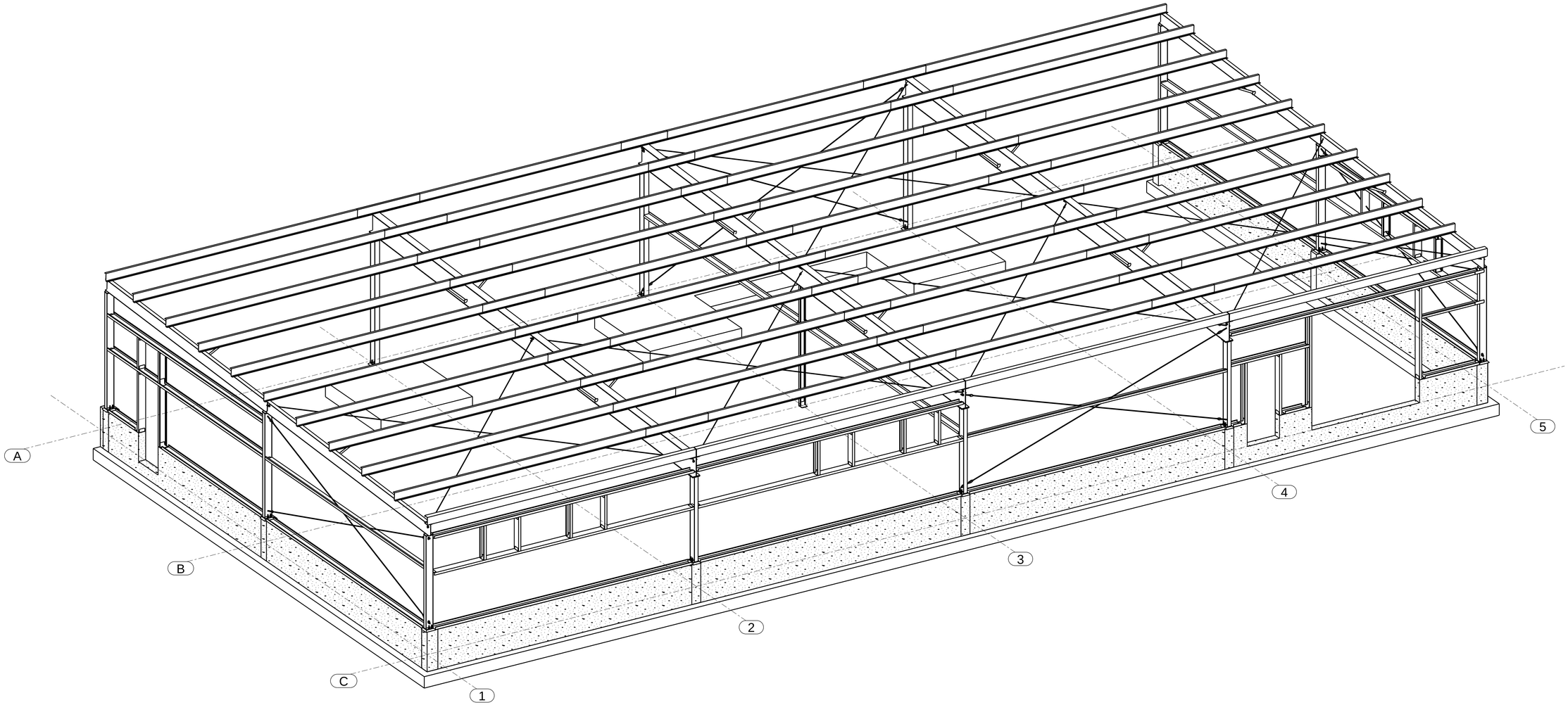
FOREMASTER

50'x96'

2397 RULON WHITE BLVD
OGDEN, UT 84404



STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB

Drawn by: SES

Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER

2397 RULON WHITE BLVD
OGDEN UT, 84404

COVER SHEET

Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

DATE 03/24/2020

PROJ. NO. 20053

DRAWING NO.
SS001

Tekla Structures

GENERAL NOTES:

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. ALL WORK TO BE IN STRICT ACCORDANCE WITH THE IBC 2018, AISC, AND LOCAL ORDINANCES.
3. ALL DIMENSIONS AND CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND ERECTION.
4. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
5. SEE ARCHITECTURAL SHEETS FOR DECK BEARING ELEVATIONS, STRUCTURAL DETAILER SHALL DETERMINE ALL BEARING PLATE ELEVATIONS FROM ARCHITECTURAL DECK ELEVATIONS.
6. SEE ARCHITECTURAL SHEETS FOR ADDITIONAL DIMENSIONS.
7. SEE ARCHITECTURAL SHEETS FOR ACCESS HATCHES, DRAFT STOPS, ETC.
8. SUBMIT SHOP DRAWINGS OF ALL STRUCTURAL STEEL, STEEL JOISTS, STEEL DECKING AND MISCELLANEOUS STEEL UPON REQUEST.
9. SEE FRAMING PLANS FOR ADDITIONAL NOTES AND REQUIREMENTS.
10. PROJECT NORTH AND NORTH ARE NOT ALWAYS THE SAME DIRECTION. PROJECT NORTH SHOWN IS FOR ORIENTATION OF STRUCTURAL STEEL ONLY.

DESIGN CRITERIA:

1. WIND LOAD
   - A. BASIC WIND SPEED (3-SECOND GUST): 103 mph
   - B. EXPOSURE CATEGORY: C
   - C. IMPORTANCE FACTOR: 1.00
   - D. BUILDING ENCLOSURE CLASSIFICATION: ENCLOSED
   - E. INTERNAL PRESSURE COEFFICIENT: +0.18
   - F. COMPONENTS AND CLADDING: VARIES
2. SNOW LOAD
   - A. GROUND SNOW LOAD (Pg): 37 psf
   - B. SNOW EXPOSURE (Ce): 1.0
   - C. IMPORTANCE FACTOR: 1.0
   - D. THERMAL FACTOR (Ct): 1.0
   - E. FLAT ROOF SNOW LOAD (Pf): 25.9 psf
3. SEISMIC LOAD
   - A. OCCUPANCY CATEGORY: II
   - B. IMPORTANCE FACTOR: 1.0
   - C. SITE CLASS: D DEFAULT
   - D. SEISMIC DESIGN CATEGORY: D
   - E. MAPPED SPECTRAL RESPONSE ACCELERATIONS
     - SHORT PERIOD (Ss): 1.475g
     - 1-SEC PERIOD (S1): 0.534g
   - F. DESIGN SPECTRAL RESPONCE COEFFICIENTS
     - SHORT PERIOD (Sds): 1.18g
     - 1-SEC PERIOD (Sd1): 0.63g
   - G. BASIC SEISMIC FORCE RESISTING SYSTEM AND RESPONSE MODIFICATION FACTOR
     - ORDINARY STEEL CONCENTRICALLY BRACED FRAMES: R=3.25
   - H. SEISMIC RESPONSE COEFFICIENT (Cs): 0.363
   - I. DESIGN BASE SHEAR (Vs = Cs x W): 15.7 kips
   - J. ANALYSIS PROCEDURE
     - EQUIVALENT LATERAL FORCE PROCEDURE
4. LIVE LOAD
   - A. ROOF LIVE LOAD (REDUCIBLE): 20 psf
   - B. SLABS ON GRADE: 125 psf
5. SOILS
   - A. SITE CLASS: D DEFAULT
   - B. ALLOWABLE BEARING CAPACITY: 2000 psf

ANCHOR ROD/BOLT NOTES:

1. FOR GROUT BEARING, GENERAL CONTRACTOR IS TO PROVIDE AN ELEVATION NUT (PAINTED RED) AT EACH SET OF ANCHOR ROD/BOLTS. NUT IS TO BE SET AT THE PROPER ELEVATION (TOP OF NUT=BOTTOM OF BASE PLATE).
2. GENERAL CONTRACTOR IS TO ASSURE LOCATION OF ALL ANCHOR ROD/BOLT PATTERNS TO BE WITHIN .375" AS CALLED FOR ON PLANS.
3. THE CONTRACTOR/ERECTOR IS TO BRING ALL OTHER NUTS TO SAME ELEVATION AS THE GENERAL CONTRACTOR'S ELEVATION NUT.
4. WHEN BASE PLATE BEARS ON THE FINISHED CONCRETE, GENERAL CONTRACTOR IS TO ASSURE THAT ANCHOR RODS/BOLTS AND CONCRETE BEARING IS WITHIN .375" FOR LOCATION AND ELEVATION.

WELDED NOTES:

1. ALL WELDING SHALL BE EXECUTED BY A CERTIFIED WELDER.
2. E-70XX ELECTRODES SHALL BE USED FOR ALL STRUCTURAL STEEL CONNECTIONS UNLESS OTHERWISE NOTED. E60XX ELECTRODES MAY BE USED FOR WELDING ROOF DECK AND FLOOR DECK.
3. NO WELDING IS PERMITTED ON THE ANCHOR BOLTS WITHOUT PRIOR APPROVAL OF THE ENGINEER.

GENERAL STRUCTURAL NOTES:

1. THE STRUCTURAL NOTES ARE THE PROJECT SPECIFICATIONS WHICH ARE PART OF THE CONSTRUCTION DOCUMENTS. SPECIFIC NOTES AND DETAILS IN THE DRAWINGS SHALL GOVERN OVER THE STRUCTURAL NOTES AND TYPICAL DETAILS. TYPICAL DETAILS SHALL APPLY WHERE SPECIFIC DETAILS ARE NOT SHOWN.
2. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE. IF ACTUAL CONDITIONS DIFFER FROM THOSE SHOWN ON CONTRACT DOCUMENTS, CONTRACTOR SHALL NOTIFY ENGINEER PRIOR TO FABRICATION OR CONSTRUCTION OF ANY AFFECTED ELEMENTS.
3. OMISSIONS OR CONFLICTS FOUND IN THE CONSTRUCTION DOCUMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY WORK INVOLVED.
4. DO NOT SCALE STRUCTURAL DRAWINGS. REFER TO ARCHITECT'S DRAWINGS FOR ALL DIMENSIONS.
5. REVIEW OF CONSTRUCTION SUBMITTALS / SHOP DRAWINGS BY THE ENGINEER OF RECORD IS FOR GENERAL COMPLIANCE ONLY AND IS NOT INTENDED AS APPROVAL. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL PERTINENT INFORMATION AND ENSURING THAT DESIGN REQUIREMENTS ARE MET.
6. THE CONTRACTOR SHALL VERIFY AND COORDINATE LOCATIONS AND SIZES OF ALL MECHANICAL OR OTHER EQUIPMENT BEFORE FABRICATING OR ERECTING EFFECTED STRUCTURAL ELEMENTS. LOCATIONS AND SIZES THAT DIFFER FROM THOSE INDICATED IN THE CONSTRUCTION DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT.
7. THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST TO THE ENGINEER FOR APPROVAL BEFORE PROCEEDING WITH ANY CHANGES, MODIFICATIONS OR SUBSTITUTIONS AFFECTING ANY STRUCTURAL ELEMENTS.
8. DURING AND AFTER CONSTRUCTION, THE DESIGN LOADS AS INDICATED IN THESE DOCUMENTS SHALL NOT BE EXCEEDED.
9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ADEQUATE TEMPORARY SHORING AND BRACING FOR ALL STRUCTURAL COMPONENTS UNTIL THE ENTIRE STRUCTURAL SYSTEM IS COMPLETED. DESIGN OF SUCH SHORING AND BRACING IS BY OTHERS.
10. STRUCTURAL OBSERVATIONS SHALL BE CONDUCTED AS NEEDED BY A REPRESENTATIVE OF THE ENGINEER OF RECORD AND WILL CONSIST OF OBSERVING THE CONSTRUCTION OF CRITICAL STRUCTURAL ELEMENTS. THESE STRUCTURAL OBSERVATIONS SHALL NOT BE CONSTRUED AS SPECIAL INSPECTIONS OR APPROVAL OF CONSTRUCTION.
11. ENGINEER SHALL NOT BE RESPONSIBLE FOR ACTIVITIES UNDER CONTROL OF THE CONTRACTOR SUCH AS CONSTRUCTION SITE SAFETY; MEANS, METHODS, AND SEQUENCING OF CONSTRUCTION.

STRUCTURAL STEEL:

1. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL OF STEEL CONSTRUCTION, FOURTEENTH EDITION.
2. ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING (UNLESS NOTED OTHERWISE):
   - A. WIDE FLANGE SHAPES - ASTM A992 (Fy=50 KSI)
   - B. RECTANGULAR HSS - ASTM A500, GRADE B (Fy=46 KSI)
   - C. ROUND HSS - ASTM A500, GRADE B (Fy=42 KSI)
   - D. PIPE MEMBERS - ASTM A53, TYPE E OR S, GRADE B (Fy=35 KSI)
   - E. CHANNELS AND ANGELS - ASTM A36 (Fy=36 KSI)
   - F. PLATE - ASTM A36 (STANDARD)
   - G. PLATE - ASTM 572-50 (Fy=50 KSI)
   - H. ALL OTHER MEMBERS - ASTM A36 (Fy=36 KSI)
   - I. WELDS - E70XX ELECTRODES
   - J. ANCHOR BOLTS - ASTM F1554 GRADE 36 OR 55 OR 105; SEE PLANS FOR REQUIREMENT
   - K. HEADED STUDS - ASTM A307 (Fu=60 KSI)
3. ALL BOLTS FOR STEEL TO STEEL CONNECTIONS TO BE ¾" DIAMETER MINIMUM A325 HIGH STRENGTH BOLTS UNLESS NOTED OTHERWISE.
4. ALL WELDS TO BE MADE BY A CERTIFIED WELDER.
5. ALL WELDS AND BOLTING TO MEET APPROVAL OF SPECIAL INSPECTOR AS REQUIRED BY THE BUILDING OFFICIAL.
6. ALL STEEL SHALL BE PROPERLY PRIMED EXCEPT AREAS THAT REQUIRE WELDING.
7. ALL STEEL BEAMS USED AS GIRDERS SHALL HAVE WEB STIFFENERS ON EACH SIDE OF WEB AT BEARING ENDS AND AT CONCENTRATED LOADS AS REQUIRED BY THE ENGINEER.
8. ANY MODIFICATION OF STRUCTURAL MEMBERS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS IS NOT PERMITTED WITHOUT PRIOR APPROVAL.
9. ANY CONNECTIONS NOT DETAILED ON STRUCTURAL PLANS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO FABRICATION AND INSTALLATION.

STRUCTURAL CONNECTIONS:

1. THE CONTRACTOR IS ULTIMATELY RESPONSIBLE TO PROVIDE ADEQUATE STRUCTURAL CONNECTIONS. CONNECTIONS MUST CARRY THE BEARING CAPACITY OF THE MEMBER AND ANY UPLIFT OR SEISMIC FORCES GENERATED IN THE MEMBER. SPECIAL CONSIDERATION SHALL BE GIVEN TO PREVENT CRUSHING OF THE MEMBER AT BEARING.
2. THE CONTRACTOR SHALL STRICTLY ADHERE TO THE CONNECTION DETAILS SPECIFIED ON THE PLANS OR WITHIN THE CONSTRUCTION DOCUMENTS. PRIOR APPROVAL IS REQUIRED FOR ANY DEVIATION FROM THE CONSTRUCTION DOCUMENTS.
3. SUBSTITUTION OF CONNECTIONS OTHER THAN THOSE SPECIFIED ON THE PLANS REQUIRES PRIOR APPROVAL. THE ENGINEER IS NOT RESPONSIBLE FOR CONNECTIONS NOT APPROVED PRIOR TO CONSTRUCTION OR INSTALLATION.
4. IF CONNECTION DETAILS, APPROVED BY THE ENGINEER, HAVE NOT BEEN PROVIDED IN THE CONSTRUCTION DOUMENTS, IT IS THE RESPONSIBLITY OF THE CONTRACTOR TO SPECIFY AND PROVIDE ALL INFORMATION TO THE ENGINEER FOR ADDITIONAL ASSISTANCE.

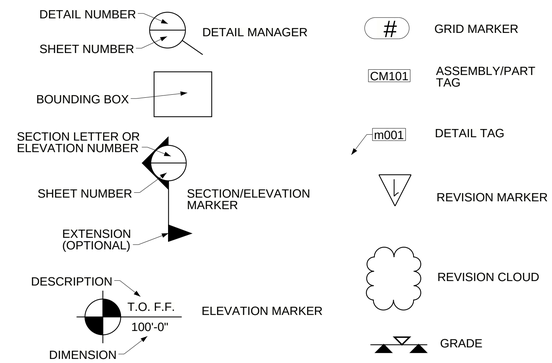
BOLTED CONNECTION NOTES:

1. ALL HIGH STRENGTH CONNECTIONS ARE TO BE MADE USING A325 BOLTS. THREADS NEED TO NOT BE EXCLUDED FROM THE SHEAR PLANE UNLESS NOTED OTHERWISE BY DESIGNATION OF BOLT.
2. BOLTS ARE TO BE INSTALLED AND TIGHTENED TO A SNUG TIGHT CONDITION. THIS CONDITION IS DEFINED AS THE TIGHTNESS THAT EXISTS WHEN ALL SURFACES ARE IN FIRM CONTACT AND IS USUALLY ATTAINED BY A FEW HITS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A WORKER USING AN ORDINARY SPUD WRENCH.
3. HARDENED WASHERS TO BE USED ON OVERSIZED HOLES AND SLOTS.
4. ALL BOLTS AND NUTS TO BE STORED IN A CLEAN DRY PLACE.

