBMISSION OF THE PRICE, THE CONTRACTOR ASKS FOR A DECISION FROM THE ARCHITECT AS TO WHICH SHALL GOVERN. ACCORDINGLY, ANY CONFLICT IN OR BETWEEN THE CONTRACT DOCUMENTS SHALL NOT

SITE VERIFICATION:

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE. CONFLICTS BETWEEN THE DRAWINGS AND ACTUAL SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK.

MEANS, METHODS AND SAFETY REQUIREMENTS

BE A BASIS FOR ADJUSTMENT IN THE CONTRACT PRICE.

THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND ALL JOB RELATED SAFETY STANDARDS SUCH AS OSHA AND DOSH (DEPARTMENT OF OCCUPATIONAL SAFETY AND HEALTH). CONTRACTOR IS RESPONSIBLE TO ADHERE TO OSHA REGULATIONS REGARDING STEEL ERECTION ITEMS SPECIFICALLY ADDRESSED ON THE LATEST OSHA REGULATIONS. BOLTING AND FIELD WELDING AT ALL MEMBER CONNECTIONS IS TO BE COMPLETED PRIOR TO THE RELEASE OF THE MEMBER FROM THE HOISTING MECHANISM UNLESS REVIEWED AND APPROVED BY THE GENERAL CONTRACTOR'S TEMPORARY BRACING AND SHORING DESIGN ENGINEER.

BRACING/SHORING DESIGN ENGINEER:

THE CONTRACTOR SHALL AT HIS DISCRETION EMPLOY AN SSE, A REGISTERED PROFESSIONAL ENGINEER FOR THE DESIGN OF ANY TEMPORARY BRACING AND SHORING.

THE CONTRACTOR IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION AND SHALL PROVIDE TEMPORARY SHORING, BRACING AND OTHER ELEMENTS REQUIRED TO MAINTAIN STABILITY UNTIL THE STRUCTURE IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE FAMILIAR WITH THE WORK REQUIRED IN THE CONSTRUCTION DOCUMENTS AND THE REQUIREMENTS FOR EXECUTING IT PROPERLY.

LOADS ON THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS AS NOTED IN DESIGN CRITERIA & LOADS BELOW OR THE CAPACITY OF PARTIALLY COMPLETED CONSTRUCTION AS DETERMINED BY THE CONTRACTOR'S SSE FOR BRACING/SHORING.

CHANGES IN LOADING:

THE CONTRACTOR HAS THE RESPONSIBILITY TO NOTIFY THE SER OF ANY ARCHITECTURAL, MECHANICAL, ELECTRICAL, OR PLUMBING LOAD IMPOSED ONTO THE STRUCTURE THAT DIFFERS FROM, OR THAT IS NOT DOCUMENTED ON THE ORIGINAL CONTRACT DOCUMENTS (ARCHITECTURAL / STRUCTURAL / MECHANICAL / ELECTRICAI OR PLUMBING DRAWINGS). PROVIDE DOCUMENTATION OF LOCATION, LOAD, SIZE AND ANCHORAGE OF ALL UNDOCUMENTED LOADS IN EXCESS OF 400 POUNDS. PROVIDE MARKED-UP STRUCTURAL PLAN INDICATING LOCATIONS OF ANY NEW EQUIPMENT OR LOADS. SUBMIT PLANS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO INSTALLATION.

PLAN AND DETAIL NOTES AND SPECIFIC LOADING DATA PROVIDED ON THE INDIVIDUAL PLANS AND DETAIL DRAWINGS SUPPLEMENTS INFORMATION IN THE STRUCTURAL GENERAL NOTES.

DIMENSIONS ARE FOR REFERENCE, CONTRACTOR TO VERIFY ALL DIMENSIONS. DIMENSIONS ARE PROVIDED BY THE ARCHITECT'S ELECTRONIC FILE. ALWAYS VERIFY THESE PLANS AND DIMENSIONS WITH THE ARCHITECT PLANS UNDER NO CIRCUMSTANCES WILL McNEIL ENGINEERING, ITS EMPLOYEES OR AGENTS BE LIABLE FOR ANY DIRECT, INDIRECT PUNITIVE OR CONSEQUENTIAL DAMAGES THAT MAY RESULT IN ANY WAY FROM YOUR USE, MISUSE, REFERENCE TO OR RELIANCE ON ANY OF THE INFORMATION PROVIDED OR THAT RESULT FROM MISTAKES, ERRORS, OMISSIONS, INTERPRETATIONS OR DEFECTS. MCNEIL ENGINEERING EXPRESSLY DISCLAIMS ALL WARRANTIES, INCLUDING ANY EXPRESS OR

IMPLIED WARRANTIES OF MERCHANTABILITY OR FITNESS FOR A PARTICULAR PURPOSE WITH RESPECT TO THE INFORMATION HEREIN.

WHILE MCNEIL ENGINEERING MAKES EVERY EFFORT TO PRESENT ACCURATE AND RELIABLE INFORMATION, MCNEIL ENGINEERING DOES NOT ENDORSE, APPROVE OR CERTIFY THE INFORMATION PROVIDED BY OTHERS. NOR DOES MCNEIL ENGINEERING GUARANTEE ITS ACCURACY, COMPLETENESS OR TIMELINESS. USE OF THIS INFORMATION IS VOLUNTARY AND RELIANCE ON IT SHOULD ONLY BE UNDERTAKEN AFTER YOU HAVE INDEPENDENTLY VERIFIED ITS ACCURACY, COMPLETENESS AND TIMELINESS.

CONTRACTOR SHALL BE RESPONSIBLE TO FIELD VERIFY DIMENSIONS AGAINST THE CORRESPONDING OFFICIAL CONSTRUCTION DRAWINGS. DIMENSIONS SHOWN ON THE CONSTRUCTION DOCUMENTS MUST BE VERIFIED WITH ARCHITECTURAL PLANS. IF ANY DISCREPANCIES ARE FOUND THE CONTRACTOR SHALL NOTIFY THE ARCHITECT BEFORE PROCEEDING WITH THE WORK. MCNEIL ENGINEERING DOES NOT GUARANTEE THAT THIS ELECTRONIC MEDIA HAS NOT BEEN DAMAGED, ALTERED OR MODIFIED DURING TRANSMISSION AND/OR STORAGE. MCNEIL ENGINEERING DOES NOT GUARANTEE CHANGES ON THE ARCHITECTURAL PLANS HAVE BEEN FULLY CONVEYED AND THE THE CONSTRUCTION DOCUMENT UPDATED. ANYONE RECEIVING ELECTRONIC MEDIA MUST VERIFY ALL INFORMATION WITH THE CORRESPONDING OFFICIAL CONSTRUCTION DRAWINGS. ANY USE OR REUSE OF THIS INFORMATION SHALL BE THE FULL RESPONSIBILITY OF THE USER.

SOIL TO BE OBSERVED PRIOR TO PLACEMENT OF FOOTINGS. ALL FOOTING DEPTHS INDICATED ON PLANS ARE MINIMUM DEPTHS. FOOTINGS MAY BE PLACED IN NEAT EXCAVATED TRENCHES. TRENCH SHALL BE APPROVED BY INSPECTOR PRIOR TO PLACEMENT OF CONCRETE. AT LOCATIONS WHERE STRUCTURAL FILL IS REQUIRED, FILL SHALL BE PLACED IN 6" LIFTS & COMPACTED AT OPTIMUM MOISTURE CONTENT.

- . MAXIMUM NET BEARING PRESSURE = 1,500 PSF (NET ALLOWABLE) ASSUMED PER IBC 2. FROST DEPTH = VERIFY W/ LOCAL JURISDICTION - FOOTINGS TO BE PLACED 36" MIN DEPTH (FROM GRADE TO BOTTOM OF FOOTING)
- 3. INTERIOR FOOTINGS = 12" (BOTTOM OF FOOTING TO TOP OF SOIL OR SLAB) 4. LATERAL SOIL PRESSURE : ACTIVE = 40 PCF; AT REST = 55 PCF; PASSIVE = 250 PCF (ASSUMED)
- 5. FRICTION COEFFICIENT: 0.45

ALL CONCRETE MATERIALS SHALL COMPLY WITH THE STANDARDS SPECIFIED IN THE LATEST EDITION OF THE ACI 318 BUILDING CODE. EACH MIX DESIGN SHALL BE REVIEWED BY AN APPROVED INDEPENDENT LABORATORY.

LOCATION	EXPOSURE CLASS	SLUMP (MAX)	AGGRE GATE (MAX SIZE)	AIR CONTENT	COMPRESSIVE STRENGTH (PS
FOOTINGS (INTERIOR)	F0, S0, P0, C0	5"	1" DIA.	1.5%	3,500 PSI
FOOTINGS (EXTERIOR)	F0, S0, P0, C0	5"	1" DIA.	1.5%	3,500 PSI
CONCRETE WALLS	F1, S0, P0, C1	4"	3/4" DIA.	5%	4,500 PSI
INTERIOR SLAB ON GRADE	F0, S0, P0, C0	5"	3/4" DIA.	1.5%	4,000 PSI

2500 PSI USED IN DESIGN

MAX FLYASH: 25%

AIR CONTENT +- 1.5% MEASURED AT POINT OF FINAL PLACEMENT. AIR-ENTRAINING ADMIXTURES SHALL COMPLY WITH ASTM C260 (WHEN USED). CALCIUM CHLORIDE SHALL NOT BE ADDED TO THE CONCRETE MIX. UNREINFORCED CONCRETE SLABS ON GRADE MAY HAVE CALCIUM CHLORIDE NOT EXCEEDING ONE PERCENT. AIR ENTRAINMENT SHALL BE ADJUSTED FOR THE USE OF ADMIXTURES AND FLY ASH.

