Weber County Design Review Application

Application submittals will be accepted by appointment only. (801) 399-8791. 2380 Washington Blvd. Suite 240, Ogden, UT 84401

Date Submitted / Completed	Fees (Office Use)	Receipt Number (Office Use)	File Number (Office Use)
4-10-12			

Property Owner Contact Information

Name of Property Owner(s)	Mailing Address of Property Owner(s)
BEN TOONE	5510 E 2200 N Eden UT 84310
Phone: 801-645-9700	**Fax:**
Email Address: Rock@oldtowneden.com	**Preferred Method of Written Correspondence:** [X] Email [ ] Fax [ ] Mail

Authorized Representative Contact Information

Name of Person Authorized to Represent the Property Owner(s)	Mailing Address of Authorized Person
Phone:	**Fax:**
Email Address:	**Preferred Method of Written Correspondence:** [ ] Email [ ] Fax [ ] Mail

Property Information

Project Name	Current Zoning
Old Town Eden	CV-2
Approximate Address: Same as above	**Land Serial Number(s):** 22-051-0079

Proposed Use: Restaurant Seating Area

Project Narrative:

This project is to put a roof over an existing brick patio.

Property Owner Affidavit

I (We), Rick Towne, depose and say that I (we) am (are) the owner(s) of the property identified in this application and that the statements herein contained, the information provided in the attached plans and other exhibits are in all respects true and correct to the best of my (our) knowledge.

Executor of Trust.
(Property Owner)

(Property Owner)

Subscribed and sworn to me this _____day of _____, 20 _____,

(Notary)

Authorized Representative Affidavit

I (We), _____, the owner(s) of the real property described in the attached application, do authorized as my (our) representative(s), _____, to represent me (us) regarding the attached application and to appear on my (our) behalf before any administrative or legislative body in the County considering this application and to act in all respects as our agent in matters pertaining to the attached application.

(Property Owner)

(Property Owner)

Dated this _____day of _____, 20 _____, personally appeared before me _____, the signer(s) of the Representative Authorization Affidavit who duly acknowledged to me that they executed the same.

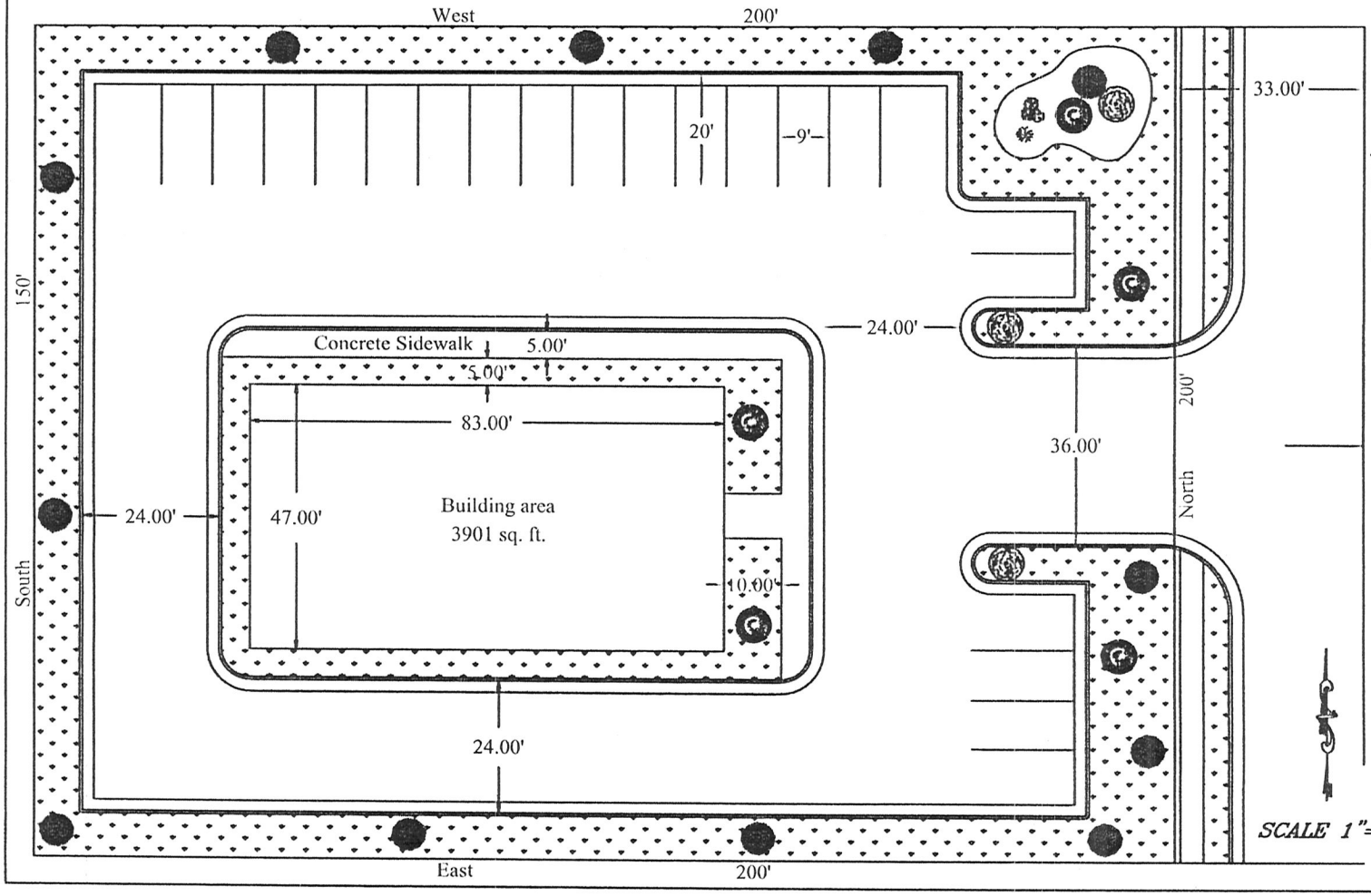
(Notary)

Check List for Site Plan Review.

