

March 12, 2020

Mr. Luke Berman Big Canyon Homes 791 North 100 East Lehi, Utah 84003

RE: Geotechnical Recommendations

Huntsville Lot 407 South 8600 East Huntsville, Utah CMT Job No. 9672

Mr. Berman,

As requested and authorized, this letter presents geotechnical recommendations for the subject site located at about 407 South 8600 East in Huntsville, Utah. Based on our field exploration and experience in the site vicinity, it is our opinion the site is suitable for the proposed construction.

### Field Exploration

The subsurface soil conditions were explored by excavating two test pits on the site at the approximate locations shown on **Site Map**, **Figure 1** (attached). The test pits extended to depths of approximately 10 to 11 feet below the existing ground surface. The subsurface soils encountered in the test pits were logged and described in general accordance with ASTM D 2488. The subsurface conditions encountered in the field exploration are discussed below. Logs of the test pits, including a description of the soil strata encountered, is presented on each individual **Test Pit Log**, **Figures 2 and 3**. A **Key to Symbols** sheet defining the terms and symbols used on the logs is provided as **Figure 4**, attached.

#### **Subsurface Conditions**

At the test pit locations we encountered approximately 6 inches of topsoil containing roots and organic material at the surface. Natural soils were encountered below the topsoil consisting of moist, stiff (estimated), brown Silty CLAY with sand (CL) overlying brown Sandy SILT (ML) and Sandy GRAVEL with silt (GP-GM), extending to the maximum depths explored of 10 to 11 feet below the existing ground surface. Groundwater was not encountered in the test pits and is not anticipated to affect construction. For a detailed description of the soil profile encountered in our exploration, see the Test Pit Logs (Figures 2 and 3), attached.

### **Earthwork Recommendations**

All deleterious materials should be stripped from the site prior to commencement of construction activities. This includes loose and disturbed soils, topsoil/organic soils, vegetation, etc. Based upon the conditions observed in the test pits there is topsoil/organic soils on the surface of the site extending to a depth of about 6 inches below the existing ground surface. We estimate that topsoil stripping will need to include the upper 6 inches except in local areas where roots/organics may extend deeper.

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The exploratory test pits dug as part of our explorations will likely contain loose and disturbed soils and possibly vegetation. If these conditions are encountered in excavations, the loose and disturbed soils should be removed and replaced with structural fill.

For temporary excavations less than 5 feet deep, either in the native soils or structural fill, slopes should not be steeper that 0.5:1 (horizontal to vertical). Deeper excavations are not anticipated. If loose sandy soils and/or groundwater are encountered, flatter slopes, shoring, bracing, and/or dewatering may be required for all conditions. All excavations should be made following OSHA safety guidelines.

Imported structural fill should consist of well-graded sandy gravel material with a maximum 20% passing the #200 sieve, a minimum 70% passing the ¾-inch sieve, a maximum particle size of 4 inches, and a maximum plasticity index of 10. The on-site sandy gravel soils appear suitable for use as structural fill if larger particles are removed/screened.

The various types of compaction equipment available have their limitations as to the maximum lift thickness that can be compacted. For example, hand operated equipment is limited to lifts of about 4 inches and most "trench compactors" have a maximum, consistent compaction depth of about 6 inches. Large rollers, depending on soil and moisture conditions can achieve compaction at 8 to 12 inches. The full thickness of each lift should be compacted to at least the following percentages of the maximum dry density as determined by ASTM D-1557:

LOCATION	TOTAL FILL THICKNESS (FEET)	MINIMUM PERCENTAGE OF MAXIMUM DRY DENSITY
Beneath an area extending at least 4 feet beyond the perimeter of structures, and below flatwork and pavement (applies to structural fill and site grading fill) extending at least 2 feet beyond the perimeter	0 to 5	95
Site grading fill outside area defined above	0 to 5	92
Utility trenches within structural areas		96
Roadbase and subbase	-	96
Non-structural fill	0 to 5	90

Structural fills greater than 5 feet thick are not anticipated at the site. For best compaction results, we recommend that the moisture content for structural fill/backfill be within 2% of optimum. Field density tests should be performed on each lift as necessary to verify that proper compaction is being achieved.

