

March 23, 2021

Structural Engineering Municipal Services Civil Engineering Land Surveying

Weber County Engineering 2380 Washington Blvd Suite #240 Ogden, Utah 84401

RE: Highland Bluff Estates Subdivision, Stormwater and Detention Pond Design Summary

Highland Bluff Estates, Project No. 9455 2225 East 6225 South, Weber County

Engineering Department,

Per your request, we have designed the stormwater drainage for the Highland Bluff Estates Subdivision to safely convey the design storm events to the proposed detention pond. Below is a summary of the storm drain system for the subdivision.

Summary:

The drainage system for the Highland Bluff Estates Subdivision was modeled using the Rational Method to determine peak runoff and detention volumes. Precipitation data for this site was obtained from the NOAA Atlas 14 Precipitation Data server for the area. The storm drain conveyance system was designed to safely convey the 10-year storm event as required by the County. The piping for the system was designed to maintain a minimum scour velocity of 2 feet per second for all the pipes in the system. The detention calculations were based on the 100-year storm event. All calculations are shown on the attached C-200 existing drainage plan exhibit and C-201 proposed drainage plan.

The site consists of undisturbed ground with natural grassy vegetation and an asphalt drive and pavilion on the site. There are two subdivisions that border the site that have storm drain pipes that go directly through the site. These subdivisions are on the south (2225 East St) and west (6225 South St) of the site. Currently the existing subdivisions have an estimated total flow of 6.152 cubic feet per sec(cfs) that would be released at the north east corner of the site in SD EX3. This is based on a 10-year storm event. See attached exhibit C-200.

In order to alleviate the amount of storm water going into the system it is proposed to install a detention basin on the western edge of the highland bluff site. The detention basin is unable to be installed on the outfall side of the site because it is not accessible. This detention basin was sized using the area of the proposed development with a release rate of 0.10 cfs/acre. The required detention is calculated to be 7,930 cubic feet. The proposed pond would provide 7,912 cubic feet. The detention calculations show the peak release needed for the proposed site. As the flows from catchment EX-1 will flow through the control structure, this flow is modeled as a bypass flow for sizing the orifice. This calculation is needed because the offsite is not detained and is not free flowing through the orifice. An orifice plate and baffle wall would be installed in the proposed storm drain manhole #213 between SD-1 and SD-2. This orifice plate will discharge 1.926 cfs. This is the sum of the allowable basin discharge and the allowable bypass

discharge. The baffle wall would be at the high-water line. The proposed storm drain system and detention basin would net a total discharge of 5.122 cfs at the north east corner of the site in SD EX3, which is less than the current discharge of 6.152 cfs discussed above. The difference between the existing flow and proposed flow would improve the system by alleviating some of the discharge by 1.03 cfs.

The storm drain conveyance system was designed to alleviate some of the discharge to the system while detaining the needed amount for the Highland Bluffs site. The existing storm drain system is free flowing with no existing detention. With the addition of a detention basin, orifice plate and baffle wall, the existing and proposed flows are mitigated as they flow through the system.

If you have any questions concerning the information noted above, feel free to contact us at any time.

Sincerely,

Reviewed by:

Ike Buckley, E.I.T. Engineer in Training

Cam Preston, PE, SE Project Manager



BENCHMARK

WEST QUARTER CORNER OF SECTION 23 TOWNSHIP 5 NORTH RANGE 1 WEST SALT LAKE BASE AND MERIDIAN

ELEV = 4971.186'

Existing Catchment Calculations (10-year storm) Time of Concentration:

Rainfall Intensity I: 2.06 in/hr 0.013 Mannings N Catchment Area (SF) C Flow (CFS) Destination 0.30 3.823 SD-EX1 0.30 1.167 SD-EX2 0.30 1.163 SD EX3 0.30 6.152 SD EX3 82,856 82,586

436,938

Existing Pipe Design (10-year storm)

Mannings N (ADS) 0.011

Mannings N (RCP) 0.013

Tributary Surface Upstream Pipe Flow Total Pipe Diameter Pipe Type Full Flow % of Full-SD-EX1 1 EX 3.823 none 0.000 3.823 0.35% 18 RCP 6.231 61.4% SD-EX2 SD EX3
 2 EX
 1.167
 none
 0.000
 1.167
 0.35%

 1
 1.163
 EX1,2
 4.990
 6.152
 0.35%
 6.231 18.7% 6.231 98.7%





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ESTATES

BLUFF **PHASE HIGHLANDS**

EXISTING DRAINAGE PLAN EXHIBIT

PRINT DATE 3/19/21 DRAWN BY J.MOSS

PROJECT MANAGER C.PRESTON

C-200



BENCHMARK

WEST QUARTER CORNER OF SECTION 23 TOWNSHIP 5 NORTH RANGE 1 WEST SALT LAKE BASE AND MERIDIAN

ELEV = 4971.186'

Detention Calculations (100-year storm) Basin Tributary Area 199,859 SF Runoff coefficient C: 0.30

Release Rate 0.10 cfs/acre Peak Release 0.459 cfs Time (min) i(in/hr) Cumulative Runoff to Basin (c.f.) Required Storage (c.f.) 138 2,906 275 4,350 413 5,323 4,625 5,736 7,730 9,564 6,904 826 1,652 **7,912** 3,303 7,796 11,100

Catchment Calculations (10-year storm) Time of Concentration: Rainfall Intensity I: 2.06 in/hr Mannings N C Flow (CFS) Destination Catchment Area (SF) 3.823 SD-EX1 2 EX 1.167 SD-EX2 17,200 0.242 Offsite 1.049 SD EX3 0.981 SD EX3 69,641

