

STRUCTURAL NOTES

A. GENERAL

- 1. THE STRUCTURAL NOTES ARE INTENDED TO COMPLEMENT THE PROJECT SPECIFICATIONS WHICH ARE PART OF THE CONSTRUCTION DOCUMENTS... 2. THESE DRAWINGS (AND, WHERE APPLICABLE, ACCOMPANYING WRITTEN SPECIFICATIONS) ARE THE ONLY CONTRACT DOCUMENTS PROVIDED BY ARCH ENGINEERS FOR THE PROJECT REPRESENTED HEREIN... 3. THE ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS... 4. SEE SPECIFICATIONS FOR REQUIRED SUBMITTALS... 5. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE... 6. THE CONTRACTOR SHALL COORDINATE AND VERIFY ALL LOCATIONS AND SIZES OF MECHANICAL EQUIPMENT... 7. THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST TO THE ARCHITECT FOR ARCHITECT AND/OR ENGINEER APPROVAL BEFORE PROCEEDING WITH ANY CHANGES, MODIFICATIONS, OR SUBSTITUTIONS... 8. OBSERVATION VISITS TO THE SITE BY ARCH ENGINEERS FIELD REPRESENTATIVES SHALL NEITHER BE CONSTRUED OR INTERPRETED AS AN APPROVAL OF THE CONSTRUCTION... 9. DURING AND AFTER CONSTRUCTION, BUILDER AND/OR OWNER SHALL KEEP LOGS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS AS NOTED IN THESE DOCUMENTS... 10. TYPICAL OR SIMILAR DETAILS AND SECTIONS SHALL APPLY WHERE SPECIFIC DETAILS ARE NOT SHOWN... 11. DRAWINGS AND DETAILS PREPARED WITH THE INTENT TO VISUALLY REPRESENT INFORMATION PROVIDED IN SCALED FORM... 12. THE CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY SHORING AND BRACING FOR ALL STRUCTURAL ELEMENTS UNTIL THE ENTIRE STRUCTURAL SYSTEM IS COMPLETED... 13. ENGINEER SHALL NOT BE RESPONSIBLE FOR ACTIVITIES UNDER CONTROL OF THE CONTRACTOR SUCH AS CONSTRUCTION SITE SAFETY... 14. THE TESTING LABORATORY WILL PERFORM VISUAL INSPECTION OF ANCHORS AND DOWELS AS SPECIFIED IN THE SPECIAL INSPECTION SCHEDULE... 15. IN ACCORDANCE WITH IBC 1704.4, THE CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTORS STATEMENT OF RESPONSIBILITY TO THE BUILDING OWNER AND OWNER... 16. THE CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTORS STATEMENT OF RESPONSIBILITY TO THE BUILDING OWNER AND OWNER... 17. THE CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTORS STATEMENT OF RESPONSIBILITY TO THE BUILDING OWNER AND OWNER...

B. STATEMENT OF SPECIAL INSPECTIONS AND SPECIAL SUBMITTALS

- 1. THE DESIGNATED SEISMIC WIND SYSTEMS AND SEISMIC WIND-FORCE-RESISTING SYSTEMS THAT ARE SUBJECT TO SPECIAL INSPECTIONS IN ACCORDANCE WITH IBC SECTION 1705.10 AND 1705.11 ARE IDENTIFIED ON THESE DOCUMENTS WITH A CIRCLE "L". ALL OTHER ITEMS REQUIRING SPECIAL INSPECTION ARE IDENTIFIED IN THE SPECIAL INSPECTION SCHEDULE ON SHEET S303... 2. SPECIAL INSPECTIONS AND TESTING ARE TO BE PROVIDED AS REQUIRED BY IBC SECTIONS 1704 THROUGH 1705 AND OTHER APPLICABLE SECTIONS OF THE IBC... 3. ALL TESTING AND SPECIAL INSPECTION SHALL BE PROVIDED BY A QUALIFIED INDEPENDENT SPECIAL INSPECTION AGENCY IN ACCORDANCE WITH IBC 1704 AND AS OUTLINED IN THE JOB SPECIFICATIONS... 4. STRUCTURAL OBSERVATION VISITS SHALL BE PERFORMED BY A REPRESENTATIVE FROM ARCH ENGINEERS IN ACCORDANCE WITH THE CONTRACT AS NEEDED TO OBSERVE THE CONSTRUCTION OF CRITICAL BUILDING ELEMENTS... 5. IN ACCORDANCE WITH IBC 1704.4, THE CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTORS STATEMENT OF RESPONSIBILITY TO THE BUILDING OWNER AND OWNER... 6. THE CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTORS STATEMENT OF RESPONSIBILITY TO THE BUILDING OWNER AND OWNER...

C. BASIS OF DESIGN

- 1. GOVERNING BUILDING CODE: INTERNATIONAL BUILDING CODE (IBC) 2012 RISK CATEGORY: II... 2. SUSPENDED FLOOR LOADS a. LIVE LOAD = 40 PSF b. DEAD LOAD = 39 PSF... 3. ROOF LOADS a. FLAT ROOF SNOW LOAD, Pf: 184 PSF 1) GROUND SNOW LOAD, Pg: 282 PSF 2) SNOW EXPOSURE FACTOR, Ce: 1.0 3) SNOW LOAD IMPORTANCE FACTOR, Is: 1.0 4) THERMAL FACTOR, Ct: 1.0 (1.2 @ OVERHANGS) b. LEVEL LOAD = 20 PSF MORE THAN 20" TO FOOTING c. DEAD LOAD = 88 PSF (GREEN ROOFS), S1 PSF (PATIOS)... 4. WIND DESIGN a. BASIC WIND SPEED (3 SECOND GUST): 115 MPH b. WIND EXPOSURE: C c. COMPONENT AND CLADDING DESIGN WIND PRESSURE SHALL BE AS REQUIRED PER ASCE 7-10... 5. SEISMIC DESIGN a. SEISMIC IMPORTANCE FACTOR, Ie: 1.0 b. SITE CLASS: B c. MAPPED SPECTRAL RESPONSE ACCELERATIONS: Ss = 0.815, S1 = 0.271 d. SPECTRAL RESPONSE COEFFICIENTS: SDS = 0.543, SD1 = 0.181 e. SEISMIC DESIGN CATEGORY: D f. BASIC SEISMIC FORCE-RESISTING SYSTEM: SPECIAL REINFORCED CONCRETE SHEAR WALLS, LIGHT FRAMED WOOD SHEAR WALLS, SPECIAL MOMENT FRAMES g. DESIGN BASE SHEAR: Vn = S5 KIPS, Vw = S5 KIPS h. SEISMIC RESPONSE COEFFICIENT: Cs: 0.11 i. RESPONSE MODIFICATION FACTOR: R: 5.0 j. ANALYSIS PROCEDURE: STATIC

D. FOUNDATION

- 1. DESIGN SOIL PRESSURE: 2500 PSF... 2. SOILS REPORT BY: IGES REPORT # 02052-01 DATED: MAY 28, 2015... 3. SOIL PREPARATION UNDER FOOTINGS AND SLABS ON GRADE SHALL BE IN ACCORDANCE WITH THE SOILS REPORT... 4. TOP OF FOOTING ELEVATIONS SHOWN ON THE FOOTING AND FOUNDATION PLAN ARE BASED ON PRELIMINARY GRADING INFORMATION AND MUST BE VERIFIED PRIOR TO CONSTRUCTION... 5. ALL WALLS (EXCEPT CANTILEVERED RETAINING WALLS) SHALL BE ADEQUATELY BRACED AGAINST LATERAL MOVEMENT PRIOR TO BACKFILLING... 6. UNLESS NOTED OTHERWISE, ALL FOOTINGS AT COLUMNS TO BE CENTERED BELOW COLUMNS... 7. UNLESS NOTED OTHERWISE, ALL FOOTINGS SHALL HAVE VERTICAL FACES FORMED WITH STANDARD FORMING MATERIALS (WOOD, METAL, ETC.) WITH PRIOR APPROVAL OF ARCHITECT AND ENGINEER... 8. UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL CONCRETE FOUNDATION WALLS SHALL BE AS FOLLOWS:

E. CONCRETE

- 1. ALL CONCRETE MIX DESIGNS SHALL COMPLY WITH THE PROJECT SPECIFICATIONS AND THE REQUIREMENTS LISTED BELOW: a. FOOTINGS, GRADE BEAMS, FOUNDATION WALLS: 1) 28 DAY COMPRESSIVE STRENGTH: 3000 PSI b. RETAINING WALLS (EXPOSURE CATEGORY F1): 1) 28 DAY COMPRESSIVE STRENGTH: 4500 PSI c. MAXIMUM W/C RATIO: 0.45 d. MAXIMUM AGGREGATE SIZE: 1" e. AIR CONTENT: 6% f. INTERIOR SLABS ON GRADE (EXPOSURE CATEGORY F0): 1) 28 DAY COMPRESSIVE STRENGTH: 3000 PSI 2) INTERIOR SUSPENDED SLABS (EXPOSURE CATEGORY F0): 1) 28 DAY COMPRESSIVE STRENGTH: 3000 PSI e. EXTERIOR SLABS (DOCKS, ETC.) (EXPOSURE CATEGORY F1): 1) 28 DAY COMPRESSIVE STRENGTH: 4500 PSI 2) MAXIMUM W/C RATIO: 0.45 3) MAXIMUM AGGREGATE SIZE: 1" 4) MINIMUM AIR CONTENT: 8% f. WATER USED IN MIXING CONCRETE SHALL CONFORM TO ASTM C1602 g. NO PIPES, DUCTS, SLEEVES, ETC. SHALL BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER... 2. UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL CONCRETE FOUNDATION WALLS SHALL BE AS FOLLOWS:

Table with 4 columns: TOP & THICKNESS, BOTTOM BARS, VERTICAL, HORIZONTAL. It lists reinforcement specifications for different wall types and thicknesses.

- 8. CONSTRUCTION JOINTS NOT SHOWN ON THE PLANS SHALL BE MADE AND LOCATED SO AS TO NOT IMPAIR THE STRENGTH OF THE STRUCTURE AND AS APPROVED BY THE STRUCTURAL ENGINEER... F. ANCHOR BOLTS/EMBEDDED BOLTS 1. ALL ANCHOR BOLTS SHALL HAVE ASTM A-363 HEAVY HEX NUT AND ASTM F-436 WASHERS AT STANDARD OR OVERSIZED... 2. ALL ANCHOR BOLTS SHALL HAVE ASTM A-363 HEAVY HEX NUT AND ASTM F-436 WASHERS AT STANDARD OR OVERSIZED... 3. ALL ANCHOR BOLTS SHALL HAVE ASTM A-363 HEAVY HEX NUT AND ASTM F-436 WASHERS AT STANDARD OR OVERSIZED... G. ADHESIVE/MECHANICAL ANCHORS 1. ALL ADHESIVE/MECHANICAL ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH AN APPROVED INDEPENDENT EVALUATION REPORT (APRO, OR APPROVED EQUAL), AS INDICATED BELOW, AND IN ACCORDANCE WITH ALL MANUFACTURERS REQUIREMENTS... 2. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION... 3. UNLESS NOTED OTHERWISE, ALL ADHESIVE ANCHORS INTO CONCRETE SHALL BE: a. HILTI HIT-RE 500-SD (ESR-2322), OR HILTI HIT-HY 200 (ESR-3187), b. SIMPSON SET XP EPOXY (ESR-2508)...

H. REINFORCING STEEL

- 1. REINFORCING BAR STRENGTH REQUIREMENTS: a. ALL REINFORCING BARS EXCEPT AS INDICATED IN NOTE 5, SHALL CONFORM TO ASTM STANDARD A615 GRADE 60 AND ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM STANDARD A-185 AND SHALL BE SUPPLIED IN FLAT SHEETS... b. ALL REINFORCING BARS FOR SPECIAL REINFORCED CONCRETE SHEARWALLS SHALL CONFORM TO ASTM A706 GR. 60... 2. HEADED SHEAR STUD ASSEMBLIES SHALL CONFORM TO ASTM A1044... 3. STEEL DISCONTINUOUS FIBER REINFORCEMENT SHALL BE DEFORMED AND CONFORM TO ASTM A820 AND SHALL HAVE A LENGTH TO DIAMETER RATIO NOT SMALLER THAN 50 AND NOT GREATER THAN 100... 4. HEADED DEFORMED BARS SHALL CONFORM TO ASTM A970... 5. ALL FIELD BENT REINFORCING STEEL SHALL BE GROUPED TO THE SPREAD OR THE SPECIAL INSPECTOR... 6. UNLESS NOTED OTHERWISE, REINFORCEMENT SHALL HAVE THE FOLLOWING CONCRETE COVERAGE: a. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: "3" b. EXPOSED TO EARTH OR WEATHER: "1" #6 & LARGER "2" #5 & SMALLER "1-1/2" c. NOT EXPOSED TO WEATHER OR EARTH: 1) SLABS, WALLS, JOISTS, #11 & SMALLER "3/4" 2) BEAMS, COLUMNS, MAIN REINFORCING OR TIES "1-1/2" d. SLAB ON GRADE: 1) PLACE REINFORCING AT CENTER OF SLAB UNLESS INDICATED OTHERWISE 2) EXCEPT WHERE NOTED ON PLANS OR DETAILS CONTINUOUS REINFORCEMENT SHALL BE SPACED AT POINTS OF MINIMUM STRESS BY LAPPING PER THE REBAR LAP SCHEDULE... 7. REINFORCING STEEL MAY BE SPLICED WITH MECHANICAL COUPLERS THAT HAVE A TENSION CAPACITY OF AT LEAST 125% OF THE STRENGTH OF THE BAR... 8. ALL VERTICAL REINFORCING STRUCTURE WITH MATCHING DOWELS EMBEDDED WITHIN THE FOOTINGS OR STRUCTURE BELOW... 9. DO NOT WELD REINFORCING EXCEPT AS NOTED ON PLANS... 10. REINFORCING BARS, TIES, AND TENDONS SHALL BE SUPPORTED BY NYLON CONES, PLASTIC COATED THE WIRES, OR PLASTIC-COATED CHAIRS... 11. UNLESS NOTED OTHERWISE, HOOKS, STIRRUPS, TIES, AND OTHER BENDS IN REINFORCING STEEL SHALL MEET THE STANDARDS SET FORTH IN ACI 318/08/11... 12. UNLESS NOTED OTHERWISE, REINFORCING SHALL BE BENT COULD AND WELDED IN CONCRETE SHALL NOT BE FIELD BENT... 13. UNLESS SPECIFICALLY NOTED AND/OR DETAILED IN THE STRUCTURAL DRAWINGS CONDUIT SHALL NOT BE IN CONTACT WITH REINFORCING STEEL.