ABBREVIATIONS

L	ANGLE	GA	GAUGE	R	RADIUS
AB	ANCHORBOLTS	GALV	GALVANIZED	RAF	RAFTER
ALT	ALTERNATE	GP	GUSSET PLATE	REF	REFERENCE
ARCH	ARCHITECT	GR	GRADE	REQ'D	REQUIRED
		GRD	GIRDER	RO	ROUGH OPENING
BM	BEAM				
BP	BASE PLATE	HB	HORIZONTAL BRIDGING	SL	STEEL (GIRT) LINE
BRG	BEARING	HED	HEADER	SCH	SCHEDULE
BLDG	BUILDING	HT	HEIGHT	SFRS	SEISMIC FORCE RESISTING SYSTEM
BOF	BOTTOM OF FOOTING	HORZ	HORIZONTAL		
BTM	BOTTOM	HSA	HEADED STUD ANCHORS	SHT	SHEET
BS	BOTH SIDES			SIM	SIMILAR
		ID	INSIDE DIAMETER	SL	STEEL LINE
CF	CONTINUOUS FOOTING	IN	INCH	SQ	SQUARE
CL	CENTERLINE	INSUL	INSULATION	SSR	STANDING SEAM ROOF
CMU	CONCRETE MASONRY UNIT	INT	INTERIOR	STAG	STAGGERED
COL	COLUMN			STD	STANDARD
CONC	CONCRETE	JB	JACK BEAM	STP	STIFFENER PLATE
CONST	CONSTRUCTION	JBS	JOIST BEARING SEAT	STL	STEEL
CONT	CONTINUOUS	JST	JOIST		
CNTR	CENTER			TAB	TOP AND BOTTOM
		K	KIPS	TCB	TENSION CONTROL BOLT
DB	DECK BEARING	KLF	KIPS PER LINEAR FOOT	THDS	THREADS
DBA	DEFORMED BAR ANCHOR	KSF	KIPS PER SQUARE FOOT	TOC	TOP OF CONCRETE
DET	DETAIL			TOD	TOP OF DECK
	DIAMETER	LBS	POUNDS	TOF	TOP OF FOUNDATION
DIM	DIMENSION			TOFF	TOP OF FINISHED FLOOR
DS	DOWN SPOUT	MAX	MAXIMUM	TOS	TOP OF STEEL
DWG	DRAWING	MECH	MECHANICAL	TOW	TOP OF WALL
		MF	MODULAR FRAME	TS	TUBE STEEL
EHS	EXTRA HIGH STRENGTH	MIN	MINIMUM	TYP	TYPICAL
ELEV	ELEVATION	MISC	MISCELLANEOUS		
EOD	EDGE OF DECK	MTP	MOMENT TRANSFER PLATE	UB	UPLIFT BRACE
EQUIP	EQUIPMENT			UNO	UNLESS NOTED OTHERWISE
EQ	EQUAL	NIC	NOT IN CONTRACT		
ES	EAVE STRUT	NS	NEAR SIDE	VERT	VERTICAL
EXP	EXPANSION	NTS	NOT TO SCALE		
EXST	EXISTING			w/	WITH
EXT	EXTERIOR	OAL	OVERALL LENGTH	w/o	WITHOUT
		OC	ON CENTER	WBC	WIND BRACE CLIP
FB	FLANGE BRACE	OD	OUTSIDE DIAMETER	WCC	WIND COLUMN CLIP
FCA	FRICTION CLIP ANGLE	OHD	OVERHEAD DOOR	WP	WORK POINT
FCP	FRICTION CLIP PLATE	OPNG	OPENING	WF or W	WIDE FLANGE
FDTN	FOUNDATION	OPP	OPPOSITE	WWF	WELDED WIRE FABRIC
F	FOOTING	OW	OPEN WEB		
FF	FINISHED FLOOR				
FL	FLOOR	PL	PLATE		
FO	FRAMED OPENING	PLF	POUNDS PER LINEAR FOOT		
FS	FAR SIDE	PSF	POUNDS PER SQUARE FOOT		
FT	FEET or FOOT	PSI	POUNDS PER SQUARE INCH		
FW	FOUNDATION WALL	PT	POINT		
		PURL	PURLIN		

SYMBOL LEGEND



INDEX TO STRUCTURAL DRAWINGS:

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT, 84404

GENERAL NOTES

Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. SS002

OPEN WEB STEEL JOISTS AND GIRDERS

1. CARE SHALL BE EXERCISED AT ALL TIMES TO AVOID DAMAGE THROUGH CARELESS HANDLING DURING UNLOADING, STORING AND ERECTING. DROPPING OF JOIST GIRDERS IS NOT PERMITTED.
2. WHERE JOIST GIRDERS ARE UTILIZED AT COLUMNS, THE JOIST GIRDER SHALL BE FIELD-BOLTED AT THE COLUMN.
3. BEFORE HOISTING CABLES ARE RELEASED AND BEFORE AN EMPLOYEE IS ALLOWED ON JOIST GIRDER OR JOIST, THE FOLLOWING CONDITIONS SHALL BE MET.
   A. THE SEAT AT EACH END OF THE JOIST GIRDER OR JOIST IS ATTACHED IN ACCORDANCE WITH NOTE 4 OR 5, RESPECTFULLY.
   B. WHERE STABILIZER PLATES ARE REQUIRED THE JOIST GIRDER BOTTOM CORD SHALL ENGAGE THE STABILIZER PLATE.
4. GIRDER BEARING SEAT ATTACHMENTS:
   A. MASONRY AND CONCRETE-ENDS OF JOIST GIRDERS RESTING ON STEEL BEARING PLATES ON MASONRY OR STRUCTURAL CONCRETE SHALL BE ATTACHED THERETO WITH A MINIMUM OF TWO ¼ INCH FILLET WELDS 2 INCHES LONG, OR WITH TWO ¾ INCH ASTM A307 BOLTS, OR EQUIVALENT.
   B. STEEL-ENDS OF JOIST GIRDERS RESTING ON STEEL SUPPORTS SHALL BE ATTACHED THERETO WITH A MINIMUM OF TWO ⅛ INCH FILLET WELDS 2 INCHES LONG, OR WITH TWO ¾ INCH ASTM A307 BOLTS, OR EQUIVALENT.
5. JOIST BRIDGING SEAT ATTACHMENTS:
   A. K SERIES JOISTS-
      - MASONRY AND CONCRETE-ENDS OF K-SERIES JOISTS RESTING ON STEEL BEARING PLATES ON MASONRY OR STRUCTURAL CONCRETE SHALL BE ATTACHED THERETO WITH A MINIMUM OF TWO ⅛ INCH FILLET WELDS 2 INCHES LONG, OR WITH TWO ½ INCH ASTM A307 BOLTS, OR EQUIVALENT AND SHALL BEAR A MINIMUM OF 2 ½ INCHES.
      - STEEL-ENDS OF K-SERIES JOISTS RESTING ON STEEL SUPPORTS SHALL BE ATTACHED THERETO WITH A MINIMUM OF TWO ⅛ INCH FILLET WELDS 2 INCHES LONG, OR WITH TWO ½ INCH ASTM A307 BOLTS, OR EQUIVALENT AND SHALL BEAR A MINIMUM OF 2 ½ INCHES.
   B. LH AND DLH SERIES JOISTS-
      - MASONRY AND CONCRETE-ENDS OF LH AND DLH SERIES JOISTS RESTING ON STEEL BEARING PLATES ON MASONRY OR STRUCTURAL CONCRETE SHALL BE ATTACHED THERETO WITH A MINIMUM OF TWO ¼ INCH FILLET WELDS 2 INCHES LONG, OR WITH TWO ½ INCH ASTM A307 BOLTS, OR EQUIVALENT AND SHALL BEAR A MINIMUM OF 4 INCHES.
      - STEEL-

JOIST SECTION NUMBER*	FILLET WELD	BEARING SEAT BOLTS FOR ERECTION	MINIMUM BEARING LENGTH
02 TO 06 INCL.	2-3/16" x 2"	2-¾" A307	2 ½"
07 TO 17 INCL.	2-¼" x 2"	2-¾" A307	4"
18 TO 25 INCL.	2-¼" x 4"	2-¾" A325	6"

* LAST TWO DIGITS OF JOIST DESIGNATION

6. JOIST GIRDERS AND JOISTS SHALL NOT BE USED AS ANCHORAGE POINTS FOR A FALL ARREST SYSTEM UNLESS WRITTEN DIRECTION TO DO SO IS OBTAINED FROM A "QUALIFIED PERSON." SEE OSHA 29 CFR PART 1926 SAFETY STANDARDS FOR STEEL ERECTION.

STEEL DECK

1. STEEL DECK SHALL BE OF THE PROFILE DEPTH AND THICKNESS AS INDICATED ON THE DRAWINGS.
2. STEEL DECK SHALL HAVE A MINIMUM END BEARING OF 2 INCHES. END JOINTS SHALL BE LAPPED 2 INCHES MINIMUM.
3. COMPOSITE AND NON-COMPOSITE STEEL DECKS MAY BE EITHER LAPPED OR BUTTED AT CONTRACTOR'S OPTION.
4. STEEL DECK SHALL BE TRIPLE SPAN CONTINUOUS WHERE POSSIBLE. DO NOT LOCATE SINGLE SPANS AT EDGES OR CORNERS.
5. WELDED ATTACHMENT OF STEEL DECK UNITS TO THE SUPPORTING MEMBERS SHALL CONFORM TO AWS D1.3. WELDING OF STEEL DECK SHALL BE PERFORMED BY CERTIFIED LIGHT GAGE STEEL WELDERS.
6. ARC SPOT OR ARC SEAM (PUDDLE) WELDS SHALL HAVE AN EFFECTIVE FUSION AREA TO SUPPORTING MEMBERS, EQUIVALENT TO AT LEAST ⅜ INCH BY 1 INCH LONG OR ½ INCH DIAMETER AND IN NO CASE ANY WELD SPACING EXCEED 3 FEET.

POST INSTALLED ANCHOR NOTES:

1. POST INSTALLED ANCHORS REFER TO EXPANSION, SLEEVE, AND ADHESIVE ANCHORS.
2. INSTALL ANCHORS PER MANUFACTURERS RECOMMENDATIONS.
3. CLEAR HOLES WITH WIRE BRUSH AND COMPRESSED AIR WHERE REQUIRED BY MANUFACTURERS INSTALLATION REQUIREMENTS.
4. OBSERVE CURE / GEL TIME FOR ALL ADHESIVE ANCHORS IN ACCORDANCE WITH MANUFACTURERS INSTALLATION REQUIREMENTS.
5. STRICTLY FOLLOW ALL SPACING, EDGE DISTANCE AND EMBEDMENT REQUIREMENTS. INCREASED EMBEDMENT DEPTH MAY CAUSE REDUCTION IN STRETCH WHEN ANCHORS ARE INSTALLED NEW EDGES.
6. ANCHOR SUBSTITUTION ARE ALLOWED ONLY WHERE SUBSTATION IS EQUIVALENT IN STRETCH AND PERFORMANCE TO THE SPECIFIED ANCHOR TYPE.

GENERAL CONCRETE NOTES:

1. CONCRETE CONSTRUCTION SHALL CONFORM TO AMERICAN CONCRETE INSTITUTE, ACI 318.
2. ALL FOOTINGS SHALL BE CENTERED UNDER WALLS, COLUMNS, PILASTERS, ETC. U.N.O. ON THE PLANS.
3. ALL FOOTING ELEVATIONS SHOWN ARE TO TOP OF FOOTING.
4. CONCRETE SHALL BE NORMAL WEIGHT AND SHALL HAVE THE FOLLOWING 28-DAY COMPRESSIVE STRENGTHS:
   a. 4,000 PSI - FOOTINGS, INTERIOR SLABS ON GRADE, SUSPENDED SLABS ON METAL DECK.
   b. 4,000 PSI - COLUMNS, FOUNDATION AND RETAINING WALLS, EXTERIOR SLABS ON GRADE CURBS, AND GUTTERS.
   c. 3,000 PSI - SLABS ON GRADE
   d. 3,000 PSI - ALL OTHER CONCRETE
5. USE ASTM TYPE I OR TYPE IA CEMENT
6. CONCRETE PLACEMENT SHALL BE IN ACCORDANCE WITH ACI 305 HOT WEATHER CONCRETING & ACI 306 COLD WEATHER CONCRETING.
7. WATER USED IN MIXING CONCRETE SHALL BE CLEAN AND FREE FROM INJURIOUS AMOUNTS OF OILS, ACIDS, ALKALIS, SALTS, ORGANIC MATERIALS OR OTHER SUBSTANCES THAT ARE DELETERIOUS TO CONCRETE OR STEEL REINFORCEMENTS.
8. ALL INSERTS, ANCHOR BOLTS, PLATES, AND OTHER ITEMS TO BE CAST IN THE CONCRETE SHALL BE HOT-DIPPED GALVANIZED ACCORDING TO ASTM A153 UNLESS OTHERWISE NOTED.
9. REINFORCING BARS, ANCHOR BOLTS, INSERTS, AND OTHER ITEMS TO BE CAST IN THE CONCRETE SHALL BE SECURED IN POSITION PRIOR TO PLACEMENT OF CONCRETE.
10. ALL CONCRETE SHALL BE CONSOLIDATED BY MECHANICAL VIBRATORS.
11. CONDUITS, PIPES, AND SLEEVES EMBEDDED WITHIN A SLAB OR WALL (OTHER THAN THOSE MERELY PASSING THROUGH) SHALL SATISFY THE FOLLOWING:
    a. NO LARGER IN OUTSIDE DIMENSIONS THAN 1/3 THE OVERALL THICKNESS OF THE SLAB, OR WALL IN WHICH THEY ARE EMBEDDED
    b. CONDUITS, PIPES, AND SLEEVES SHALL NOT BE PLACED THROUGH OR EMBEDDED IN A BEAM UNLESS SPECIFICALLY DETAILED
    c. SPACED NO CLOSER THAN 3 DIAMETERS OR WIDTHS ON CENTER.
    d. PLACED IN THE MIDDLE 1/3 OF SLAB OR WALL THICKNESS
12. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, ETC. TO BE CAST IN TO CONCRETE, AND FOR EXTENT AND LOCATION OF DEPRESSIONS, CURBS, RAMPS, ETC.
13. CONSTRUCTION JOINTS SHALL BE MADE AND LOCATED SO AS TO NOT IMPAIR THE STRENGTH OF THE STRUCTURE. PROVIDE 2 X 4 (SHAPED) KEYWAY IN ALL VERTICAL AND HORIZONTAL JOINTS UNLESS NOTED OR DETAILED OTHERWISE.
14. ALL STEEL REINFORCING SHALL BE CONTINUOUS THROUGH COLD JOINTS UNLESS NOTED OTHERWISE.
15. NON-SHRINK GROUT SHALL BE A PREMIXED NON-METALLIC FORMULA. CAPABLE OF DEVELOPING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI IN 1 DAY AND 5,000 PSI IN 28 DAYS.
16. LEAVE FRAMEWORK FOR OTHER STRUCTURAL ELEMENTS THAT SUPPORT WEIGHT OF CONCRETE IN PLACE UNTIL CONCRETE HAS ACHIEVED IT'S 28 DAY DESIGN COMPRESSIVE STRENGTH.

FOUNDATION NOTES:

1. EXCEPT WHERE NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS, THE RECOMMENDATIONS OF THE ABOVE MENTIONED GEOTECHNICAL REPORT SHALL BE FOLLOWED, WHICH INCLUDE BUT ARE NOT LIMITED TO, SOIL CORRECTIONS / OVER-EXCAVATIONS, SUBGRADE PREPARATIONS, FILL REQUIREMENTS, AND COMPACTION REQUIREMENTS.
2. CONTRACTOR SHALL PROVIDE FOR DE-WATERING OF EXCAVATION FROM SURFACE WATER, GROUND WATER OR SEEPAGE.
3. EXCAVATIONS FOR ANY PURPOSE SHALL NOT REMOVE LATERAL SUPPORT FROM ANY FOOTING OR FOUNDATION WITHOUT FIRST UNDERPINNING OR PROTECTING THE FOOTING OR FOUNDATION AGAINST SETTLEMENT OR LATERAL SUPPORT.
4. CONTRACTOR SHALL BRACE OR PROTECT ALL WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHING FLOORS ARE COMPLETELY IN PLACE AND HAVE ATTAINED THEIR FULL DESIGN STRENGTH.
5. ALL FOOTINGS SHALL HAVE VERTICAL FACES FORMED WITH STANDARD FORMING MATERIALS (WOOD, METAL, ETC.) UNLESS NOTED OTHERWISE.
6. TOP OF FOOTING ELEVATIONS SHOWN ON THE FOOTING AND FOUNDATION PLAN ARE BASED ON PRELIMINARY GRADING INFORMATION AND MUST BE VERIFIED PRIOR TO CONSTRUCTION.

REINFORCING STEEL NOTES:

1. ALL REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.
2. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, SHALL BE SUPPLIED IN FLAT SHEETS, AND SHALL HAVE A MINIMUM SIDE LAP OF 8 INCHES.
3. ADEQUATELY TIE AND SUPPORT ALL REINFORCING STEEL AS SPECIFIED BY ACI 315 TO MAINTAIN EXACT REQUIRED POSITION.
4. CAST DOWELS IN FORMING FOR CONCRETE WALLS AND COLUMNS ABOVE DOWELS TO BE SAME QUANITY, SIZE AND SPACING AS THE VERTICAL WALL AND COLUMN REINFORCING. DOWELS ARE TO PROJECT FROM FOOTING TO PROVIDE THE REQUIRED LAP SPLICING REQUIREMENTS. DOWELS INTO FOOTINGS SHALL TERMINATE WITH A SPLICING HOOK, AND SHALL EXTEND TO WITHIN 4 INCHES OF THE BOTTOM OF THE FOOTING BUT NEED NOT EXTEND MORE A DEVELOPMENT LENGTH INTO THE FOOTING.
5. CLEAR CONCRETE COVER FOR REINFORCING BARS SHALL BE AS FOLLOWS, UNLESS OTHERWISE NOTED:
   a. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 INCHES
   b. CONCRETE FORMED AND EXPOSED TO EARTH OR WEATHER:
      i. NO. 6 AND LARGER : 2 INCHES
      ii. NO. 5 AND SMALLER: 1.5 INCHES
   c. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
      i. WALLS, SLABS, JOISTS:
         i.a. NO. 11 BAR AND SMALLER: 0.75 INCHES
      ii. PEDESTALS, COLUMNS, BEAMS:
         ii.a. PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS: 1.5 INCHES
   d. CONCRETE TILT-UP PANELS CAST AGAINST A RIGID HORIZONTAL SURFACE SUCH AS CONCRETE SLAB EXPOSED TO THE WEATHER:
      i. NO. 8 AND SMALLER: 1 INCH
      ii. NO. 9 AND LARGER: 2 INCHES
6. DO NOT WELD REINFORCING EXCEPT AS NOTED ON PLANS. WHERE WELDING REINFORCEMENT IN REQUIRED, USE ASTM A708.
7. CLEAR DISTANCE BETWEEN THE SURFACE OF A BAR AND ANY SURFACE OF A MASONRY UNIT SHALL NOT BE LESS THAN ½ INCH, UNLESS NOTED OTHERWISE.
8. REINFORCING STEEL SHALL BE SPLICED PER TYPICAL DETAILS AND SCHEDULES, UNLESS NOTED OTHERWISE.
9. MECHANICAL SPLICE CONNECTORS SHALL DEVELOP IN TENSION 125 % OF THE SPECIFIED MINIMUM YIELD STRENGTH OF REINFORCEING BARS.