19,532

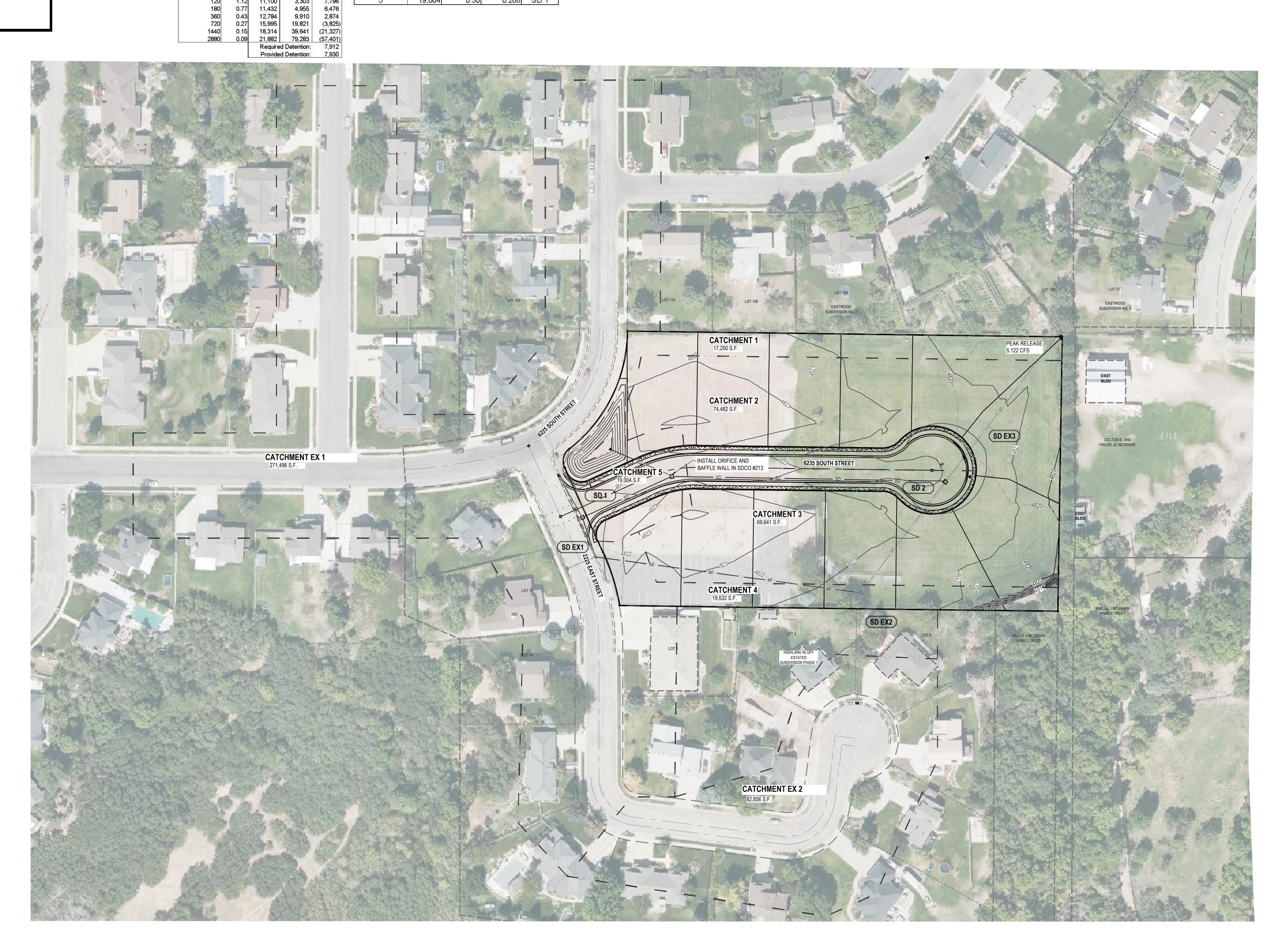
19,004

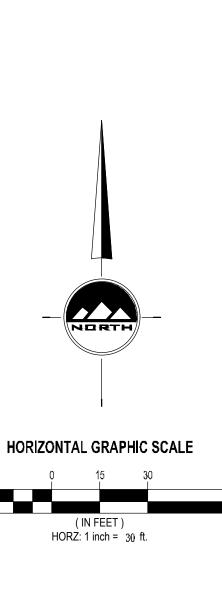
0.275 Offsite

0.268 SD 1

Pipe Design (10-year storm) Mannings N (ADS) 0.011 Mannings N (RCP) 0.013 Tributary Surface Upstream Pipe Flow Total Pipe Diameter Pipe Type Full Flow % of Full-SD-EX1 0.000 3.823 0.35% 6.231 61.4% 3.823 none 18 RCP 1 EX SD-EX2 0.000 1.167 0.35% 6.231 18.7% 2 EX 1.167 18 RCP 6.231 65.6% SD 1 5 0.268 EX1 3.823 4.090 0.35% 18 RCP SD 2 None 0.000 Orifice 1.926 1.926 0.15% 18 RCP 4.079 47.2% SD EX3 2, 3 2.029 SD2, EX2 3.093 5.122 0.35% 18 RCP 6.231 82.2%

Orifice Calculations: Q=C_dA₀(2gh)^{1//} High Water Elevation: 4,821.45 FT Box Invert 4,816.30 FT Allowed Basin Discharge* 0.459 CFS Allowable Bypass 1.468 CFS Total Discharge 1.926 CFS 0.62 Orifice Area 24.57 IN² Orifice Diameter 5.6 IN







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ESTATES BLUFF **HIGHLANDS**

PHA

OVERALL DRAINAGE

DRAWN BY J.MOSS

PROJECT MANAGER C.PRESTON

C-201