I. STRUCTURAL STEEL

- 1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE FOLLOWING: a. ANSIAISC 360-10 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", WITH "COMMENTARY" AND "SUPPLEMENTS" AS REQUIRED BY BUILDING CODE... b. AISC 303-10 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" EXCLUDING THE FOLLOWING SECTIONS: 4.4, 4.4.1, AND 4.4.2... c. AISI "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" d. AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" e. AWS D1.1 AND 1.3, "STRUCTURAL WELDING CODE" (EXCEPT SPECIFIC ITEMS DO NOT APPLY IF THEY CONFLICT WITH AISI) f. ANSIAISC 341-10 "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS" 2. STRUCTURAL STEEL SHALL COMPLY WITH THE FOLLOWING: a. WIDE FLANGE SHAPES AND WELDED SHAPES: ASTM A992 b. OTHER SHAPES AND PLATES: ASTM A-36 (UNO) c. TUBES (TS) AND HOLLOW STRUCTURAL SECTIONS (HSS): ASTM A-500, GRADE B (SQUARE AND RECTANGULAR SHAPES) OR ASTM A-501, GRADE B (CIRCULAR SHAPES) d. PIPE COLUMNS: ASTM A-53, GRADE B TYPE E OR S e. STAINLESS STEEL SHAPES, PLATES, AND FASTENERS: ASTM 304 f. DEFORMED BAR ANCHORS (DBA): ASTM A-496, WELDED IN ACCORDANCE WITH AWS D1.1 g. HEADED STUD ANCHORS (HSA): ASTM D-1180 AND WELDED IN ACCORDANCE WITH AWS D1.1 FOR TYPE "B" USE 3/4" DIAMETER STUDS, UNLESS NOTED OTHERWISE... 3. CONNECTIONS SHALL COMPLY WITH THE STRUCTURAL DRAWINGS UNLESS WRITTEN APPROVAL TO CHANGE IS GIVEN BY THE STRUCTURAL ENGINEER... 4. ALL SHOP FABRICATIONS SHALL BE PERFORMED BY AN APPROVED FABRICATOR IN ACCORDANCE WITH SECTIONS 1702 AND 1704 OF THE IBC OR WITH SHOP INSPECTION BY AN INDEPENDENT AGENCY... 5. WELDING a. ALL WELDING AND CUTTING SHALL BE PERFORMED BY AWS QUALIFIED WELDERS IN ACCORDANCE WITH ANSIAWS D1.1 (LATEST EDITION)... b. USE E-70XX ELECTRODES UNLESS NOTED OTHERWISE... c. ALL INTERSECTING STEEL SHAPES WHICH ARE NOT CONNECTED WITH BOLTS SHALL BE WELDED TOGETHER WITH A FILLET WELD... 6. BOLTING a. UNLESS NOTED OTHERWISE, ALL STRUCTURAL STEEL TO STEEL CONNECTIONS SHALL USE HIGH STRENGTH BOLTS CONFORMING TO ASTM A-325... b. UNLESS NOTED OTHERWISE, ALL BOLTING IS CLASSIFIED AS NON-SLIP CRITICAL BEARING TYPE CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLATE... 7. UNLESS NOTED OTHERWISE, WHERE STEEL BEAMS SUPPORT WOOD FRAMING OR WOOD SHEATHING, PROVIDE A CONTINUOUS DOUBLE 2x OR SINGLE 3x NAILER PLATE ON THE TOP OF THE BEAM THAT EXTENDS AT LEAST THE FULL WIDTH OF THE BEAM FLANGE... 8. ALL COLUMNS ADJACENT TO OR EMBEDDED IN WOOD STUD WALLS SHALL HAVE (1) 1/2" DIAMETER X 3'-1/2" THREADED STEEL ROD SHOP WELDED TO THE FACE OF THE COLUMN AND EXTENDING EACH WAY INTO THE ADJACENT STUD WALLS... 9. SECONDARY FRAMING POINTS, STIFFENER PLATES SHALL BE THICKNESS SHOWN UNLESS NOTED OTHERWISE AND SHALL BE WELDED BOTH SIDES WITH FILLET WELDS ALL AROUND... 10. UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL CONCRETE FOUNDATION WALLS SHALL BE AS FOLLOWS:

- 10. FABRICATORS AND SUPPLIERS SHALL COORDINATE PAINT FINISHES WITH REQUIREMENTS FOR DIRECT APPLIED INSULATION, FIREPROOFING, ETC. AS NOTED IN THE PROJECT SPECIFICATIONS... 11. WHEN DETERMINING THE FIRE RESISTANCE OF ASSEMBLIES, USE THE FOLLOWING: STEEL ROOF MEMBERS ARE CONSIDERED UNRESTRAINED AND STEEL FLOOR FRAMING MEMBERS ARE CONSIDERED RESTRAINED... 12. UNLESS NOTED OTHERWISE, ALL HORIZONTAL FRAMING MEMBERS SHALL BE ERECTED WITH THE NATURAL CROWN UP... 13. UNLESS OTHERWISE SHOWN OR DETAILED IN THE PLANS, ALL STEEL COLUMNS, BEAMS, BRACES, STRUTS, ETC. SHALL BE CONTINUOUS BETWEEN CONNECTIONS OR SUPPORTS... J. MOMENT FRAMES 1. STRUCTURAL STEEL IN MOMENT FRAMES SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC SPECIFICATIONS NOTED IN THE STRUCTURAL STEEL NOTES ABOVE... 2. STRUCTURAL STEEL IN MOMENT FRAMES SHALL COMPLY WITH THE REQUIREMENTS NOTED IN THE STRUCTURAL STEEL NOTES ABOVE... 3. UNLESS OTHERWISE SHOWN OR DETAILED IN THE PLANS, ALL STEEL COLUMNS, BEAMS, BRACES, STRUTS, ETC. SHALL BE CONTINUOUS BETWEEN CONNECTIONS OR SUPPORTS... 4. WELDED CONNECTIONS BETWEEN THE PRIMARY MEMBERS OF MOMENT FRAMES SHALL BE TESTED FOR COMPLIANCE... 5. ALL COMPLETE-JOINT-PENETRATION WELDS USED IN MOMENT FRAMES SHALL BE MADE WITH A FILLER METAL THAT CAN PRODUCE WELDS THAT HAVE A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT-LB AT 0 DEGREES F, AS DETERMINED AS DETERMINED BY THE APPROPRIATE AWS AS CLASSIFICATION TEST METHOD OR MANUFACTURER CERTIFICATION... 7. IN SPECIAL AND INTERMEDIATE MOMENT FRAMES THE REGION AT THE END OF THE BEAM AS SHOWN IN DETAIL 25531 IS REQUIRED AS A PROTECTIVE MEASURE... 8. UNLESS OTHERWISE SHOWN OR DETAILED IN THE PLANS, ALL STEEL COLUMNS, BEAMS, BRACES, STRUTS, ETC. SHALL BE CONTINUOUS BETWEEN CONNECTIONS OR SUPPORTS... 9. UNLESS OTHERWISE SHOWN OR DETAILED IN THE PLANS, ALL STEEL COLUMNS, BEAMS, BRACES, STRUTS, ETC. SHALL BE CONTINUOUS BETWEEN CONNECTIONS OR SUPPORTS... 10. UNLESS OTHERWISE SHOWN OR DETAILED IN THE PLANS, ALL STEEL COLUMNS, BEAMS, BRACES, STRUTS, ETC. SHALL BE CONTINUOUS BETWEEN CONNECTIONS OR SUPPORTS... 11. UNLESS OTHERWISE SHOWN OR DETAILED IN THE PLANS, ALL STEEL COLUMNS, BEAMS, BRACES, STRUTS, ETC. SHALL BE CONTINUOUS BETWEEN CONNECTIONS OR SUPPORTS... 12. UNLESS OTHERWISE SHOWN OR DETAILED IN THE PLANS, ALL STEEL COLUMNS, BEAMS, BRACES, STRUTS, ETC. SHALL BE CONTINUOUS BETWEEN CONNECTIONS OR SUPPORTS... 13. UNLESS OTHERWISE SHOWN OR DETAILED IN THE PLANS, ALL STEEL COLUMNS, BEAMS, BRACES, STRUTS, ETC. SHALL BE CONTINUOUS BETWEEN CONNECTIONS OR SUPPORTS...

K. TIMBER

- 1. WOOD GRADES (UNLESS NOTED OTHERWISE) a. ALL FRAMING LUMBER SHALL BE DOUGLAS FIR/LARCH CLEARLY MARKED WITH A STAMP BY WWPA APPROVED AGENCY AND SHALL BE GROUPED TO THE SPREAD OR THE SPECIAL INSPECTOR... b. ALL FRAMING IN CONTACT WITH FOOTINGS, FOUNDATIONS, OR SLABS ON GRADE SHALL BE PRESURE TREATED OR TIMBER TRAND LSL... c. UNLESS NOTED OTHERWISE, ALL ENGINEERED LUMBER SHALL BE FURNISHED BY TRUS-JOIST CORPORATION OR APPROVED EQUAL... d. ALL WALL SHEATHING SHALL BE FASTENED TO THE WALL FRAMING PER THE WOOD SHEAR WALL SCHEDULE ON SHEET S302... 2. SHEATHING SHALL BE APA RATED SHEATHING, EXPOSURE I, TRUS-JOIST AND PANEL INDEX RATING AS NOTED BELOW UNLESS NOTED OTHERWISE... 3. INDIVIDUAL PIECES OF SHEATHING AT ROOF, FLOOR, AND SHEAR WALLS SHALL NOT BE SMALLER THAN 24" IN EITHER DIRECTION AND SHALL SPAN A MINIMUM OF TWO FRAMING SPACES, UNO... 4. ALL WALL SHEATHING SHALL BE FASTENED TO THE WALL FRAMING PER THE WOOD SHEAR WALL SCHEDULE ON SHEET S302... 5. ALL WALLS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES: LVL: 1,900,000 PSI PSL: 2,000,000 PSI LSL: 1,500,000 PSI... 6. A CONTINUOUS BEAD OF PERMANENT BOND TIMBERWOOD ADHESIVE COMPOUND SHALL BE USED TO FASTEN ALL PLYWOOD FLOOR SHEATHING TO FLOOR JOISTS... 7. UNLESS NOTED OTHERWISE, ALL WALL BOTTOM PLATES TO BE ANCHORED TO FOUNDATIONS OR FOOTINGS WITH 3/4" DIAMETER ANCHOR BOLTS AT 32" O.C. WITH 8" MINIMUM EMBEDMENT... 8. UNLESS NOTED OTHERWISE, ALL ROOF AND FLOOR SHEATHING AND WALL SHEATHING AT SHEAR WALLS SHALL HAVE SOLID BLOCKING AT ALL PANEL EDGES... 9. PROVIDE DOUBLE JOIST UNDER PARALLEL NONBEARING WALLS AND SOLID BLOCKING UNDER PERPENDICULAR NONBEARING WALLS... 10. AT ALL OVERBUILD LOCATIONS, ROOF SHEATHING SHALL BE COMPLETE BOLD OVERBUILDS PRIOR TO OVERBUILD CONSTRUCTION... 11. PROVIDE SOLID 2" (NOMINAL) FULL DEPTH BLOCKING AT ENDS AND SUPPORT LOCATIONS FOR ALL JOISTS AND RAFTERS... 12. UNLESS NOTED OTHERWISE, ALL BEARING WALLS SHALL BE 2x6 SPACED AT 16" O.C. BLOCK ALL NON-SHEATHED BEARING WALLS AT 16" O.C... 13. VERIFY THE STUD SPACING WITH THE ANCHOR BOLT LAY-OUT... 14. EXTERIOR WALLS SHALL HAVE DOUBLE 2x TOP PLATES SPLICED WITH A MINIMUM OF 48" OF OVERLAP AND SHALL BE CONNECTED WITH A MINIMUM OF (12) 16d NAILS... 15. EXCEPT WHERE NOTED OTHERWISE, THE NUMBER AND SIZE OF NAILS CONNECTING WOOD MEMBERS SHALL NOT BE LESS THAN THAT SET FORTH IN IBC TABLE 2304.4.1... 16. UNLESS NOTED OTHERWISE, ALL HORIZONTAL FRAMING MEMBERS SHALL BE INSTALLED WITH THE NATURAL CROWN UP... 17. UNLESS NOTED OTHERWISE, ALL HORIZONTAL FRAMING MEMBERS SHALL BE INSTALLED WITH THE NATURAL CROWN UP... 18. UNLESS NOTED OTHERWISE, ALL HORIZONTAL FRAMING MEMBERS SHALL BE INSTALLED WITH THE NATURAL CROWN UP... 19. UNLESS NOTED OTHERWISE, ALL HORIZONTAL FRAMING MEMBERS SHALL BE INSTALLED WITH THE NATURAL CROWN UP... 20. UNLESS NOTED OTHERWISE, ALL HORIZONTAL FRAMING MEMBERS SHALL BE INSTALLED WITH THE NATURAL CROWN UP...

- 2. DEFERRED SUBMITTALS 1. DEFERRED SUBMITTALS ARE COMPLETE PACKAGES TO BE SUBMITTED FOR REVIEW THAT INCLUDE DRAWINGS AND CALCULATIONS FOR ALL ELEMENTS AND CONNECTIONS OF ITEMS LISTED BELOW... 2. DEFERRED SUBMITTALS SHALL INCLUDE, BUT ARE NOT LIMITED TO: a. SEISMIC BRACING OF ALL ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL ITEMS WHERE REQUIRED BY ASCE 7-10 AND THE PROJECT CONTRACT DOCUMENTS b. PERMANENT WALL SHORING

LEGEND OF SYMBOLS AND ABBREVIATIONS

Table with 2 columns: Symbol and Description. It lists various construction symbols and their corresponding abbreviations, such as AB for Anchor Bolt, ARCH for Architect, and various steel and concrete symbols.

39 SUMMIT, LLC  
Summit at Powder Mountain Lot 39  
8355 E. SUMMIT PASS

39 SUMMIT, LLC  
Summit at Powder Mountain Lot 39  
8355 E. SUMMIT PASS

June 26, 2015  
November 2, 2015  
July 27, 2017  
September 22, 2017

FOR CONSTRUCTION

Structural Sheet Index table with columns: SHEET NUMBER and SHEET NAME. It lists sheets S001 through S401 and their corresponding titles, such as Structural Notes, Schedules, Footing and Foundation Plan, etc.