MASONRY NOTES:

1. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, TYPE N-1, AND SHALL HAVE A MINIMUM UNIT STRENGTH OF 1,900 PSI.
2. ALL MASONRY WALLS SHALL HAVE A MINUMIM NET COMPRESSIVE STRENGTH OF 1,500 PSI.
3. GROUT SHALL CONFORM TO ASTM C476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI AT 28 DAYS AND SHALL BE PLACED WITH SUFFICENT WATER FOR POURING WITHOUT SEGREGATION.
4. GROUT STOP SHALL BE RESIN COATED FIBERGLASS MESH 1.5 INCHES NARROWER THAN WALL THICKNESS. OTHER GROUT STOP MATERIALS ARE NOT PERMITTED WITHOUT PRIOR APPROVAL FROM THE ENGINEER.
5. MORTAR SHALL BE TYPE "S" CONFORMING TO ASTM C270 AND SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 1,800 PSI AT 28 DAYS.
6. ENTIRE MASONRY WALLS SHALL NOT BE GROUTED SOLID EXCEPT AS SPECIFICALLY NOTED. ALL CELLS CONTAINING REINFORCING STEEL, EMBEDDED ITEMS, ANCHOR BOLTS, ETC. SHALL BE GROUTED SOILD.
7. WHERE WALLS ARE NOT GROUTED SOLID, EACH GROUT POUR SHALL TERMINATE FLUSH WITH THE TOP OF THE UPPERMOST UNIT EXCEPT AT CELLS WITH VERTICAL REINFORCING WHERE GROUT SHALL BE 1.5 INCHES BELOW THE TOP OF THE UNIT TO PROVIDE A CONSTRUCTION KEY.
8. GROUT POURS SHALL BE LIMITED 4 FEET UNLESS HIGH LIFT GROUTING PROCEDURES ARE FOLLOWED.
9. ALL MASONRY BEAMS SHALL BE CONSTRUCTED INTEGRAL WITH SUPPORTS. TOOTHING OR DOWELING IS NOT ACCEPTABLE. UNITS WITH ONE END OPEN SHALL BE USED AT ALL MASONRY BEAMS.
10. UNLESS NOTED OTHERWISE, ALL OPENINGS GREATER THAN 6 FEET SHALL HAVE A MASONRY COLUMN EACH SIDE 8" x 16" WITH (4) #5 VERTICAL BARS AND #3 TIES AT 8" O.C.
11. UNLESS OTHERWISE NOTED, SINGLE VERTICAL REINFORCING BARS SHALL BE PLACED IN THE CENTER OF THE WALLS, AND DOUBLE VERTICAL BARS SHALL BE PLACED NEAR EACH FACE OF THE MASONRY UNITS.
12. UNLESS NOTED OTHERWISE, PROVIDE VERTICAL CONTROL JOINTS AS INDICATED BY ARCHITECT AT A MAXIMUM PACING OF 40 FEET AND AT ALL CHANGES IN WALL ELEVATION AND MASONRY THICKNESS. CONTROL JOINTS HALL NOT BE LOCATED DIRECTLY OVER OR LOWER THAN 24 INCHES TO ANY WALL OPENINGS.
13. VERTICAL REINFORCEMENT SHALL BE PROVIDED AT EACH SIDE OF CONTROL JOINTS. HORIZONTAL REINFORCEMENT SHALL TERMINATE AT EACH SIDE OF CONTROL JOINTS EXCEPT AT FLOOR AND ROOF LEVEL BOND BEAMS AND AT TOP OF PARAPET.
14. WALL OPENINGS EXCEEDING 24 INCHES IN EITHER DIRECTION SHALL BE REINFORCED WITH A SINGLE #5 BAR (MINIMUM) ON ALL SIDES. HORIZONTAL BARS SHALL EXTEND 24 INCHES BEYOND THE CORNERS OF THE OPENING AND VERTICAL BARS SHALL EXTEND THE FULL HEIGHT OF THE WALL.
15. UNLESS NOTED OTHERWISE MINIMUM REINFORCING IN ALL 8 INCH MASONRY WALLS SHALL BE AS FOLLOWS:
    a. VERTICAL: #5 BARS ADJACENT TO ALL OPENINGS, AT ALL CORNERS, AT EACH SIDE OF CONTROL JOINTS, AND AT A MAXIMUM SPACING OF 32 INCHES ON CENTER THROUGHOUT THE WALL.
    b. HORIZONTAL: (2) #4 BARS IN 8 INCH DEEP "H" BLOCK BOND BEAM UNITS AT FLOORS, ROOF, TOP OF WALL, AND AT A MAXIMUM SPACING OF 48 INCHES ON CENTER THROUGHOUT THE WALL. BOND BEAMS AT A SLOPING ROOF MUST FOLLOW THE SAME SLOPE.
16. SECOND-HAND MASONRY UNITS SHALL NOT BE REUSED UNLESS THEY CONFORM TO THE REQUIREMENTS OF NEW UNITS. THE UNITS SHALL BE OF WHOLE, SOUND MATERIALS AND FREE FROM CRACKS AND OTHER DEFECTS THAT WILL INTERFERE WITH PROPER LAYING AND USE. OLD MORTAR SHALL BE CLEANED FROM THE UNIT BEFORE REUSE.

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT, 84404

INSTALLATION NOTES

Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. SS003

Tekla Structures

SPECIAL INSPECTION SCHEDULE

AREAS REQUIRING SPECIAL INSPECTION:	FREQUENCY CONT.	FREQUENCY PERIODIC	COMMENTS
FABRICATORS *(IBC 1704.2.5)*			
	X		IF FABRICATOR IS APPROVED, ON-SITE INSPECTION IS NOT REQUIRED BUT A CERTIFICATION OF COMPLETION MUST BE PROVIDED TO THE B.O. (IBC 1704.2.5.2)
SOILS *(IBC 1705.6)*			
VERIFY ADEQUATE MATERIALS BELOW FOOTINGS		X	PRIOR TO PLACEMENT OF CONCRETE
EXCAVATION EXTEND TO PROPER DEPTH AND MATERIALS		X	PRIOR TO PLACEMENT OF COMPACTED FILL OR CONCRETE
CLASSIFICATION AND TESTING OF FILL MATERIALS		X	CHECK CLASSIFICATION AND GRADATIONS AT EACH LIFT, BUT NOT LESS THAN ONCE FOR EACH 10,000 FT OF SURFACE AREA
VERIFY PROPER FILL MATERIALS, LIFT THICKNESSES AND IN-PLACE DENSITIES	X		PRIOR TO PLACEMENT OF CONCRETE
VERIFY PROPERLY PREPARED SITE AND SUBGRADE		X	PRIOR TO PLACEMENT OF CONCRETE
COLD-FORMED STEEL CONSTRUCTION *(IBC 1705.11.3)*			
COMPONENTS OF WIND-AND SEISMIC-FORCE RESISTING SYSTEMS		X	VERIFY PROPER SCREW ATTACHMENT, BOLTING AND ANCHORING OF SHEAR WALLS, BRACES AND HOLD-DOWNS HAVING A FASTENER SPACING LESS THAN OR EQUAL TO 4" O.C.
OTHER THAN STRUCTURAL CONSTRUCTION*(IBC 1705.2.2)*			
STEEL ROOF & FLOOR DECK:			
MATERIAL VERIFICATION OF STEEL DECK		X	IDENTIFICATION MARKINGS PER APPPLICABLE ASTM STANDARD
ROOF AND DECK WELDS		X	VERIFY THAT THE WELDS CONFORM TO AWS D1.3
WELDING OF REINFORCING STEEL:			
VERIFICATION OF WELDABILITY (EXCEPT A706 BAR)		X	VERIFY MATERIAL IS ABLE TO CONFORM TO AWS D1.4
STRUCTURAL STEEL CONSTRUCTION*(IBC 1705.2, 1705.11, 1705.12)*			
PRIOR TO WELDING (TABLE N5.4-1, AISC 360-10):			
VERIFY WELDING PROCEDURES	X		
MATERIAL IDENTIFICATION		X	VERIFY TYPE AND GRADE OF MATERIAL
WELDER IDENTIFICATION		X	VERIFY THERE IS A SYSTEM IN PLACE TO IDENTIFY THE WELDER WHO HAS WELDED A JOINT OR MEMBER
FIT-UP GROOVE WELDS		X	VERIFY JOINT PREPARATION, DIMENSIONS, CLEANLINESS, TACKING AND BACKING
ACCESS HOLES		X	VERIFY CONFIGURATION AND FINISH
FIT-UP WELDS		X	VERIFY ALIGNMENT, GAPS AT ROT, CLEANLINESS OF STEEL, SURFACES, TACK WELD QUALITY AND LOCATION
DURING WELDING (TABLE N5.4-2, AISC 360-10):			
USE OF QUALIFIED INSPECTORS		X	VERIFY THAT WELDERS ARE APPROPRAITELY QUALIFIED
CONTROL AND HANDLING OF WELDING CONSUMABLES		X	VERIFY PACKAGING AND EXPOSURE CONTROL
CRACKED TACK WELDS		X	VERIFY WELDING IS NOT OVER CRACKED TACK WELD
ENVIRONMENTAL CONDITIONS		X	VERIFY WIND SPEED IS WITHIN LIMITS AS WELL AS PRECIPITATION AND TEMPERATURE
WPS FOLLOWED		X	VERIFY ITEMS SUCH AS WELDING EQUIPMENT SETTINGS, TRAVEL SPEED, WELDING MATERIALS, SHIELDING GAS, TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED, AND PROPER POSITION.
WELDING TECHNIQUES		X	VERIFY INTERPASS AND FINAL CLEANING EACH PASS IS WITHIN PROFILE LIMITATIONS, AND QUALITY OF EACH PASS

SPECIAL INSPECTION SCHEDULE (CONTINUED)

AREAS REQUIRING SPECIAL INSPECTION:	FREQUENCY CONT.	FREQUENCY PERIODIC	COMMENTS
STRUCTURAL STEEL CONSTRUCTION (CONTINUED)			
AFTER WELDING (TABLE N5.4-3, AISC 360-10):			
WELDS CLEANED		X	VERIFY THAT WELDS HAVE BEEN PROPERLY CLEANED
SIZE, LENGTH AND LOCATION OF WELDS	X		
WELDS MEET VISUAL ACCEPTANCE CRITERIA	X		
ARC STRIKES	X		
K-AREA	X		
BACKING & WELDING TABS REMOVED	X		
REPAIR ACTIVITIES	X		
DOCUMENT ACCEPTANCE/REJECTION OF WELD	X		
NONDESTRUCTIVE TESING (TABLE N5.5, AISC 360-10):			
CJP WELDS (RISK CAT. II)		X	ULTRASONIC TESTING SHALL BE PERFORMED ON 10% OF CJP GROOVE WELDS IN BUTT, T- AND CORNER JOINTS SUBJECT TO TRANSVERSELY APPLIED TENSION LOADING IN MATERIALS 5/16 INCH THICK OR GREATER. TESTING RATE MUST BE INCREASED IF >5% OF WELDS HAVE BEEN UNACCEPTABLE DEFECTS.
ACCESS HOLES (FLANGE >2")	X		
WELDED JOINTS SUBJECT TO FATIGUE	X		
OTHER STEEL INSPECTIONS (TABLE N5.7, AISC 360-10; TABLES J8-1 AND J10-1, AISC 341-10):			
STRUCTURAL STEEL DETAILS		X	ALL FABRICATED STEEL AND THEIR CONNECTIONS SHALL BE INSPECTED TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN AND IN THE APPROVED PLANS
ANCHOR RODS/EMBEDS SUPPORTING STRUCTURAL STEEL		X	SHALL BE ON THE PREMISES DURING THE PLACEMENT OF ANCHOR RODS / EMBEDMENTS. VERIFY DIAMETER, GRADE, TYPE, AND LENGTH OF ELEMENT AND THE EXTENT OF DEPTH OF EMBEDMENT PRIOR TO PLACEMENT OF CONCRETE.
REDUCED BEAM SECTIONS (RBS)		X	VERIFY CONTOUR AND FINISH AS WELL AS DIMENSIONAL TOLERANCES (SEE TABLE J8-1 OR AISC 341)
PROTECTED ZONES		X	VERIFY THAT NO HOLES OR UNAPPROVED ATTACHMENTS ARE MADE WITHIN THE PROTECTED ZONE (SEE TABLE J8-1 OR AISC 341)
MASONARY CONSTRUCTION *(IBC 1705.4)*			
MINIMUM TESTING (TABLE 1.19.2, TMS-402/ACI 530-11):			
VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) FOR SELF-CONSOLIDATING GROUT		X	COMPRESSIVE STRENGTH TESTS PER ASTM C 1019 FOR SLUMP FLOW AND ASTM C 1611 FOR VSI
VERIFICATION OF F'm"			DETERMINE COMPRESSIVE STRENGTH PET "UNIT STRENGTH" OF "PRISM TEST" AS SPECIFIED IN ARTICLE 1.4.B OF ACI 530.1 PRIOR TO CONSTRUCTION
PRIOR TO CONSTRUCTION (TABLE 1.15, TMS-602/ACI 530-11):			
REVIEW MATERIAL CERTIFICATES, MIX DESIGNS, TEST RESULTS AND CONSTRUCTION PROCEDURES		X	CERTIFY MATERIALS CONFORM TO APPROVED CONSTRUCTION DOCUMENTS. MIX DESIGN, TEST RESULTS, MATERIAL CERTIFICATES, AND CONSTRUCTION PROCEDURES SHOULD BE SUBMITTED FOR REVIEW. MORTAR MIX DESIGNS SHALL CONFORM TO ASTM C 270 WHILE GROUT SHALL CONFORM TO ASTM C 476. MATERIAL CERTIFICATES SHALL BE PROVIDED FOR THE FOLLOWING REINFORCEMENT; ANCHORS, TIES, FASTENERS, AND METAL ACCESSORIES; MASONRY UNITS; MORTAR AND GROUT MATERIALS. REVIEW COLD-WEATHER OR HOT-WEATHER CONSTRUCTION PROCEDURES
AS CONSTRUCTION BEGINS (TABLE 1.19.2, TMS-402/ACI 530-11):			
PROPORTIONS OF SITE-PREPARED MORTAR		X	VERIFY THAT MORTAR IS TYPE AND COLOR SPECIFIED ON APPROVED PLANS, IT CONFORMS TO ASTM C 270, AND IS MIXED PER ARTICLE 2.6.A OF ACI 530.1
CONSTRUCTIONS OF MORTAR JOINTS		X	VERIFY MORTAR JOINTS MEET ARTICLE 3.3.B OF ACI 530.1
LOCATION OF REINFORCEMENT, CONNECTORS AND ANCHORAGES		X	VERIFY REINFORCEMENT IS PLACED IN ACCORDANCE WITH ARTICLE 3.4 OR ACI 530.1
PRIOR TO GROUTING (TABLE 1.19.2, TMS-402/ACI 530-11):			
GROUT SPACE		X	VERIFY THAT GROUT SPACE IS FREE OF MORTAR DROPPINGS, DEBRIS, LOOSE AGGREGATE, AND OTHER DELETERIOUS MATERIALS AND THAT CLEANOUTS ARE PROVIDED PER ARTICLE 3.2.D AND 3.2.F OF ACI 530.1
GRADE, TYPE AND SIZE OF REINFORCEMENT, CONNECTORS AND ANCHORAGES		X	VERIFY REINFORCEMENT, JOINT REINFORCEMENT, ANCHOR BOLTS AND VENEER ANCHORS COMPLY WITH APPROVED PLANS AND SECTION 1.6 OF ACI 530
PLACEMENT OF REINFORCEMENT, ANCHOR BOLTS AND ANCHORAGES		X	VERIFY REINFORCEMENT, JOINT REINFORCEMENT, ANCHOR BOLTS AND VENEER ANCHORS ARE INSTALLED PER APPROVED PLANS AND ARTICLES 3.2.E, 3.4 AND 3.6.A OF ACI 530.1
PROPORTIONS OF SITE-PREPARED GROUT		X	VERIFY GROUT PROPORTIONS MEET ASTM C 476 AND A SLUMP BETWEEN 8-11 INCHES. SELF-CONSOLIDATED GROUT SHALL NOT BE PROPORTIONED ONSITE
CONSTRUCTION OF MORTAR JOINTS		X	VERIFY MORTAR JOINTS PLACED IN ACCORDANCE WITH ARTICLE 3.3.B OF ACI 530.1

SPECIAL INSPECTION SCHEDULE (CONTINUED)

AREAS REQUIRING SPECIAL INSPECTION:	FREQUENCY CONT.	FREQUENCY PERIODIC	COMMENTS
MASONRY CONSTRUCTION (CONTINUED)			
DURING TO CONSTRUCTION (TABLE 1.19.2, TMS-402/ACI 530-11):			
SIZE AND LOCATION OF STRUCTURAL ELEMENTS		X	VERIFY LOCATIONS OF STRUCTURAL ELEMENTS PER APPROVED PLANS AND CONFIRM TOLERANCES MEET ARITCLE 3.3.F OF ACI 530.1
TYPE, SIZE AND LOCATION OF ANCHORS, FRAMES, ETC.		X	VERIFY CORRECT ANCHORAGES AND CONNECTIONS ARE PROVIDED PER APPROVED PLANS AND SECTIONS 1.18.4.3 AND 1.17.1 OF ACI 530
PLACEMENT OF GROUT	X		
PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (<40 DEGREE F) OR HOT WEATHER (>90 DEGREE F)		X	VERIFY COLD WEATHER CONSTRUCTION COMPLIES WITH ARTICLE 1.8.C OF ACI 530.1 AND HOT WEATHER CONSTRUCTION PER ARTICLE 1.8.D OF ACI 530.1
OBSERVATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS		X	CONFIRM SPECIMENS/PRISMS ARE PERFORMED AS REQUIRED BY ARTICLE 1.4 OF ACI 530.1

REQUIRED SPECIAL INSPECTION AND TESTS OF CONCRETE CONSTRUCTION TABLE 1705.3

TYPE	FREQUENCY OF INSPECTION CONTINUOUS	FREQUENCY OF INSPECTION PERIODIC	REFERENCE FOR CRITERIA REFERENCED STANDARD	REFERENCE FOR CRITERIA IBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	-	X	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706; b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND c. INSPECT ALL OTHER WELDS	- X X	X X	AWS D1.4, ACI 318: 26.6.4	-
3. INSPECT ANCHORS CAST IN CONCRETE	-	X	ACI 318: 17.8.2	-
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERSb a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a	X	 X	ACI 318: 17.8.2.4 ACI 318: 17.8.2.4	- -
5. VERIFY USE OF REQUIRED DESIGN MIX.	-	X	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	-	ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	-	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	X	ACI 318: 26.5.3-26.5.5	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR; a. APPLICATION OF PRESTRESSING FORCES; AND b. GROUTING OF BONDED PRESTRESSING TENDONS.	X X	- -	ACI 318: 26.10	-
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS	-	X	ACI 318: 26.9	-
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS	-	X	ACI 318: 26.11.2	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	-	X	ACI 318: 26.11.1.2(b)	-

FOR SI: INCH = 25.4MM.
a. WHERE APPLICABLE, SEE SECTION 1705.12, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE.
b. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318, OR OTHER QUALIFICATION PROCEDURES. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.