Name of the proposed development
Name and address of the owner of property
Name and address of the preparer of the site plan
Statement describing the intended use of the development
A north arrow and scale not less than 1:50
The tax ID number of the development site
The land use and zoning of the development site
Adjacent land use and zoning
* Identify the percentage of the property covered by buildings and hard surface

Adjacent streets shall be shown and identified, along with distance from centerline to property
Building setbacks and distances
Easement on property and on abutting property, that could be affected
A letter from the Water and Sewer company serving the project or a septic tank approval letter
* Elevation drawings depicting architectural theme, building features, materials and colors is required
* A grading and drainage plan is required
Landscaping plan

* Lighting plan
Detailed sign information including color and material
Fire hydrant location
Parking information - size and number of stalls
The geometric layout and dimensions of proposed building, driveways, parking areas, loading areas, signs and other features of the development
Existing structures
Storm water management plan



Site Plan Requirement

	sq. ft	Percentage
Total Parcel area		
Building coverage		
Asphalt Coverage		
Landscaping		
Existing Zoning		
Landscaping type, size and number		
Parking Stalls Required		
Parking Stalls Shown		
Proposed Elevation		
Land Use Proposed		

* Does not apply to Home Occupation Conditional Use Applications

Note: This is not a substitution for reading the Weber County Zoning Ordinance.

Gerald B. Taggart, P.E.
2080 North 5700 East, Eden, Utah 84310, Phone 801/ 745-1731

August 14, 2000

Mr. Ben Toone
5522 East 2200 North
Eden, Utah 84310

REF: Privacy Fence Structure

Dear Ben:

I have reviewed the design of the privacy fence you are building around the new outdoor courtyard at you restaurant, and find the fence structure as built sufficient to withstand a 70 mph wind load for a exposure B wind condition as required. However, the concrete footings holding the vertical posts are not of sufficient size to withstand the overturning moments created from the wind blowing on the 8'-0" high fence above. These will need to be modified.

At the stage of construction you are in, I believe the easiest fix would be to dig down to the existing footings, and pour an additional 14" thick x 20" wide by 3'-3" footing that is dowelled into the existing footings with (3) #4 bars x 48" long, top and bottom. The bars should extend into the existing footing a minimum of 12". Allow 3" of concrete cover on top, bottom and sides of bars. Provide (3) cross bars of #4 size in the top and bottom mats.

These additional footings are not required at the corner posts, only the intermediate and end posts.

If you have any questions, please give me a call.

Sincerely,

Gerald B. Taggart

Gerald B. Taggart, P.E.

PRIVACY FENCE
BEN TOONE
EDEN, UT.

REGISTERED PROFESSIONAL ENGINEER
3074
STATE OF UTAH
Gerald B. Taggart
GERALD B. TAGGART
Registered Professional Engineer

DESIGN WIND PRESSURE

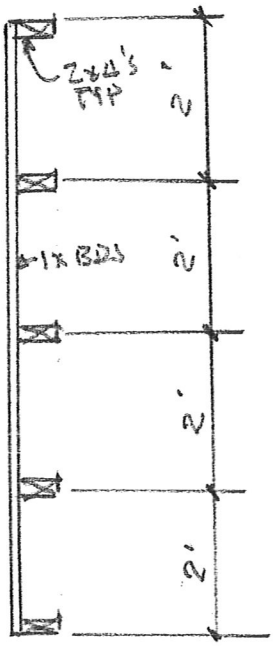
$P = C_e C_q Q_s I_w$

$C_e = 1.06$ TABLE 16G
$C_q = 1.3$ TABLE 16H
$Q_s = 12.6$ 70 MPH
$I_w = 1.0$ TABLE 16K

$P = 1.06(1.3)(12.6)(1.0) = 17.4$ PSF.

MAXIMUM POST SPACING IS 9'

FENCE SECTION IS AS SHOWN



FOR THE 3 CENTER SUPPORTS

$w = 17.4 \times 2 = 34.8$ LBS/FT

FOR THE TOP & BOTM SUPPORTS

$w = 17.4 \times 1 = 17.4$ LBS/FT

SEE ATTACHED COMPUTER PRINTED SHEETS FOR FENCE RAILS

FOR THE SUPPORT COLUMNS

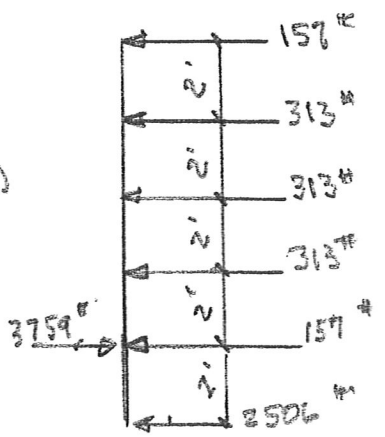
$\Sigma M_{BTM} = 0$

$\Sigma R_1 = 2(157) + 4(313) + 6(313) + 8(313) + 10(157)$
$= 7518$

$R_1 = 3759$ LBS

$R_2 = 2(157) + 3(313) - 3759 = 2506$#

$M_{MAX} = 2506 \times 24 = 60,144$ IN-LBS



Gerald B. Taggart, P.E.
2080 North 5700 East
Eden, Utah 84310
Phone: 801/ 745-1731
FAX: 801/ 745-2256