### **Foundation Recommendations**

We recommend that footings be constructed on suitable undisturbed natural soils or on structural/ engineered fill which extends to natural soils. Footings may then be designed using a net bearing pressure of 2,000 psf. The term "net bearing pressure" refers to the pressure imposed by the portion of the structure located above lowest adjacent final grade, thus the weight of the footing and backfill to lowest adjacent final grade need not

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be considered. The allowable bearing pressure may be increased by 1/3 for temporary loads such as wind and seismic forces.

We also recommend the following:

- 1. Exterior footings subject to frost should be placed at least 30 inches below final grade.
- 2. Interior footings not subject to frost should be placed at least 16 inches below grade.
- 3. Continuous footing widths should be maintained at a minimum of 18 inches.
- 4. Spot footings should be a minimum of 24 inches wide.

Foundations designed and constructed in accordance with our recommendations could experience some settlement, but we anticipate that settlement of footings founded as recommended above will be 1 inch or less. We expect approximately 50 percent of this settlement to take place during construction.

#### **Seismic Recommendations**

There are no faults that are mapped crossing or projecting toward the subject site. The nearest mapped fault trace is the Weber segment of the Wasatch fault, located approximately 9.7 miles west of the site.

The 2014 USGS mapping utilized by the IBC provides values of peak ground, short period and long period accelerations for the Site Class B/C boundary and the Maximum Considered Earthquake (MCE). This Site Class B boundary represents average bedrock values for the Western United States and must be corrected for local soil conditions. The Seismic Design Categories in the International Residential Code (IRC 2018 Table R301.2.2.1.1) are based upon the Site Class as addressed in the previous section. For Site Class D at site grid coordinates of 41.2573 degrees north latitude and -111.7434 degrees west longitude, **S**<sub>DS</sub> is 0.617 and the **Seismic Design Category** is D<sub>0</sub>.

### **Drainage Recommendations**

The International Residential Code recommends that drains be provided around "foundations that retain earth and enclose habitable or usable space below grade." An exception is allowed if the foundation is installed on "well drained" ground consisting of Group 1 soils. These soils include those defined by the Unified Soil Classification System as GW, GP, GM, SW, SP and SM. The natural soils below depths of about 2 to 6 feet consist of Group 1 soils (GP-GM). Thus, it is our opinion that foundation drains are not needed.

### **Limitations/Closure**

The recommendations provided herein were developed by evaluating the information obtained from the test pit and site exploration. Soil and groundwater conditions may differ from conditions encountered at the actual exploration locations. The nature and extent of any variation in the explorations may not become evident until during the course of construction. If variations do appear, it may become necessary to re-evaluate the recommendations of this report after we have observed the variation.

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Our professional services have been performed, our findings obtained, and our recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all other warranties, either expressed or implied.

If you have any questions, please call.