SPECIAL INSPECTIONS NOTES:

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. SPECIAL INSPECTION NOTES ARE CONSTRUCTION DOCUMENTS THAT SHALL BE INCLUDED WITH THE STRUCTURAL PLANS AND PROJECT SPECIFICATIONS.
3. SPECIAL INSPECTION SHALL BE PROVIDED BY OWNER OR OWNER AGENT ACCORDING TO IBC SECTION 17. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR SHALL SEND REPORTS TO THE OWNER, THE BUILDING OFFICAL, THE ARCHITECT, THE ENGINEER AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING THAT THE SPECIAL INSPECTION WORK WAS, TO THE BEST OF HIS KNOWLEDGE, IN CONFORMANCE WITH THE PLANS, SPECIFICATIONS AND APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC.
4. SPECIAL INSPECTIONS SHALL BE IN ACCORDANCE WITH THE IBC AND AS DIRECTED BY THE BUILDING OFFICIAL. THE OWNER SHALL BE RESPONSIBLE FOR EMPLOYING SPECIAL INSPECTORS WHO MEET THE QUALIFICATIONS STATED IN THE IBC.
5. SPECIAL INSPECTIONS FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360 AND THE QUALITY ASSURANCE REQUIREMENTS OF THE AISC 341. TESTING FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341. SPECIAL INSPECTION FOR STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH SECTION 1705.2.2 OF THE IBS.
6. SPECIAL INSPECTIONS REQUIRED BY SECTION 1705 ARE NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND PEROIDIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATION OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:
Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT, 84404

SPECIAL INSPECTION NOTES

Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. SS004

Tekla Structures

FOUNDATION WALL SCHEDULE						
MARK	WIDTH (IN.)	MAX HEIGHT WALL (HW)	REINFORCEMENT			
			VERTICAL		HORIZONAL	
			SIZE	SPACE	SIZE	SPACE
FW 8	8''	3'-6''	#5	18''	#5	18''

FOUNDATION WALL NOTES:

1) SEE FOOTING AND FOUNDATION PLAN FOR FOUNDATION WALL CALLOUTS.
2) PLACE TWO HORIZONTAL BARS IN TOP AND BOTTOM OF EACH CONCRETE WALL.
3) SEE FOOTING AND FOUNDATION PLAN FOR TOP OF FOUNDATION WALL ELEVATION.

SQUARE FOOTING SCHEDULE					
				REINFORCEMENT (BARS)	
MARK	WIDTH	LENGTH	THICK	TRANS	LENGTH
F6.5	8'-6''	8'-6''	12''	(8) #6	(8) #6

FOOTING NOTES:

1) SEE FOOTING AND FOUNDATION PLAN FOR FOOTING MARKS
2) RUN CONTINUOUS FOOTING REINFORCEMENT THROUGH SPOT FOOTINGS.
3) PLACE ALL FOOTING REINFORCEMENTS IN BOTTOM OF FOOTING WITH 3" OF CLEAR UNLESS NOTED OTHERWISE.
4) IF FOOTINGS ARE EARTHFORMED, FOOTINGS SHALL BE EXTENDED 6" IN EACH DIRECTION FROM WHAT IS NOTED IN SCHEDULE.
5) SEE FOOTING AND FOUNDATION PLAN FOR TOP OF SQUARE FOOTING ELEVATION.

CONTINUOUS FOOTING SCHEDULE					
MARK	WIDTH	LENGTH	THICK	TRANS	LENGTH
CF	2'-0''	CONT.	12''	#5 @ 15'' O.C.	(2) #5 BAR

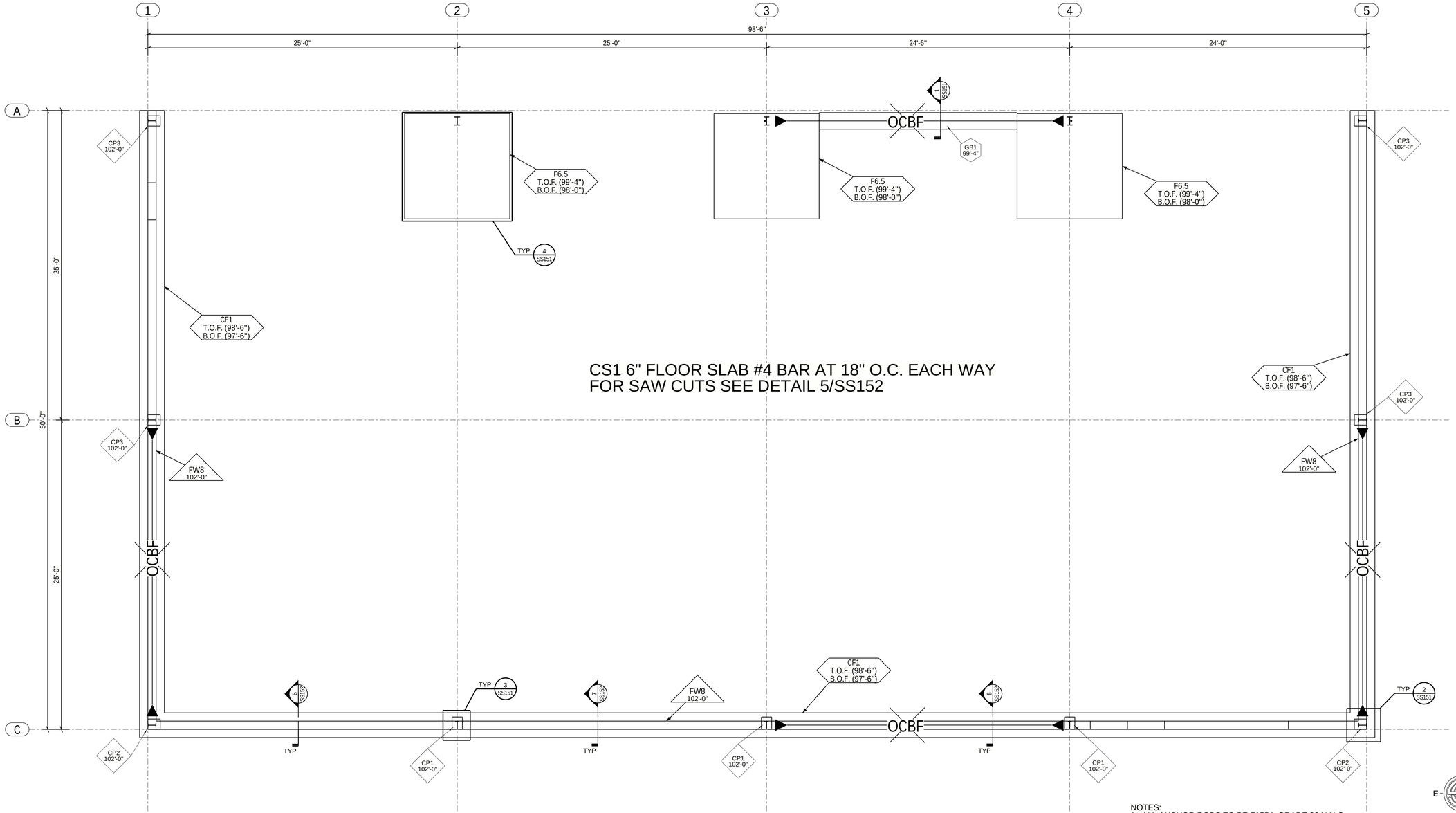
CONTINUOUS FOOTING NOTES:

1) SEE FOOTING AND FOUNDATION PLAN FOR FOOTING MARKS
2) RUN CONTINUOUS FOOTING REINFORCEMENT THROUGH SPOT FOOTINGS.
3) PLACE ALL FOOTING REINFORCEMENTS IN BOTTOM OF FOOTING WITH 3" OF CLEAR UNLESS NOTED OTHERWISE.
4) IF FOOTINGS ARE EARTHFORMED, FOOTINGS SHALL BE EXTENDED 6" IN EACH DIRECTION FROM WHAT IS NOTED IN SCHEDULE.
5) SEE FOOTING AND FOUNDATION PLAN FOR TOP OF CONTINUOUS FOOTING ELEVATION

PIER SCHEDULE					
MARK	CONFIG	LENGTH	WIDTH	VERTICAL	SHEAR TIES/SPACING
CP1	A	12''	10''	(4) #5 BAR	#3 BAR @ 8'' O.C.
CP2	B	12''	10''	(4) #5 BAR	#3 BAR @ 8'' O.C.
CP2	C	12''	10''	(4) #5 BAR	#3 BAR @ 8'' O.C.

PIER NOTES:

1) SEE FOOTING AND FOUNDATION PLAN FOR FOOTING MARKS
2) PIER VERTICAL REINFORCEMENT TO BE PLACED UNIFORMLY AROUND PERIMETER WITH 2" CLEAR FROM OUTSIDE FACE. (SEE PIER CONFIGURATIONS ON SS151)
3) SEE FOOTING AND FOUNDATION PLAN FOR TOP OF PIER ELEVATION.
4) SEE SHEET SS252 FOR ANCHOR EMBEDMENT



8" FOUNDATION WALL TOP OF WALL (102'-0")
BOTTOM OF WALL (98'-6")

NOTES:
1. ALL ANCHOR RODS TO BE F1554, GRADE 36 U.N.O.
2. FINISH FLOOR SLAB ELEVATION 100'-0"
3. TOP OF STEM WALL ELEVATION 102'-0"

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT, 84404

FOUNDATION PLAN

Mark	Description	Date	By
A	For Approval	02/14/2020	SB
B	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. SS101

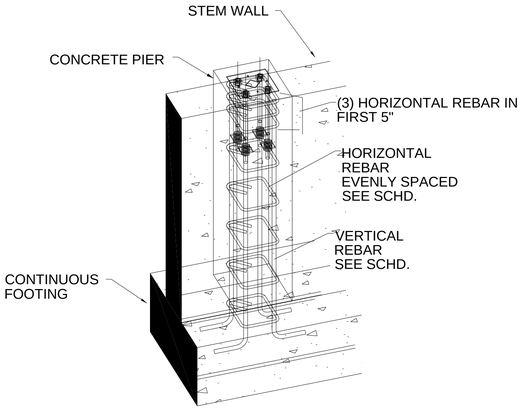
Tekla Structures

1) REFERENCE TABLES ON SHEET SS101 FOR REBAR SIZE AND QUANTITY REQUIREMENTS.

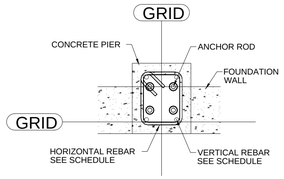
2) VERTICAL PIER REINFORCEMENT TO BE UNIFORMLY SPACED AROUND PERIMETER. 2" CLEAR FROM OUTSIDE FACE OF PIER.

3) TOP OF PIER ELEVATION IS 102'-0" EXCEPT WHERE NOTED ON PLAN.

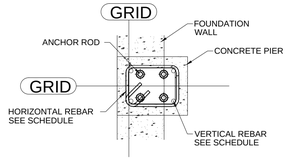
4) FOR ANCHOR ROD EMBEDMENT, SEE SS252



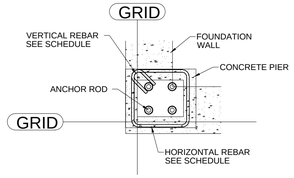
3 SS151 PIER REINFORCEMENT



CONFIGURATION A



CONFIGURATION C



CONFIGURATION B

GRADE BEAM SCHEDULE											
			BOTTOM REINF.			TOP REINF.			STIRRUP REINF.		
MARK	WIDTH	MIN. DEPTH (d)	QTY	SIZE	SPACING	QTY	SIZE	SPACING	QTY	SIZE	SPACING
GB1	16"	16"	(3)	#6 BAR	–	(3)	#6 BAR	–	–	#4 BAR	12" O.C.

1) FIRST VERTICAL BAR TO BE WITHIN 8" OF END OF BEAM

2) HORIZONTAL WALL REINFORCEMENT TO RUN CONTINUOUS THROUGH CONCRETE BEAM. WHERE REINFORCEMENT OCCURS IN THE SAME LOCATION, USE LARGER BAR DIAMETER

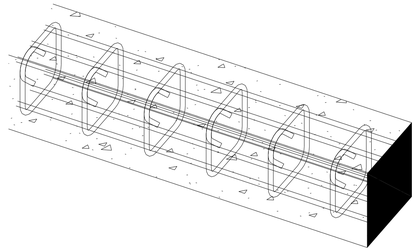
3) HOOK SHEAR TIES AROUND LONG REINFORCEMENT.

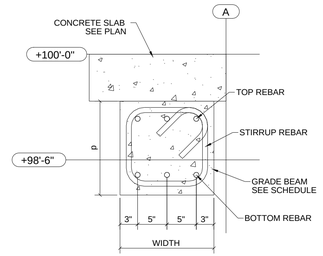
4) DO NOT SPLICE HORIZONTAL REINFORCEMENT WITHIN OPENING.

5) SHEAR REINFORCEMENT (STIRRUP/TIES) SHALL BE SINGLE BAR (IE. NOT SPLICED) W/ 180 DEGREE HOOK AT EACH END.

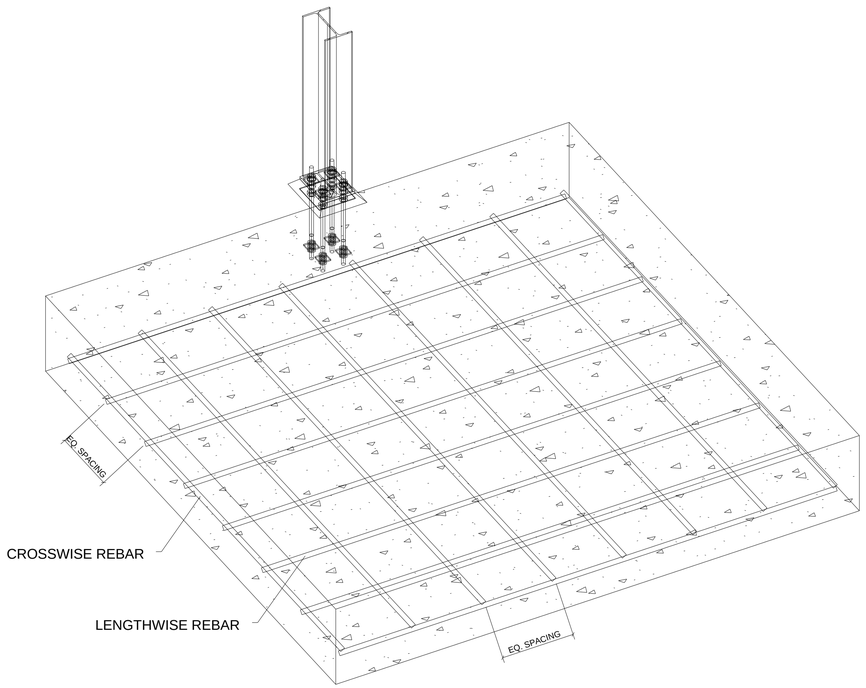
6) SEE CONCRETE EMBED SCHEDULE FOR REQUIRED "Ld" (2'-6" MIN)

7) FOR REINFORCEMENT CLEAR COVER SEE GNS.





1 SS151 GRADE BEAM

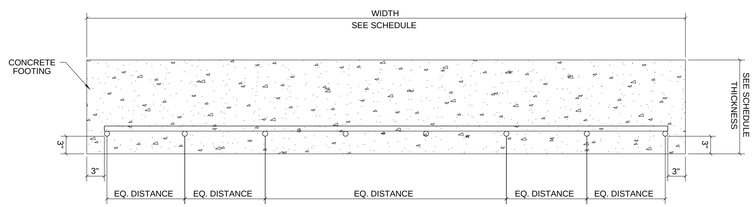


4 SS151 FOOTING REINFORCEMENT

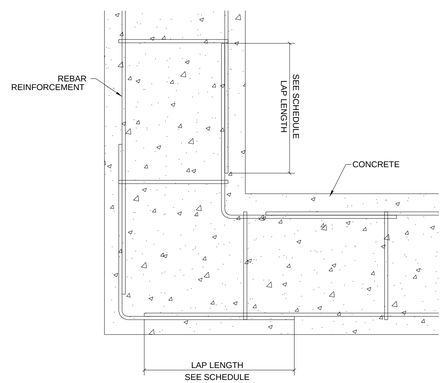
1) REFERENCE TABLES ON SHEET SS101 FOR REBAR SIZE AND QUANTITY REQUIREMENTS.

2) ALLOW 3" OF CLEAR SPACING ON ALL SIDES FOR REINFORCEMENT.

3) REINFORCEMENT TO BE INSTALLED WITH 3" CLEAR SPACING FROM BOTTOM OF FOOTING.



1) SEE SCHEDULE FOR LAP REQUIREMENT
2) VERTICAL BARS NOT SHOWN
3) THIS DETAIL IS APPLICABLE AT ALL FOOTINGS AND FOUNDATION WALLS.