Title :
Dsgnr:
Description :

Job #
Date: 9:43AM, 11 AUG 00

Scope :

General Timber Beam

Description Center Fence Rails

General Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Section Name	2x4	Center Span	9.00 ft	.....Lu	0.00 ft
Beam Width	1.500 in	Left Cantilever	ft	.....Lu	0.00 ft
Beam Depth	3.500 in	Right Cantilever	ft	.....Lu	0.00 ft
Member Type	Sawn	Douglas Fir - Larch, No.2			
		Fb Base Allow	875.0 psi		
Load Dur. Factor	1.330	Fv Allow	85.0 psi		
Beam End Fixity	Pin-Pin	Fc Allow	625.0 psi		
		E	1,300.0 ksi		

Full Lengh Uniform Loads

Center	DL	#/ft	LL	34.80	#/ft
Left Cantilever	DL	#/ft	LL		#/ft
Right Cantilever	DL	#/ft	LL		#/ft

Summary

Beam Design OK

Span= 9.00ft, Beam Width = 1.500in x Depth = 3.5in, Ends are Pin-Pin

Max Stress Ratio	0.791 : 1		
Maximum Moment	0.4 k-ft	Maximum Shear * 1.5	0.2 k
Allowable	0.4 k-ft	Allowable	0.6 k

Max. Positive Moment	0.35 k-ft	at	4.500 ft	Shear:	@ Left	0.16	k
Max. Negative Moment	0.00 k-ft	at	0.000 ft		@ Right	0.16	k
Max @ Left Support	0.00 k-ft			Camber:	@ Left	0.000	in
Max @ Right Support	0.00 k-ft				@ Center	0.000	in
Max. M allow	0.45				@ Right	0.000	in

				Reactions...			
fb	1,380.64 psi	fv	44.74 psi	Left DL	0.00 k	Max	0.16 k
Fb	1,745.63 psi	Fv	113.05 psi	Right DL	0.00 k	Max	0.16 k

Deflections

Center Span...	Dead Load	Total Load	Left Cantilever...	Dead Load	Total Load
Deflection	0.000 in	-0.737 in	Deflection	0.000 in	0.000 in
...Location	9.000 ft	4.500 ft	...Length/Defl	0.0	0.0
...Length/Defl	0.0	146.47	**Right Cantilever...**		
			Deflection	0.000 in	0.000 in
			...Length/Defl	0.0	0.0

Stress Calcs

Bending Analysis

Ck	27.106	Le	0.000 ft	Sxx	3.063 in3	Area	5.250 in2
Cf	1.500	Rb	0.000	Cl	0.000		

	Max Moment	Sxx Req'd	Allowable fb
@ Center	0.35 k-ft	2.42 in3	1,745.63 psi
@ Left Support	0.00 k-ft	0.00 in3	1,745.63 psi
@ Right Support	0.00 k-ft	0.00 in3	1,745.63 psi

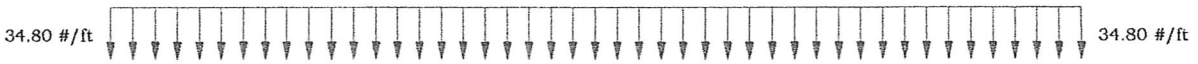
Shear Analysis	@ Left Support	@ Right Support
Design Shear	0.23 k	0.23 k
Area Required	2.078 in2	2.078 in2
Fv: Allowable	113.05 psi	113.05 psi

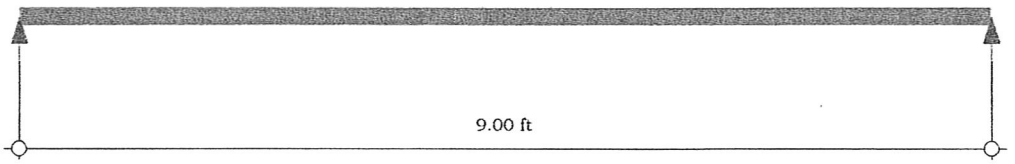
Bearing @ Supports

Max. Left Reaction	0.16 k	Bearing Length Req'd	0.167 in
Max. Right Reaction	0.16 k	Bearing Length Req'd	0.167 in

Query Values

M, V, & D @ Specified Locations		Moment	Shear	Deflection
@ Center Span Location =	0.00 ft	0.00 k-ft	0.16 k	0.0000 in
@ Right Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in
@ Left Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in





Mmax = 0.35 k-ft

Dmax = -0.7373 in

Rmax = 0.156 k

Vmax @ left = 0.156 k

Rmax = 0.156 k

Vmax @ rt = 0.156 k

Gerald B. Taggart, P.E.
2080 North 5700 East
Eden, Utah 84310
Phone: 801/ 745-1731
FAX: 801/ 745-2256

Title :
Dsgnr:
Description :

Job #
Date: 9:49AM, 11 AUG 00

Scope :