ANY CONCRETE THAT FAILS TO MEET SPECIFICATIONS SHALL BE REMOVED AND REPLACED AT THE EXPENSE OF THE CONTRACTOR.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION, DESIGN, PLACEMENT AND REMOVAL OF ALL FORMWORK. ALL SHORING DURING PLACEMENT OF CONCRETE IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

SEE CIVIL DRAWINGS FOR SITE CONCRETE REQUIREMENTS. IN ABSENCE OF

INFORMATION, USE VALUES LISTED.

CONFORM TO THE FOLLOWING COVER AND CORROSION PROTECTION REQUIREMENTS

UNLESS NOTED OTHERWISE IN THE DRAWIN	NGS:
REINFORCEMENT LOCATION	MIN. COVER
FOOTING BOTTOM REINFORCEMENT	3"
FOOTING TOP REINFORCEMENT	2"
SLAB ON GRADE REINFORCEMENT	2" FROM TOP
WALLS IN CONTACT WITH EARTH	2"
WALLS NOT IN CONTACT WITH EARTH	3/4"

POST-INSTALLED ANCHORS TO CONCRETE:

ANCHOR LOCATION, TYPE, DIAMETER AND EMBEDMENT SHALL BE AS INDICATED ON DRAWINGS. REFERENCE THE "POST INSTALLED ANCHORS" SECTION FOR APPLICABLE POST-INSTALLED ANCHOR ADHESIVES. ANCHORS SHALL BE INSTALLED AND INSPECTED IN STRICT ACCORDANCE WITH THE APPLICABLE ICC.

ALL REINFORCING BARS SHALL CONFORM TO ASTM A-615 GRADE 60, Fy=60,000 PSI MIN., UNLESS NOTED OTHERWISE. BARS SHALL BE TIED SECURE PRIOR TO PLACEMENT OF CONCRETE TO MAINTAIN PROPER PLACEMENT AFTER CONCRETE IS IN PLACE. LAP ALL BARS 40 DIAMETERS UNLESS NOTED OTHERWISE. SPLICE BARS ONLY WHERE SHOWN ON

NORMAL WEIGHT CONCRETE SHALL HAVE A UNIT WEIGHT OF POUNDS PER CUBIC FOOT. USE OF CALCIUM CHLORIDE IS NOT PERMITTED IN ANY CONCRETE MIXES. ALL OTHER ADDITIVES AND ADMIXTURES MUST HAVE THE WRITTEN APPROVAL OF THE ENGINEER.

THE ENGINEER SHALL HAVE 10 BUSINESS DAYS TO REVIEW SHOP DRAWINGS.

ALL STRUCTURAL STEEL COMPONENTS SHALL BE FABRICATED AND ERECTED ACCORDING TO THE LATEST EDITION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATIONS FOR DESIGN FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", WITH " COMMENTARY", AND THE AISC "CODE OF

STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AS WELL AS THE FOLLOWING AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS"

APPROVED BY THE RESEARCH COUNCIL ON RIVETED AND BOLTED STRUCTURAL JOINTS OF THE ENGINEERING FOUNDATION. AWS D1.1 "STRUCTURAL WELDING CODE".

ASTM A-6 "GENERAL REQUIREMENTS FOR DELIVERY OF ROLLED STEEL PLATES, SHAPES, SHEET PILING AND BARS FOR STRUCTURAL USE".

ALL STEEL SECTIONS SHALL CONFORM TO THE FOLLOWING: WIDE FLANGE SHAPES: ASTM A572 GRADE 50 OR ASTM A992 GRADE 50.

HOLLOW STRUCTURAL SECTIONS: ASTM A500 GRADE B FY MIN. = 46 KSI

ANGLES, CHANNELS, PLATES & BARS: ASTM A36.

ALL LAMINATED VENEER LUMBER SHALL CONFORM TO THE SPECIFICATIONS OF BOISE CASCADE CORPORATION FOR VENEER LUMBER, OR ENGINEER APPROVED EQUIVALENT. DESIGN VALUES SHALL MEET OR EXCEED THOSE PUBLISHED VALUES IN THE BOISE CASCADE PRODUCT GUIDE, LATEST EDITION.

SHEATHING SHALL BE A.P.A. RATED, SEE PLAN FOR SPAN RATING AND THICKNESS. SHEATHING INSTALLATION:

ROOF AND FLOOR SHEATHING SHALL BE LAID WITH THE FACE GRAIN PERPENDICULAR TO THE FRAMING MEMBERS U.N.O. AND END JOINTS SHALL BE STAGGERED. WALL

ALL NAILS SHALL BE COMMON WIRE NAILS U.N.O. EQUIVALENT PNEUMATIC DRIVEN NAILS MAY BE USED IF FASTENER MANUFACTURER HAS CURRENT I.C.C. APPROVAL. FASTENERS TO BE USED SHALL BE EQUIVALENT IN LATERAL AND WITHDRAWAL STRENGTH TO THE SIZE COMMON NAIL SPECIFIED.

USE EXTERIOR GRADE SHEATHING AT DECKS AND CORRIDORS.

SHEATHING MAY BE APPLIED HORIZONTALLY OR VERTICALLY

EDGE BLOCKING OF UNSUPPORTED EDGES OF SHEATHING AS NOTED ON PLANS. PLY CLIPS OR APPROVED EOUAL CONNECTOR SHALL BE INSTALLED AT MID SPAN BETWEEN EACH SUPPORT WHEN RAFTER SPACING EXCEEDS 16" AND EDGE BLOCKING IS NOT SPECIFIED.

TYPICAL NAILING SHALL BE 8d @ 6" O.C. AT SUPPORTED EDGES AND OVER SHEAR WALLS AND 8d AT 12" O.C. AT INTERMEDIATE SUPPORTS, U.N.O.

EDGE BLOCKING OF UNSUPPORTED EDGES OF SHEATHING AS NOTED ON PLANS.

TYPICAL NAILING SHALL BE 10d @ 6" O.C. ALL SUPPORTED EDGES AND OVER SHEAR WALLS, AND 10d @ 12" O.C. ALL INTERMEDIATE SUPPORTS U.N.O. USE RING SHANK NAILS.

ALL FLOOR SHEATHING SHALL BE GLUED TO JOISTS. THE FIELD-GLUED FLOOR SYSTEM SHALL BE INSTALLED ACCORDING TO THE RECOMMENDATION OF THE AMERICAN PLYWOOD ASSOCIATION. GLUE SHALL BE APPLIED TO THE JOISTS AND TO THE GROOVE IN THE EDGE OF THE T & G PANELS. GLUE SHALL MEET THE REQUIREMENTS OF THE AMERICAN PLYWOOD ASSOCIATION ADHESIVE SPEC. AFG-D1 AND SHALL BE APPLIED AS DIRECTED BY THE GLUE MANUFACTURER. GLUE MAY BE APPLIED MANUALLY OR WITH PNEUMATIC OF ELECTRIC EQUIPMENT

TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE FOLLOWING STANDARDS: NATIONAL DESIGN SPECS. AMERICAN INSTITUTE OF TIMBER CONSTRUCTION RULE NO. 16. DESIGN SPECS FOR LT. MTL. PLATE CONNECTED WOOD TRUSSES BY THE TRUSS PLATE INSTITUTE.

CAMBER TRUSSES FOR 1 1/2 TIMES THE DEAD LOAD DEFLECTION. TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MIN. SUPERIMPOSED LOADING: ROOF

TRUSSES, ASSUMED FOR TOTAL LOADING OF 20 PSF NON-CONCURRNET WITH THE TOP CHORD DL = 13 psf

MAXIMUM DEFLECTION OF TRUSSES SHALL BE (DEAD + LIVE).

SL = SEE SNOW LOADING Cd = 1.0

BTM. CHORD DL= 7 psf

LL = 10 psf (NOT TO BE APPLIED AT THE SAME TIME AS TOP CHORD LL)

ROOF - L/240 WEB MEMBERS MAY NOT BE SHOWN ON DRAWINGS. MANUFACTURER TO DETERMINE WEB MEMBER CONFIGURATION AND TRUSS CONNECTION. NO TRUSS MAY BE ALTERED OR MODIFIED IN THE FIELD WITHOUT THE WRITTEN APPROVAL FROM THE ENGINEER. A COMPLETE SET OF SHOP DRAWINGS, ICC REPORTS AND CALCULATIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO THE FABRICATION OF ANY

MEMBERS. SUCH CALCULATIONS SHALL BE PREPARED UNDER THE DIRECTION OF A

LICENSED ENGINEER IN THE STATE IN WHICH THE BUILDING IS TO BE BUILT.

FRAMING LUMBER SHALL BE KILN DRIED AND SHALL MEET THE FOLLOWING MINIMUM

STANDARD U.N.O. STANDARD OR BETTER. SILL PLATES 2 x 4 2 x 6, 2 x 8 NO. 2 OR BETTER.

ALL SILL PLATES IN CONTACT WITH CONCRETE OR MASONRY, SHALL BE PRESSURE TREATED OR CALIFORNIA REDWOOD.

HORIZONTAL FRAMING LUMBER: (UNO)

4x4 AND SMALLER 2x ROOF JOISTS & RAFTERS D.F. NO. 2 2x FLOOR JOISTS D.F. NO. 2 3x LEDGERS D.F. NO. 1 4x HEADERS & BEAMS D.F. NO. 1 6x6 & LARGER BEAMS D.F. NO. 1

VERTICAL FRAMING LUMBER: (U.N.O.) ALL STUDS

PERPENDICULAR PARTITIONS AT 32" O.C.