2 SS151 CORNER REINF. TRANSITION

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:
Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

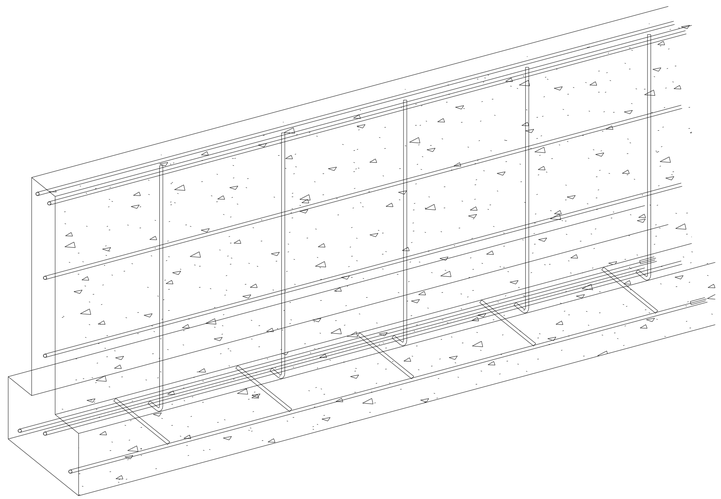
FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT, 84404

FOUNDATION DETAILS

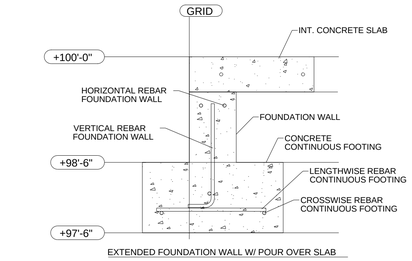
Mark	Description	Date	By
A	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. SS151

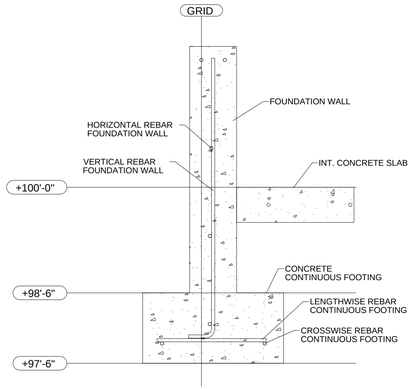
Tekla Structures



7 SS152 CONCRETE WALL AND STRIP FOOTING REINF

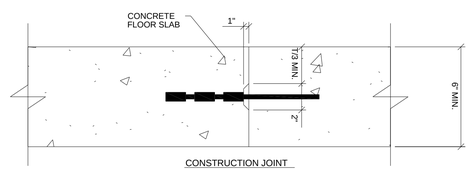


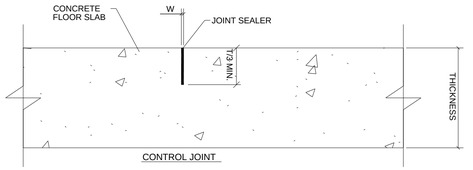
EXTENDED FOUNDATION WALL W/ POUR OVER SLAB

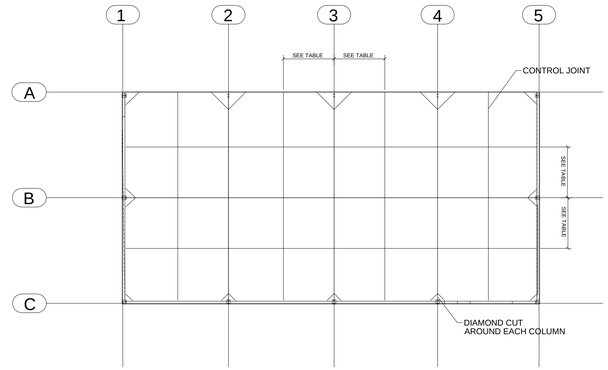


CONCRETE SLAB-ON-GRADE SCHEDULE							
			LONG.		TRANS.		
MARK	Ts (IN.)	Ts / 4	SIZE	SPACING	SIZE	SPACING	MAX JOINT SPACING
CS1	6"	1 1/2"	#4 BAR	18"	#4 BAR	18"	12'-0"

1) THICKENING OF THE SLAB IS NOT REQUIRED FOR SLABS WITH Ts = 6" OR GREATER. SEE PLAN FOR 'T' AND SLAB REINFORCEMENT SIZE AND SPACING.
2) COORDINATE JOINT LOCATIONS WITH ARCHITECT.
3) FILL ALL INTERIOR CONTROL JOINTS WITH SEALER AFTER 28 DAYS FOLLOWING CONCRETE PLACEMENT.
4) SLAB SHALL BE UNDERLAIN BY FREE DRAINING MATERIAL. SEE GEOTECH REPORT WHERE APPLICABLE.
5) REINFORCEMENT TO RUN CONTINUOUSLY THROUGH COLD JOINTS AS SHOWN.
6) SLAB ON GRADE SHALL BE PER SCHED. SLAB SHALL BE UNDERLAIN BY FREE DRAINING MATERIAL AS PRESCRIBED IN THE SOILS REPORT (WHERE APPLICABLE).



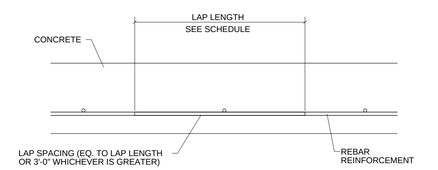


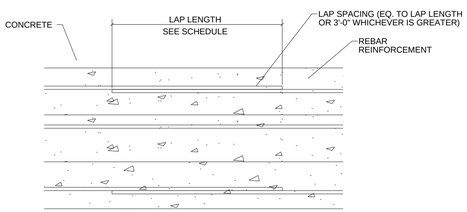


SAW CUT PLAN
SEE TABLE

5 SS152 CONCRETE SLAB REINFORCEMENT

CONCRETE REINFORCING BAR DEVELOPMENT & LAP SCHEDULE																		
	fc = 3000 psi				fc = 4000 psi				fc = 5000 psi				fc = 6000 psi				fc = ALL	
BAR SIZE	Ld	Lt	Ls	Lst	Ld	Lt	Ls	Lst	Ld	Lt	Ls	Lst	Ld	Lt	Ls	Lst	Ldc	Lsc
#4	24"	31"	32"	41"	21"	27"	28"	36"	19"	24"	25"	32"	17"	22"	23"	29"	11"	15"
#5	30"	39"	39"	51"	26"	34"	34"	45"	23"	30"	30"	39"	21"	28"	28"	37"	14"	19"
#6	36"	47"	47"	62"	31"	40"	41"	52"	28"	36"	37"	47"	26"	33"	34"	43"	17"	23"
#7	42"	54"	55"	71"	36"	47"	47"	62"	33"	42"	43"	55"	30"	39"	39"	51"	20"	27"
#8	48"	62"	63"	81"	41"	54"	54"	71"	37"	48"	49"	63"	34"	44"	45"	58"	22"	30"
#9	54"	70"	71"	91"	47"	61"	62"	80"	42"	54"	55"	71"	38"	50"	50"	65"	25"	34"
#10	61"	79"	80"	103"	53"	68"	69"	89"	47"	61"	62"	80"	43"	56"	56"	73"	28"	39"
#11	67"	87"	88"	114"	58"	76"	76"	99"	52"	68"	68"	89"	48"	62"	63"	81"	31"	43"

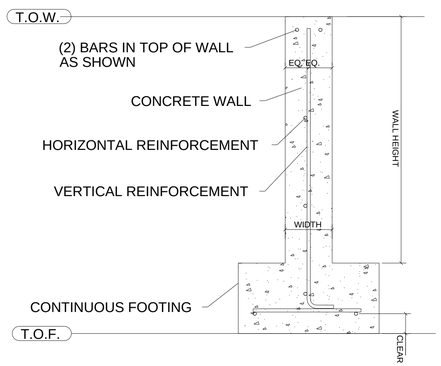


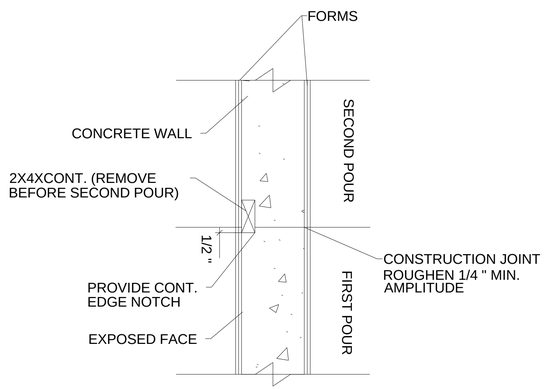


1) DEFINITIONS
Ld: TENSION DEVELOPMENT LENGTH SATISFYING THE FOLLOWING:
SLABS & WALLS: CLEAR SPACING >2*db AND CLEAR COVER > db
BEAMS & COLUMNS: CLEAR COVER SPACING AND CONCRETE CLEAR COVER > db
Lt: TENSION DEVELOPMENT LENGTH FOR TOP BARS. TOP BARS ARE ANY HORIZONTAL REINFORCEMENT WITH MORE THAN 12"OF CONCRETE CAST BELOW (EXAMPLE: MOST WALL REINFORCEMENT)
Ls: TENSION LAP SPLICE LENGTH FOR CLASS B BARS. THIS INCLUDES ALL OTHER BARS THAT ARE NOT CONSIDERED TOP BARS AND HAVE LESS THAN 12" OF CONCRETE CAST BELOW (EXAMPLE: SLABS)
Lst: TENSION LAP SPLICE LENGTH OF TOP BARS.
Ldc: COMPRESSION DEVELOPMENT LENGTH
Lsc: COMPRESSION LAP SPLICE LENGTH FOR COLUMNS
db: NORMAL BAR DIAMETER (IN.)

2) DEVELOPMENT LENGTHS MUST BE INCREASED IF THERE ARE CASES WHERE THERE IS LESS COVER. FOR Ld. IF THE CLEAR SPACING REQUIREMENTS IN NOTE 1 ARE NOT MET, MULTIPLY TABULATED VALUES BY (1.5).

8 SS152 REINFORCEMENT LAP SPLICING AND DEVELOPMENT LENGTHS





6 SS152 CONCRETE WALL AND STRIP FOOTING REINF PLACEMENT

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:
Designed by: JB
Drawn by: SES
Checked by: JL

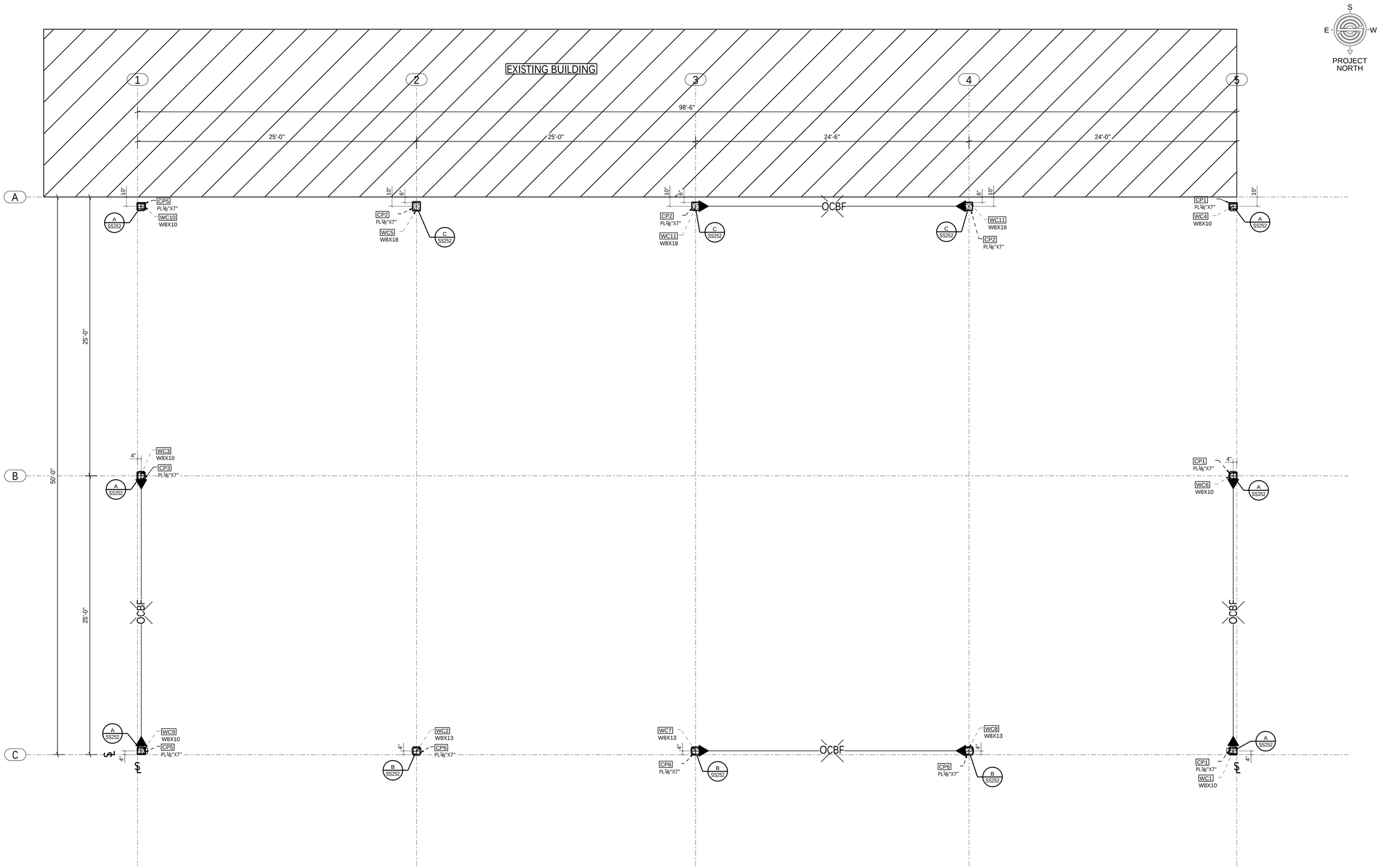
PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT. 84404

FOUNDATION DETAILS

Mark	Description	Date	By
A	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. SS152



NOTES:
1) ALL ANCHOR RODS TO BE F1554, GRADE 36, U.N.O.
2) FINISH FLOOR SLAB ELEVATION: 100'-0"
3) SEE SHEET SS252 FOR BASE PLATE ELEVATIONS AND DIMENSIONS AND ANCHOR ROD PATTERNS
4) TOP OF STEM WALL ELEVATION: 102'-0" (U.N.O.)
5) SFRS IS ORDINARY CONCENTRICALLY BRACED FRAMES R=3.25, ρ=1.3, Ω=2.0
6) FOR STANDARD CONCRETE DETAILS SEE SS151 - SS152

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER

2397 RULON WHITE BLVD
OGDEN UT. 84404

ANCHOR ROD PLAN

Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. **SS201**

SETTING ANCHOR RODS IN THE FOUNDATION (CONCRETE AND FOUNDATION SUB CONTRACTOR)

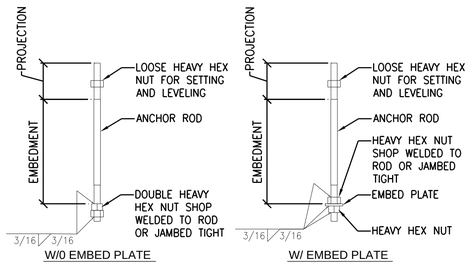
3/4" DIAMETER ANCHOR RODS IN A 4" BY 4" PATTERN SHOWN FOR REFERENCE. SEE ANCHOR ROD DETAILS FOR SIZE AND PATTERN SPECIFIC TO EACH LOCATION.

ANCHOR ROD PLACEMENT NOTES

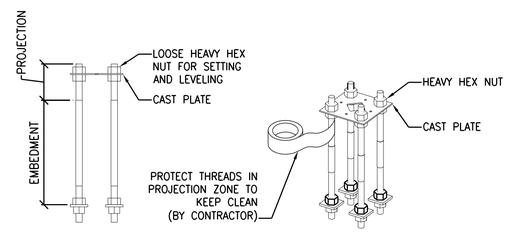
1) ANCHOR RODS IN CONCRETE TO BE PLACED PER AISC 303-16 SECTION 7.5.1 AND AISC DESIGN GUIDE 1 SECTION 2.8., SHOP FABRICATED 1/8" THICK STEEL TEMPLATES "CAST PLATE" MAY HAVE BEEN PROVIDED IN LIEU OF FIELD FABRICATED PLYWOOD TEMPLATES BY CUSTOMER REQUEST. (SEE CONTRACT)
   a. INDICATES; TOP OF ANCHOR ROD FROM SPECIFIED ELEVATION VERTICAL DEVIATION ±1/2IN.

** 1" DIFFERENCES IN ELEVATION MAY OCCUR. LEVELING BASE PLATES TO THE LOCATION WITH THE HIGHEST CONCRETE LEVEL WILL ENSURE THE MOST EFFECTIVE GROUTING AND SIMPLEST LEVELING.

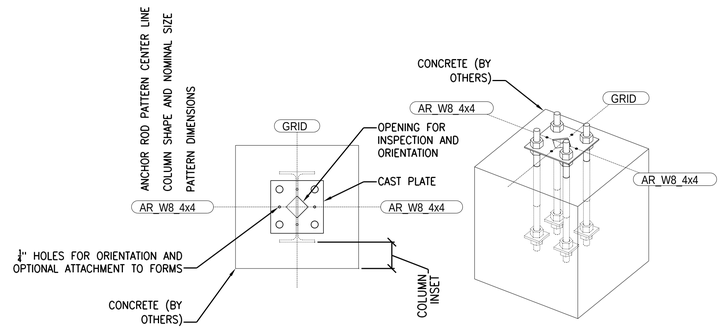
   b. INDICATES; CENTERLINE OF INDIVIDUAL ANCHOR BOLTS FROM SPECIFIED LOCATION HORIZONTAL DEVIATION 3/4- AND 7/8-IN.-DIAMETER BOLTS: ±1/4 IN; 1-, 1-1/4-, AND 1-1/2-IN. -DIAMETER BOLTS: ±1/2 IN; 1-3/4, 2-, AND 2-1/2-IN.-DIAMETER BOLTS: ±3/4 IN



ANCHOR ROD ASSEMBLY
STEP 1



CAST PLATE ASSEMBLY / GROUP TEMPLATE
STEP 2



ANCHOR ROD PLACEMENT
STEP 3

SETTING ELEVATION AND LEVELING (GENERAL CONTRACTOR)

3/4" DIAMETER ANCHOR RODS IN A 4" BY 4" PATTERN SHOWN U.N.O. FOR REFERENCE. SEE ANCHOR ROD DETAILS FOR SIZE AND PATTERN SPECIFIC TO EACH LOCATION.

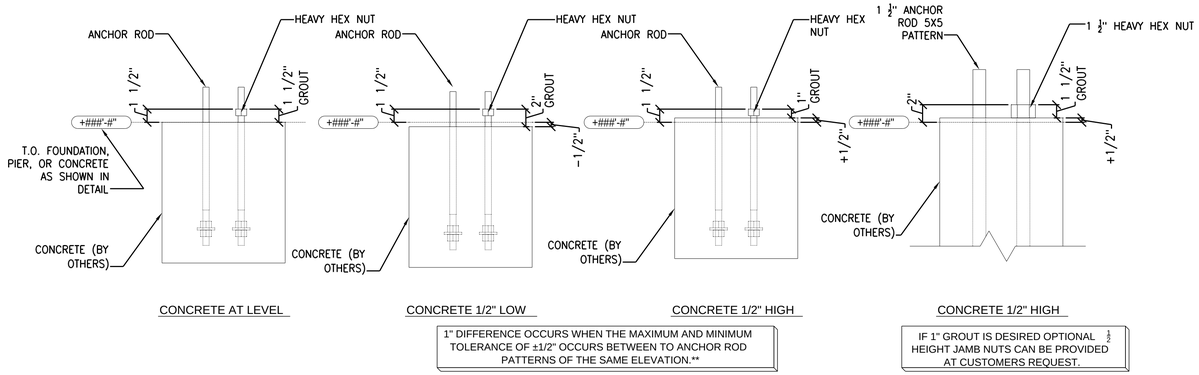
ANCHOR ROD INSTALLATION NOTES

1) COMMENCEMENT OF ERECTION WORK DOES NOT CONSTITUTE ACCEPTANCE OF THE SITE, CONCRETE, OR ANCHOR BOLT PLACEMENT. ANCHOR RODS SHALL BE STRAIGHT, CLEAN, AND FREE OF THREAD DAMAGE.
2) ACCURACY OF BOLT LOCATIONS IS HIGHLY CRITICAL TO AN EXPEDITED ERECTION. COSTLY DELAYS WILL OCCUR IF AN ERROR IN SQUARENESS, DIMENSIONS, OR ELEVATION PROHIBITS THE ERECTOR FROM MAINTAINING REQUIRED TOLERANCE. ANY DEVIATION FROM PLANS, DETAILS, NOTES, AND/OR TOLERANCES MUST BE COMMUNICATED TO THE METAL BUILDING SUPPLIER.
3) STEEL TEMPLATES "CAST PLATE" MUST BE REMOVED BEFORE LEVELING NUTS ARE SET.
4) (1) LEVELING NUT PER BOLT PATTERN MUST BE LOCATED AT BOTTOM OF BASE PLATE ELEVATION AND SPRAY PAINTED TO ESTABLISH PROPER GROUT THICKNESS AND ELEVATION PRIOR TO SETTING COLUMNS.

** 1" DIFFERENCES IN ELEVATION MAY OCCUR. LEVELING BASE PLATES TO THE LOCATION WITH THE HIGHEST CONCRETE LEVEL WILL ENSURE THE MOST EFFECTIVE GROUTING AND SIMPLEST LEVELING.

5) BASE PLATES DESIGNED TO BE PLACED AT THE TOP OF 1 ½" GROUT U.N.O... SEE NOTED GROUT THICKNESS FOR ANCHOR RODS GREATER THAN 1 ½" DIAMETER.

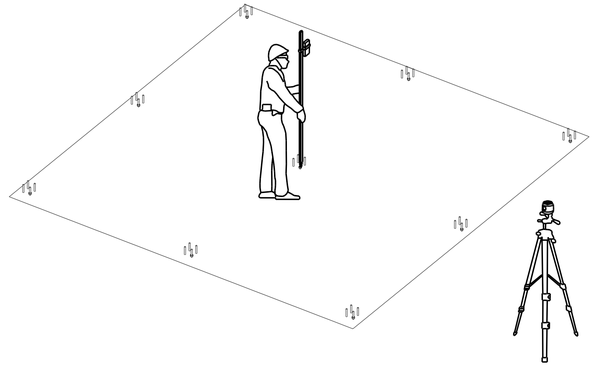
6) BASE PLATE LEVELING AND/OR SHIMMING TO ±1/2 IN. OF DESIGNED GROUT IS ALLOWED.



1" DIFFERENCE OCCURS WHEN THE MAXIMUM AND MINIMUM TOLERANCE OF ±1/2" OCCURS BETWEEN TO ANCHOR ROD PATTERNS OF THE SAME ELEVATION.**

IF 1" GROUT IS DESIRED OPTIONAL ½" HEIGHT JAMB NUTS CAN BE PROVIDED AT CUSTOMERS REQUEST.

HIGHEST FOUNDATION LOCATION **
STEP 1



LEVELING NUTS ON SAME ELEVATION ANCHOR ROD GROUPS
STEP 2 (LOCATE ONE LEVELING NUT PER BOLT PATTERN AT BOTTOM OF BASE PLATE ELEVATION ESTABLISHED AT HIGHEST LOCATION AND SPRAY PAINT.)

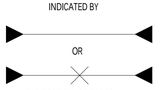
(CONDITIONS SHOWN ARE WITHIN TOLERANCES GIVEN BY AISC 303-16 SECTION 7.5.1 AND AISC DESIGN GUIDE 1 SECTION 2.8.) (LOCATE ONE LEVELING NUT FOR THIE HIGHEST BOLT PATTERN AT BOTTOM OF BASE PLATE ELEVATION AND SPRAY PAINT.)

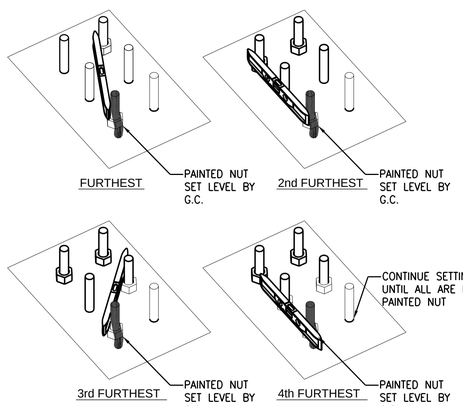
SETTING COLUMNS (STEEL ERECTION SUB CONTRACTOR)

WIDE FLANGE COLUMN AND 5X5X5 PATTERN SHOWN FOR REFERENCE. SEE ROOF FRAMING PLAN, ANCHOR ROD PLAN, AND DETAILS FOR COLUMN TYPE AND SIZE SPECIFIC TO EACH LOCATION.

COLUMN ERECTION NOTES

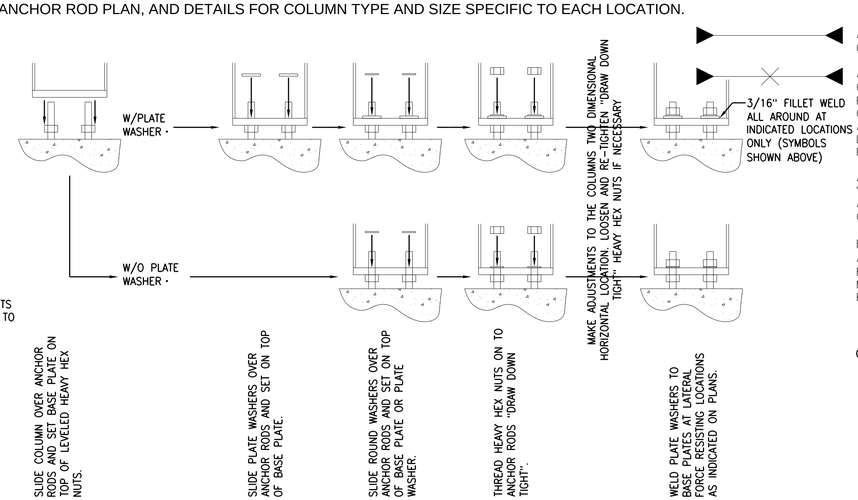
1) VERIFY THAT THE DRAWINGS IN POSSESSION ARE FOR CONSTRUCTION AND HAVE THE MOST CURRENT REVISIONS
2) VERIFY ELEVATION NUT IS SET AND PAINTED BY THE GENERAL CONTRACTOR AT ALL ANCHOR ROD PATTERNS BEFORE STAGING STEEL.
3) NON-SHRINKING GROUT MUST BE INSTALLED PRIOR TO ANY ADDITIONAL LOADING TO THE STEEL FRAMING.
4) PLATE WASHERS AT OVERSIZED BASE PLATE HOLES MUST BE WELDED AT ALL ANCHOR ROD GROUPINGS WITH LATERAL FORCE RESISTING SYSTEMS (LFRS) INDICATED. NON LATERAL FORCE RESISTING SYSTEMS DO NOT REQUIRE PLATE WASHERS TO BE WELDED AT ANCHOR ROD GROUPINGS. LATERAL FORCE RESISTING SYSTEM INDICATED BY





(SET PAINTED LEVELING NUT TO THE SAME POSITION AS WHEN LEVELED.) (LEVEL ALL NUTS IN PATTERN TO THE PAINTED LEVELING NUT.)

SETTING LEVELING NUTS IN ANCHOR ROD PATTERNS
STEP 1



STANDING COLUMNS
STEP 2

GROUTING COLUMNS (GENERAL CONTRACTOR)

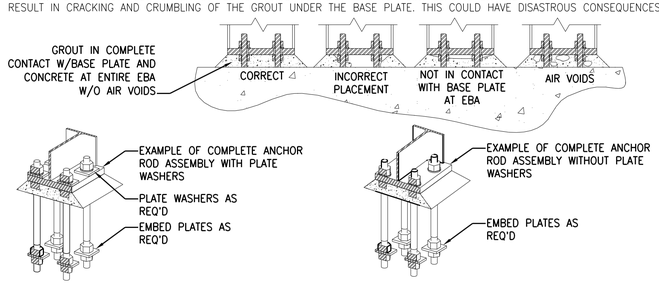
3/4" DIAMETER ANCHOR RODS IN A 4" BY 4" PATTERN SHOWN U.N.O. FOR REFERENCE.

ADJUST HEAVY HEX LEVELING NUTS AND LOOSENING/RE-TIGHTENING "DRAW DOWN TIGHT" HEAVY HEX ANCHOR NUTS TO PLUMB THE COLUMN WHILE INSTALLING BRACING ELEMENTS WILL HELP TO ENSURE PROPER SQUARENESS, DIMENSIONS, AND LOCATION.

AISC SECTION 16.3-40 PARAGRAPH 7.7.
GROUTING SHALL BE THE RESPONSIBILITY OF THE OWNER'S DESIGNATED REPRESENTATIVE FOR CONSTRUCTION (G.C.). LEVELING PLATES AND LOOSE BASE AND BEARING PLATES SHALL BE PROMPTLY GROUTED AFTER THEY ARE SET AND CHECKED FOR LINE AND GRADE. COLUMNS WITH ATTACHED BASE PLATES, BEAMS WITH ATTACHED BEARING PLATES AND OTHER SIMILAR MEMBERS WITH ATTACHED BEARING DEVICES THAT ARE TEMPORALLY SUPPORTED ON LEVELING NUTS AND WASHERS, SHIMS OR OTHER SIMILAR LEVELING DEVICES, SHALL BE PROMPTLY GROUTED AFTER THE STRUCTURAL STEEL FRAME OR PORTION THEREOF HAS BEEN PLUMBED.

AISC SECTION 14-7
"LEVELING BOLTS OR NUTS SHOULD NOT BE USED TO SUPPORT THE COLUMN DURING ERECTION. IF GROUTING IS DELAYED UNTIL AFTER STEEL ERECTION, THE BASE PLATE MUST BE SHIMMED TO PROPERLY DISTRIBUTE LOADS TO THE FOUNDATION WITHOUT OVERSTRESSING EITHER THE BASE PLATE, [LEVELING NUTS], OR THE CONCRETE."

EFFECTIVE BEARING AREA (EBA) IS DEFINED BY THE AREA DIRECTLY UNDER THE COLUMN BASE PLATE. THE EBA MUST MATCH THE AREA OF THE BASE PLATE IN ORDER TO PROPERLY TRANSMIT LOAD FROM THE COLUMN TO THE FOUNDATION/PIER. THE EBA CAN BE AFFECTED BY GROUT LEAKAGE, UNLEVELED PLATE, SHRINKAGE OF GROUT, INADEQUATE MIXING OF GROUT, INCORRECT PLACEMENT METHOD, POOR GROUT FLOW OR ENTRAPPED AIR VOIDS. EBA CAN LEAD TO POINT LOADING ON THE GROUT WHICH IN TURN CAN RESULT IN CRACKING AND CRUMBLING OF THE GROUT UNDER THE BASE PLATE. THIS COULD HAVE DISASTROUS CONSEQUENCES.



PLUMBING AND GROUTING COLUMNS
STEP 3

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:
Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

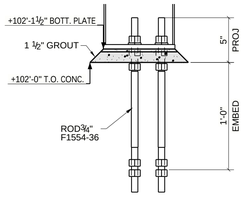
FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT. 84404

ANCHOR ROD NOTES

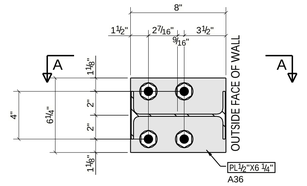
Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. SS251

Tekla Structures

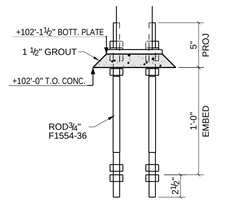


A - A

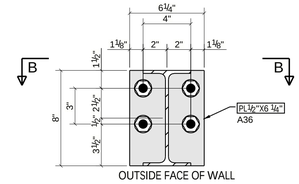


A SS252 ANCHOR ROD PATTERN

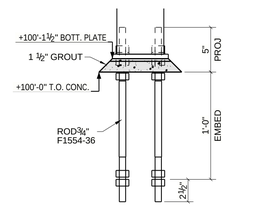
COLUMN ORIENTATION MAY VARY



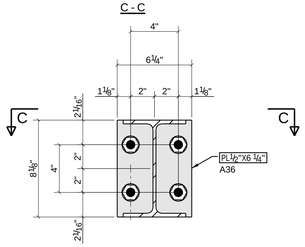
B - B



B SS252 ANCHOR ROD PATTERN



C - C



C SS252 ANCHOR ROD PATTERN

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB

Drawn by: SES

Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER

2397 RULON WHITE BLVD
OGDEN UT. 84404

ANCHOR ROD DETAILS

Mark	Description	Date	By
A	For Approval	02\14\2020	SES
B	For Permit	03\24\2020	SES

DATE 03/24/2020

PROJ. NO. 20053

DRAWING NO. **SS252**

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

S
E W
PROJECT NORTH

EXISTING BUILDING

1 2 3 4 5

98'-6"
25'-0" 25'-0" 24'-6" 24'-0"

A B C

50'-0"
25'-0" 25'-0"

WC10 W8X10 · WC5 W8X18 · WC11 W8X18 · GRADE BEAM 16"X16" · WC11 W8X18 · WC4 W8X10
WC3 W8X10 · WC6 W8X10
WC9 W8X10 · WC2 W8X13 · WC7 W8X13 · WC8 W8X13 · WC1 W8X10

PZ4 CFS 8X2.5Z12 · PZ13 CFS 8X2.5Z14 · PZ14 CFS 8X2.5Z14 · PZ7 CFS 8X2.5Z12
PZ4 CFS 8X2.5Z12 · PZ5 CFS 8X2.5Z14 · PZ6 CFS 8X2.5Z14 · PZ7 CFS 8X2.5Z12
PZ4 CFS 8X2.5Z12 · PZ11 CFS 8X2.5Z14 · PZ12 CFS 8X2.5Z14 · PZ7 CFS 8X2.5Z12
PZ1 CFS 8X2.5Z12 · PZ2 CFS 8X2.5Z14 · PZ3 CFS 8X2.5Z14 · PZ8 CFS 8X2.5Z12
PZ1 CFS 8X2.5Z12 · PZ9 CFS 8X2.5Z14 · PZ10 CFS 8X2.5Z14 · PZ8 CFS 8X2.5Z12

FB1 L2X2X3/16 · FB2 L2X2X3/16 · FB3 L2X2X3/16 · FB4 L2X2X3/16

WB4 W12X14 · WB5 W12X14 · WB2 W12X14 · WB1 W12X14
WB3 W21X48 · WB9 W21X48 · WB7 W21X48

OCBF · OCBF · OCBF

CB11 CABLE 3/8" · CB11 CABLE 3/8" · CB11 CABLE 3/8" · CB11 CABLE 3/8" · CB11 CABLE 3/8" · CB11 CABLE 3/8" · CB11 CABLE 3/8" · CB11 CABLE 3/8"

ES1 LES 8X5_5X.5D 12GA · ES4 LES 8X5_5X.5D 12GA · ES2 LES 8X5_5X.5D 12GA · ES3 LES 8X5_5X.5D 12GA

TYP 17 SS506 · TYP 18 SS506 · TYP 13 SS506 · 16 SS506 TYP · TYP 5 SS500 · TYP 7 SS500 · TYP 14 SS506 · TYP 15 SS506 · A SS301

1'-6" 4'-0" · 2'-0" 2'-0" · 4'-0" 1'-6"
PURLIN GAP

3'-2" · 1'-0 7/8" · 1'-0 3/4"
10' · 4'-2 1/4" · 5'-0" · 5'-0" · 5'-0" · 5'-0" · 5'-0" · 5'-0" · 4'-0"

ROOF NOTES:
1) ROOF SLOPE 1/2:12
2) UPLIFT BRACES AS SHOWN BY MARKS UB TO BE ANGLE 2x2x3/16
3) PURLINS ALIGNED WITH TOP FLANGE DIRECTED TOWARD HIGH EAVE