Description Top & Btm Fence Rails

General Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Section Name	2x4	Center Span	9.00 ft	.....Lu	0.00 ft
Beam Width	1.500 in	Left Cantilever	ft	.....Lu	0.00 ft
Beam Depth	3.500 in	Right Cantilever	ft	.....Lu	0.00 ft
Member Type	Sawn	Douglas Fir - Larch, No.2			
		Fb Base Allow	875.0 psi		
Load Dur. Factor	1.330	Fv Allow	85.0 psi		
Beam End Fixity	Pin-Pin	Fc Allow	625.0 psi		
		E	1,300.0 ksi		

Full Lengh Uniform Loads

Center	DL	#/ft	LL	17.40 #/ft
Left Cantilever	DL	#/ft	LL	#/ft
Right Cantilever	DL	#/ft	LL	#/ft

Summary

Beam Design OK

Span= 9.00ft, Beam Width = 1.500in x Depth = 3.5in, Ends are Pin-Pin

Max Stress Ratio	0.395 : 1				
Maximum Moment	0.2 k-ft		Maximum Shear * 1.5		0.1 k
Allowable	0.4 k-ft		Allowable		0.6 k
Max. Positive Moment	0.18 k-ft at	4.500 ft	Shear:	@ Left	0.08 k
Max. Negative Moment	0.00 k-ft at	0.000 ft		@ Right	0.08 k
Max @ Left Support	0.00 k-ft		Camber:	@ Left	0.000 in
Max @ Right Support	0.00 k-ft			@ Center	0.000 in
Max. M allow	0.45			@ Right	0.000 in
		Reactions...			
fb 690.32 psi	fv 22.37 psi	Left DL	0.00 k	Max	0.08 k
Fb 1,745.63 psi	Fv 113.05 psi	Right DL	0.00 k	Max	0.08 k

Deflections

Center Span...	Dead Load	Total Load	Left Cantilever...	Dead Load	Total Load
Deflection	0.000 in	-0.369 in	Deflection	0.000 in	0.000 in
...Location	9.000 ft	4.500 ft	...Length/Defl	0.0	0.0
...Length/Defl	0.0	292.95	**Right Cantilever...**		
			Deflection	0.000 in	0.000 in
			...Length/Defl	0.0	0.0

Stress Calcs

Bending Analysis

Ck	27.106	Le	0.000 ft	Sxx	3.063 in3	Area	5.250 in2
Cf	1.500	Rb	0.000	Cl	0.000		

	Max Moment	Sxx Req'd	Allowable fb
@ Center	0.18 k-ft	1.21 in3	1,745.63 psi
@ Left Support	0.00 k-ft	0.00 in3	1,745.63 psi
@ Right Support	0.00 k-ft	0.00 in3	1,745.63 psi

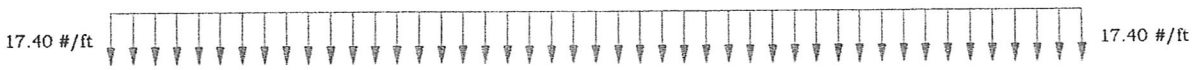
Shear Analysis	@ Left Support	@ Right Support
Design Shear	0.12 k	0.12 k
Area Required	1.039 in2	1.039 in2
Fv: Allowable	113.05 psi	113.05 psi

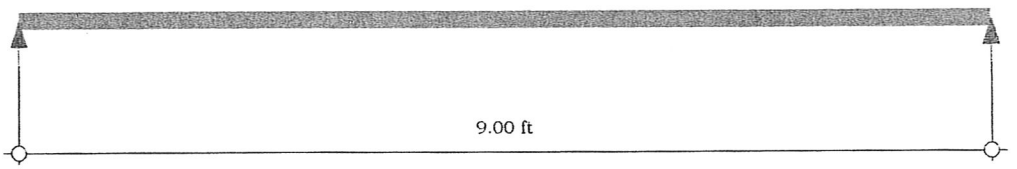
Bearing @ Supports

Max. Left Reaction	0.08 k	Bearing Length Req'd	0.084 in
Max. Right Reaction	0.08 k	Bearing Length Req'd	0.084 in

Query Values

M, V, & D @ Specified Locations		Moment	Shear	Deflection
@ Center Span Location =	0.00 ft	0.00 k-ft	0.08 k	0.0000 in
@ Right Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in
@ Left Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in





Mmax = 0.17 k-ft

Dmax = -0.3686 in

Rmax = 0.078 k

Vmax @ left = 0.078 k

Rmax = 0.078 k

Vmax @ rt = 0.078 k

POSTS ARE TS 3½ x 3½ x ¼

$$f_b = \frac{60,144}{3.02} = 19,915 \text{ PSI}$$

WEIGHT OF FENCE

(5) 2x4's 5 x 1.02 = 5.1 LBS/FT
1x6's 1.3 LBS/FT
6.4 LBS/FT

WEIGHT OF WALL 1.5 x .83 x 150 186.8 LBS/FT
193.2 LBS/FT

TOTAL WEIGHT ON FOOTING = 193.2 x 9 = 1739 LBS
WEIGHT OF POST = 10.51 x 10 105 LBS
WEIGHT OF INSERT TS 4x4x¼x35" 2.5 x 12.21 76 LBS
1920 LBS

WEIGHT OF FOOTING 1.67 x 1.67 x 1.17 x 150 489 LBS

WEIGHT OF GROUND ABOVE FOOTING
1.67 x 1.25 x 1.5 x 100 x 2 626 LBS

TOTAL WEIGHT 3035 LBS

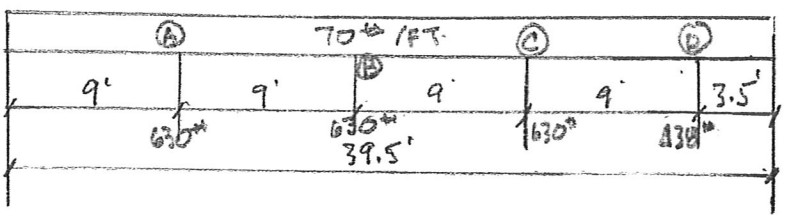
RESTORING MOMENT = 3035 x 10 = 30350 IN-LBS

REQD RESTORING MOMENT = 1.4 x 60,144 = 84,201 IN-LBS > 30350

NO GOOD !