S001



**STRUCTURAL STEEL SPECIAL INSPECTION SCHEDULE**

ESTABLISHED PER 2012 IBC SECTION 1705.2.1

INSPECTION TASKS PRIOR TO WELDING (TABLE N5.4-1)	FABRICATOR QUALITY CONTROL		SPECIAL INSPECTOR QUALITY ASSURANCE		NOTES	INSPECTION TASKS PRIOR TO BOLTING (TABLE N5.6-1)				NOTES										
	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC											
WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	●		●		1. PERIODIC - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS. 2. CONTINUOUS - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER. 3. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR. 4. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), APPLICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE, EXCEPT AS PERMITTED IN ACCORDANCE WITH SECTION N7. 5. QC AND QA INSPECTORS SHALL BE QUALIFIED IN ACCORDANCE WITH AISC 360-10 CHAPTER N4. 6. NONDESTRUCTIVE TESTING PERSONNEL SHALL BE QUALIFIED IN ACCORDANCE WITH AISC 360-10 CHAPTER N4.3. 7. NONDESTRUCTIVE TESTING OF WELDED JOINTS SHALL COMPLY WITH AISC 360-10 CHAPTER N5a AND b. 8. OBSERVATION OF WELDING OPERATIONS AND VISUAL INSPECTION OF IN-PROCESS AND COMPLETED WELDS SHALL BE THE PRIMARY METHOD TO CONFIRM THAT THE MATERIALS, PROCEDURES AND WORKMANSHIP ARE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS. FOR STRUCTURAL STEEL, ALL PROVISIONS OF AWS D1.1 / D1.1M STRUCTURAL WELDING CODE - STEEL FOR STATICALLY LOADED STRUCTURES SHALL APPLY. 9. THERMALLY CUT SURFACES OF ACCESS HOLES SHALL BE TESTED BY QA USING MT OR PT, WHEN THE FLANGE THICKNESS EXCEEDS 2 IN. (50mm) FOR ROLLED SHAPES, OR WHEN THE WEB THICKNESS EXCEEDS 2 IN. (50mm) FOR BUILT-UP SHAPES. ANY CRACK SHALL BE DEEMED UNACCEPTABLE REGARDLESS OF SIZE OR LOCATION. 10. WHEN REQUIRED BY APPENDIX 3, TABLE A-3.1, WELDED JOINTS REQUIRING WELD SOUNDNESS TO BE ESTABLISHED BY RADIOGRAPHICS OR ULTRASONIC INSPECTION SHALL BE TESTED BY QA AS PRESCRIBED. REDUCTION IN THE RATE OF UT IS PROHIBITED. 11. REDUCTION OF RATE OF ULTRASONIC TESTING - THE RATE OF UT IS ONLY PERMITTED TO BE REDUCED IF APPROVED BY THE EOR AND THE AHJ PER AISC 360-10 CHAPTER N5g. 12. FOR STRUCTURES IN RISK CATEGORY II, WHERE THE INITIAL RATE FOR UT IS 10%, THE NOT RATE FOR AN INDIVIDUAL WELDER OR WELDING OPERATOR SHALL BE INCREASED TO 100% SHOULD THE REJECT RATE, THE NUMBER OF WELDS CONTAINING UNACCEPTABLE DEFECTS DIVIDED BY THE NUMBER OF WELDS COMPLETED, EXCEEDS 5% OF THE WELDS TESTED FOR THE WELDER OR WELDING OPERATOR. A SAMPLING OF AT LEAST 20 COMPLETED WELDS FOR A JOB SHALL BE MADE PRIOR TO IMPLEMENTING SUCH AN INCREASE. WHEN THE REJECT RATE FOR THE WELDER OR WELDING OPERATOR, AFTER A SAMPLING OF AT LEAST 40 COMPLETED WELDS, HAS FALLEN TO 5% OR LESS, THE RATE OF UT SHALL BE RETURNED TO 10%. FOR EVALUATING THE REJECT RATE OF CONTINUOUS WELDS OVER 3 FT (1M) IN LENGTH WHERE THE EFFECTIVE THROAT IS 1 IN. (25mm) OR LESS, EACH 12 IN. (300mm) INCREMENT OR FRACTION THEREOF SHALL BE CONSIDERED AS ONE WELD. FOR EVALUATING THE REJECT RATE ON CONTINUOUS WELDS OVER 3 FT (1M) IN LENGTH WHERE THE EFFECTIVE THROAT IS GREATER THAN 1 IN. (25mm), EACH 6 IN. (150mm) OF LENGTH OR FRACTION THEREOF SHALL BE CONSIDERED ON WELD. 13. ALL NDT PERFORMED SHALL BE DOCUMENTED. FOR SHOP FABRICATION, THE NDT REPORT SHALL IDENTIFY THE TESTED WELD BY PIECE MARK AND LOCATION IN THE PIECE. FOR FIELD WORK, THE NDT REPORT SHALL IDENTIFY THE TESTED WELD BY LOCATION IN THE STRUCTURE, PIECE MARK, AND LOCATION IN THE PIECE. WHEN A WELD IS REJECTED ON THE BASIS OF NDT, THE NDT RECORD SHALL INDICATE THE LOCATION OF THE DEFECT AND THE BASIS OF REJECTION. 14. DEMAND CRITICAL WELDS SHALL MEET THE PROVISION FOUND IN AISC 341-10 AND WELDING METHODS, PROCEDURES AND QUALITY CONTROL SHALL COMPLY WITH AWS D1.1 AND THE FOLLOWING: a. ARC STRIKES, GOUGES AND OTHER IMPERFECTIONS WITHIN OR ADJACENT TO THE JOINT, SHALL BE REPAIRED OR REMOVED. b. PREHEAT AND INTER-PASS REQUIREMENTS AS OUTLINED IN SECTION 5.5. c. UNREPAIRED CRACKS, GOUGES, AND NOTCHES WILL NOT BE PERMITTED IN THE JOINT AREA. d. USE ELECTRODES WITH CHARPY V-NOTCH ABSORBED ENERGY EQUAL TO OR GREATER THAN 20 FT-LB AT -20 DEGREES FAHRENHEIT UNDER AWS AS CLASSIFICATION TEST METHODS, AND 40 FT-LBS AT 70 DEGREES FAHRENHEIT USING TEST PROCEDURES PRESCRIBED IN APPENDIX X OF AISC 358. ACCEPTABLE ELECTRODES INCLUDE E70T-K2, E71 T-1.	●	●	●	●	●	●	●	●							
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	●		●			MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	●	●	●	●	1. PERIODIC - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS. 2. CONTINUOUS - PERFORM THESE TASKS FOR EACH BOLTED CONNECTION. 3. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR. 4. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), APPLICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE, EXCEPT AS PERMITTED IN ACCORDANCE WITH SECTION N7. 5. FOR SNUG-TIGHT JOINTS, PRE-INSTALLATION VERIFICATION TESTING AS SPECIFIED IN TABLE N5.6-1 AND MONITORING OF THE INSTALLATION PROCEDURES AS SPECIFIED IN TABLE N5.6-2 ARE NOT APPLICABLE. THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS IN SNUG-TIGHT JOINTS. 6. FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE TURN-OF-NUT METHOD WITH MATCH-MARKING TECHNIQUES, THE DIRECT-TENSION INDICATOR METHOD, OR THE TWIST-OFF-TYPE TENSION CONTROL BOLT METHOD, MONITORING OF BOLT PRETENSIONING PROCEDURES SHALL BE AS SPECIFIED IN TABLE N5.6-2. THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS WHEN THESE METHODS ARE USED BY THE INSTALLER. 7. FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE CALIBRATED WRENCH METHOD OR THE TURN-OF-NUT METHOD WITHOUT MATCH-MARKING, MONITORING OF BOLT PRETENSIONING PROCEDURES SHALL BE AS SPECIFIED IN TABLE N5.6-2. THE QCI AND QAI SHALL BE ENGAGED IN THEIR ASSIGNED INSPECTION DUTIES DURING INSTALLATION OF FASTENERS WHEN THESE METHODS ARE USED BY THE INSTALLER. 8. OBSERVATION OF BOLTING OPERATIONS SHALL BE THE PRIMARY METHOD USED TO CONFIRM THAT THE MATERIALS, PROCEDURES AND WORKMANSHIP INCORPORATED IN CONSTRUCTION ARE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS AND THE PROVISIONS OF THE RCSC SPECIFICATION.									
MATERIAL IDENTIFICATION (TYPE / GRADE)	●		●			FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	●	●	●	●										
WELDER IDENTIFICATION SYSTEM <sup>1</sup>	●		●			PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	●	●	●	●										
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)	●		●			PROPER BOLTING PROCEDURES SELECTED FOR JOINT DETAIL	●	●	●	●										
* JOINT PREPARATION	●		●			CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FINISH SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	●	●	●	●										
* DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)	●		●			PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	●	●	●	●										
* CLEANLINESS (CONDITION OF STEEL SURFACES)	●		●			PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	●	●	●	●										
* TACKING (TACK WELD QUALITY AND LOCATION)	●		●				●	●	●	●										
* BACKING TYPE AND FIT (IF APPLICABLE)	●		●				●	●	●	●										
CONFIGURATION AND FINISH OF ACCESS HOLES	●		●				●	●	●	●										
FIT-UP OF FILLET WELDS	●		●				●	●	●	●										
* DIMENSIONS (ALIGNMENT, GAPS AT ROOT)	●		●				●	●	●	●										
* CLEANLINESS (CONDITION OF STEEL SURFACES)	●		●			●	●	●	●											
* TACKING (TACK WELD QUALITY AND LOCATION)	●		●			●	●	●	●											
CHECK WELDING EQUIPMENT	●		●			●	●	●	●											
<sup>1</sup> THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.						<b>INSPECTION TASKS DURING WELDING (TABLE N5.4-2)</b>				<b>INSPECTION TASKS AFTER BOLTING (TABLE N5.6-2)</b>										
USE OF QUALIFIED WELDERS		●		●																
CONTROL AND HANDLING OF WELDING CONSUMABLES		●		●																
* PACKAGING		●		●																
* EXPOSURE CONTROL		●		●																
NO WELDING OVER CRACKED TACK WELDS		●		●																
ENVIRONMENTAL CONDITIONS		●		●																
* WIND SPEED WITHIN LIMITS		●		●																
* PRECIPITATION AND TEMPERATURE		●		●																
WPS FOLLOWED		●		●																
* SETTINGS ON WELDING EQUIPMENT		●		●																
* TRAVEL SPEED		●		●																
* SELECTED WELDING MATERIALS		●		●																
* SHIELDING GAS TYPE / FLOW RATE		●		●																
* PREHEAT APPLIED		●		●																
* INTERPASS TEMPERATURE MAINTAINED (MIN. / MAX)		●		●																
* PROPER POSITION (F, V, H, OH)		●		●																
WELDING TECHNIQUES		●		●																
* INTERPASS AND FINAL CLEANING		●		●																
* EACH PASS WITHIN PROFILE LIMITATIONS		●		●																
* EACH PASS MEETS QUALITY REQUIREMENTS		●		●																
<b>INSPECTION TASKS AFTER WELDING (TABLE N5.4-3)</b>						<b>INSPECTION TASKS AFTER BOLTING (TABLE N5.6-3)</b>				<b>INSPECTION OF STEEL ELEMENTS OF COMPOSITE CONSTRUCTION PRIOR TO CONCRETE PLACEMENT (TABLE N6.1)</b>										
WELDS CLEANED		●		●																
SIZE, LENGTH AND LOCATION OF WELDS		●		●																
WELDS MEET VISUAL ACCEPTANCE CRITERIA		●		●																
* CRACK PROHIBITION		●		●																
* WELD / BASE-METAL FUSION		●		●																
* CRATER CROSS SECTION		●		●																
* WELD PROFILES		●		●																
* WELD SIZE		●		●																
* UNDERCUT		●		●																
* POROSITY		●		●																
ARC STRIKES		●		●																
K-AREA <sup>1</sup>		●		●																
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)		●		●																
REPAIR ACTIVITIES		●		●																
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER		●		●																
<sup>1</sup> WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. (75mm) OF THE WELD.						<b>GENERAL STEEL SPECIAL INSPECTION NOTES :</b>				1. QUALITY ASSURANCE (QA) INSPECTION OF FABRICATED ITEMS SHALL BE MADE AT THE FABRICATOR'S PLANT. THE QUALITY ASSURANCE INSPECTOR (QAI) SHALL SCHEDULE THIS WORK TO MINIMIZE INTERRUPTION TO THE WORK OF THE FABRICATOR. 2. QA INSPECTION OF THE ERECTED STEEL SYSTEM SHALL BE MADE AT THE PROJECT SITE. THE QAI SHALL SCHEDULE THIS WORK TO MINIMIZE INTERRUPTION TO THE WORK OF THE ERECTOR. 3. WHERE A TASK IS NOTED TO BE PERFORMED BY BOTH QC AND QA, IT IS PERMITTED TO COORDINATE THE INSPECTION FUNCTION BETWEEN THE QCI AND QAI SO THAT THE INSPECTION FUNCTIONS ARE PERFORMED BY ONLY ONE PARTY. WHERE QA RELIES UPON INSPECTION FUNCTIONS PERFORMED BY QC, THE APPROVAL OF THE ENGINEER OF RECORD AND THE AUTHORITY HAVING JURISDICTION IS REQUIRED. 4. THE FABRICATOR'S QCI SHALL INSPECT THE FABRICATED STEEL TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE SHOP DRAWINGS, SUCH AS PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. THE ERECTOR'S QCI SHALL INSPECT THE ERECTED STEEL FRAME TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE ERECTION DRAWINGS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. 5. THE QAI SHALL BE ON THE PREMISES FOR INSPECTION DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. AS A MINIMUM, THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE VERIFIED PRIOR TO PLACEMENT OF THE CONCRETE. 6. THE QAI SHALL INSPECT THE FABRICATED STEEL OR ERECTED STEEL FRAME, AS APPROPRIATE, TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. 7. QUALITY ASSURANCE (QA) INSPECTIONS, EXCEPT NONDESTRUCTIVE TESTING (NDT), MAY BE WAIVED WHEN THE WORK IS PERFORMED IN A FABRICATING SHOP OR BY AN ERECTOR APPROVED BY THE AUTHORITY HAVING JURISDICTION (AHJ) TO PERFORM THE WORK WITHOUT QA. NOT OF WELDS COMPLETED IN AN APPROVED FABRICATOR'S SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE AHJ. WHEN THE FABRICATOR PERFORMS THE NDT, THE QA AGENCY SHALL REVIEW THE FABRICATOR'S NDT REPORTS. 8. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE FABRICATOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS. AT COMPLETION OF ERECTION, THE APPROVED ERECTOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE ERECTOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS. 9. IDENTIFICATION AND REJECTION OF MATERIAL OR WORKMANSHIP THAT IS NOT IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS, SHALL BE PERMITTED AT ANY TIME DURING THE PROGRESS OF THE WORK. HOWEVER, THIS PROVISION SHALL NOT RELIEVE THE OWNER OR THE INSPECTOR OF THE OBLIGATION FOR TIMELY, IN-SEQUENCE INSPECTIONS. NONCONFORMING MATERIAL AND WORKMANSHIP SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE FABRICATOR OR ERECTOR, AS APPLICABLE. 10. NONCONFORMING MATERIAL OR WORKMANSHIP SHALL BE BROUGHT INTO CONFORMANCE, OR MADE SUITABLE FOR ITS INTENDED PURPOSE AS DETERMINED BY THE ENGINEER OF RECORD. 11. CONCURRENT WITH THE SUBMITTAL OF SUCH REPORTS TO THE AHJ, EOR OR OWNER, THE QA AGENCY SHALL SUBMIT TO THE FABRICATOR AND ERECTOR: (1) NONCONFORMANCE REPORTS (2) REPORTS OF REPAIR, REPLACEMENT OR ACCEPTANCE OF NONCONFORMING ITEMS.										

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SCHEDULES

June 26, 2015	
November 2, 2015	▲
June 27, 2017	▲
September 22, 2017	▲



2014168

**S002**

FOR CONSTRUCTION



SPECIAL INSPECTION SCHEDULE 1,2				
ESTABLISHED PER 2012 IBC SECTION 110 AND CHAPTER 17				
ITEM	CONTINUOUS <sup>3</sup>	PERIODIC <sup>3</sup>	REFERENCE	COMMENTS
<b>PRE-FAB CONSTRUCTION (IBC 1704.2)</b>			REFERENCE NOTES P1 & P2	P1. SPECIAL INSPECTION IS NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION, PROVIDED THE FABRICATOR COMPLETES WITH IBC INSPECTION FOR PREFABRICATED CONSTRUCTION SHALL BE THE SAME AS IF THE MATERIAL USED IN THE CONSTRUCTION TOOK PLACE ON SITE. SPECIAL INSPECTION WILL NOT BE REQUIRED DURING PREFABRICATION IF THE APPROVED SPECIALIST CERTIFIES THE CONSTRUCTION AND FURNISHES EVIDENCE OF COMPLIANCE. (SEE NOTE 2). P2.
<b>CONCRETE CONSTRUCTION (IBC 1705.3)</b>			SEE IBC TABLE 1705.3 - REF. NOTE C1	C1. SPECIAL INSPECTION IS NOT REQUIRED FOR CONC. ISOLATED SPREAD FOOTINGS, CONTINUOUS FOOTINGS, NON-STRUCTURAL SLABS, FOUNDATION WALLS, PATIOS, DRIVEWAYS, AND SIDEWALKS PROVIDED THE REQUIREMENTS OF IBC 1705.3 ARE MET. C2. PERIODIC SPECIAL INSPECTION IS ALLOWED FOR VERIFICATION OF THE WELDABILITY OF REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES. BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR WALLS AND SHEAR REINFORCEMENT. PERIODIC SPECIAL INSPECTION IS ALLOWED FOR WELDING OF OTHER ASTM A 709 REINFORCING STEEL NOT INCLUDED IN THE CONTINUOUS SPECIAL INSPECTION REQUIREMENTS NOTED ABOVE. C3. PERFORM AIR SLUMP AND TEMP. TESTS WHEN CONCRETE SAMPLES ARE CAST. C4. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR VERIFICATION OF IN-SITU CONCRETE STRENGTH FOR POST-TENSIONED CONCRETE PRIOR TO TENSIONING TENDONS OR REMOVING SHORING OR FORMS. C5. EPOXY AND EXPANSION ANCHORS TO MASONRY OR CONCRETE MAY BE USED ONLY WHEN APPROVED BY ARCHITECT AND/OR ENGINEER USING AN APPROVED PRODUCT WITH CURRENT PUBLISHED ICC RESEARCH REPORT NUMBERS. COORDINATE CONTINUOUS/PERIODIC SPECIAL INSPECTION REQUIREMENTS WITH ICC REPORT.
<b>WOOD (IBC 1705.5)</b>			REFERENCE NOTE W1	W1. WOOD STRUCTURAL PANEL SHEATHING SHALL BE INSPECTED TO ASCERTAIN THAT GRADE AND THICKNESS ARE IN COMPLIANCE WITH APPROVED BUILDING PLANS. NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES, THE NAIL OR STAPLE DIAMETER AND LENGTH, THE NUMBER OF FASTENER LINES, AND SPACING BETWEEN FASTENERS IN EACH LINE AND AT EDGE MARGINS SHALL ALSO BE INSPECTED AND VERIFIED FOR COMPLIANCE WITH APPROVED BUILDING PLANS. W2. SPECIAL INSPECTION IS NOT REQUIRED FOR WOOD SHEAR WALLS, WOOD DIAPHRAGMS, INCLUDING NAILING, BOLTING, AND OTHER FASTENING TO OTHER COMPONENTS WHERE THE SPACING OF THE SHEATHING FASTENERS IS GREATER THAN 4"x.c.
<b>SOILS (IBC 1705.6)</b>			REFERENCE NOTE F1	F1. SPECIAL INSPECTION OF SOILS SHALL REFERENCE THE APPROVED SOILS REPORT TO DETERMINE COMPLIANCE. WHERE SOILS REPORT IS NOT PROVIDED SPECIAL INSPECTIONS ARE REQUIRED TO VERIFY THAT THE IN-PLACE DRY DENSITY OF THE COMPACTED FILL IS NOT LESS THAN 90 PERCENT OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT DETERMINED IN ACCORDANCE WITH ASTM D 1557.
REINFORCING STEEL PLACEMENT			REFERENCE NOTE C2	
WELDING OF REINFORCING STEEL				
EMBEDDED BOLTS & PLATES			REFERENCE NOTE C3	
VERIFYING REQUIRED DESIGN MIX				
CONCRETE PLACEMENT / SAMPLING			REFERENCE NOTE C4	
CURING TEMPERATURE / TECHNIQUES				
VERIFICATION OF IN-SITU STRENGTH			REFERENCE NOTE C5	
EPOXY / EXPANSION ANCHOR PLACEMENT				
HIGH LOAD DIAPHRAGMS (ROOF / FLOOR)			REFERENCE NOTE W1	
SITE-BUILT ASSEMBLIES				
SHEAR WALL & DIAPHRAGM NAILING			REFERENCE NOTE W2	
DRAG STRUTS				
BRACES & SHEAR PANELS				
HOLD-DOWNS				
GLUING OPERATIONS				
VERIFY ADEQUATE MATERIALS BELOW FOOTINGS			REFERENCE NOTE F1	
EXCAVATIONS EXTEND TO PROPER DEPTH AND REACH PROPER MATERIAL			REFERENCE NOTE F2	
CLASSIFY & TEST CONTROLLED FILL MATERIALS			REFERENCE NOTE F2	
PERFORM MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.			REFERENCE NOTE F1	
PROPERLY PREPARED SITE AND SUB-GRADE PRIOR TO FILL.			REFERENCE NOTE F1	

- GENERAL SPECIAL INSPECTION NOTES :**
- THE ITEMS MARKED WITH A "●" IN THE SPECIAL INSPECTION SCHEDULE SHALL BE INSPECTED IN ACCORDANCE WITH IBC CHAPTER 17 BY A CERTIFIED SPECIAL INSPECTOR FROM AN ESTABLISHED TESTING AGENCY. FOR MATERIAL SAMPLING AND TESTING REQUIREMENTS, REFER TO THE MATERIAL SAMPLING AND TESTING SECTION, THE PROJECT SPECIFICATIONS, AND THE SPECIFIC GENERAL NOTES SECTIONS. THE TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE ARCHITECT, ENGINEER, CONTRACTOR, AND BUILDING OFFICIAL. ANY ITEMS WHICH FAIL TO COMPLY WITH THE APPROVED CONSTRUCTION DOCUMENTS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF DISCREPANCIES ARE NOT CORRECTED, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL, ARCHITECT, AND ENGINEER PRIOR TO COMPLETION OF THAT PHASE OF WORK. SPECIAL INSPECTION TESTING REQUIREMENTS APPLY EQUALLY TO ALL BIDDER DESIGNED COMPONENTS.
  - ANY CONSTRUCTION OR MATERIAL THAT HAS FAILED INSPECTION SHALL BE SUBJECT TO REMOVAL AND REPLACEMENT.
  - CONTINUOUS SPECIAL INSPECTION MEANS THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. PERIODIC SPECIAL INSPECTION MEANS THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK. (IBC SECTION 1702)

BEAM CONNECTIONS SCHEDULE						
CONNECTION SCHEDULE						
BEAM DEPTH	SHEAR PLATE INFORMATION		BOLTS W/ STANDARD WASHERS OVER SLOTS		WELD 'A'	COMMENTS
	PL. DIMENSIONS W/ SHORT-SLOTTED HOLES	Lev	No.	SIZE		
W8x, W10x	PL. 1/4" x 4"	1 1/2"	2"	2	3/4" Ø	3/16"
W12x	PL. 5/16" x 4"	1 1/2"	2"	3	3/4" Ø	1/4"
W14 x 90 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	3	3/4" Ø	1/4"
W16 x 77 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	4	3/4" Ø	1/4"
W18 x 65 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	5	3/4" Ø	1/4"
W21 x 75 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	6	3/4" Ø	1/4"
W24 x 94 & LIGHTER	PL. 3/8" x 4"	1 1/2"	2"	7	7/8" Ø	1/4"
W27 x 114 & LIGHTER	PL. 3/8" x 4"	1 1/2"	2"	7	7/8" Ø	1/4"
W30 x 124 & LIGHTER	PL. 1/2" x 4"	1 3/4"	2"	8	1" Ø	5/16"
W33 x 130 & LIGHTER	PL. 1/2" x 4"	1 3/4"	2"	9	1" Ø	5/16"
W36 x 160 & LIGHTER	PL. 1/2" x 4 1/2"	2"	2 1/4"	10	1-1/8" Ø	5/16"

STANDARD HOOK & BEND SCHEDULE						
BAR SIZE	DIMENSION OF STANDARD 180-DEG HOOKS, ALL GRADES			DIMENSION OF STANDARD 90-DEG HOOKS, ALL GRADES		
	A or G	J	D	A or G	J	D
#3	5"	3"	2 1/4"	6"	2"	2 1/4"
#4	6"	4"	3"	8"	3"	3"
#5	7"	5"	3 3/4"	10"	3 3/4"	3 3/4"
#6	8"	6"	4 1/2"	12"	4 1/2"	4 1/2"
#7	10"	7"	5 1/4"	15"	5 1/4"	5 1/4"
#8	11"	8"	6"	16"	6"	6"
#9	13"	11 3/4"	9 1/2"	17"	9 1/2"	9 1/2"
#10	15"	14 1/4"	10 3/4"	20"	10 3/4"	10 3/4"
#11	17"	16 3/4"	12"	24"	12"	12"

BEAM FRAMING SCHEDULE						
BEAM SIZE	ANGLE SIZE (EA. SIDE)	WELD 'A'	BOLTS	EMBED PLATE	# OF H.S.A.	H.S.A. PATTERN
W8x, C8x	3 x 5 x 1/4	3/16"	(2) 3/4" Ø	8" x 8"	4	••
W10x	3 x 5 x 5/16	1/4"	(2) 3/4" Ø	12" x 1'-4"	6	•••
W12x, C12x	3 x 5 x 5/16	1/4"	(3) 3/4" Ø	12" x 1'-4"	6	••••
W14x	3 x 5 x 5/16	1/4"	(3) 3/4" Ø	15" x 1'-4"	9	•••••
W16x	3 x 5 x 5/16	1/4"	(4) 3/4" Ø	15" x 2'-0"	12	••••••
W18x	3 x 5 x 5/16	1/4"	(5) 3/4" Ø	15" x 2'-0"	12	•••••••
W21x	3 x 5 x 3/8	1/4"	(6) 7/8" Ø	15" x 2'-0"	12	••••••••
W24x	3 x 5 x 3/8	1/4"	(7) 7/8" Ø	15" x 2'-0"	12	•••••••••
W27x	3 x 5 x 3/8	3/8"	(8) 7/8" Ø	15" x 2'-6"	15	••••••••••
W30x	3 x 5 x 3/8	3/8"	(9) 7/8" Ø	15" x 2'-6"	15	•••••••••••

2012 IBC CONC. REBAR LAP SPlice SCHEDULE																										
FOR CONCRETE APPLICATIONS (ACI 318 - 11)																										
BAR LOCATION	CONCRETE TYPE	STRENGTH	CONCRETE REINFORCING & SPLICE LENGTHS (IN)																							
			BAR SIZE																							
			#3	#4	#5	#6	#7	#8	#9	#10	#11															
			l <sub>d</sub>	l <sub>s</sub>	l <sub>d</sub>	l <sub>s</sub>	l <sub>d</sub>	l <sub>s</sub>	l <sub>d</sub>	l <sub>s</sub>	l <sub>d</sub>	l <sub>s</sub>	l <sub>d</sub>	l <sub>s</sub>												
VERT. WALL BARS, FILL ON METAL DECK	NWC	3000 PSI	17	22	8	22	29	8	28	36	10	33	43	12	48	62	13	55	72	15	62	17	69	19	76	30
HORIZ. WALL BARS, FOOTING TOP BARS	NWC	3000 PSI	17	22	8	22	29	8	28	36	10	33	43	12	48	62	13	55	72	15	62	17	69	19	76	30
BEAM BOTTOM BARS, COLUMN BARS	NWC	3000 PSI	17	22	8	22	29	11	28	36	14	33	43	16	48	62	19	55	72	22	62	25	69	27	76	30
FOOTING BOTTOM BARS	NWC	3000 PSI	12	16	8	14	18	8	17	22	10	20	26	12	29	38	13	33	43	15	37	17	42	19	46	30
BEAM TOP BARS	NWC	3000 PSI	22	29	8	29	38	11	36	47	14	43	56	16	63	82	19	72	94	22	81	25	90	27	98	30
SLAB ON GRADE	NWC	3000 PSI	12	16	8	14	18	8	17	22	10	20	26	12	32	42	13	42	55	15	53	17	69	19	76	30

CONCRETE REINFORCING & SPLICE LENGTHS (IN)																										
BAR LOCATION	CONCRETE TYPE	STRENGTH	BAR SIZE																							
			#3	#4	#5	#6	#7	#8	#9	#10	#11															
			l <sub>d</sub>	l <sub>s</sub>	l <sub>d</sub>	l <sub>s</sub>	l <sub>d</sub>	l <sub>s</sub>	l <sub>d</sub>	l <sub>s</sub>	l <sub>d</sub>	l <sub>s</sub>	l <sub>d</sub>	l <sub>s</sub>												
VERT. WALL BARS, FILL ON METAL DECK	NWC	4500 PSI	14	18	7	18	23	6	23	30	8	27	35	9	40	52	11	45	59	13	51	14	56	16	62	25
HORIZ. WALL BARS, FOOTING TOP BARS	NWC	4500 PSI	14	18	7	18	23	6	23	30	8	27	35	9	40	52	11	45	59	13	51	14	56	16	62	25
BEAM BOTTOM BARS, COLUMN BARS	NWC	4500 PSI	14	18	7	18	23	9	23	30	11	27	35	13	40	52	16	45	59	18	51	20	56	22	62	25
FOOTING BOTTOM BARS	NWC	4500 PSI	12	16	7	12	16	6	14	18	8	17	22	9	24	31	11	27	35	13	31	14	34	16	37	25
BEAM TOP BARS	NWC	4500 PSI	18	23	7	24	31	9	30	39	11	35	46	13	51	66	16	59	77	18	66	20	73	22	80	25
SLAB ON GRADE	NWC	4500 PSI	12	16	7	12	16	6	14	18	8	17	22	9	27	35	11	34	44	13	44	14	56	16	62	25

**NOTES:**

- MECHANICAL COUPLERS MAY BE USED IN LIEU OF LAP SPLICES SHOWN. SEE STRUCTURAL NOTES FOR MINIMUM COUPLER CAPACITY.
- WHERE MECHANICAL COUPLERS ARE USED, STAGGER ADJACENT SPLICES A MINIMUM OF 24" AS INDICATED ABOVE.
- DEVELOPMENT LENGTHS SHALL BE INCREASED BY 50% FOR STRAIGHT BAR DEVELOPMENT AND 20% FOR HOOKED BARS WHERE EPOXY COATING IS USED.
- SPLICE BARS LARGER THAN #11 USING MECHANICAL COUPLERS.

**ENGINEERS**  
 structural consultants  
 1584 W. Park, Ok. Ogden, Utah 84403  
 PH: 801.726.6028 FAX: 801.726.4856

**39 SUMMIT, LLC**  
 Summit at Powder Mountain Lot 39  
 8365 E. SUMMIT PASS

SCHEDULES  
 June 26, 2015  
 November 2, 2015  
 June 27, 2017  
 September 22, 2017