Tekla Structures

FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT, 84404

ROOF PLAN

Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

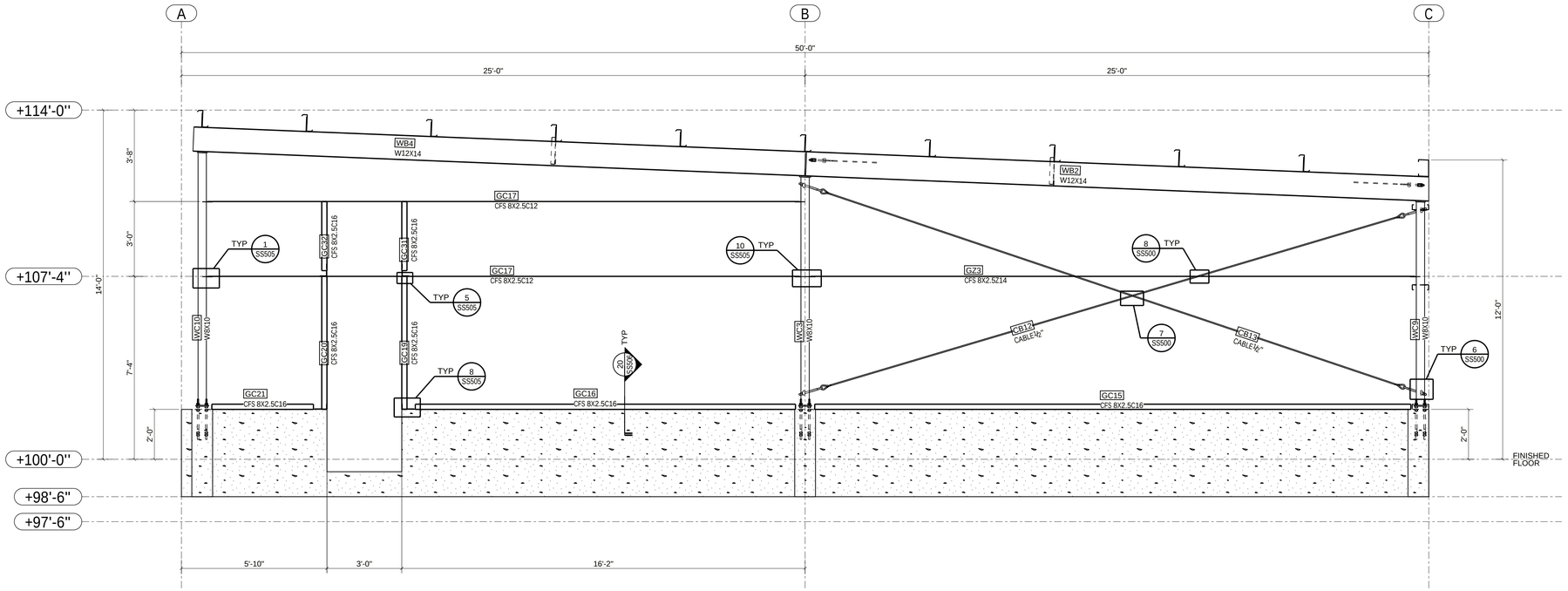
DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. SS301

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH



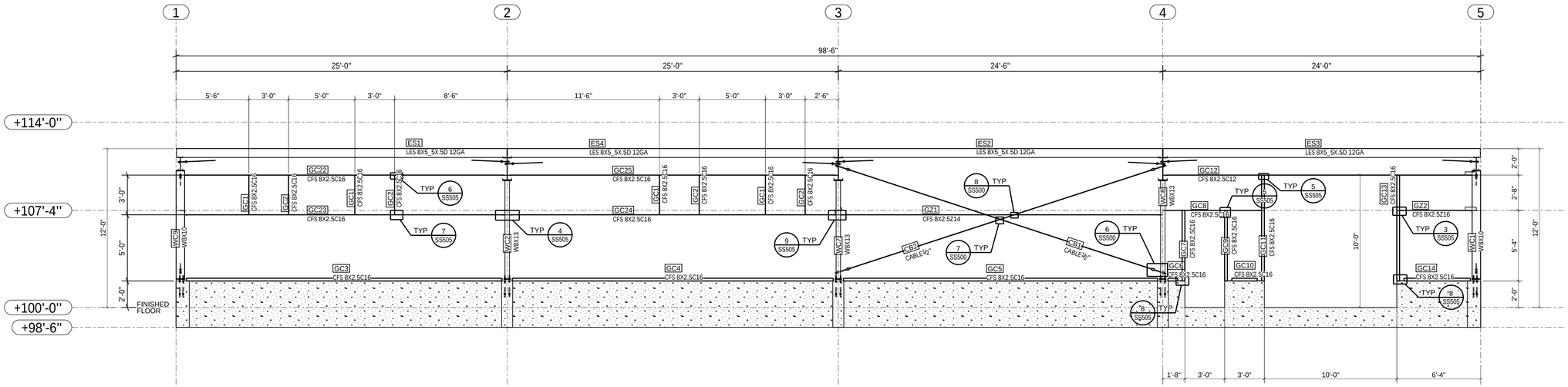
Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT, 84404

ELEVATION AT GRID 1

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. **SS302**

Todd Structures



STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER

2397 RULON WHITE BLVD
OGDEN UT. 84404

ELEVATION AT GRID A

Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. **SS303**

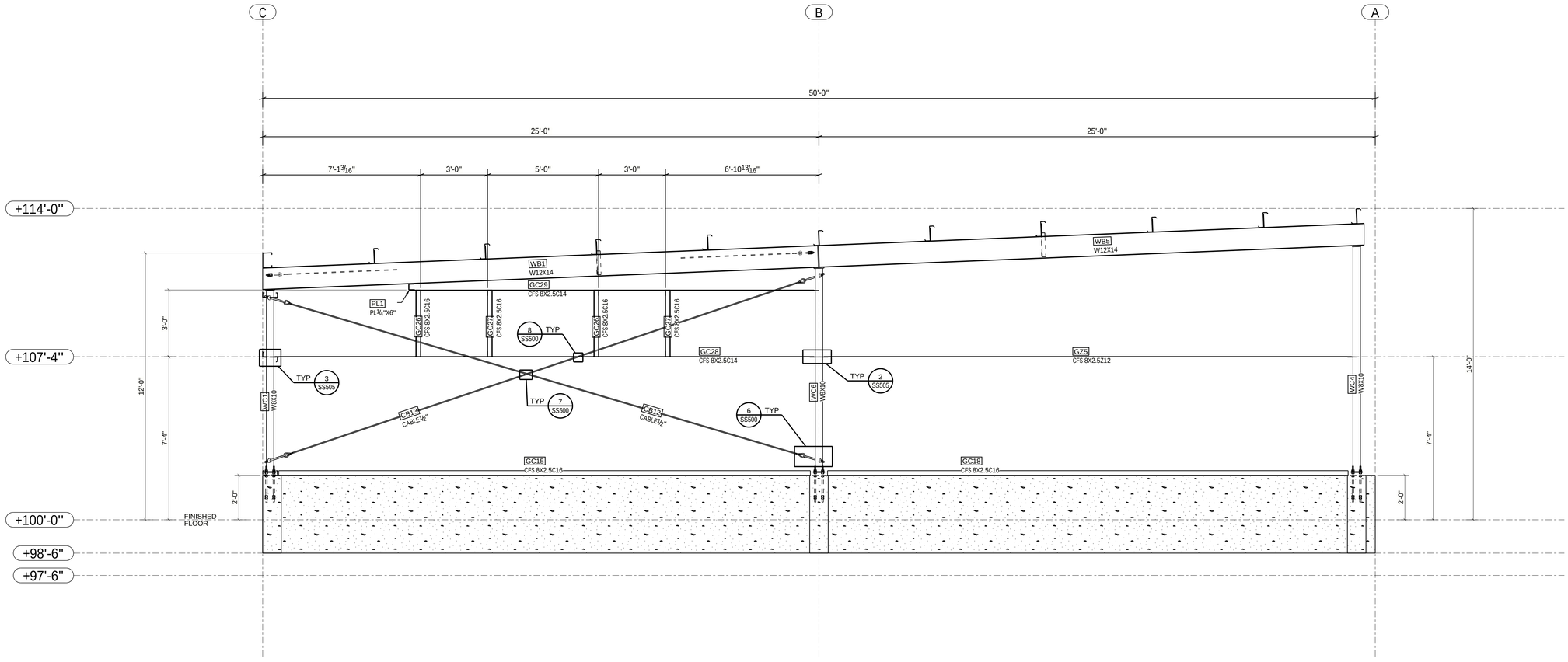
Tekla Structures

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH



Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

DATE 03/24/2020

PROJ. NO. 20053

DRAWING NO. SS304

FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT, 84404

ELEVATION AT GRID 5

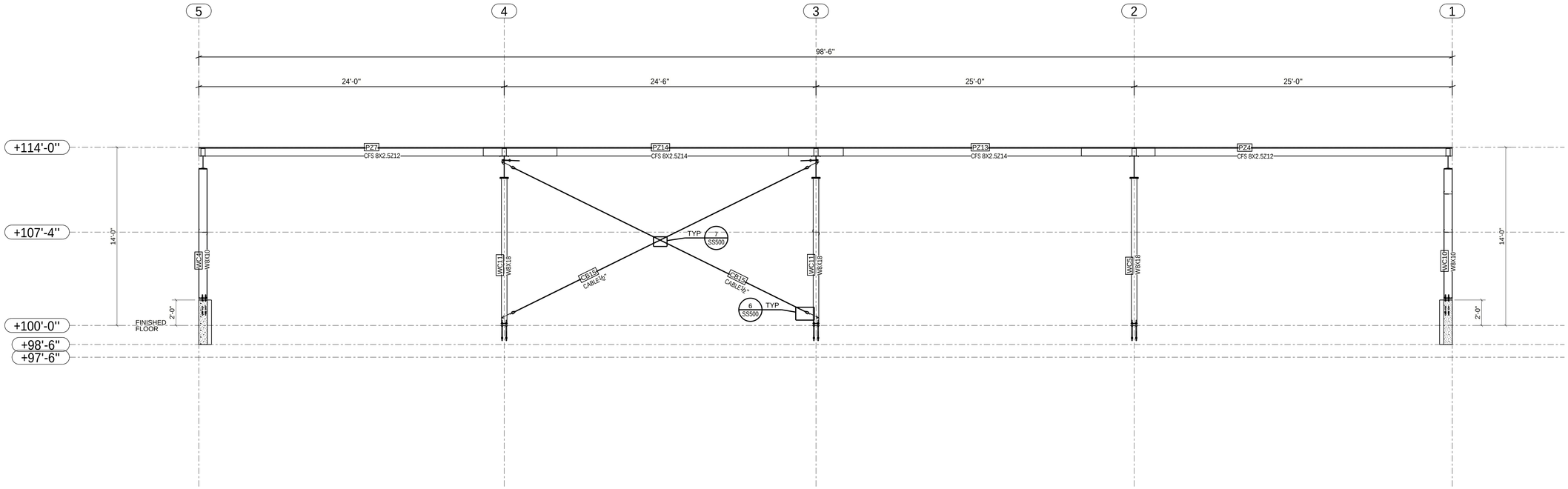
Tekla Structures

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH



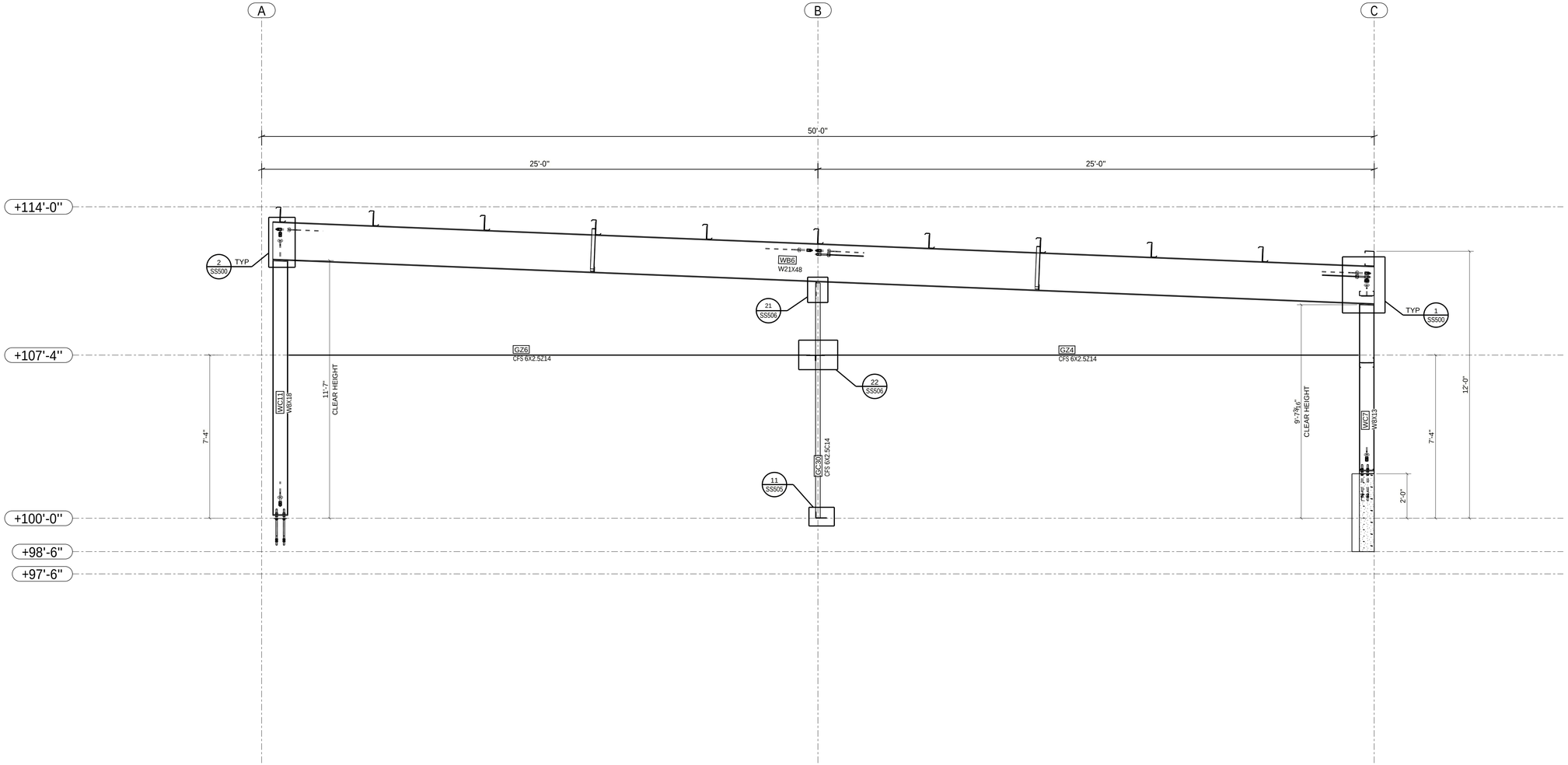
FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT, 84404

ELEVATION AT GRID C

Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. SS305

Trelis Structures



STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB

Drawn by: SES

Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER

2397 RULON WHITE BLVD
OGDEN UT. 84404

CROSS SECTION AT GRID 3

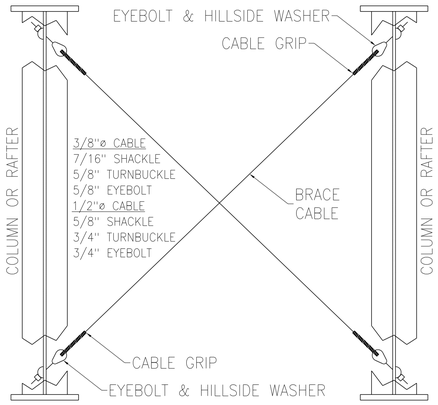
Mark	Description	Date	By
A	For Approval	02/13/2020	SES
B	For Permit	03/24/2020	SES

DATE 03/24/2020

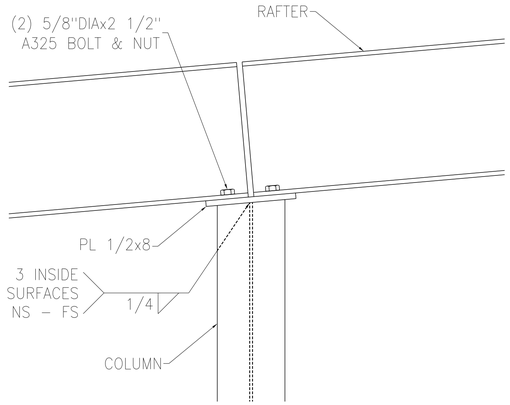
PROJ. NO. 20053

DRAWING NO. **SS401**

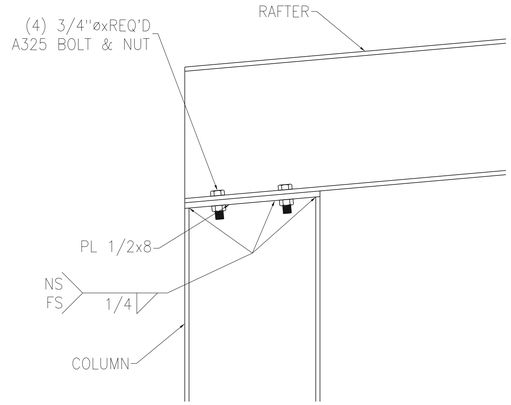
Tekla Structures



7 SS500 CABLE BRACING TO COLUMN OR RAFTER



4 SS500 RAFTER TO ROTATED COLUMN

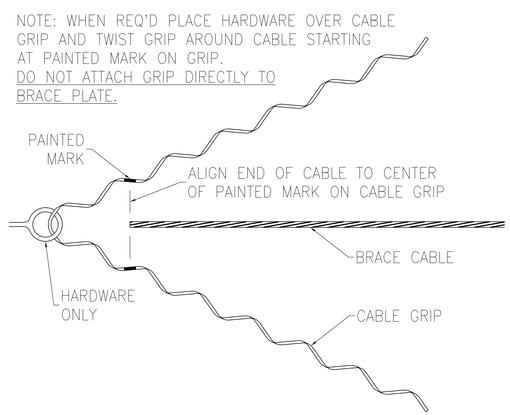


1 SS500 RAFTER TO COLUMN LOW EAVE

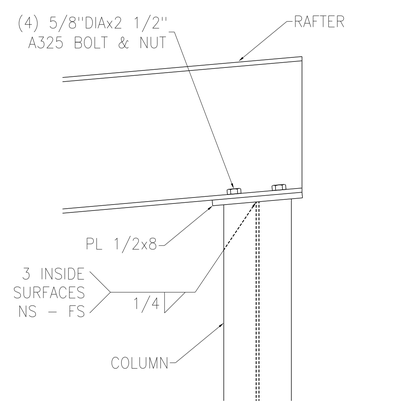
8 SS500 BRACE CABLE OR ROD THROUGH GIRT

5 SS500 RAFTER TO ROTATED COLUMN LOW EAVE

2 SS500 RAFTER TO COLUMN HIGH EAVE



6 SS500 CABLE GRIP TO BRACE CABLE



3 SS500 RAFTER TO ROTATED COLUMN HIGH EAVE

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT, 84404

PRIMARY FRAMING DETAILS

Mark	Description	Date	By
A	For Permit	03/24/2020	SES

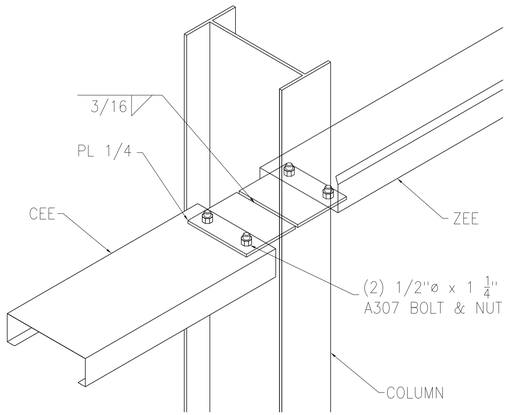
DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. **SS500**