ASSUME A MEMBER PLACED AT THE TOP OF THE WALL TO SUPPORT THE TOP OF THE FENCE. FOR THE EAST WALL, ASSUME SPAN AS INDICATED

W = 17.4 x 4 = 70#/FT



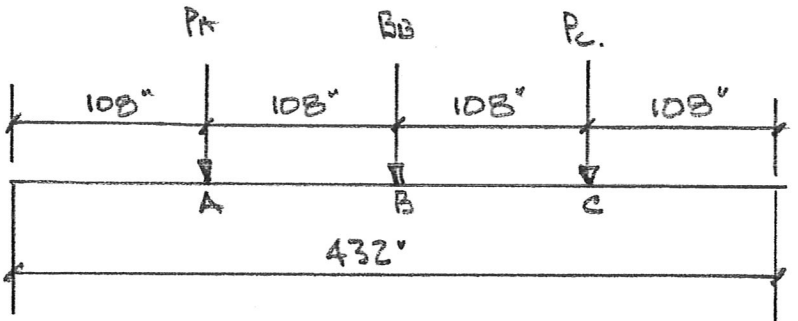
FOR THE CANTILEVER POST THE DEFLECTION AT THE TOP IS

$$\Delta_1 = \frac{PL_1^3}{15EI} = \frac{P_1(96)^3}{15 \times 29\times10^6 \times 5.29} = 0.0004\,P_1$$

FOR A BEAM LOADED AS SHOWN THE DEFLECTION AT THE LOAD POINTS ARE

$P_A = P_C$

$\Delta_A = \Delta_C$



$$\Delta A = \Delta C = \frac{P_A(108)^2(324)^2}{3(29\times10^6)(432)I} + \frac{P_B(216)(108)}{6(29\times10^6)(432)I}\left[432^2-216^2-108^2\right] + \frac{P_A(108)(108)}{6(29\times10^6)(432)I}\left[432^2-108^2-108^2\right]$$

$$= .0326\frac{P_A}{I} + .0398\frac{P_B}{I} + .0253\frac{P_A}{I}$$

$$= .0579\frac{P_A}{I} + .0398\frac{P_B}{I}$$

$$\Delta_B = \frac{P_B(432)^3}{48(29\times10^6)I} + \frac{2(P_A)(108)(216)}{6(29\times10^6)(432)I}\left[432^2-216^2-108^2\right]$$

$$= .0579\frac{P_B}{I} + .0796\frac{P_A}{I}$$

$\Delta_A = \Delta_1$

$P_1 = 630 - P_A$

$$.0004P_1 = .0579\frac{P_A}{I} + .0398\frac{P_B}{I}$$

$$.252 - .0004\,P_A = .0579\frac{P_A}{I} + .0398\frac{P_B}{I}$$

$$\Delta_B = 0.0004 P_2 = .0579 \frac{P_B}{I} + .0796 \frac{P_A}{I}$$

$P_2 = 630 - P_B$

FOR TS 8x4x1/4, $I = 45.1$

$11.365 - .018 P_A = .0579 P_A + .0398 P_B$

$11.365 = 0.0759 P_A + .0398 P_B$

$11.365 - .018 P_B = .0579 P_B + .0796 P_A$

$11.365 = .0796 P_A + .0759 P_B$

$11.365 = .0796 P_A + .0759 P_B$
$11.919 = .0796 P_A + .0417 P_B$

$0.554 = .0342 P_B$

$P_B = 16.2$ LBS $\qquad P_2 = 630 - 16.2 = 613.8$ LBS.

97% OF LOAD WILL GO INTO POST.

MAKE FOOTINGS CAPABLE OF RESTORING MOMENT

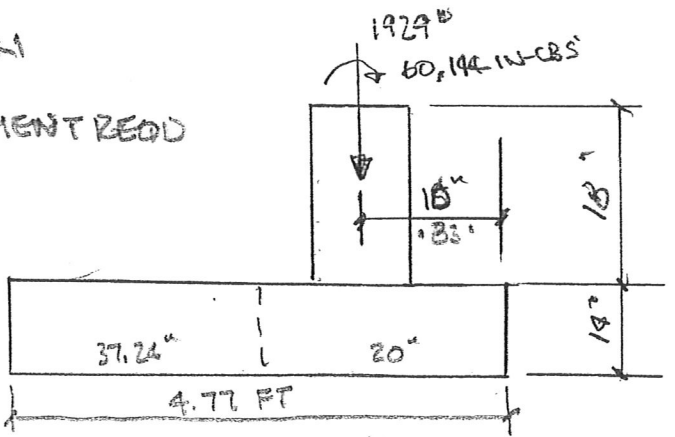
ASSUME FOOTING AS SHOWN

ADDITIONAL RESTORING MOMENT REQD

$M_R = 1.4 \times 60,144 - 1929 \times 10$

$= 64,865$ IN-LBS

$= 5405$ FT-LBS.