ALL POSTS ALL OTHER LUMBER U.N.O

> LOCATIONS. AT EXTERIOR LOCATIONS, DECKS EXPOSED CORRIDORS, USE APA RATED SHEATHING EXTERIOR. WHERE CONSTRUCTION DELAYS ARE EXPECTED PRIOR TO PROVIDING PROTECTION USE APA RATED SHEATHING EXPOSURE 1 COMMONLY KNOWN AS "CDX" PROVIDE A MINIMUM OF (2) STUDS UNDER ALL BEAM BEARING LOCATIONS UNO. PROVIDE A MINIMUM OF (3) STUDS UNDER ALL GIRDER TRUSS BEARING LOCATIONS UNO. WHERE POSTS OR MULTIPLE STUDS UNDER BEAMS OR HEADERS ARE IDENTIFIED ON DRAWINGS,

THOSE POSTS OR MULTIPLE STUDS SHALL BE CARRIED TO THE FOUNDATION. BLOCK

JOISTS AT ALL SUPPORTS. DOUBLE JOISTS UNDER PARALLEL PARTITIONS. BLOCK UNDER

D.F.

D.F.

FINGER-JOINTED LUMBER MAY BE USED EXCEPT AT SHEARWALL HOLDOWNS

NO. 1

STUD GRADE OR #2 (SEE PLAN)

STANDARD OR BETTER.

JOISTS HANGERS AND OTHER METAL FRAMING ACCESSORIES ARE REFERRED TO ON PLANS BY PARTICULAR TYPE AS MANUFACTURED BY SIMPSON COMPANY, SAN LEANDRO CALIFORNIA. ACCESSORIES OF OTHER MANUFACTURER WITH EQUIVALENT LOAD CARRYING CHARACTERISTICS MAY BE USED, WHEN APPROVED BY ENGINEER.

BOLTS: HOLES IN WOOD 1/16" OVERSIZE MAX. USE WASHERS AGAINST WOOD. RE-TIGHTEN ALL BOLTS BEFORE CLOSING IN. PRE-DRILL HOLES FOR LAG BOLTS AND TURN BOLTS INTO HOLES, DO NOT DRIVE-IN. FIRE STOPPING, BACKING FOR INTERIOR FINISHES, NON-BEARING WALLS AND OTHER NON-STRUCTURAL FRAMING IS NOT NECESSARILY SHOWN ON THE STRUCTURAL DRAWINGS. SEE FASTENING SCHEDULE (U.N.O.) PER IBC CHAPTER 23

FASTENERS IN PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOOD: FASTENERS SHALL BE OF HOT DIPPED ZINC-COATED GALVANIZED STEEL. STAINLESS STEEL, SILICON BRONZE OR COPPER. THE COATING WEIGHTS FOR ZINC-COATED STEEL WITH COATING WEIGHTS IN ACCORDANCE WITH ASTM A 153 PER IBC CHAPTER 23



KEYNOTES



REVISIONS DESCRIPTION DATE

0 DATE: 20003.010 04/20/2022 **BASEMENT LEVEL:** 1,101 S.F.

MAIN LEVEL:

UPPER LEVEL: 451 S.F.