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

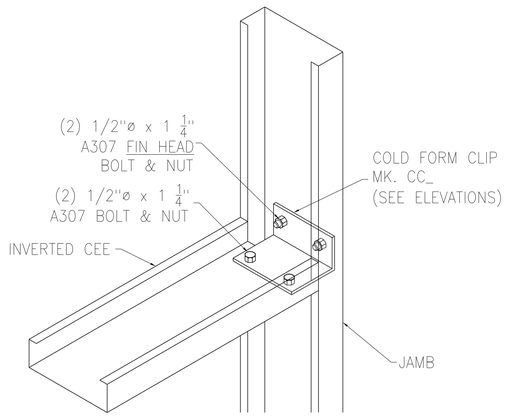
PREPARED BY:

Designed by: JB
Drawn by: SES
Checked by: JL

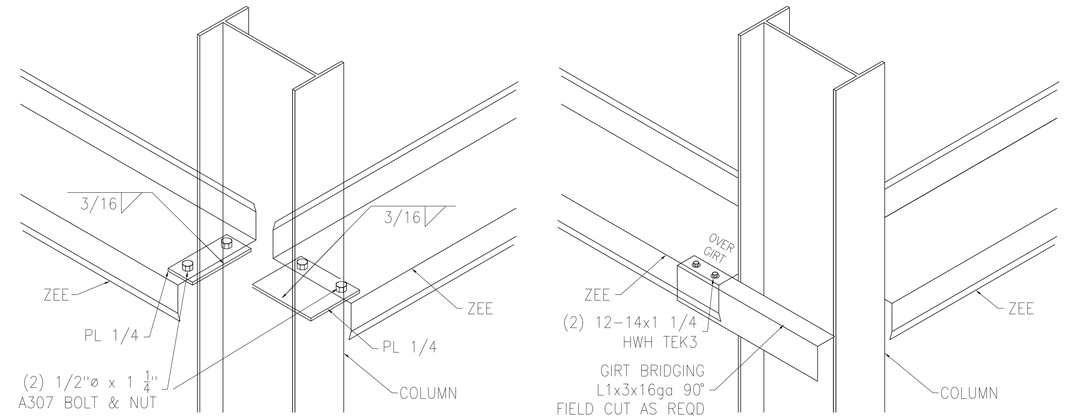
PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH



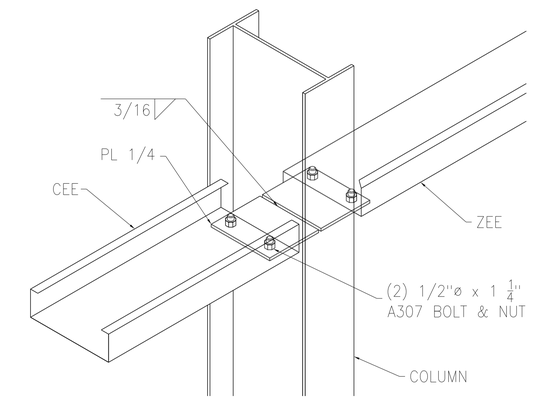
9 SS505 CEE AND INVERTED ZEE TO FLUSH COLUMN



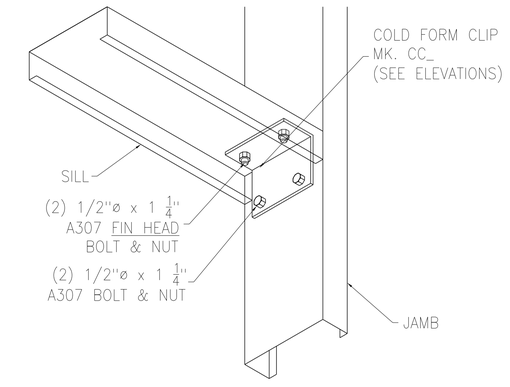
6 SS505 INVERTED CEE TO JAMB



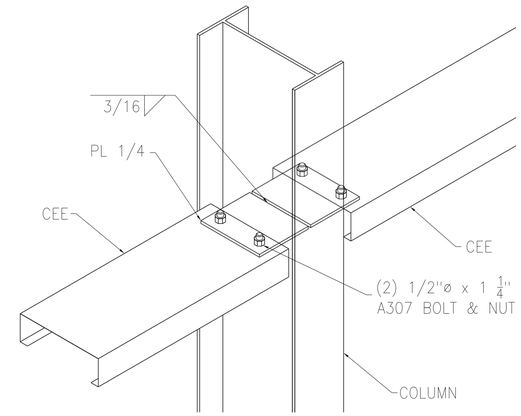
1 SS505 ZEE TO COLUMN



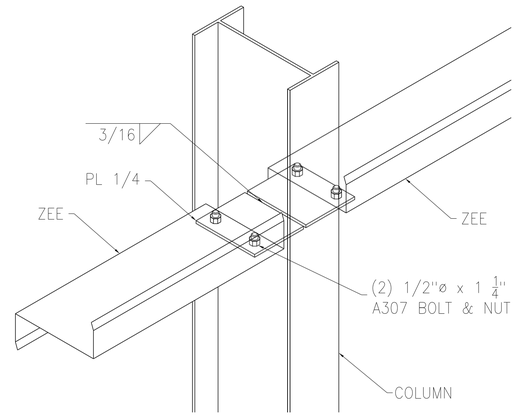
10 SS505 INVERTED CEE & INVERTED ZEE TO FLUSH COLUMN



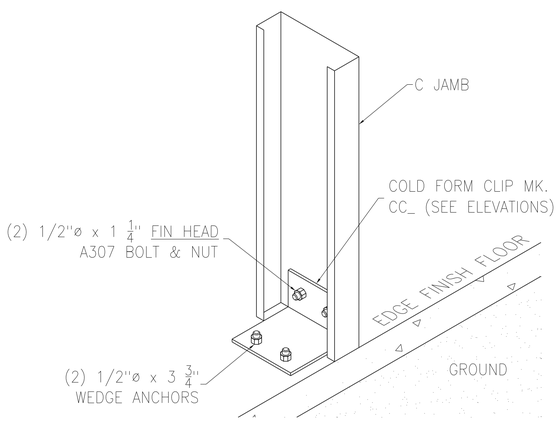
7 SS505 SILL TO JAMB



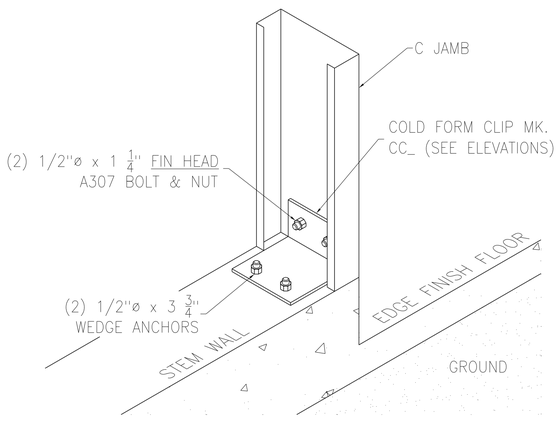
4 SS505 CEE TO FLUSH COLUMN



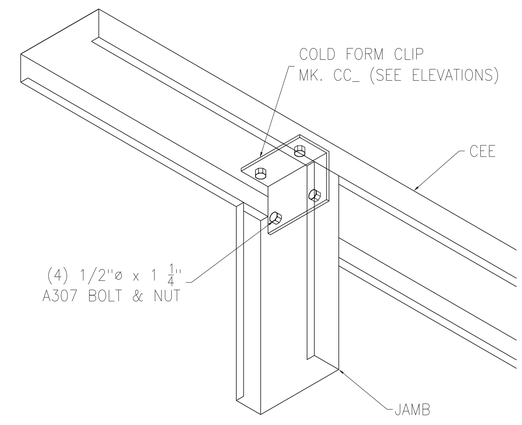
2 SS505 INVERTED ZEE TO FLUSH COLUMN



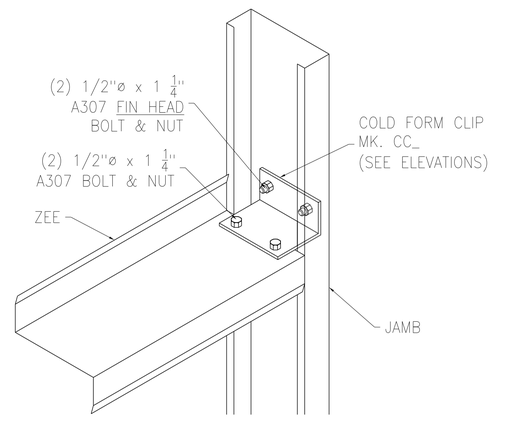
11 SS505 JAMB TO FLOOR



8 SS505 JAMB TO STEM WALL



5 SS505 JAMB TO CEE



3 SS505 ZEE TO JAMB

Tekla Structures

FOREMASTER

2397 RULON WHITE BLVD
OGDEN UT. 84404

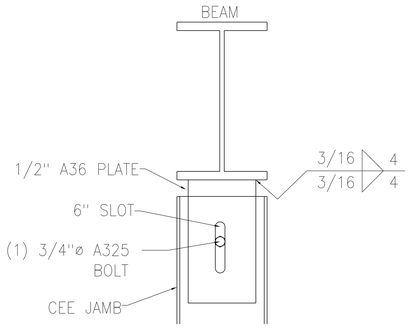
SECONDARY FRAMING DETAILS

Mark	Description	Date	By
A	For Permit	03/24/2020	SES

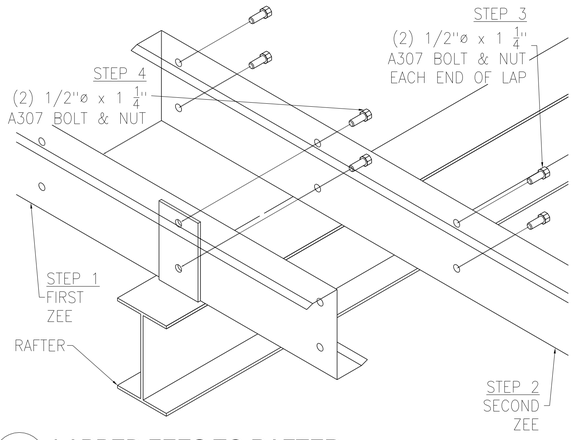
DATE 03/24/2020

PROJ. NO. 20053

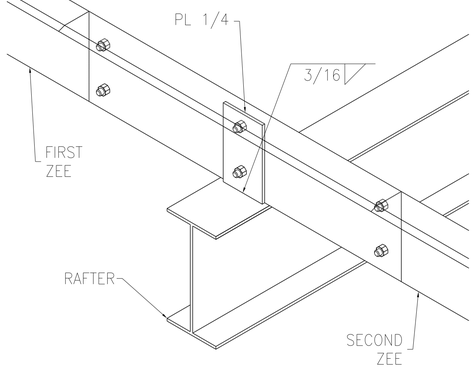
DRAWING NO. **SS505**

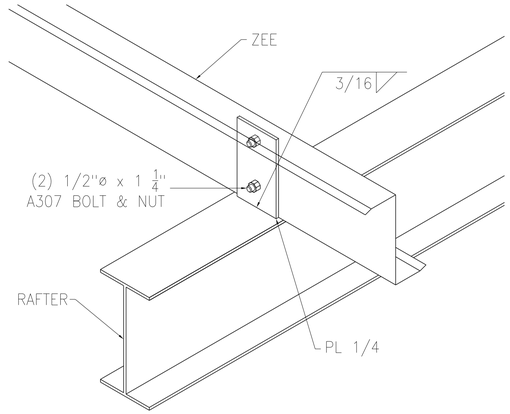


21 SS506 SLOTTED SHEAR TAB

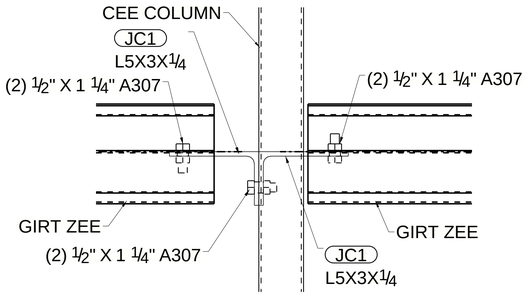


16 SS506 LAPPED ZEES TO RAFTER

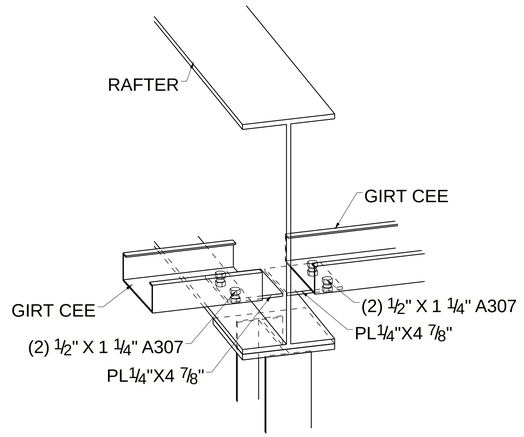




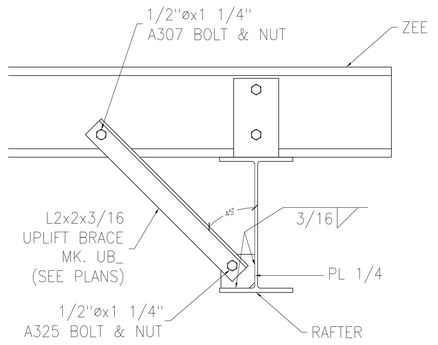
13 SS506 ZEE TO RAFTER



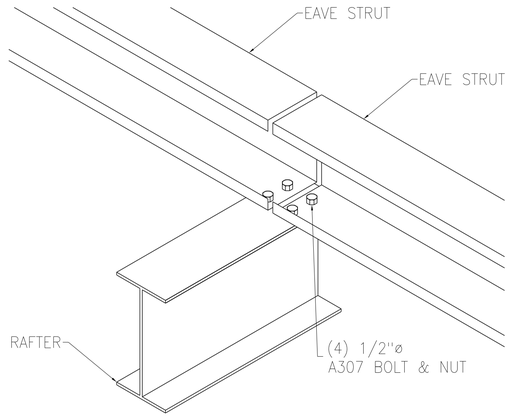
22 SS506 FLUSH GIRTS TO CEE JAMB



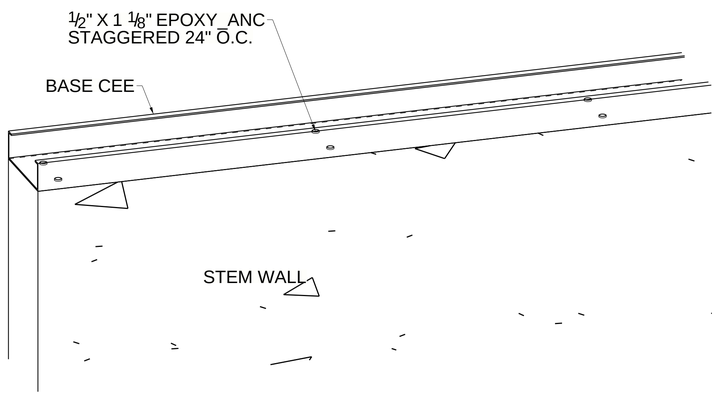
19 SS506 GIRT TO RAFTER



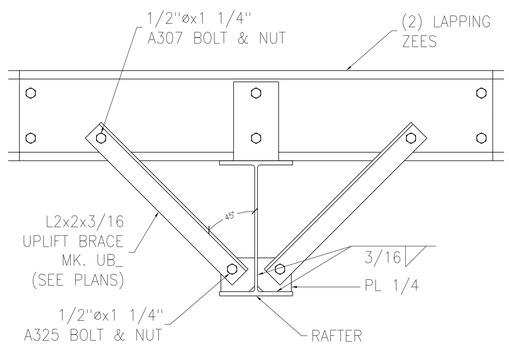
17 SS506 UPLIFT BRACE TO RAFTER & ZEE



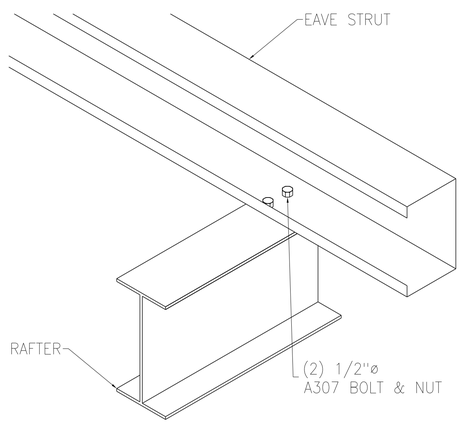
14 SS506 EAVE STRUT TO RAFTER



20 SS506 BASE CEE TO STEM WALL



18 SS506 UPLIFT BRACE TO RAFTER & PURLIN



15 SS506 EAVE STRUT TO RAFTER

STEEL CONCEPTS
1981 PAINTER LANE
WEST HAVEN, UTAH 84401
PHONE: 801-452-6699
FAX: 801-452-6698
www.steel-concepts.com

PREPARED BY:
Designed by: JB
Drawn by: SES
Checked by: JL

PROFESSIONAL STRUCTURAL ENGINEER
No. 368583
03/24/2020
JARED E. LEE
STATE OF UTAH

FOREMASTER
2397 RULON WHITE BLVD
OGDEN UT. 84404

SECONDARY FRAMING DETAILS

Mark	Description	Date	By
A	For Permit	03/24/2020	SES

DATE 03/24/2020
PROJ. NO. 20053
DRAWING NO. SS506