$$5405 = (1.1667 \times 1.6667 \times 150)\left(\frac{L^2}{2}\right) + 1.5(1.6667)(L - 1.25)\left(\frac{L - 1.25}{2} + 1.25\right)$$

$5405 = 145.84 L^2 + 125 L^2 - 156 L$

$270.84 L^2 - 156 L - 5405 = 0$

$$L = \frac{-(-156) \pm \sqrt{(-156)^2 - 4(270.84)(-5405)}}{2(270.84)} = 4.77 \text{ FT}$$

CHECK WALL THE TOTAL RESTORING MOMENT

$$M_{RT} = \frac{1929 \times 10}{12} + 4.77(1.1667)(1.6667)(150)\left(\frac{4.77}{2}\right)$$

$$+ 1.5(1.6667)(4.777 - 1.25)\left(\frac{3.527}{2} + 1.25\right)(100)$$

$$= 1607.5 + 3518.3 + 3286.4$$

$$= 8212 \text{ FT-LBS} = 98,544 \text{ IN-LBS} > 84,202 \text{ IN-LBS}$$

MOMENT @ BUTT OF EXIST. FOUNDATION

$$M = 150(3.11)(1.6667)(1.1667)\left(\frac{3.11}{2}\right) + 100(1.5)(1.6667)\left(\frac{3.11}{2}\right)^2$$

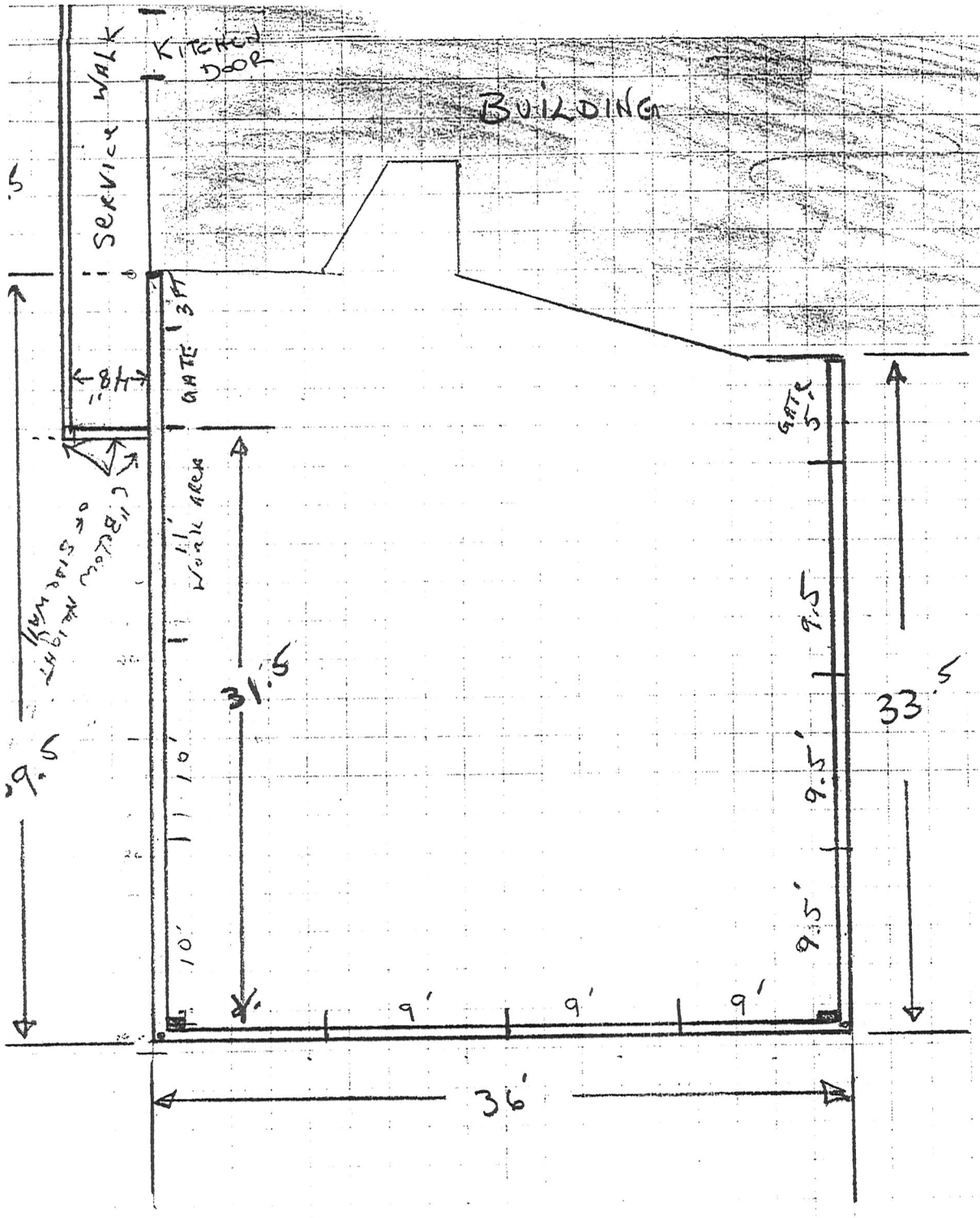
$$= 1410 + 1209 = 2619 \text{ FT-LBS}$$

$$= 31,428 \text{ IN-LBS}$$

$$f = \frac{2(31,428)}{.403(.8657)(20)(10.75)^2} = 78 \text{ PSI}$$

$$A_s = \frac{31,428}{20,000(.8657)(10.75)} = 0.17 \text{ IN}^2$$

USE (3) #4 BARS DRILLED INTO EXISTING FOOTING, TOP & BTM.