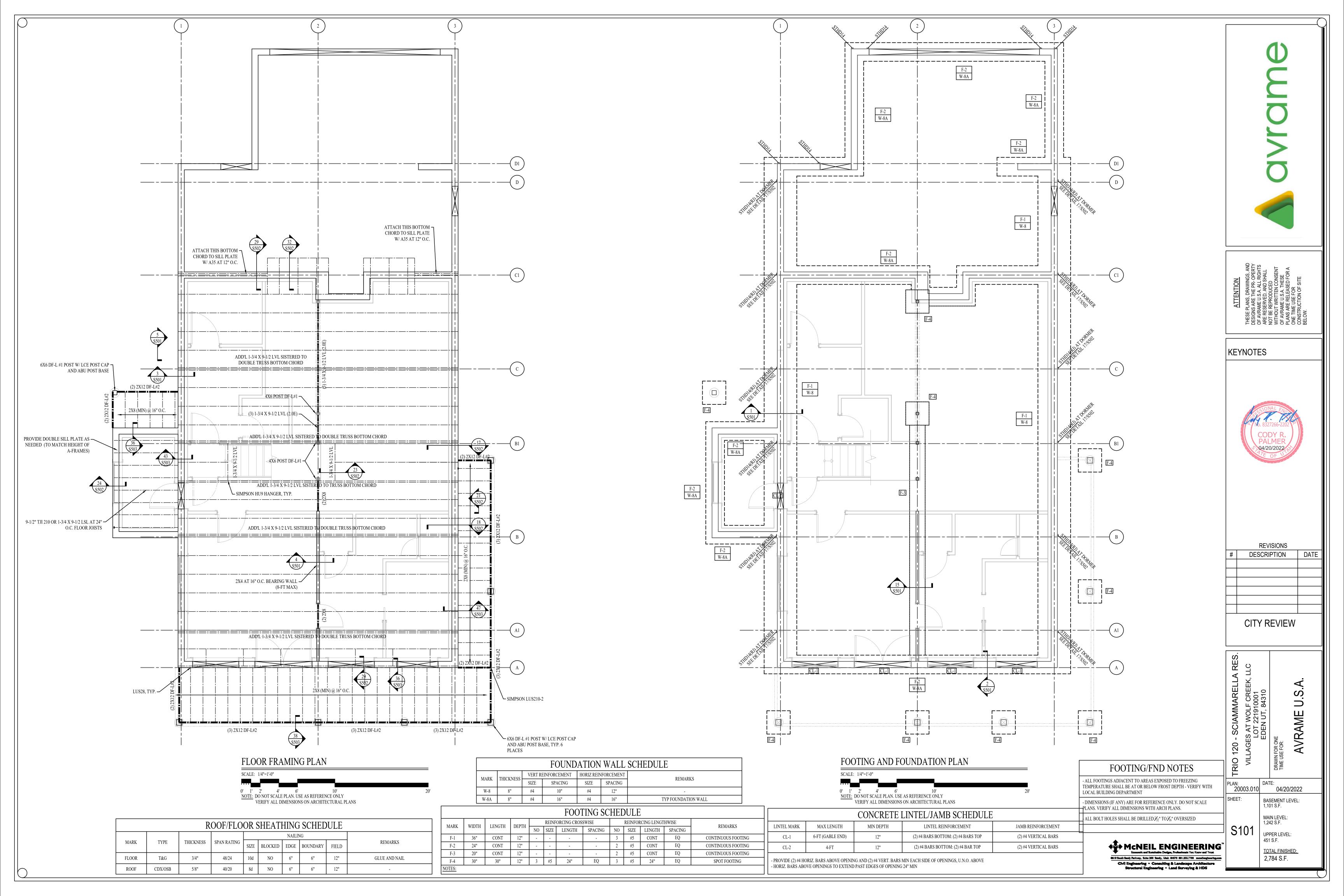
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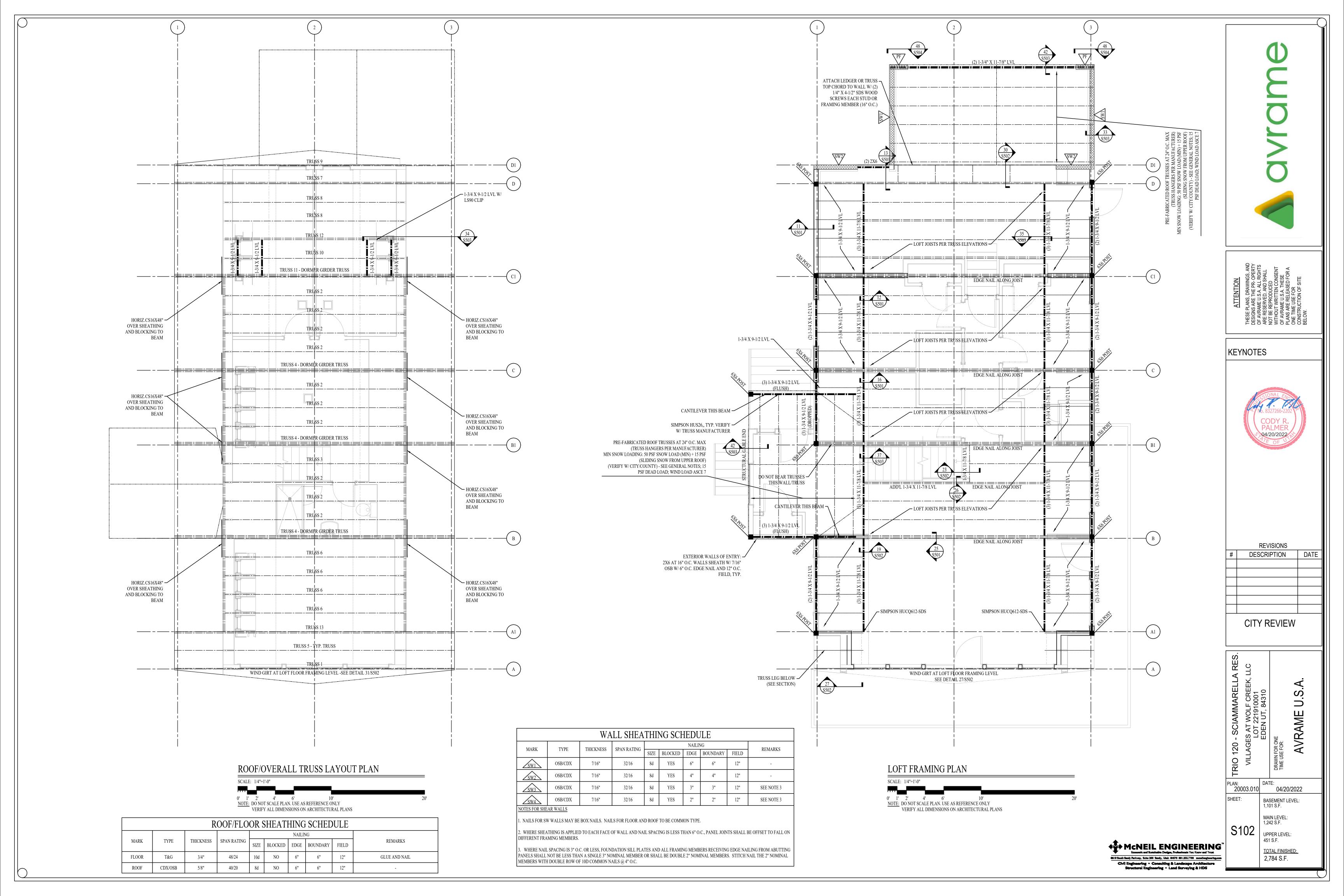
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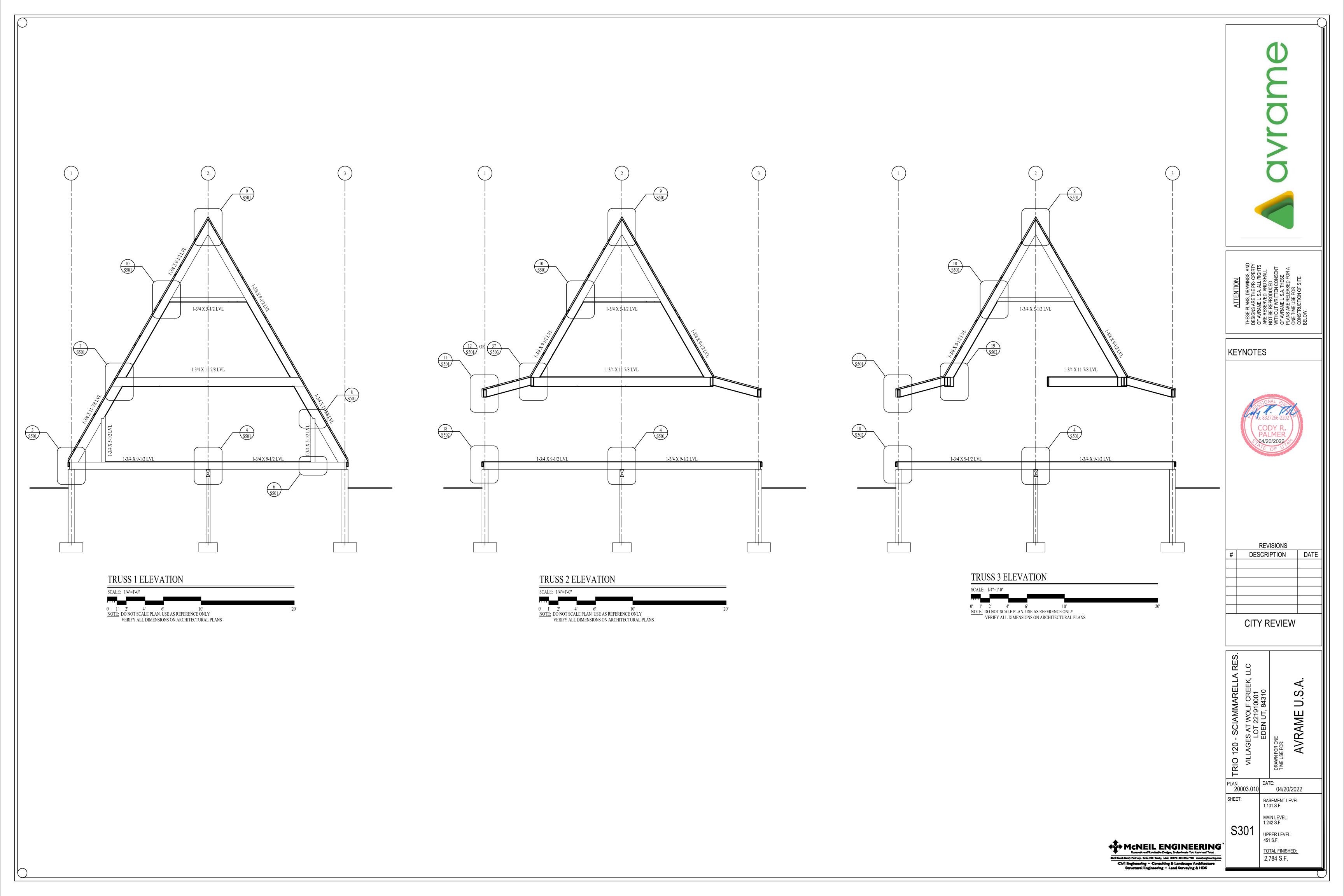
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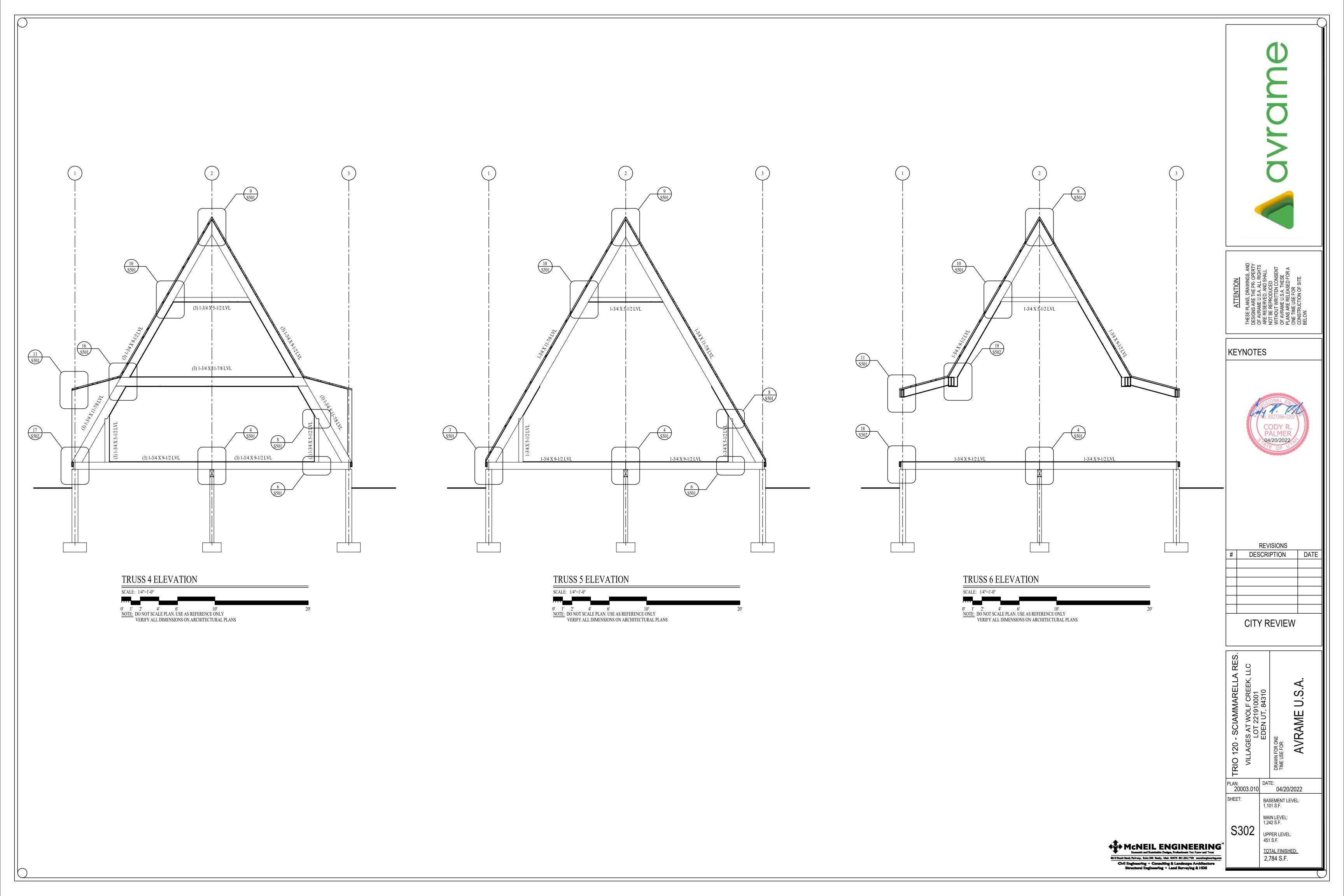
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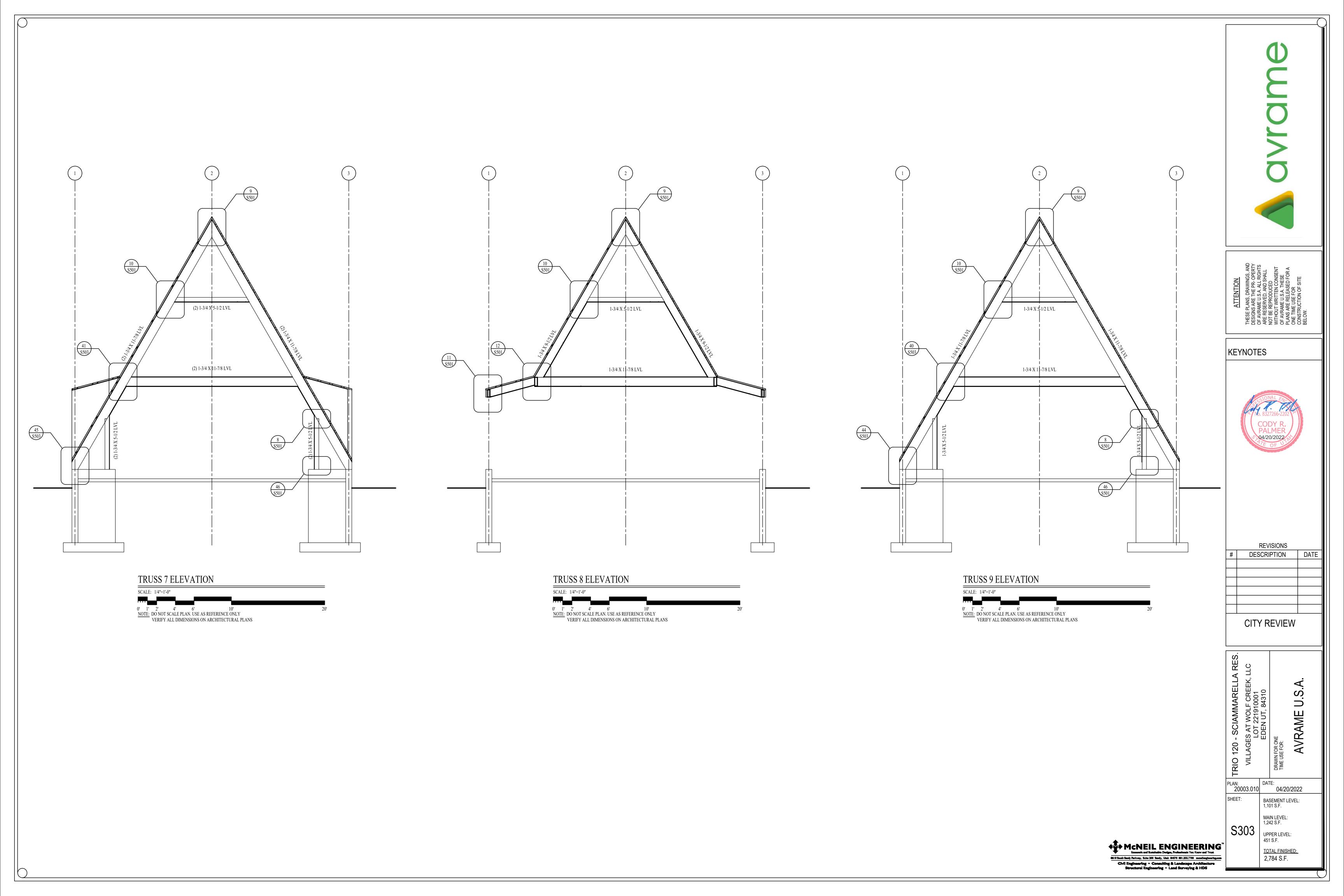
McNEIL ENGINEERING

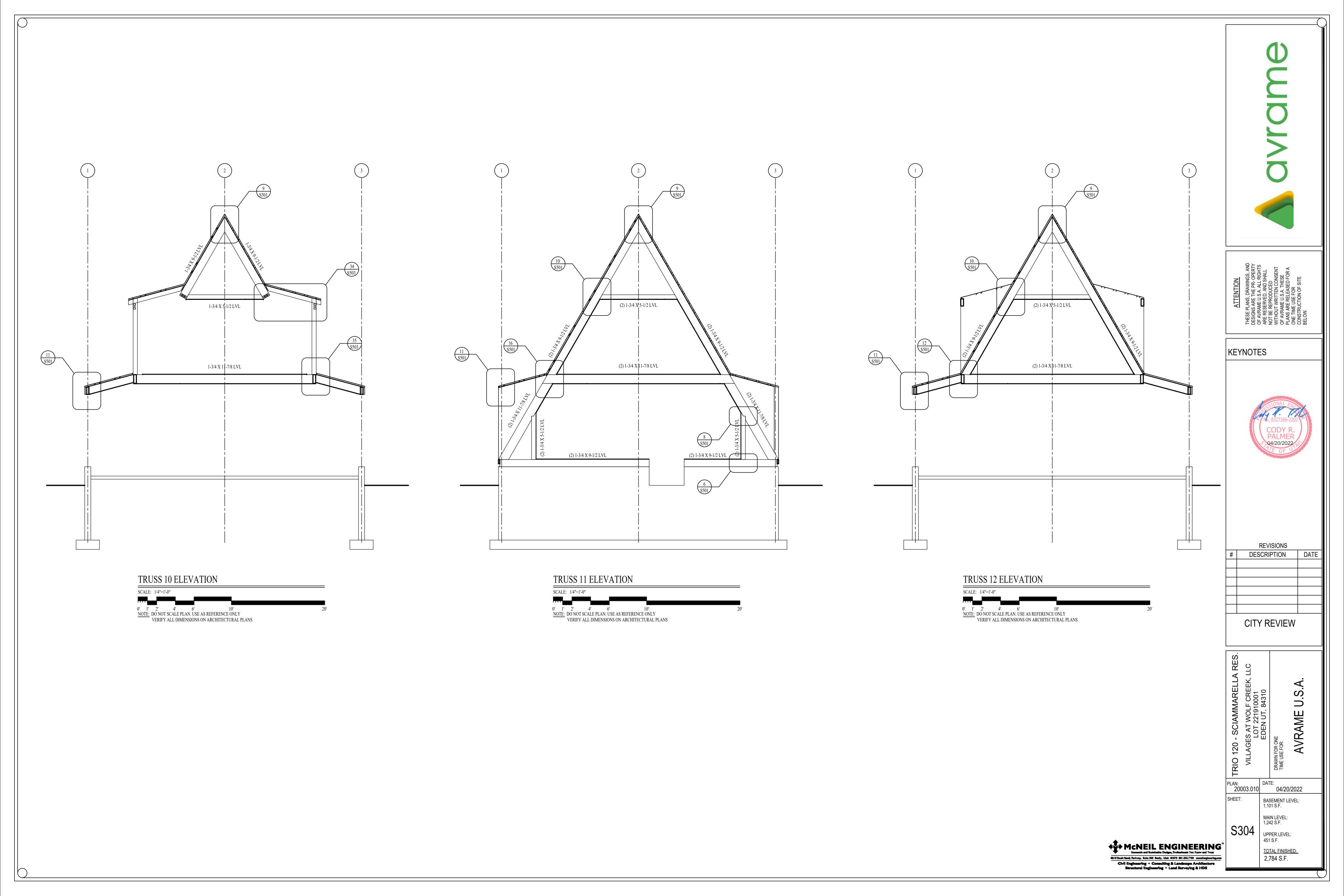


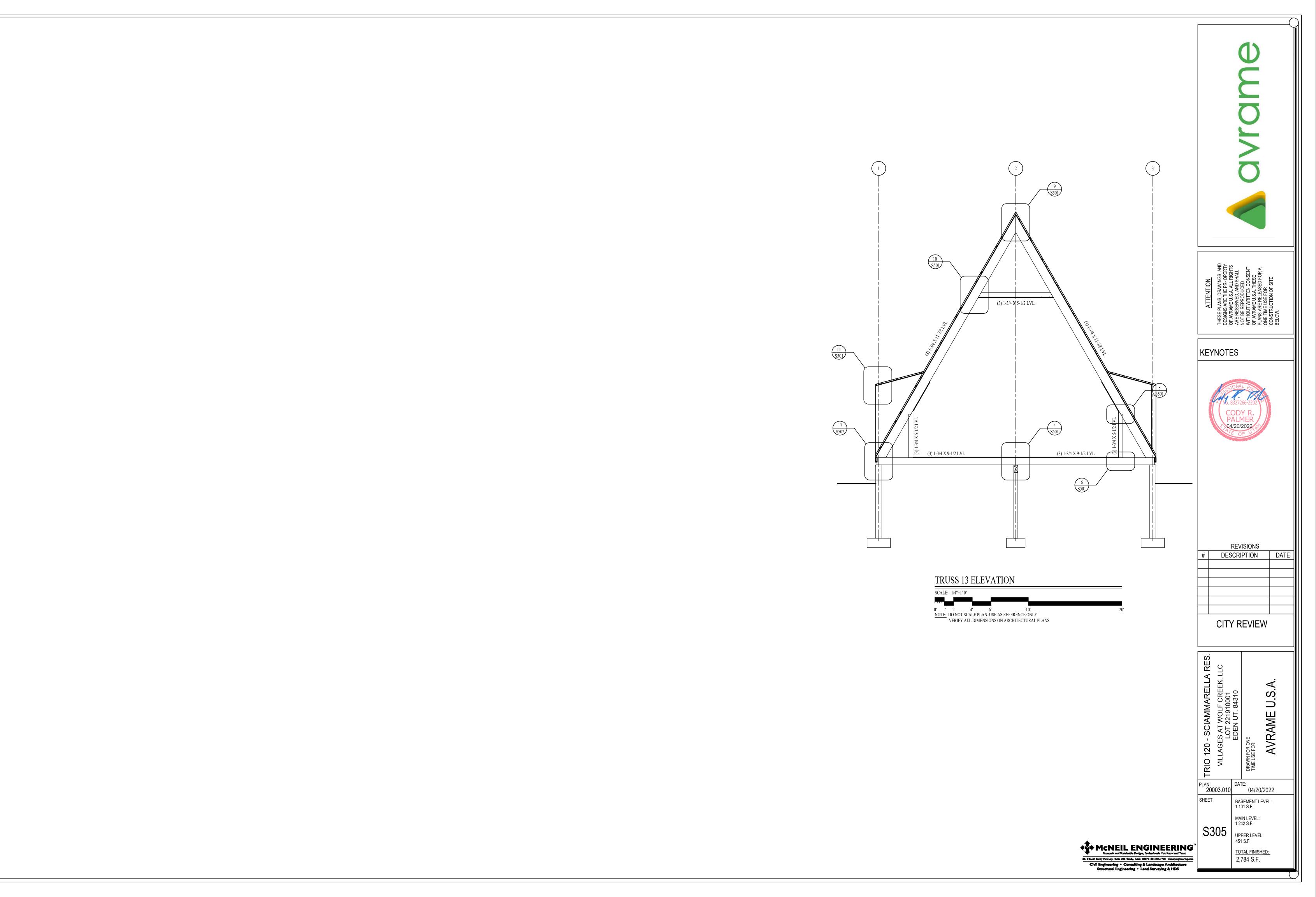


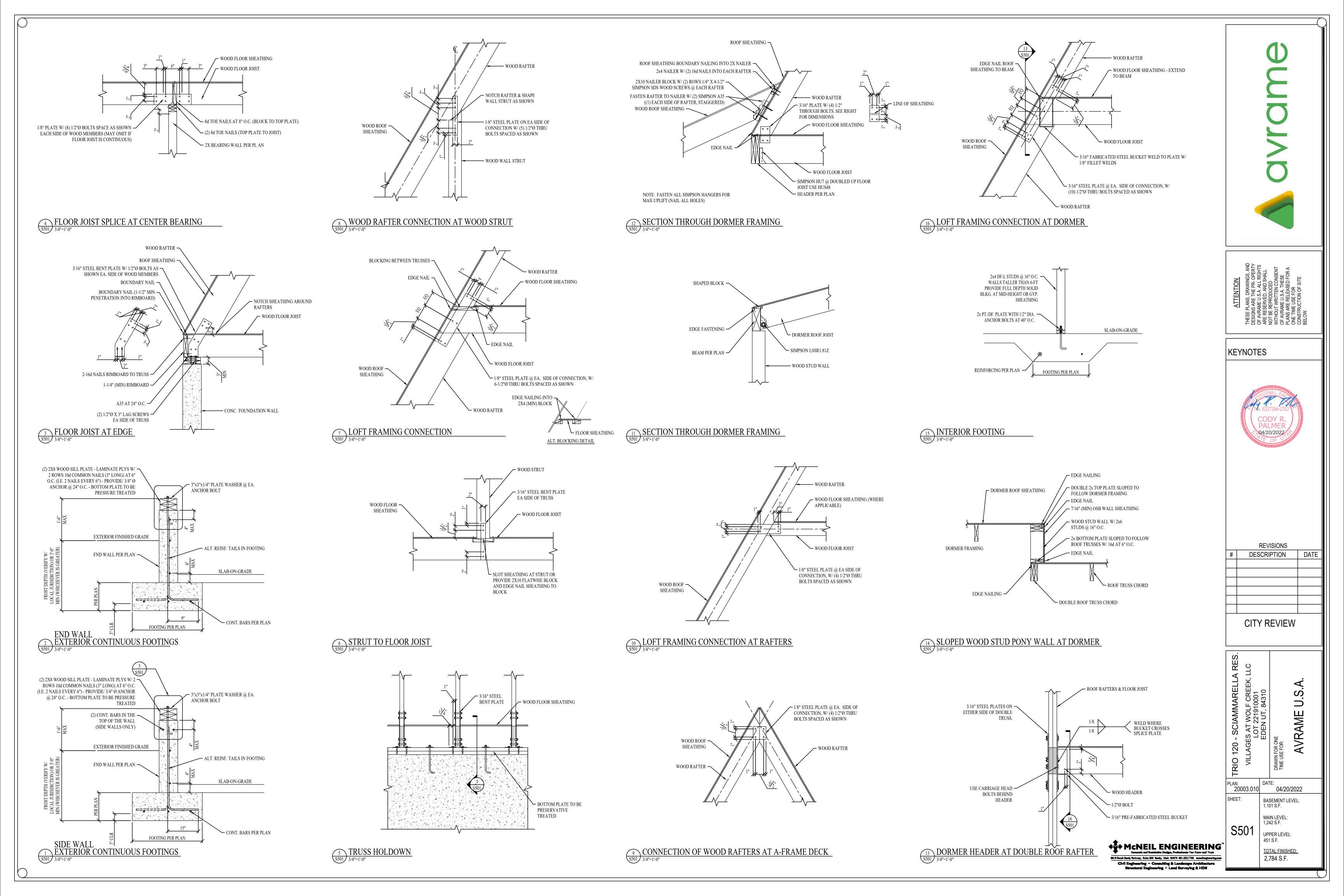


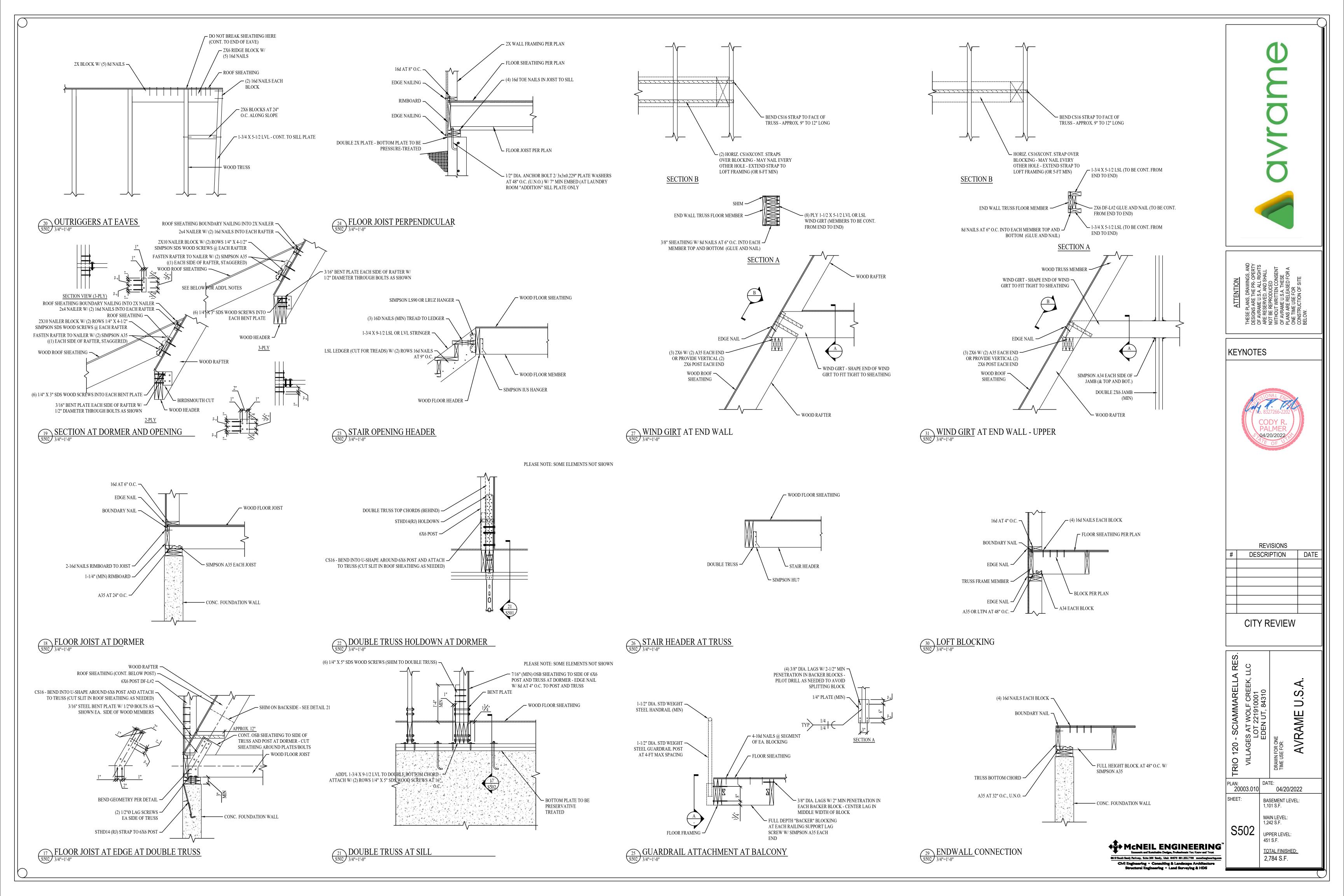


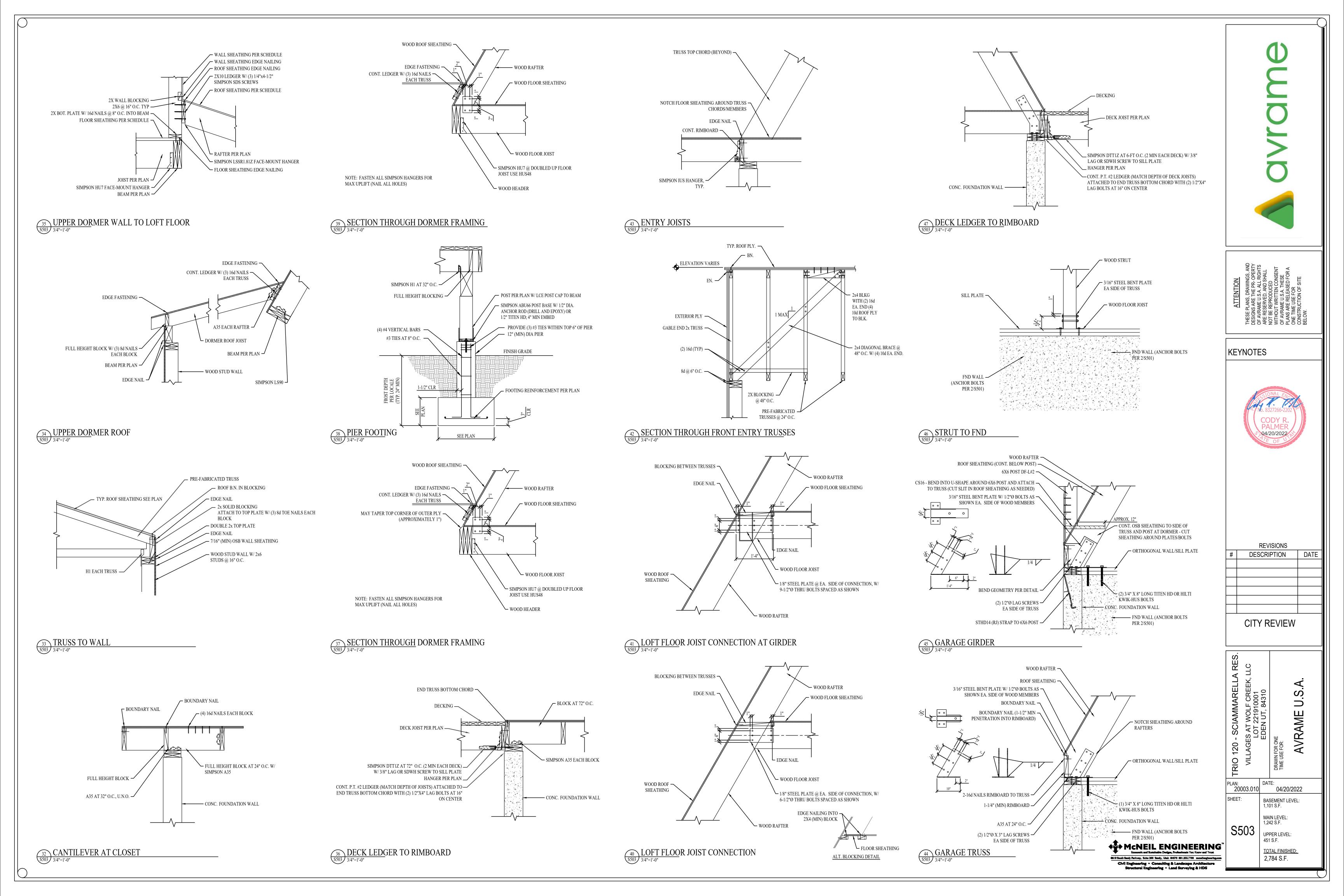


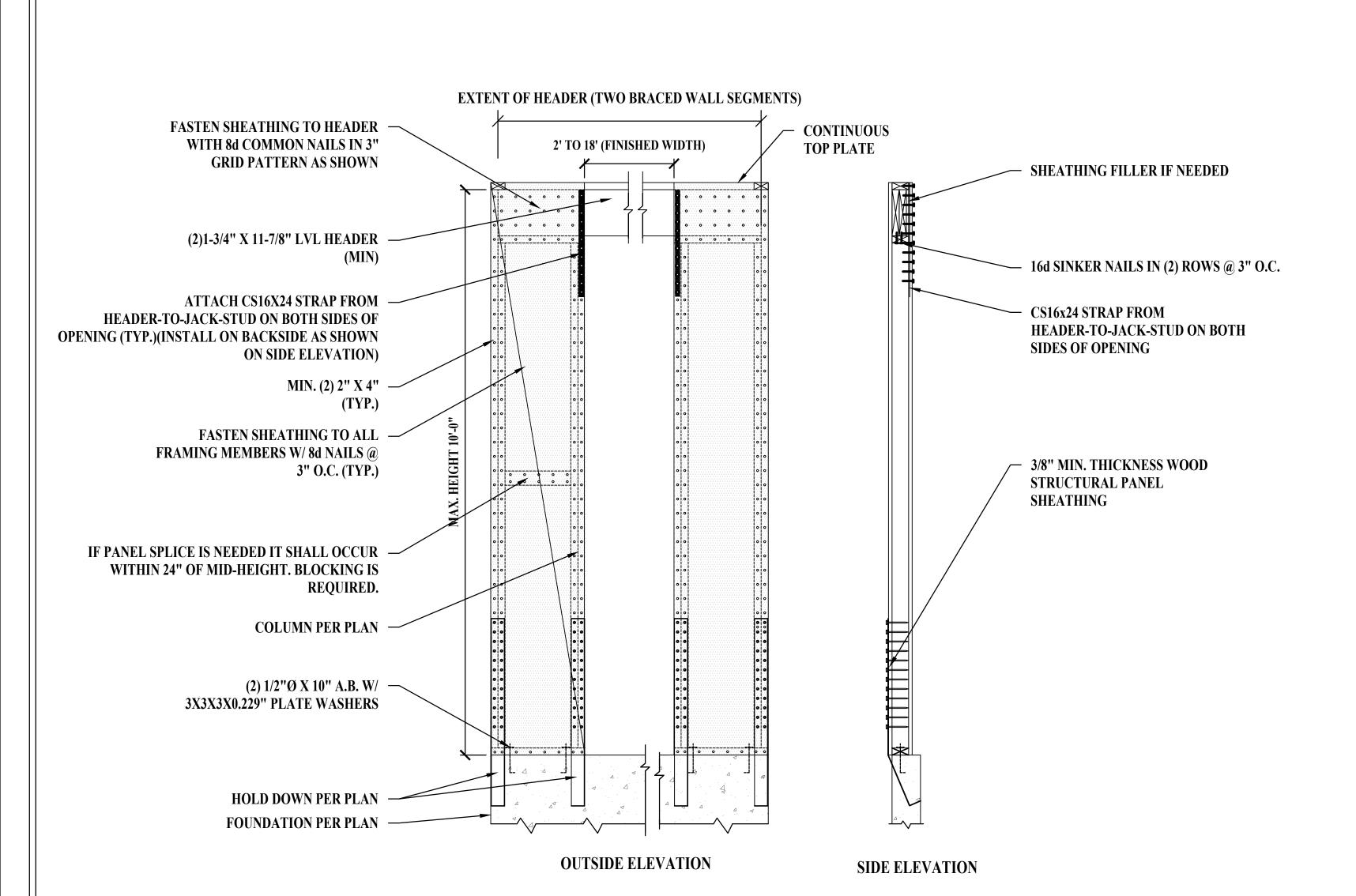












PORTAL FRAME
N.T.S.



KEYNOTES