**FOR CONSTRUCTION**

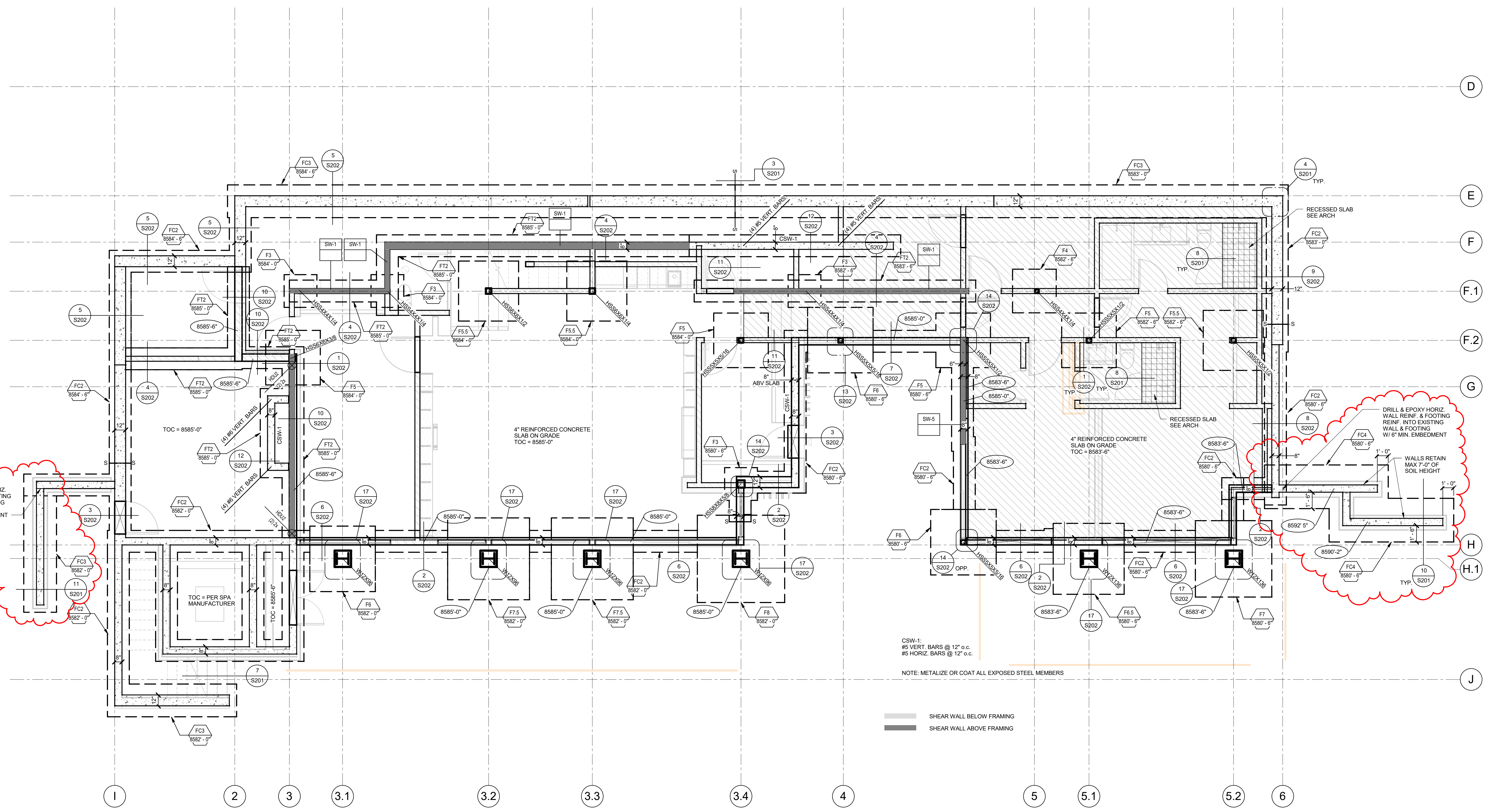
No. 37896  
 Top Me-Div.  
 07/27/17

2014168

**S003**



FOR CONSTRUCTION



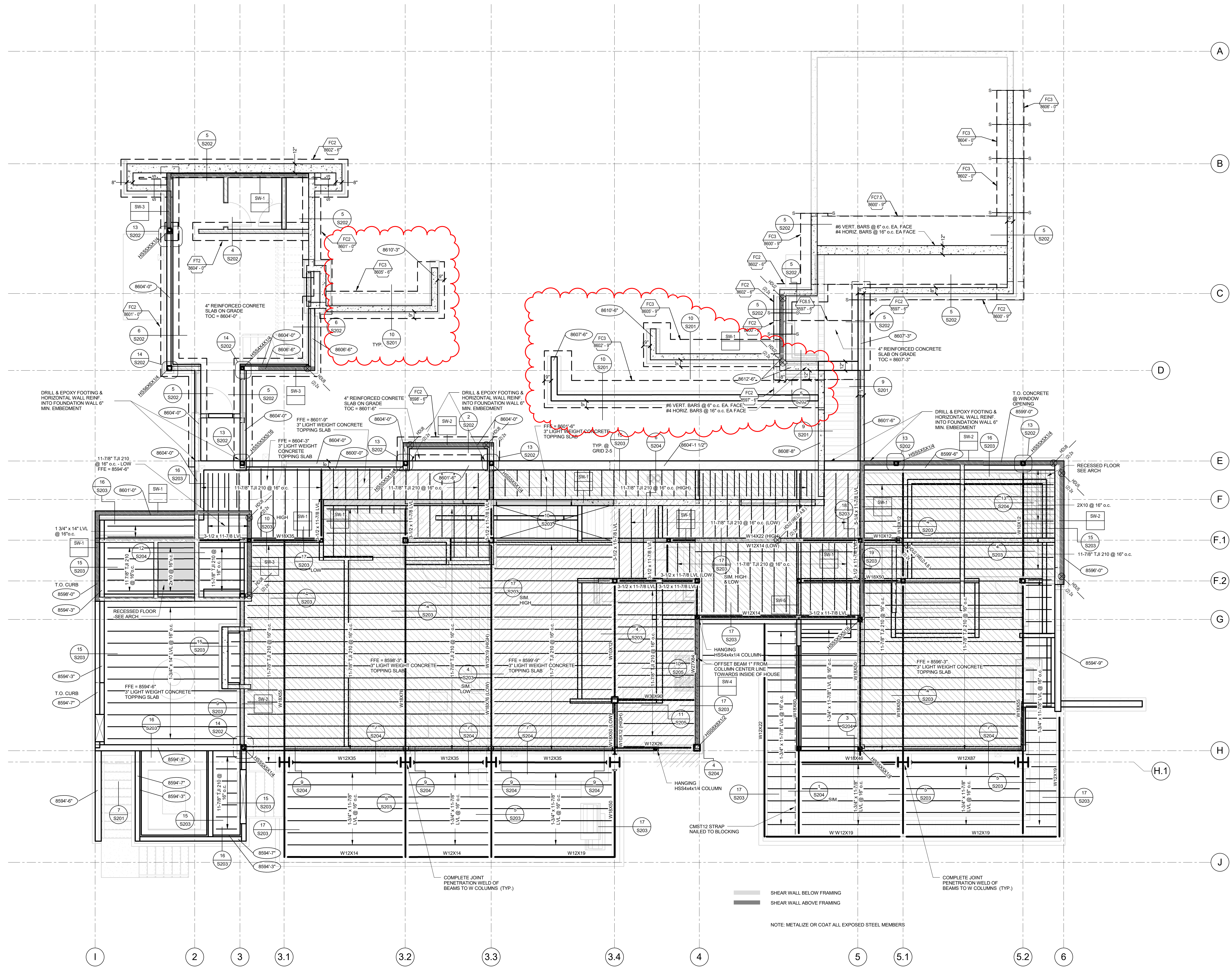
T.O. SUB-SLAB @ BASEMENT LEVEL  
 SCALE: 1/4" = 1'-0"

A  
 S101

CSW-1:  
 #5 VERT. BARS @ 12" o.c.  
 #5 HORIZ. BARS @ 12" o.c.  
 NOTE: METALIZE OR COAT ALL EXPOSED STEEL MEMBERS

— SHEAR WALL BELOW FRAMING  
 — SHEAR WALL ABOVE FRAMING





T.O. SUB-FLOOR @ KITCHEN & DINING  
SCALE: 1/4" = 1'-0"

A  
S102

A

B

C

D

E

F

F.1

F.2

G

H

H.1

J

1

2

3

3.1

3.2

3.3

3.4

4

5

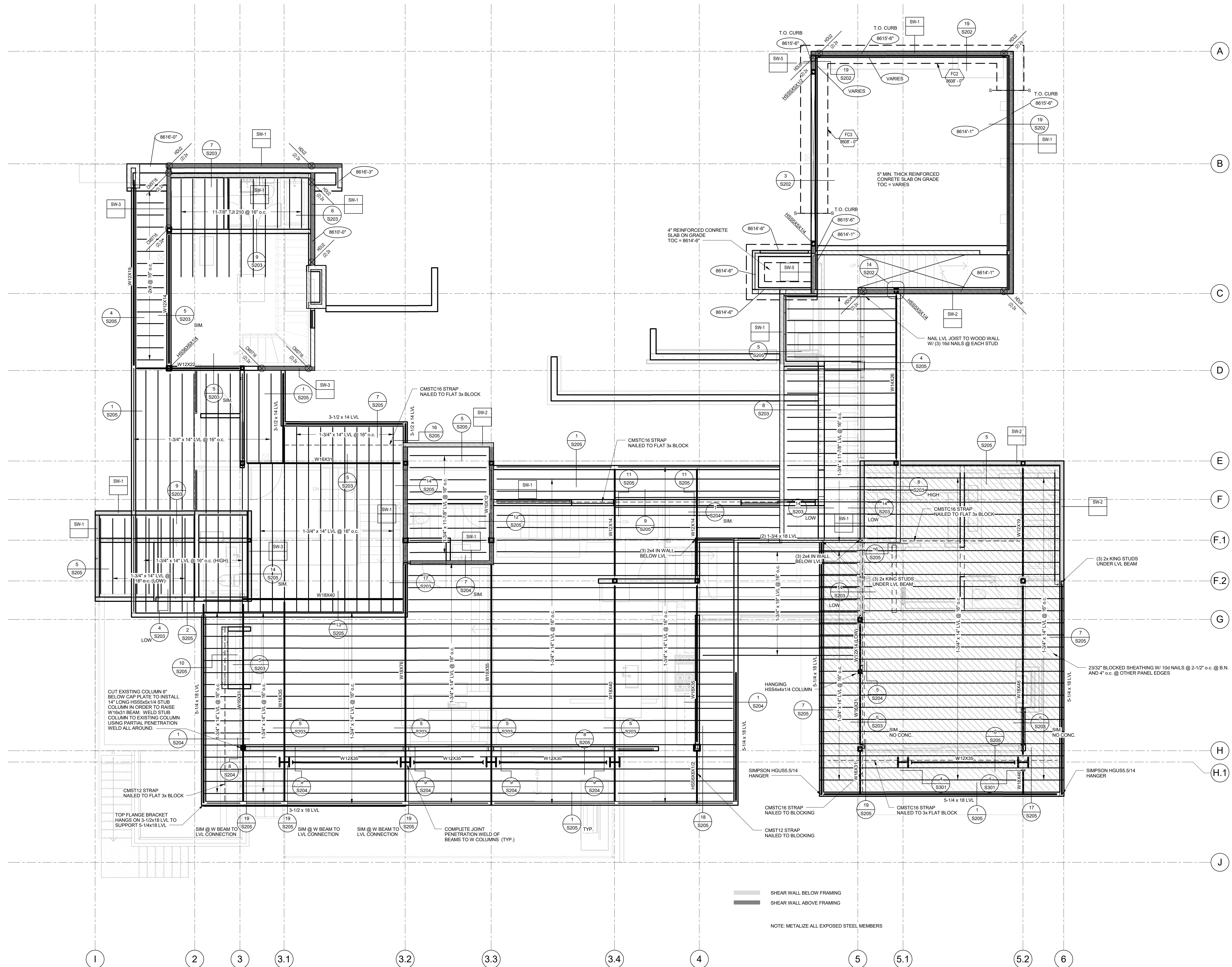
5.1

5.2

6

FOR CONSTRUCTION





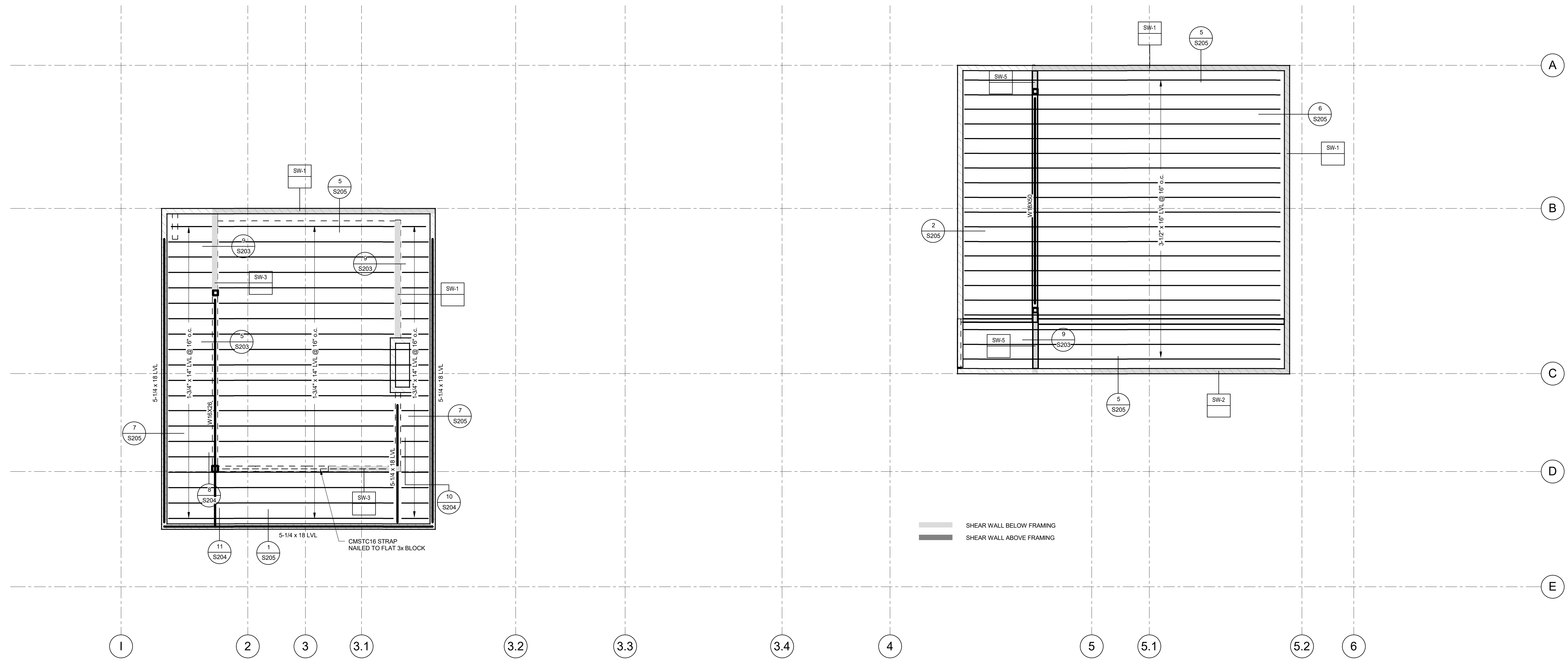
**ROOF FRAMING PLAN**  
SCALE: 1/4" = 1'-0"

A  
S103

— SHEAR WALL BELOW FRAMING  
— SHEAR WALL ABOVE FRAMING

NOTE: METALIZE ALL EXPOSED STEEL MEMBERS





**HIGH ROOF FRAMING PLAN**  
SCALE: 1/4" = 1'-0"

A  
S104

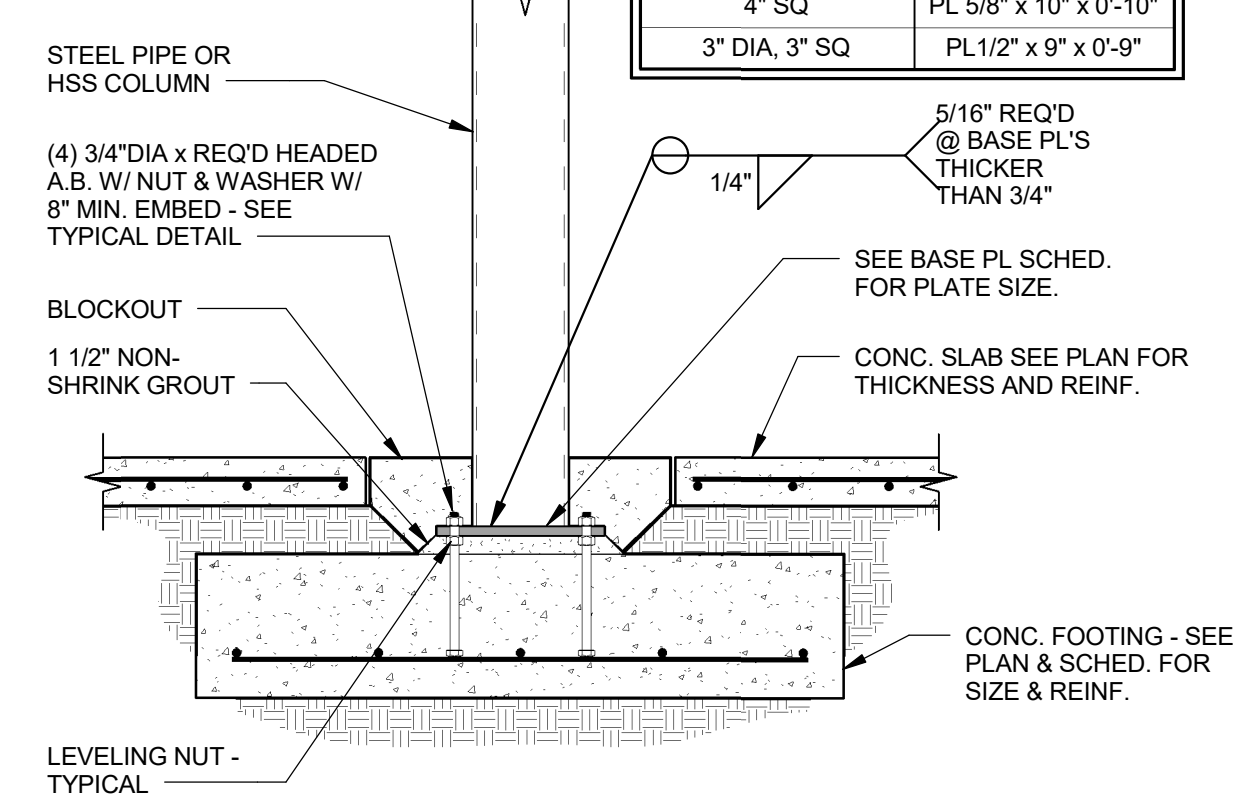
FOR CONSTRUCTION



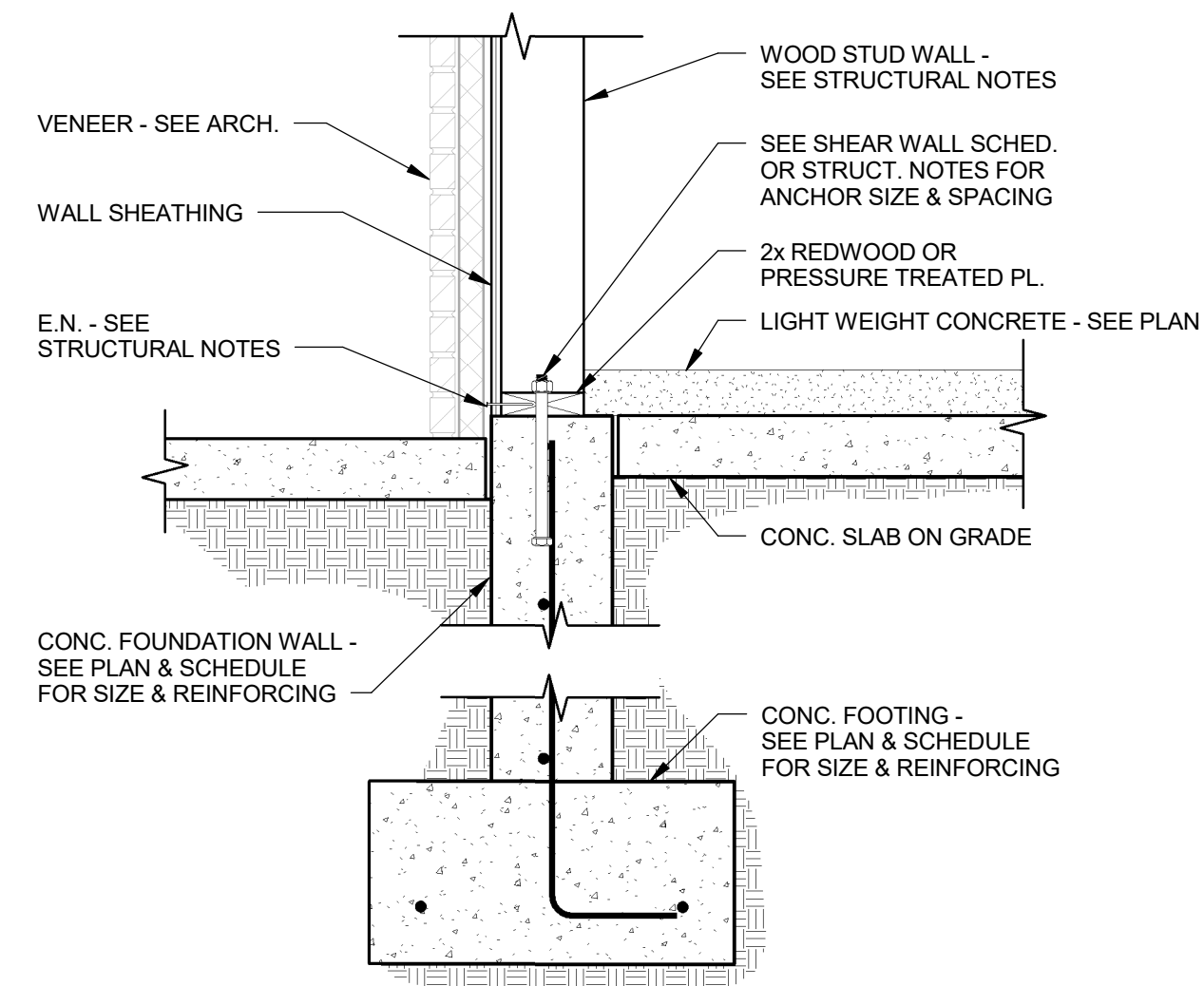




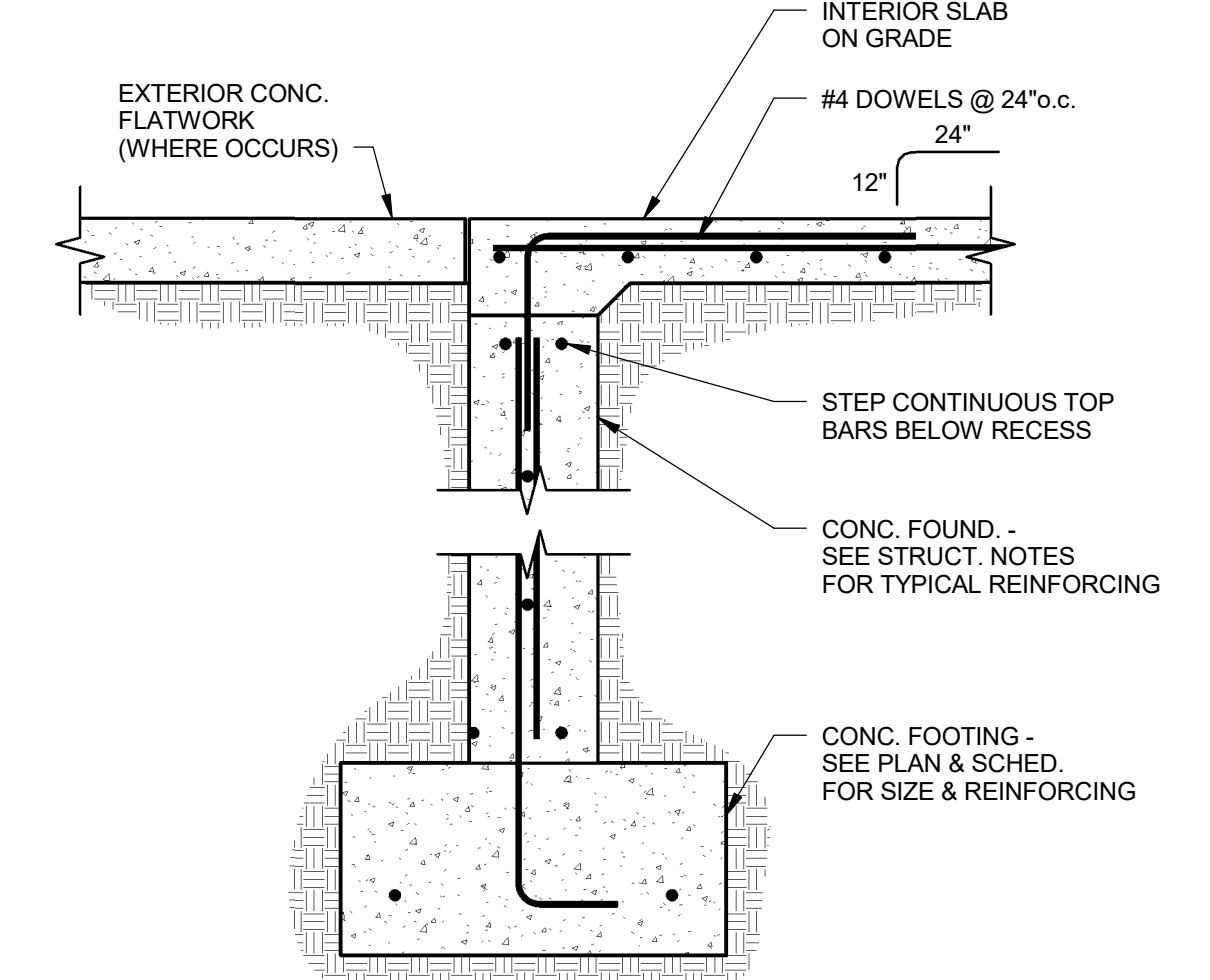
BASE PLATE SCHEDULE	
COLUMN SIZE	BASE PLATE SIZE
8" SQ, 10" DIA	PL 3/4" x 14" x 1'-2"
7" SQ	PL 3/4" x 13" x 1'-1"
6" SQ, 8" DIA	PL 3/4" x 12" x 1'-0"
5" SQ	PL 3/4" x 12" x 1'-0"
4" SQ	PL 5/8" x 10" x 0'-10"
3" DIA, 3" SQ	PL 1/2" x 9" x 0'-9"



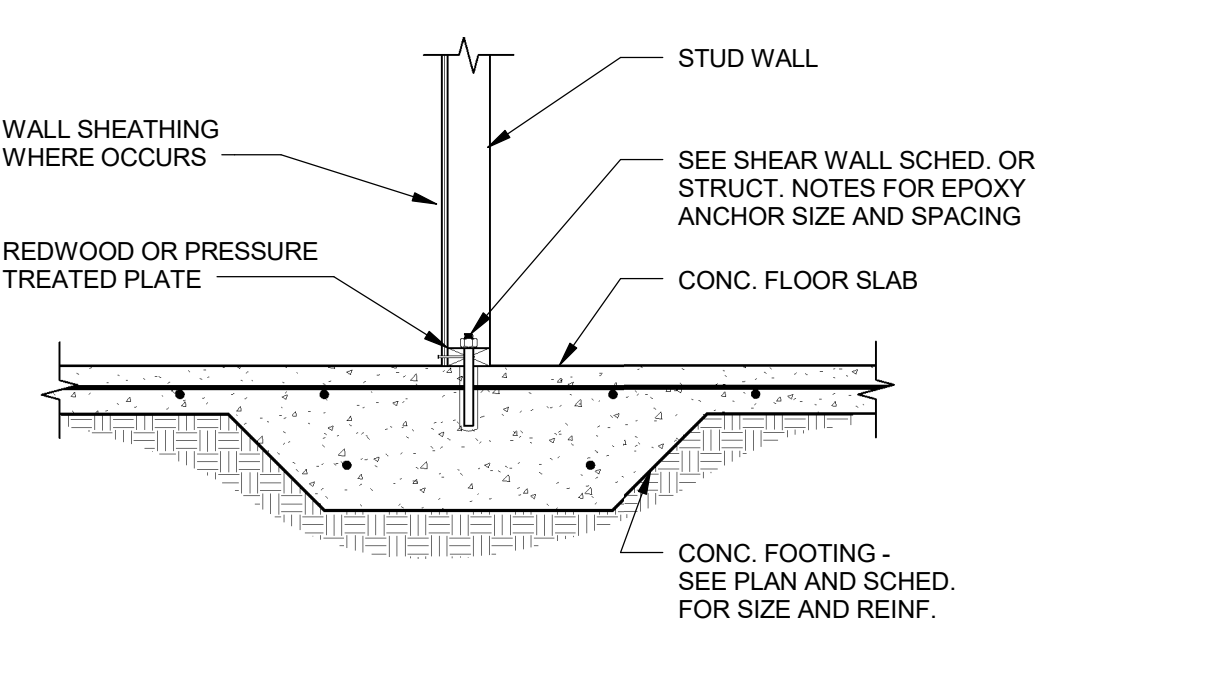
**1**  
COLUMN TO SPOT FOOTING  
SCALE: NONE  
S202



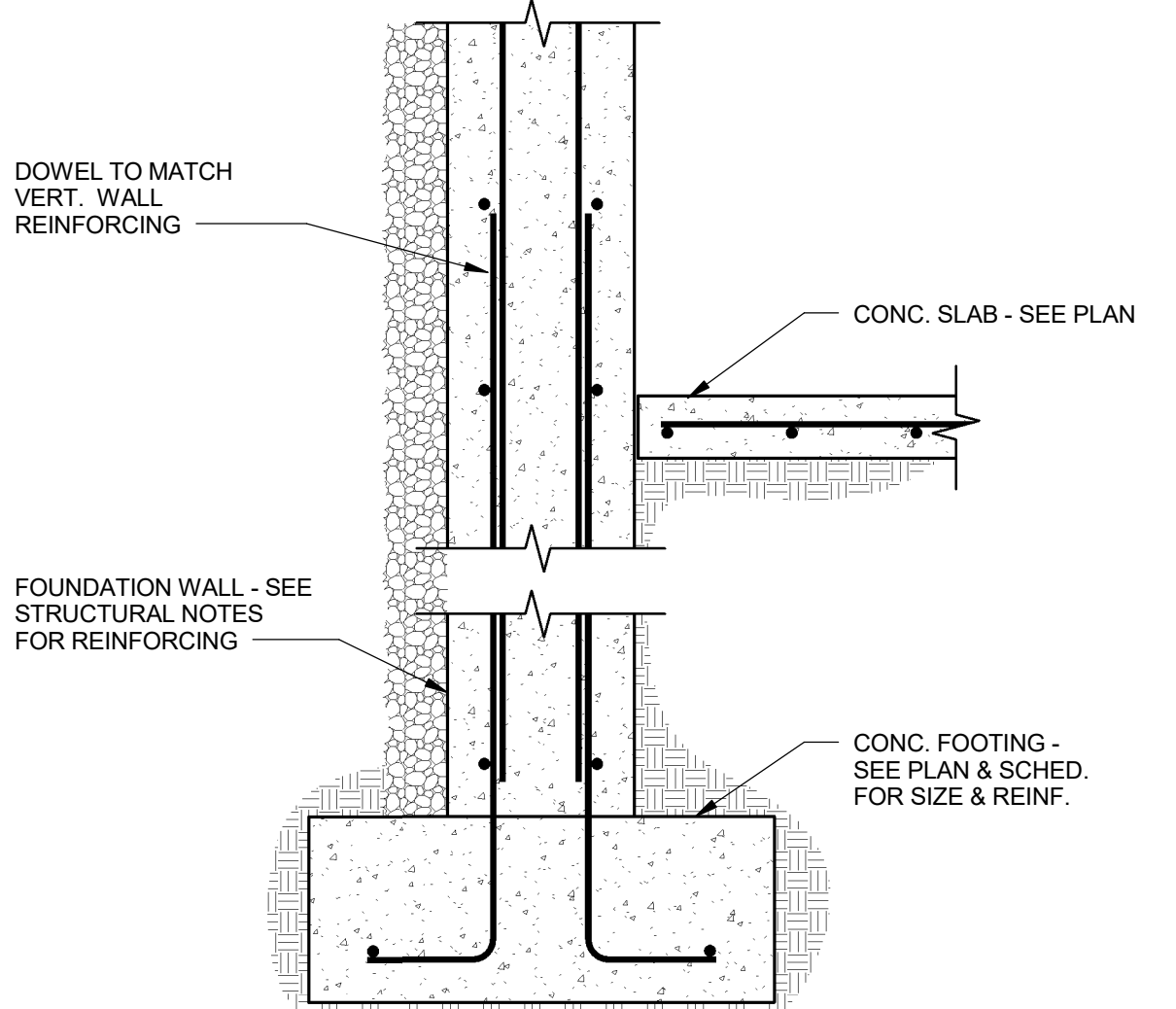
**2**  
SECTION @ EXTERIOR TIMBER  
SCALE: NONE  
S202