Service Walk	34	lin ft
East Wall	40	lin ft
North Wall	36	lin ft
West Wall	34	lin ft
Total	144	lin ft

4x8x¾ ply	22 sheets
2x10 Reinforcement	288 lin ft
Rebar/metal stakes	70

Rebar Schd. 60

Footings	17.6 ft
Wall	57C ft
Total	900 ft = 45 pc

Tie Wire 4 Rolls

10 lbs 2" Grabbers

1 can Surveyor Paint

4x4 m Post or Sleeve 10' post → 12 posts

36" sleeve 12 sleeves

Concrete Footings 1 C yrd.

Wall 5.28 yd

Randy Rogers 745-2763

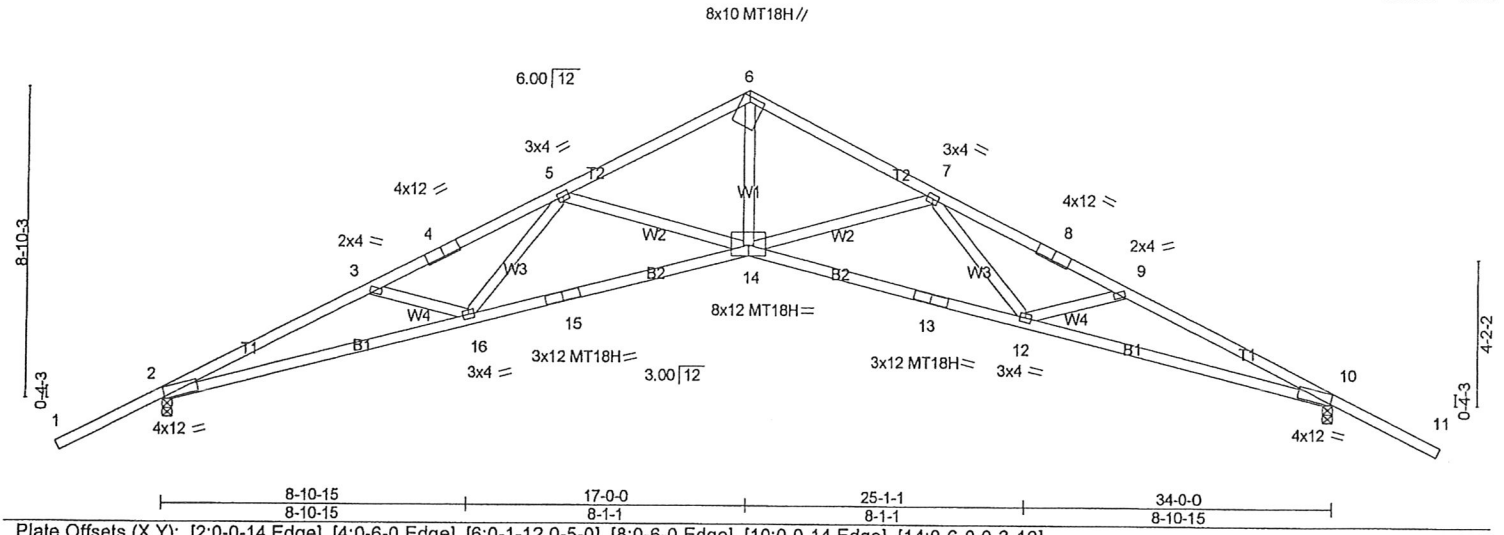
Job	Truss	Truss Type	Qty	Ply	
Q136535	A1	SCISSORS	14	1	Job Reference (optional)

Stock Components

7.250 s Jan 28 2011 MiTek Industries, Inc. Fri Feb 10 11:54:11 2012 Page 1
ID:R1a5nyBjYRp1h2YjCe6IcmyDyV_-cow0eRkgco9d6abtoL7VCDKJ835BSqx4_j1QfbzmXcg

-3-0-0	6-2-10	11-7-5	17-0-0	22-4-11	27-9-6	34-0-0	37-0-0
3-0-0	6-2-10	5-4-11	5-4-11	5-4-11	5-4-11	6-2-10	3-0-0

Scale = 1:66.3
Camber = 1/2 in



8-10-15	17-0-0	25-1-1	34-0-0
8-10-15	8-1-1	8-1-1	8-10-15

Plate Offsets (X,Y): [2:0-0-14,Edge], [4:0-6-0,Edge], [6:0-1-12,0-5-0], [8:0-6-0,Edge], [10:0-0-14,Edge], [14:0-6-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plates Increase	1.15	TC	0.86	Vert(LL)	-0.82	14-16	>495	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.85	Vert(TL)	-1.31	14-16	>309	180	MT18H	197/144
BCLL	0.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.91	10	n/a	n/a		
BCDL	5.0	Code IRC2009/TPI2007		(Matrix)		Wind(LL)	0.36	14-16	>999	240	Weight: 131 lb	FT = 20%