REVISIONS DESCRIPTION DATE

CITY REVIEW

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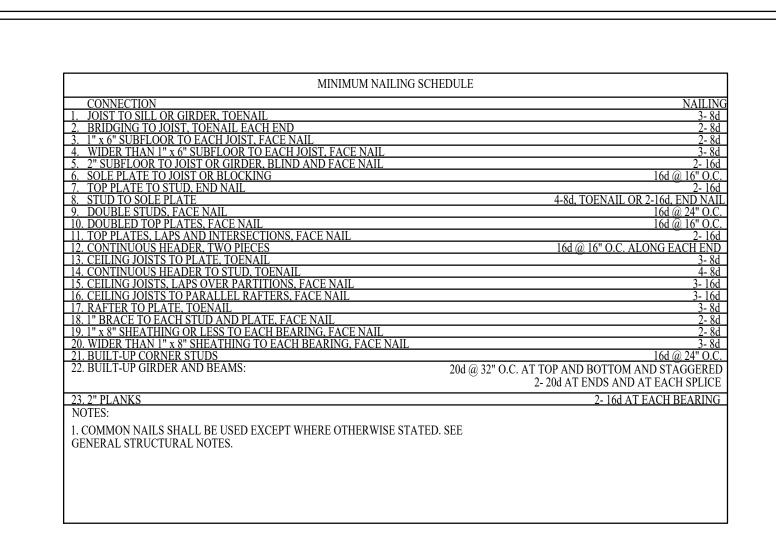
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MAIN LEVEL: 1,242 S.F. S504

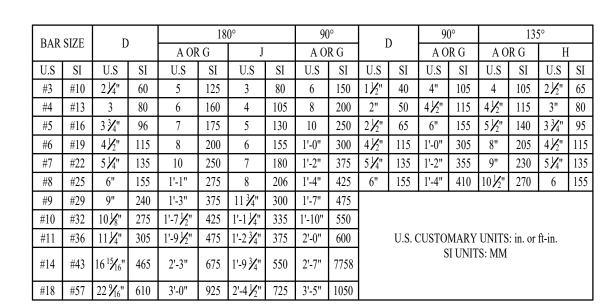
UPPER LEVEL: 451 S.F. TOTAL FINISHED: 2,784 S.F.

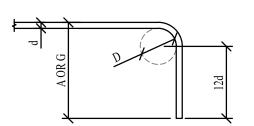
MCNEIL ENGINEERING

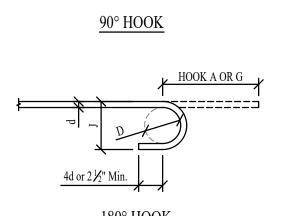
Economic and Sustainable Designs, Professionals You Know and Trust



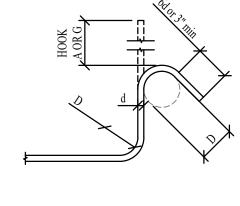
1170	f'c = 2500 PSI				
SIZE	Ld	Lt	Lsb	Lsbt	
#3	18"	23"	23"	30"	
#4	24"	31"	31"	41"	
#5	30"	39"	39"	51"	
#6	36"	47"	47"	61"	
2 мп	Lsbt Ldc: Lsc: db: TOP	E TENSI DEVE TIED O NOMIN BARS:	ION LA LOPME COLUM NAL BA HORIZ S IN SC	P SPLICI ENT LEN IN LAP S IR DIAM ONTAL I HEDULI	E LENGTH FOR OTHER THAN TOP BARS (CLASS B) E LENGTH OF TOP BARS. GTH FOR BARS IN COMPRESSION PLICE IN COMPRESSION ETER (INCHES) BEAM REINFORCEMENT WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW E BY 1.5 IF CLEAR SPACING OR CONCRETE COVER DO NOT MEET REQUIREMENTS FOR Ld IN NOTE 1. E BY 1.3 FOR USE IN LIGHTWEIGHT AGGREGATE CONCRETE. EPLY VALUES IN SCHEDULE BY 1.5 FOR BARS WITH CLEAR COVER < 3db OR CLEAR SPACING < 6db. OTHERWISE

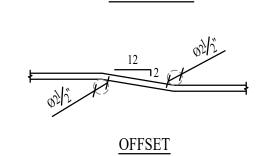


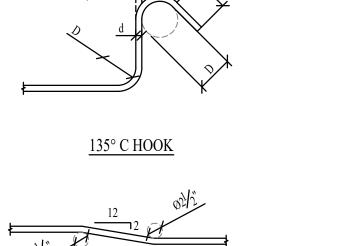


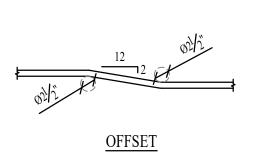


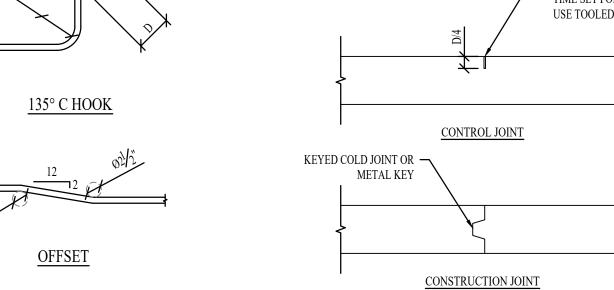
REINF. BENDS, HOOKS, AND OFFSET





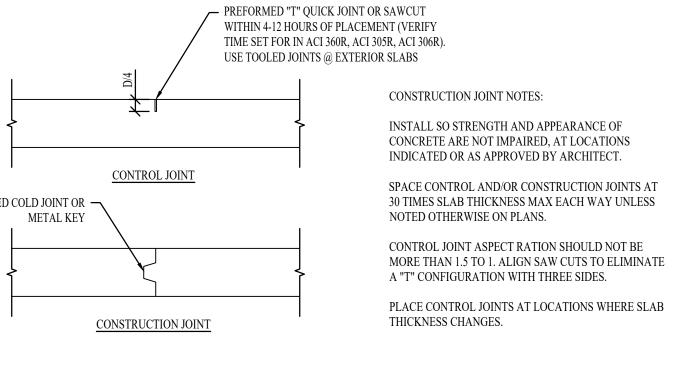


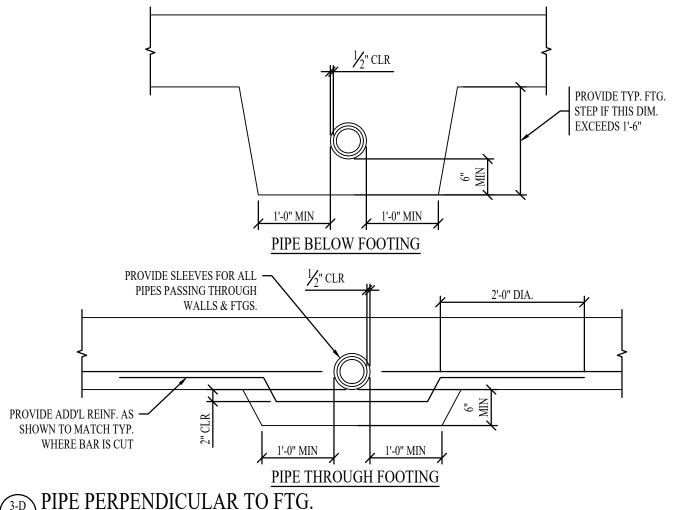




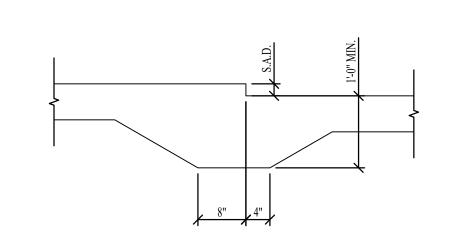
BACKFILLED — TRENCH SPREAD FTG. ➤ BOTTOM OF TRENCH TO BE ABOVE THIS LINE

PIPE PARALLEL TO FTG. 3/4"=1'-0"

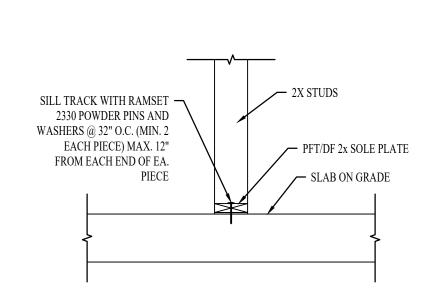




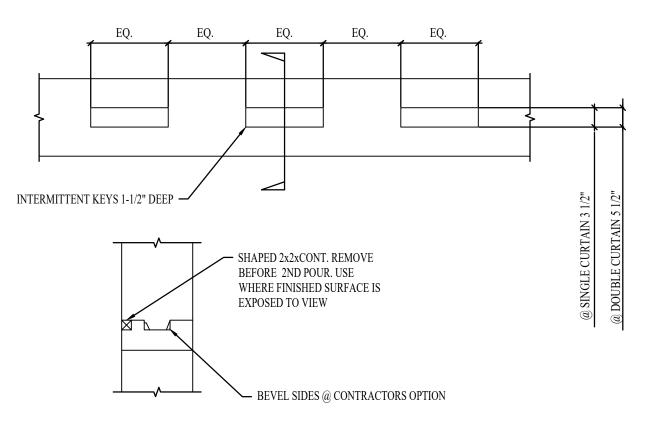




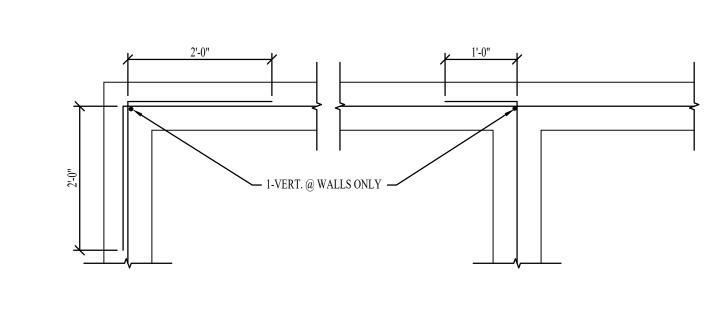
3-C DEPRESSED SLAB ON GRADE 3/4"=1'-0"



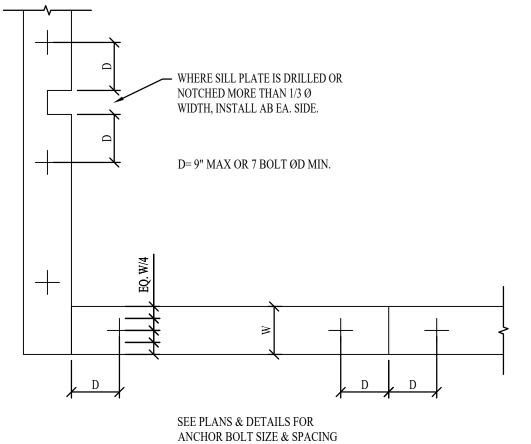
SLAB ON GRADE NON-BEARING WALL



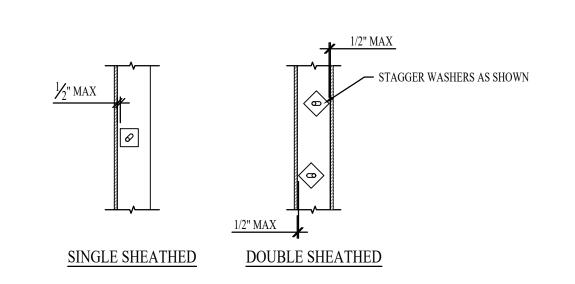




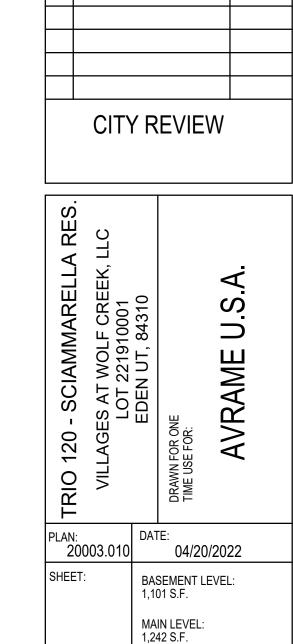
REINF. @ INTERSECTIONS
3/4"=1'-0"



SILL PLATE BOLTING
3/4"=1'-0"



ANCHOR BOLT WASHER PLACEMENT
3/4"=1'-0"



UPPER LEVEL: 451 S.F.

TOTAL FINISHED: 2,784 S.F.

KEYNOTES

04/20/2022

REVISIONS

DATE

DESCRIPTION

2-A SLAB ON GRADE JOINT

3-A CONST. JT. IN WALL OR FTG
3/4"=1'-0" APPLIES TO BOTH VERT. & HORIZ. JOINTS

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