Sincerely,

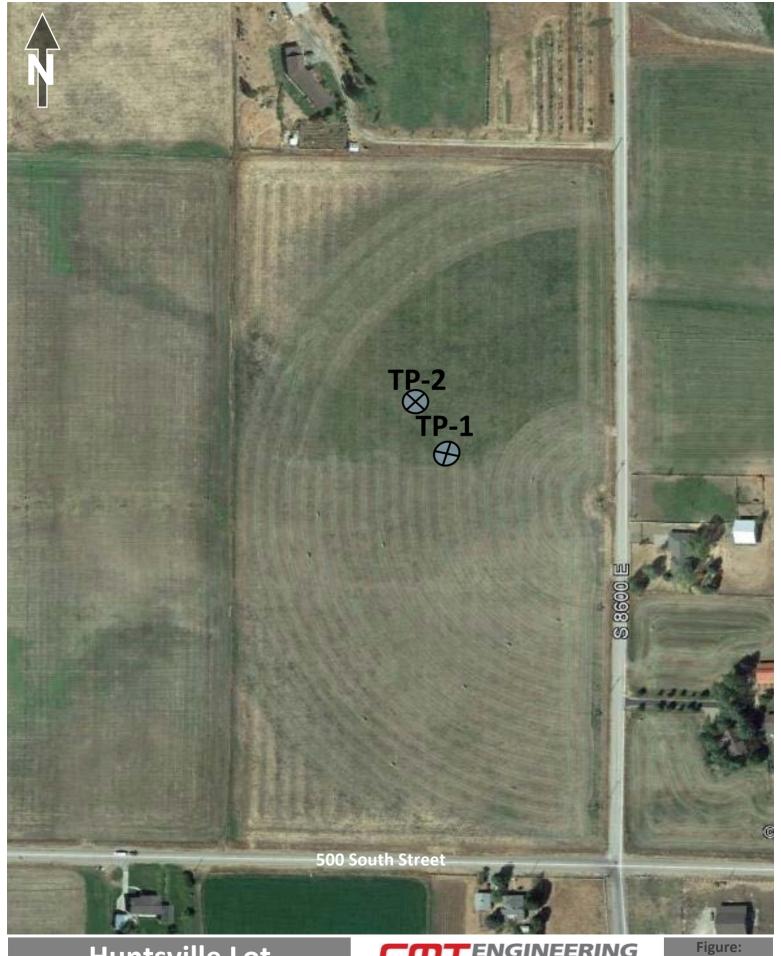
**CMT** Engineering Laboratories

William G. Turner, P.E.

Senior Geotechnical Engineer

Encl: Figure 1, Site Map

Figures 2-3, Test Pit Log Figure 4, Key to Symbols



407 South 8600 East, Huntsville, Utah

Date: Site Map Job#

6-Mar-20 12366

**Test Pit Log** 

TP-1

407 South 8600 East, Huntsville, Utah

Equipment: Trackhoe Surface Elev. (approx):

Total Depth: 11'
Water Depth: (see Remarks)

Date: 2/18/20 Job #: 12366

t)	o a		ype		(%	(pcf)		Gradatio		on Atte		erg
Depth (ft)	GRAPHIC LOG	Soil Description	Sample Type	Sample #	Moisture (%)	Dry Density(pcf)	Gravel %	Sand %	Fines %	П	PL	Ы
0		TOPSOIL										
1 -		Brown Silty Clay with sand (CL) moist, stiff (estimated)										
2 -		Brown Sandy GRAVEL with silt (GP-GM), some cobble dry to slightly moist, medium dense (estimated)										
3 -	***			1								
	• • •											
4 -												
5 -	***											
6 -												
7 -												
8 -	• • •											
0 -	41.			2								
9 -												
10 -	***											
11 -	47.	END AT 11'										
12 -												
13 -												
14		Groundwater not encountered during excavation									iaur	

Remarks: Groundwater not encountered during excavation.

Figure:

2

**Test Pit Log** 

407 South 8600 East, Huntsville, Utah

Equipment: Trackhoe Surface Elev. (approx):

Total Depth: Water Depth: (see Remarks)

Job #: 12366

f)	ე ი	(2)	ype		(%)	(pcf)		adation		Att	erg	
Depth (ft)	GRAPHIC	$  \circ  $		Sample #	Moisture (%)	Ory Density(pcf)	Gravel %	Sand %	Fines %	П	PL	₫
0		TOPSOIL										
1 -		Brown Silty Clay with sand (CL) moist, stiff (estimated)										
2 -		Brown Fine Sandy SILT (ML), trace gravel moist, stiff (estimated)										
3 -				3	11				52			
4 -												
5 -												
6 -		Brown Sandy GRAVEL with silt (GP-GM), some cobble dry to slightly moist, medium dense (estimated)										
7 -	• • •			4	3				9			
8 -												
9 -												
10 -		END AT 10'	-									
11 -	_											
12 -	-											
13 -												
14 Pom	arka:	Croundwater not appointered during execution									iaur	<u>.                                    </u>
Reili	Remarks: Groundwater not encountered during excavation. Figure:											

Excavated By: Logged By:

Client Hogan Wright

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### Key to Symbols

407 South 8600 East, Huntsville, Utah

Date: 2/18/20 Job #: 12366

Depth (ft)	GRAPHIC LOG	Soil Description	Sample Type	Sample #	Moisture (%)	Dry Density(pcf)	Gravel %	ada <sup>®</sup> % yang %	Fines % OI	Att	terberg 9	erg
1	2	3	4	(5)	6	7	l.					
		COLUMN DESCRIPTIONS										

- **Depth (ft.):** Depth (feet) below the ground surface (including groundwater depth - see water symbol below).
- **Graphic Log:** Graphic depicting type of soil encountered (see 2 below).
- Soil Description: Description of soils encountered, including Unified Soil Classification Symbol (see below).
- Sample Type: Type of soil sample collected at depth interval **(4)** shown; sampler symbols are explained below-right.
- Sample #: Consecutive numbering of soil samples collected (5) during field exploration.
- Moisture (%): Water content of soil sample measured in laboratory (percentage of dry weight of sample).
- Dry Density (pcf): The dry density of a soil measured in laboratory (pounds per cubic foot).