LUMBER
TOP CHORD 2 X 4 SPF 1650F 1.5E
BOT CHORD 2 X 4 SPF 1650F 1.5E *Except*
B1: 2 X 4 SPF 2100F 1.8E
WEBS 2 X 4 DF Stud/Std *Except*
W1: 2 X 4 DF No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-2-4 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2175/0-3-8 (min. 0-2-10), 10=2175/0-3-8 (min. 0-2-10)
Max Horz 2=160(LC 5)
Max Uplift2=-487(LC 5), 10=-487(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/133, 2-3=-6145/992, 3-4=-5512/843, 4-5=-5395/862, 5-6=-4078/579, 6-7=-4078/593, 7-8=-5395/726, 8-9=-5512/707, 9-10=-6145/831, 10-11=0/133
BOT CHORD 2-16=-890/5471, 15-16=-637/4684, 14-15=-633/4697, 13-14=-470/4697, 12-13=-477/4684, 10-12=-638/5471
WEBS 6-14=-388/3002, 3-16=-538/222, 5-16=-66/516, 5-14=-1111/350, 7-14=-1111/353, 7-12=-72/516, 9-12=-538/228

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
3) All plates are MT20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 487 lb uplift at joint 2 and 487 lb uplift at joint 10.
7) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

WEBER COUNTY CMS RECEIPTING SYSTEM

OFFICIAL RECEIPT

*** REPRINT ***

Date: 10-APR-2012 Receipt Nbr: 705 ID# 3206

Employee / Department: ANGELA MARTIN - 4181 - PLANNING
Monies Received From: OLD TOWN EDEN LLC
Template: PUBLIC WORKS
Description: DESIGN REVIEW - ADMINISTRATIVE APP

The following amount of money has been received and allocated to the various accounts listed below:

Total Currency	$	.00
Total Coin	$	.00
Total Debit/Credit Card	$	.00
Pre-deposit	$	.00
Total Checks	$	100.00
Grand Total	$	100.00

Account Number	Account Name	Comments	Total
2012-01-4181-3419-0550-000	ZONING FEES		100.00

TOTAL $ 100.00

Check Amounts

100.00

Total Checks: 1 Total Check Amounts: $ 100.00

*** SAVE THIS RECEIPT FOR YOUR RECORDS ***

Weber County

Weber County Planning Division
www.co.weber.ut.us/planning
2380 Washington Blvd., Suite 240
Ogden, Utah 84401-1473
Voice: (801) 399-8791
Fax: (801) 399-8862

Design Review (Commercial, Manufacturing, and other Main Uses)

Design review allows the Planning Division an opportunity to review specified proposed developments, with the goals established by the General Plan and standards listed in county ordinances, which implement the goals of the General Plan.

A pre-application meeting is required prior to application submittal; please call (801) 399-8791 to make an appointment. Date of pre-application review meeting: _____ Time: _____

- **Staff member assigned to process application:** _____

APPLICATION DEADLINE: Thirty (30) days prior to the applicable Planning Commission meeting

The Western Weber County Township Planning Commission holds their meetings on the 2nd Tuesday of the month.
The Ogden Valley Township Planning Commission holds their meetings on the 4th Tuesday of the month.

First Determination

☐ Is this a small building with a total footprint of less than 10,000 sq ft and a project area of less than one acre

If **Yes,** the application can be approved administratively without Planning Commission review.
If **No,** the application will be reviewed by the Planning Commission.

Application Submittal Checklist

The Planning Division will only accept complete applications with supporting documents as outlined below. Submitting an application does not guarantee that this application will be placed on the next Planning Commission agenda.

The following is required as part of the application form submittal:

☒ Complete Application Form

☐ A non-refundable fee made payable to Weber County (see *Fee Schedule*)

☐ Obtain signature of the owner(s) on the application and any authorized representatives

☐ All documents submitted in the application shall be accompanied by a PDF file of the respective document. All plans (including but not limited to site plans, architectural elevations/renderings, etc), and subsequent submittals and revisions, shall be accompanied by a full scale set of PDF files of the respective plans.

☐ Culinary water and waste water letter

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6. Considerations relating to prior development concept plan approval associated with any Rezoning Agreement, Planned Commercial or Manufacturing Rezoning or Planned Residential Unit Development Approval.

Appeal Process

Appeals of Staff administrative approvals are made to the Planning Commission within 15 days of the decision.

Appeals of the Planning Commission decision are made to the County Commission within 15 days of the decision.

For Your Information

If construction of any development for which design approval has been granted has not been commenced within eighteen months from date of Design Review approval, the approval shall be deemed automatically revoked. Upon application, the Planning Commission may grant an extension of time.

This application can be filled out online at the following Planning Division web site: www.co.weber.ut.us/planning Copies of the applicable Weber County Zoning Ordinances and other helpful information are also available at this web site.

Weber County Planning Division

WEBER COUNTY AGENCY REVIEW OF DESIGN REVIEW (COMMERCIAL, MANUFACTURING, & ADMINISTRATIVE)

PAPER	ELECTRONIC	AGENCY
○	●	ENGINEERING
○	●	BUILDING INSPECTION
○	○	ASSESSORS
○	○	HEALTH
○	●	FIRE

OTHER AGENCY REVIEW

PAPER	ELECTRONIC	AGENCY
○	○	WEBER PATHWAYS
○	○	_____
○	○	_____
○	○	_____
○	○	_____
○	○	_____

--If processing by paper, please respond to this review request **by returning this form** and the attached plan **within 14 days** to:

Weber County Planning Commission, 2380 Washington Blvd., Ste 240, Ogden, UT 84401-1473

--If processing through Miradi, submit your response **within 14 days**

-- If you have any questions or need further information, please call 399-8791, Fax 399-8862

Thank You, **Kary Serrano**