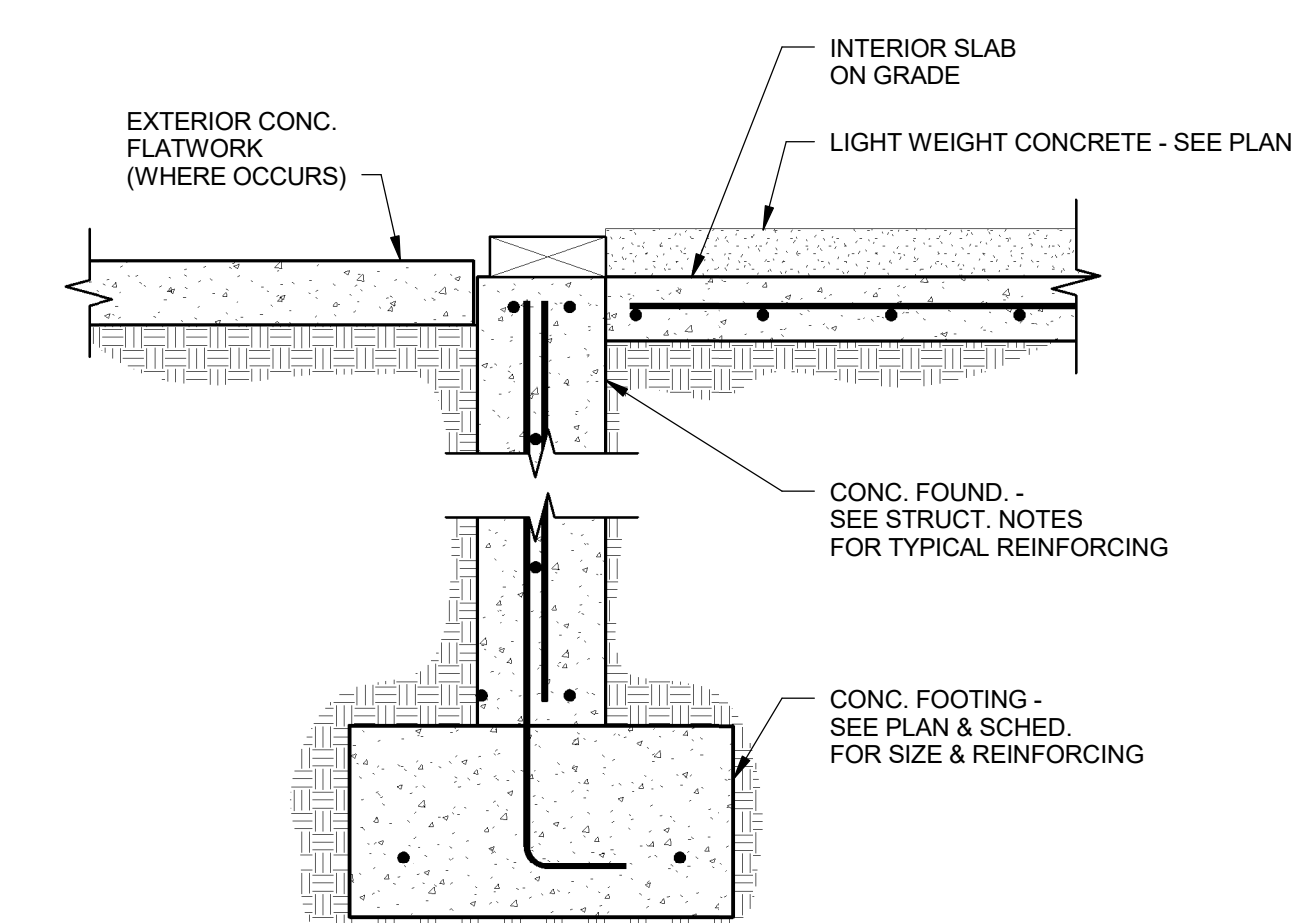
**3**  
CONCRETE FOUNDATION @ OPENING  
DETAIL  
SCALE: NONE  
S202



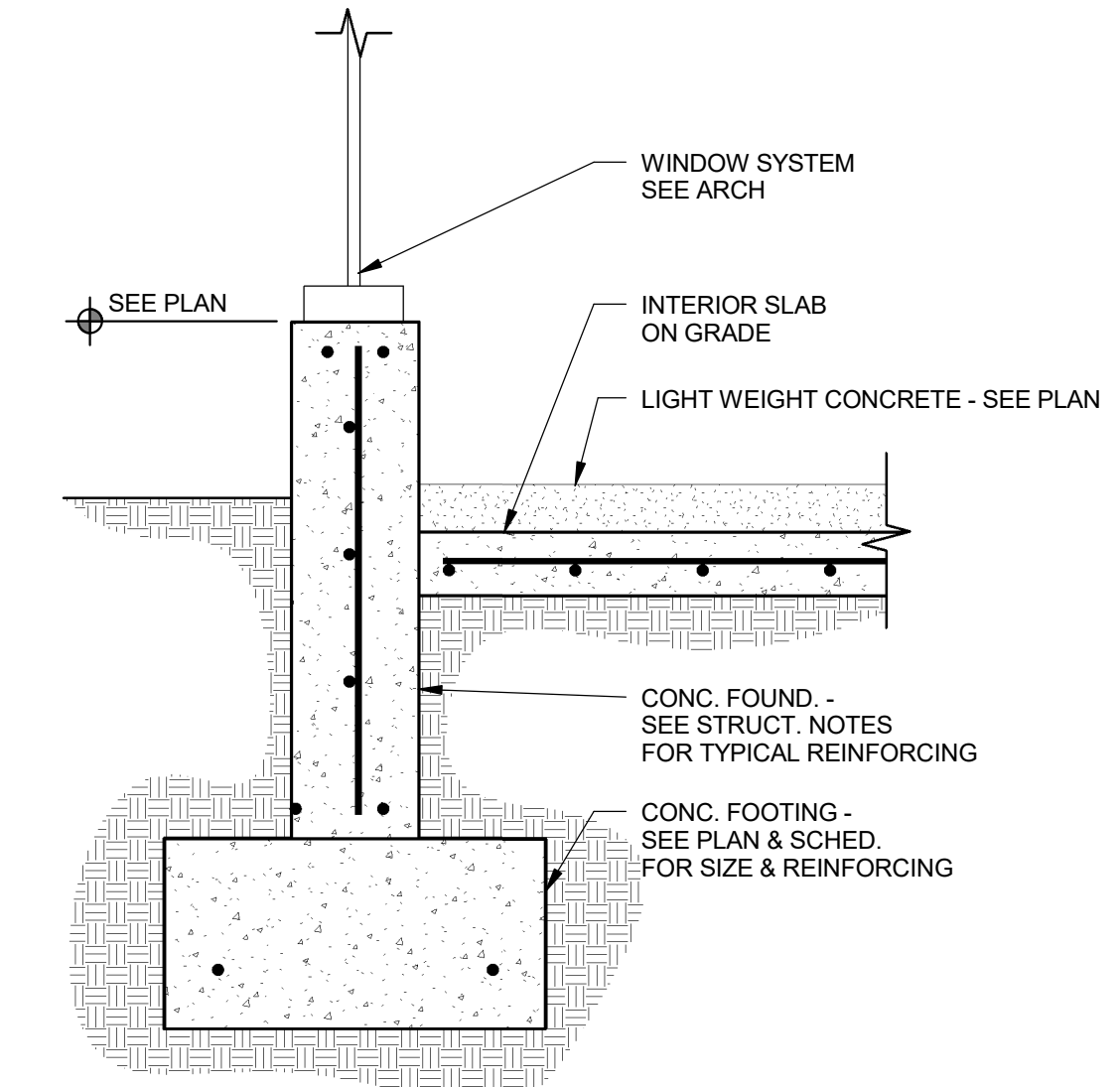
**4**  
STUD WALL ON THICKENED SLAB  
SCALE: NONE  
S202