**Gradation:** Percentages of Gravel, Sand and Fines (Silt/Clay), obtained from lab test results of soil passing the No. 4 and No. 200 sieves.

- (9) Atterberg: Individual descriptions of Atterberg Tests are as follows:
  - LL = Liquid Limit (%): Water content at which a soil changes from plastic to liquid behavior.

PL = Plastic Limit (%): Water content at which a soil changes from liquid to plastic behavior.

PI = Plasticity Index (%): Range of water content at which a soil exhibits plastic properties (= Liquid Limit - Plastic Limit).

STF	MODIFIERS			
Description	Description Thickness			
Seam	Up to ½ inch	<5%		
Lense	Up to 12 inches	Some		
Layer	Greater than 12 in.	5-12%		
Occasional	1 or less per foot	With		
Frequent	> 12%			

MOISTURE CONTENT									
Dry: Absence of moisture,									
dusty, dry to the touch.									

Moist: Damp / moist to the touch, but no visible water

Saturated: Visible water, usually soil below groundwater.

	MA	JOR DIVISI	ONS	USCS SYMBOLS	2	TYPICAL DESCRIPTIONS
cs)		GRAVELS	CLEAN GRAVELS	GW		Well-Graded Gravels, Gravel-Sand Mixtures, Little or No Fines
(USC		The coarse fraction	(< 5% fines)	GP	3	Poorly-Graded Gravels, Gravel-Sand Mixtures, Little or No Fines
STEM	COARSE- GRAINED	retained on No. 4 sieve.	GRAVELS WITH FINES	GM	•	Silty Gravels, Gravel-Sand-Silt Mixtures
STI	SOILS	No. 4 Sieve.	( ≥ 12% fines)	GC		Clayey Gravels, Gravel-Sand-Clay Mixtures
I SY	More than 50% of material is	SANDS	CLEAN SANDS	SW		Well-Graded Sands, Gravelly Sands, Little or No Fines
0	larger than No. 200 sieve size.	The coarse	(< 5% fines)	SP		Poorly-Graded Sands, Gravelly Sands, Little or No Fines
SIFICATION		fraction passing through No. 4 sieve.	SANDS WITH FINES	SM		Silty Sands, Sand-Silt Mixtures
)FI		No. 4 Sieve.	( ≥ 12% fines)	SC		Clayey Sands, Sand-Clay Mixtures
ဟ	FINE- GRAINED SOILS			ML		Inorganic Silts and Sandy Silts with No Plasticity or Clayey Silts with Slight Plasticity
CLA		0.2.07.	ND CLAYS less than 50%	CL		Inorganic Clays of Low to Medium Plasticity, Gravelly Clays, Sandy Clays, Silty Clays, Lean Clays
SOIL				OL		Organic Silts and Organic Silty Clays of Low Plasticity
	More than 50% of material is			МН	$\prod$	Inorganic Silts, Micacious or Diatomacious Fine Sand or Silty Soils
UNIFIEI	smaller than No. 200 sieve size.		ND CLAYS reater than 50%	CH		Inorganic Clays of High Plasticity, Fat Clays
N N				ОН		Organic Silts and Organic Clays of Medium to High Plasticity
	HIGHL	Y ORGANIC	SOILS	PT		Peat, Soils with High Organic Contents

#### **SAMPLER SYMBOLS**

**Block Sample** 



Bulk/Bag Sample



Modified California Sampler 3.5" OD, 2.42" ID



Rock Core



Standard Penetration Split Spoon Sampler Thin Wall



(Shelby Tube)

### **WATER SYMBOL**



**Encountered Water** Level

Measured Water Level

(see Remarks on Logs)

Note: Dual Symbols are used to indicate borderline soil classifications (i.e. GP-GM, SC-SM, etc.). The results of laboratory tests on the samples collected are shown on the logs at the respective sample depths

2. The subsurface conditions represented on the logs are for the locations specified. Caution should be exercised if interpolating between or extrapolating beyond the exploration locations

3. The information presented on each log is subject to the limitations, conclusions, and recommendations presented in this report.



Figure:

