#### Design Calculations for Motorcycle Rack for Kenny Watkins (2015 IBC)

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Having analyzed the existing motorcycle rack it has been determined that it is sufficiently strong to support the motorcycle and snowmobile loads along with the snow load in Eden Utah. However, given the height and slenderness of the rack, it is my recommendation that the owner remove the top section and use the middle section as just a covering (keep unloaded). Doing this drastically reduces any risk of overturning due to wind or earthquake and will allow for multiple single level racks that are easier and safer to load and unload. It will also reduce any adverse visual appearance. It is also my recommendation that the owner laterally support the rack by welding either expanded metal or diagonal bracing to the back side. Currently the sections of rack are only connected in a few locations. The owner has also been advised to properly connect all sections at all available locations using properly sized timber blocking and bolts of the same size and type as existing in the few already connected locations. The owner has been advised of all of these recommendations and is willing to comply.





### Design Calculation for Motorcycle Rack for Kenny Watkins (IBC 2015) Main Support 10/23/2017

# Input Variables

$$\phi := 0.9$$

$$E_{Steel} := 29000 \text{ ksi}$$
  $F_{V} := 36000 \text{ psi}$ 

$$F_{v} := 36000 ps$$

$$W_T := 2ft$$

Tributary Width

$$H_{Beam} := 4in$$

$$W_{Beam} := 1.75in$$

$$T_{Beam} := .125in$$

### Live Loads:

$$L_R := 38.4 psf$$

Live Load of 2 snowmobiles @ 700 lbs each

$$L_{\mathbf{Snow}} := 70 \mathbf{psf}$$

Snow Load

## Dead Loads:

$$DL := 5psf$$

## Main Beam Design

## Factored loads:

$$\mathbf{w_{LL}} := 1.6 \cdot \left( \mathbf{L_R} \cdot \mathbf{W_T} + \mathbf{L_{Snow}} \cdot \mathbf{W_T} \right) = 346.88 \cdot \mathbf{plf}$$

$$\mathbf{w_{DL}} := 1.2(\mathbf{DL} \cdot \mathbf{W_T}) = 12 \cdot \mathbf{plf}$$

$$\mathbf{w} := \mathbf{w_{LL}} + \mathbf{w_{DL}} = 358.88 \cdot \mathbf{plf}$$

#### Reaction

$$\mathbf{R_1} := \frac{(\mathbf{w} \cdot \mathbf{span})}{2} = 1.615 \cdot \mathbf{kip}$$

$$R_2 := R_1 = 1.615 \cdot kip$$

Bending

$$\mathbf{M_{U}} := \frac{\left(\mathbf{w \cdot span}^2\right)}{8} = 3.634 \cdot \mathbf{kip \cdot ft}$$

$$\mathbf{Z}_{\mathbf{Required}} := \frac{\mathbf{M}_{\mathbf{U}}}{\mathbf{\phi} \cdot \mathbf{F}_{\mathbf{y}}} = 1.346 \cdot \mathbf{in}^3$$

$$\mathbf{Z_{Main\_Support}} := \frac{\left[\mathbf{W_{Beam}} \cdot \mathbf{H_{Beam}}^3 - \left(\mathbf{W_{Beam}} - 2 \cdot \mathbf{T_{Beam}}\right) \cdot \left(\mathbf{H_{Beam}} - 2 \cdot \mathbf{T_{Beam}}\right)^3\right]}{6 \cdot \mathbf{H_{Beam}}} = 1.371 \text{ in}^3$$

 $Z_{Main\_Support} \geq Z_{Required}$ 

## Bending is Ok

## Shear

$$V_U := R_1 = 1.615 \cdot kip$$

$$A_{\mathbf{W}} := 2 \cdot H_{\mathbf{Beam}} \cdot T_{\mathbf{Beam}} = 1 \text{ in}^2$$

Area of the Web

$$V_{\mathbf{n}} := 0.6 \cdot \phi \cdot \mathbf{F}_{\mathbf{y}} \cdot \mathbf{A}_{\mathbf{w}} = 19.44 \cdot \mathbf{kip}$$

$$\mathbf{V}_n \geq \mathbf{V}_U$$

## Shear is Ok

## **Deflection**

$$\delta_{\mbox{\bf Allow}} := \frac{\mbox{\bf span}}{120} = 0.9 \cdot \mbox{\bf in}$$

$$I_{\mathbf{Required}} := \frac{\left(5 \cdot \mathbf{w} \cdot \mathbf{span}^{4}\right)}{\left(384 \cdot \mathbf{E}_{\mathbf{Steel}} \cdot \delta_{\mathbf{Allow}}\right)} = 2.03 \cdot \mathbf{in}^{4}$$

$$\mathbf{I_{Main\_Beam}} := \frac{\left[\mathbf{W_{Beam}} \cdot \mathbf{H_{Beam}}^3 - \left(\mathbf{W_{Beam}} - 2 \cdot \mathbf{T_{Beam}}\right) \cdot \left(\mathbf{H_{Beam}} - 2 \cdot \mathbf{T_{Beam}}\right)^3\right]}{12} = 2.742 \text{ in}^4$$

 $I_{Main\_Beam} \geq I_{Required}$ 

Deflection is Ok

#### Design Calculation for Motorcycle Rack for Kenny Watkins (IBC 2015) Lipped C-Channel Support 10/23/2017

## Input Variables

$$\phi := 0.9$$

$$\mathbf{E_{Steel}} := 29000 \, \mathbf{ksi}$$
  $\mathbf{F_y} := 36000 \, \mathbf{psi}$ 

$$F_{v} := 36000 ps$$

$$\mathbf{W_T} := 2.17 \mathbf{ft}$$

Tributary Width

#### Live Loads:

$$L_R := 38.4psf$$

Live Load of 2 snowmobiles @ 700 lbs each

$$L_{Snow} := 70psf$$

Snow Load

$$Z_{Lipped\_C\_Channel} := 0.29in^3$$

Section modulus. See attached sheet of properties for Lipped C-Channel.

Modulus of Elasticity. See attached sheet of properties for Lipped C-Channel.

#### Dead Loads:

$$DL := 5psf$$

## **Lipped C-Channel Design**

### Factored loads:

$$w_{LL} := 1.6 \cdot (L_R \cdot W_T + L_{Snow} \cdot W_T) = 376.365 \text{ plf}$$

$$\mathbf{w_{DL}} := 1.2(\mathbf{DL} \cdot \mathbf{W_T}) = 13.02 \cdot \mathbf{plf}$$

$$w := w_{LL} + w_{DL} = 389.385 \, plf$$

## Reaction

$$\mathbf{R_1} := \frac{(\mathbf{w} \cdot \mathbf{span})}{2} = 0.779 \cdot \mathbf{kip}$$

$$R_2 := R_1 = 0.779 \text{ kip}$$

## Bending

$$\mathbf{M_{U}} := \frac{\left(\mathbf{w \cdot span}^2\right)}{8} = 0.779 \cdot \mathbf{kip \cdot ft}$$

$$\mathbf{Z_{Required}} := \frac{\mathbf{M_U}}{\phi \cdot \mathbf{F_v}} = 0.288 \cdot \mathbf{in}^3$$

 $Z_{Lipped\_C\_Channel} \geq Z_{Required}$ 

## Bending is Ok

## **Shear**

$$V_U := R_1 = 0.779 \cdot kip$$

$$A_w := 0.64in^2$$
 Area of the Web

$$V_{\mathbf{n}} := 0.6 \cdot \phi \cdot F_{\mathbf{y}} \cdot A_{\mathbf{w}} = 12.442 \cdot \mathbf{kip}$$

$$\boldsymbol{\mathrm{V}}_n \geq \boldsymbol{\mathrm{V}}_U$$

# Shear is Ok

## **Deflection**

$$\delta_{Allow} := \frac{span}{120} = 0.4 \cdot in$$

$$I_{\mathbf{Required}} := \frac{\left(5 \cdot \mathbf{w} \cdot \mathbf{span}^{4}\right)}{\left(384 \cdot \mathbf{E_{\mathbf{Steel}}} \cdot \delta_{\mathbf{Allow}}\right)} = 0.193 \cdot \mathbf{in}^{4}$$

 $I_{Lipped\_C\_Channel} \ge I_{Required}$ 

# Deflection is Ok

### https://www.ns-kenzai.co.jp/english/023light.html