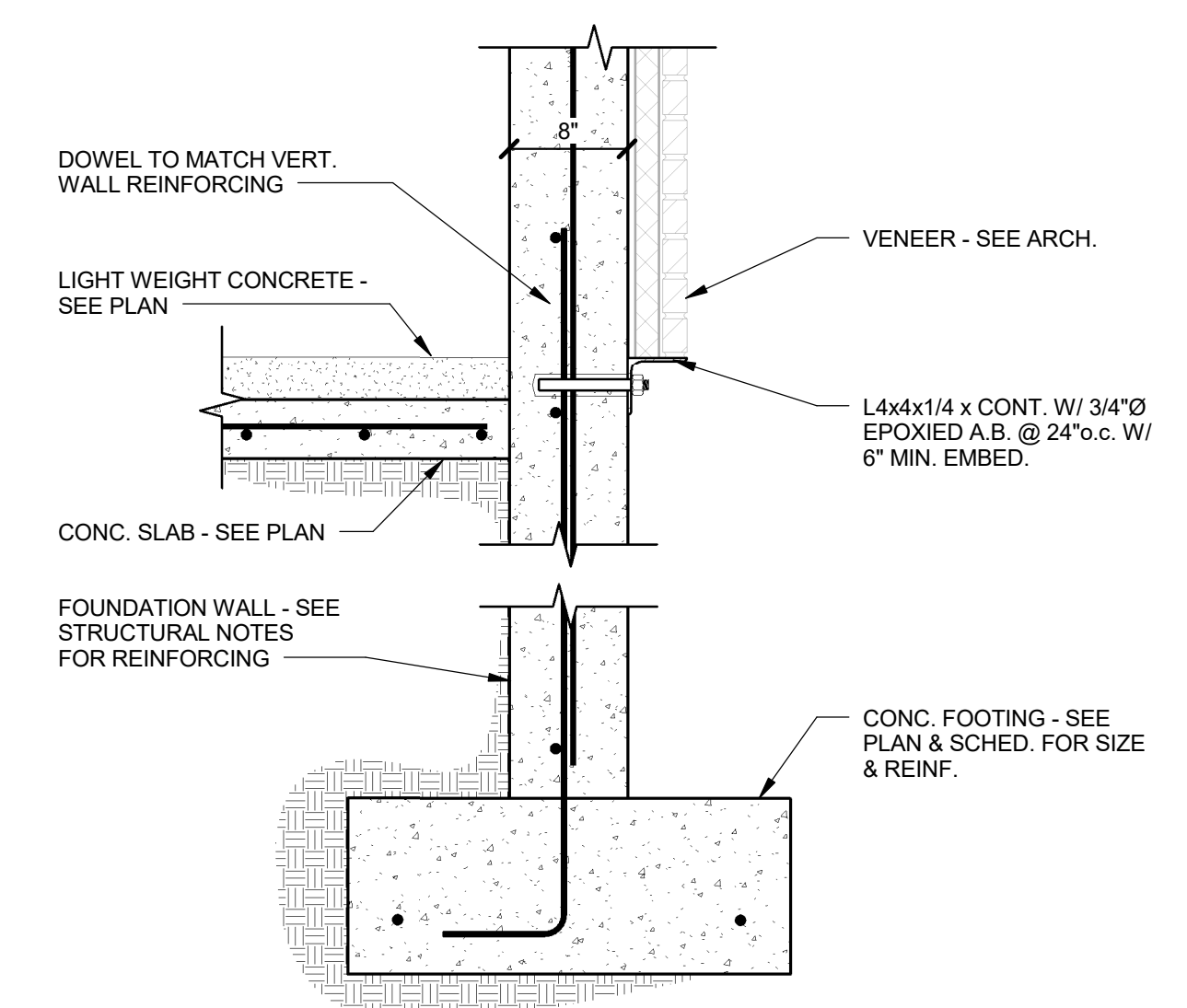
**5**  
DETAIL  
SCALE: NONE  
S202



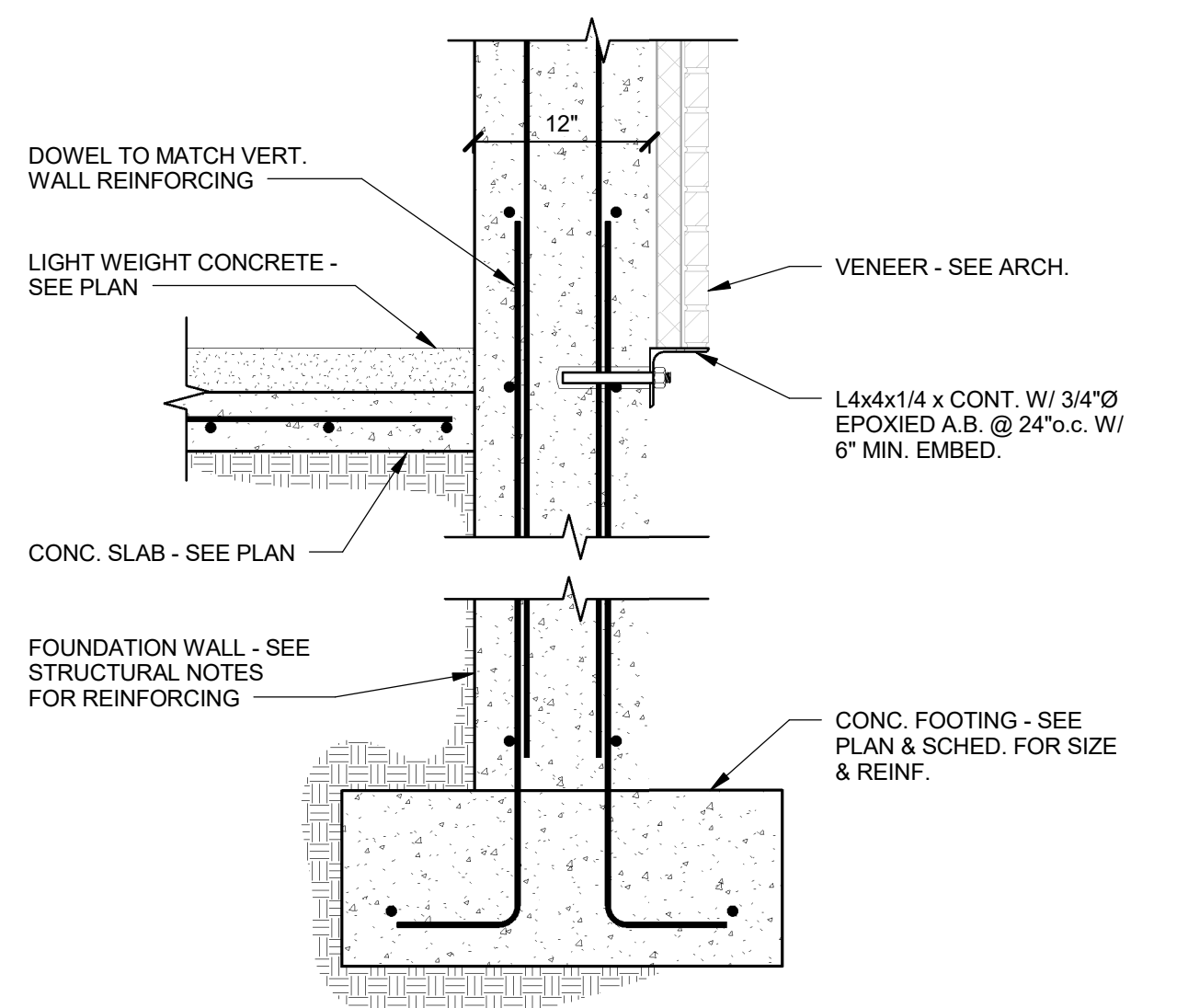
**6**  
DETAIL  
SCALE: NONE  
S202



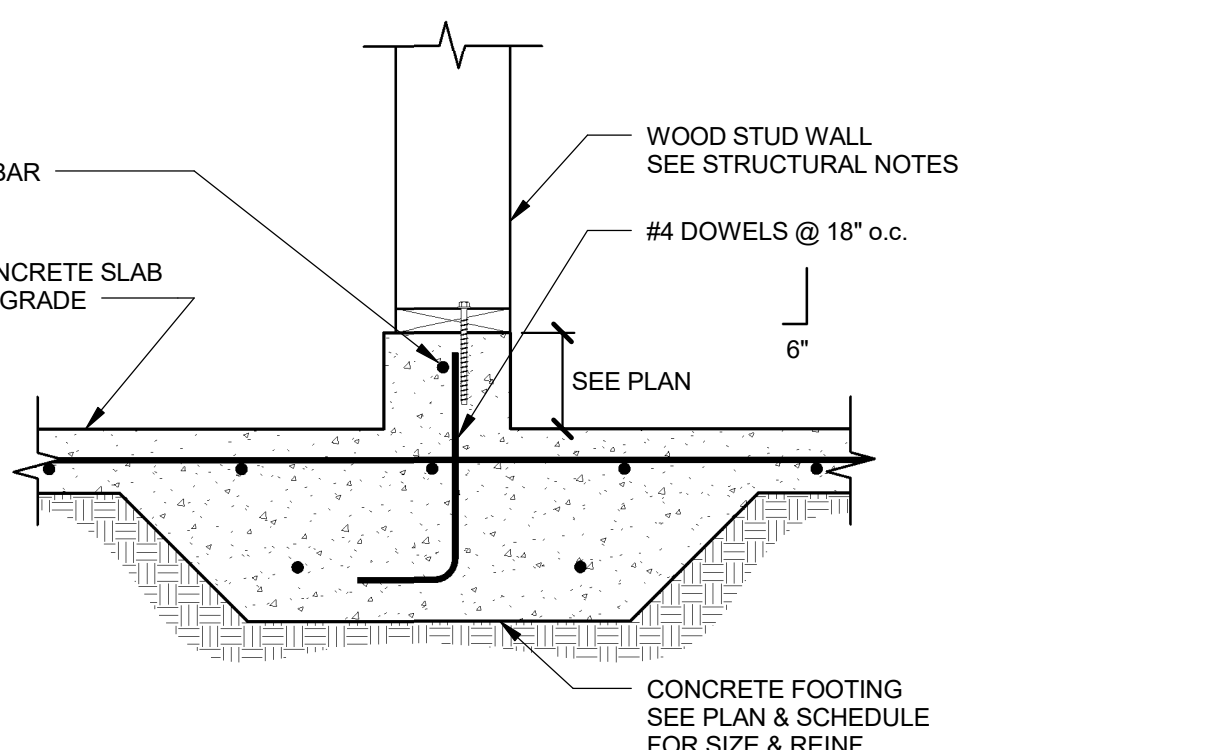
**7**  
DETAIL  
SCALE: NONE  
S202



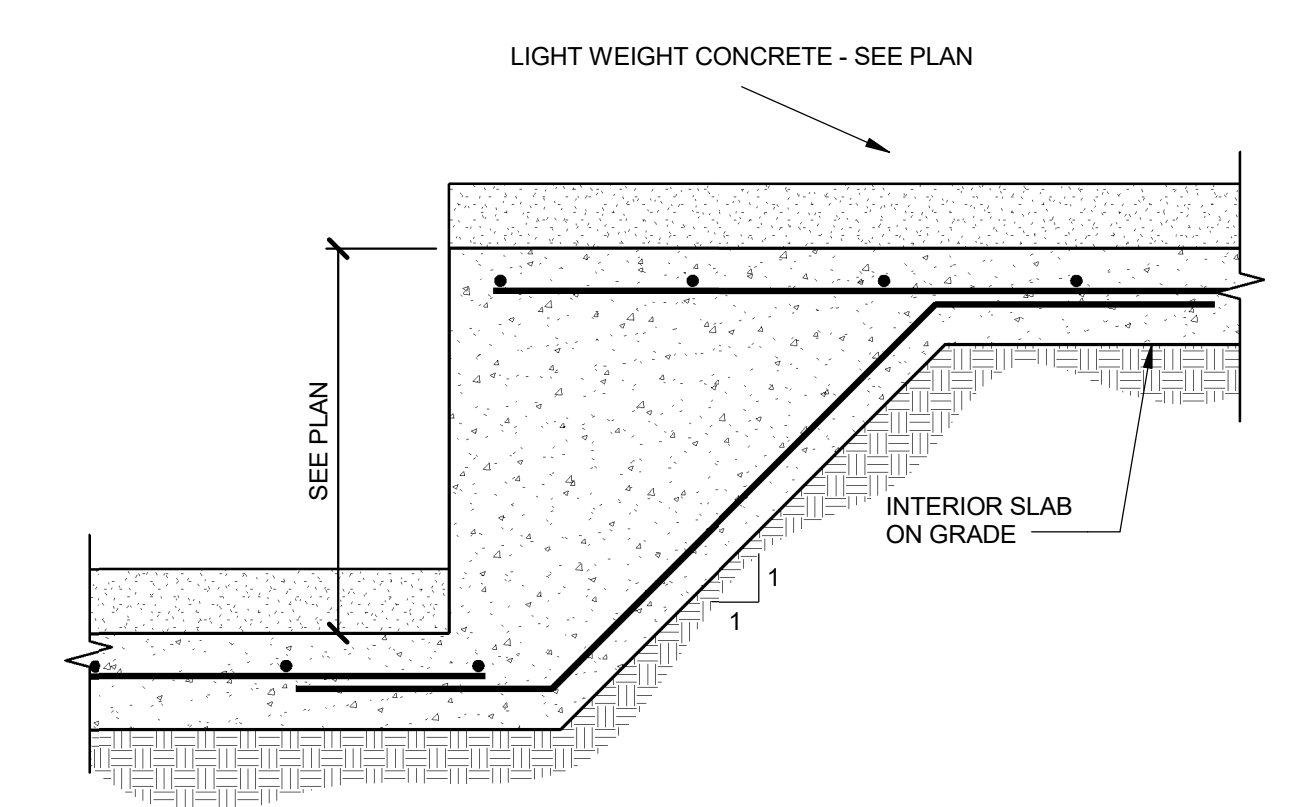
**8**  
DETAIL  
SCALE: NONE  
S202



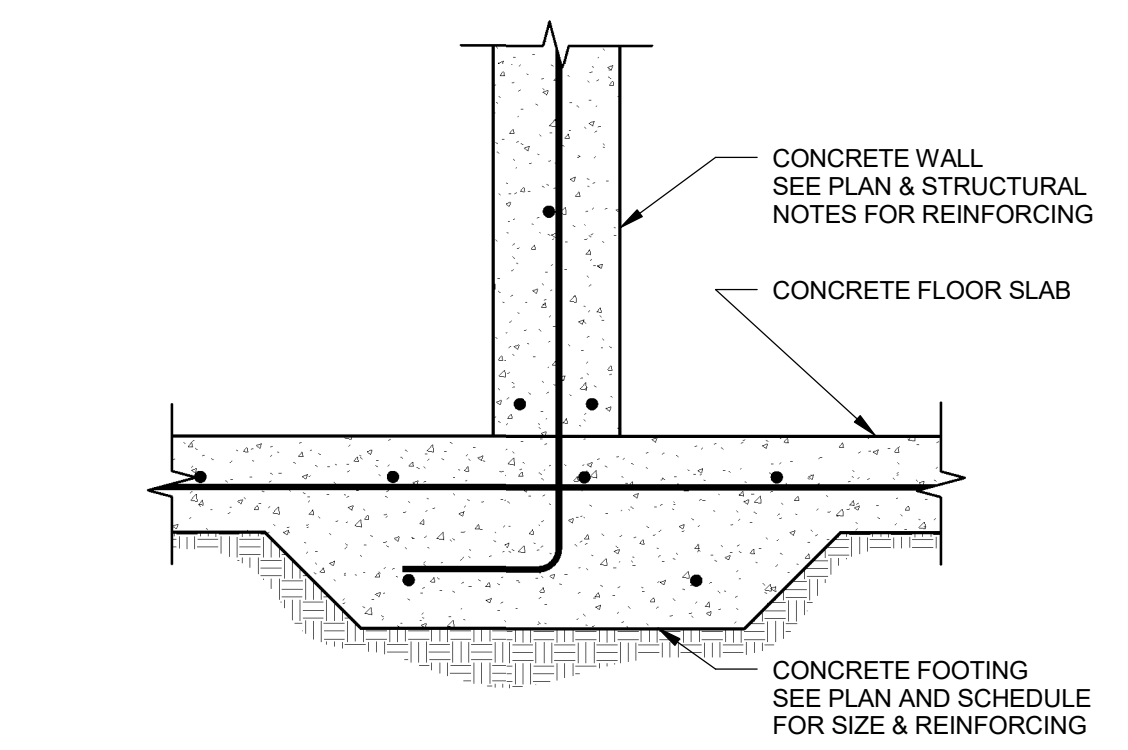
**9**  
DETAIL  
SCALE: NONE  
S202



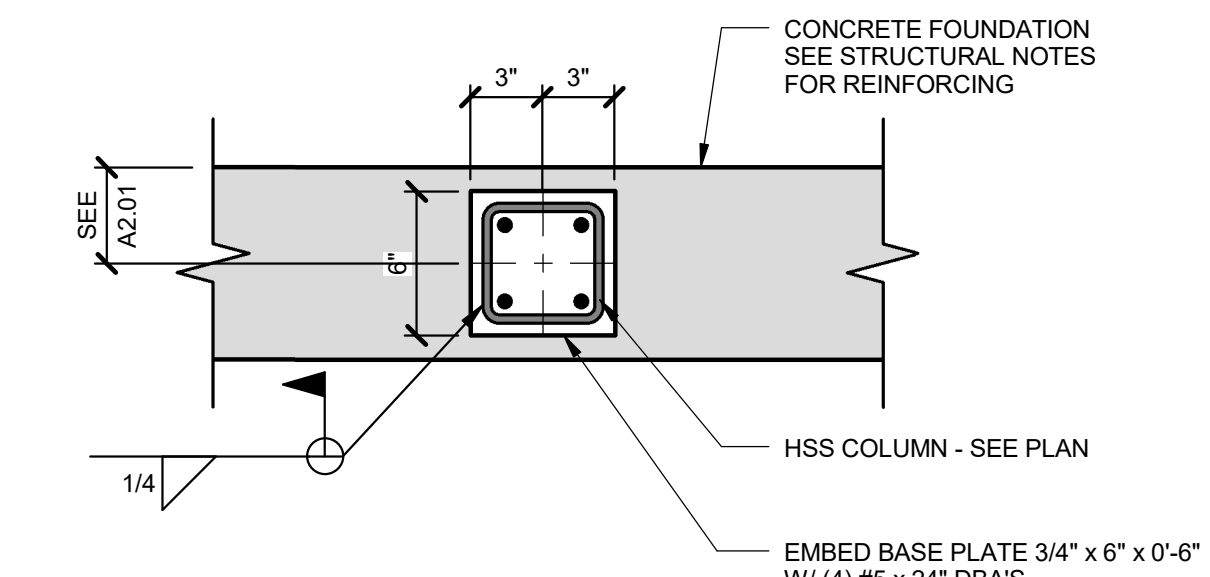
**10**  
DETAIL  
SCALE: NONE  
S202



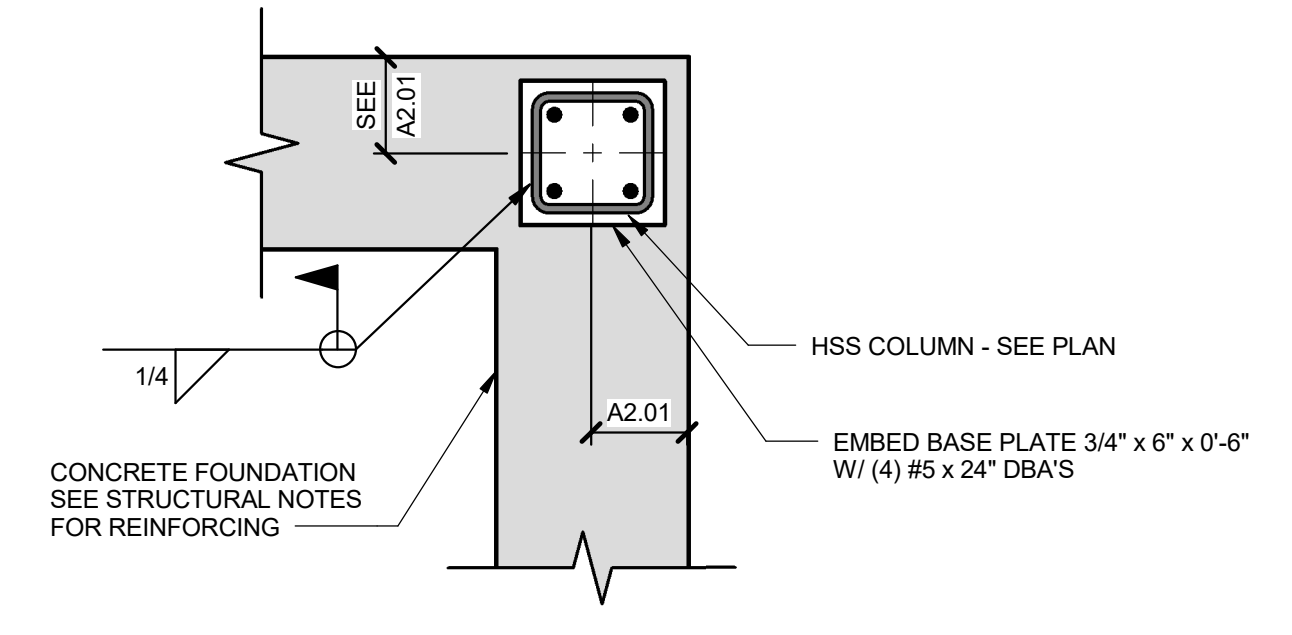
**11**  
DETAIL  
SCALE: NONE  
S202



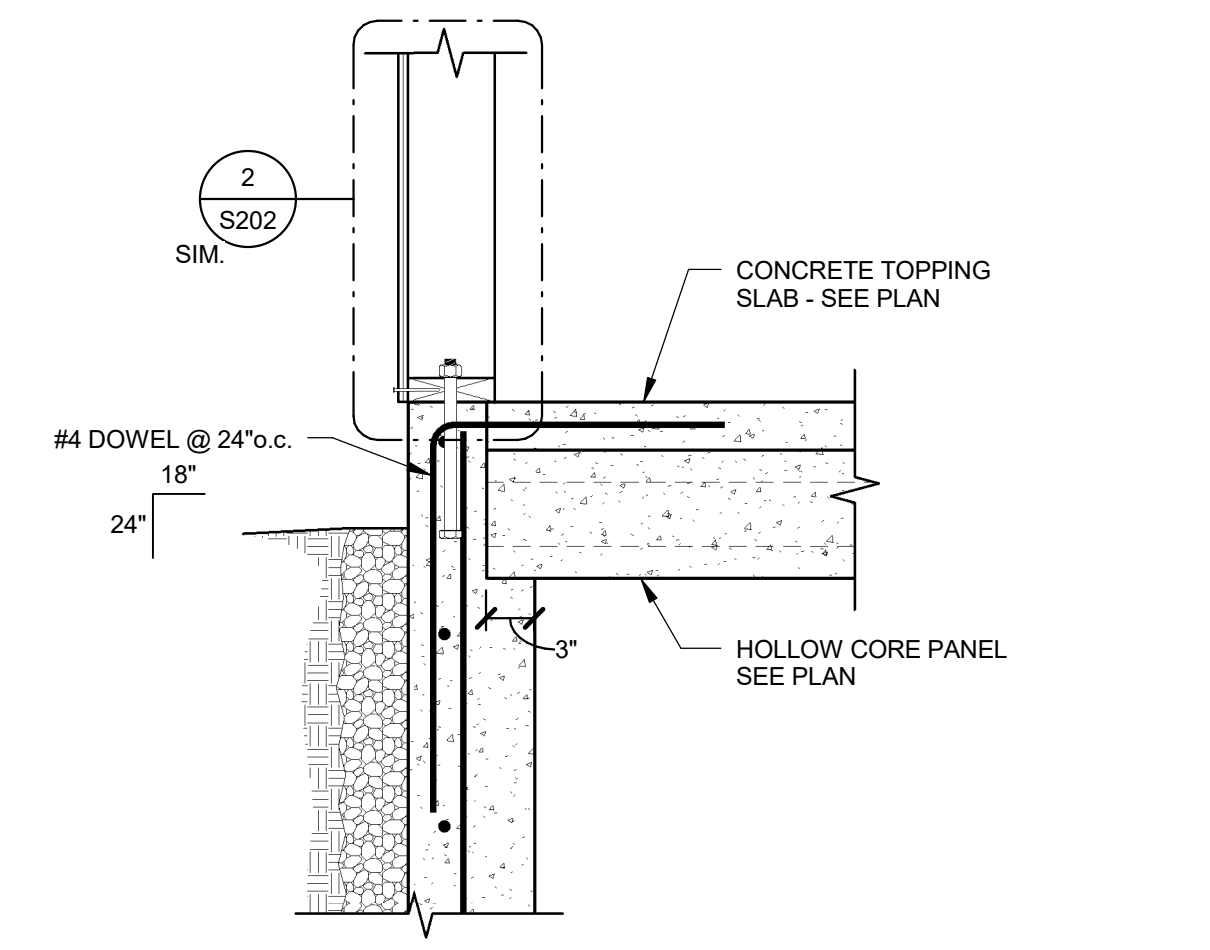
**12**  
DETAIL  
SCALE: NONE  
S202



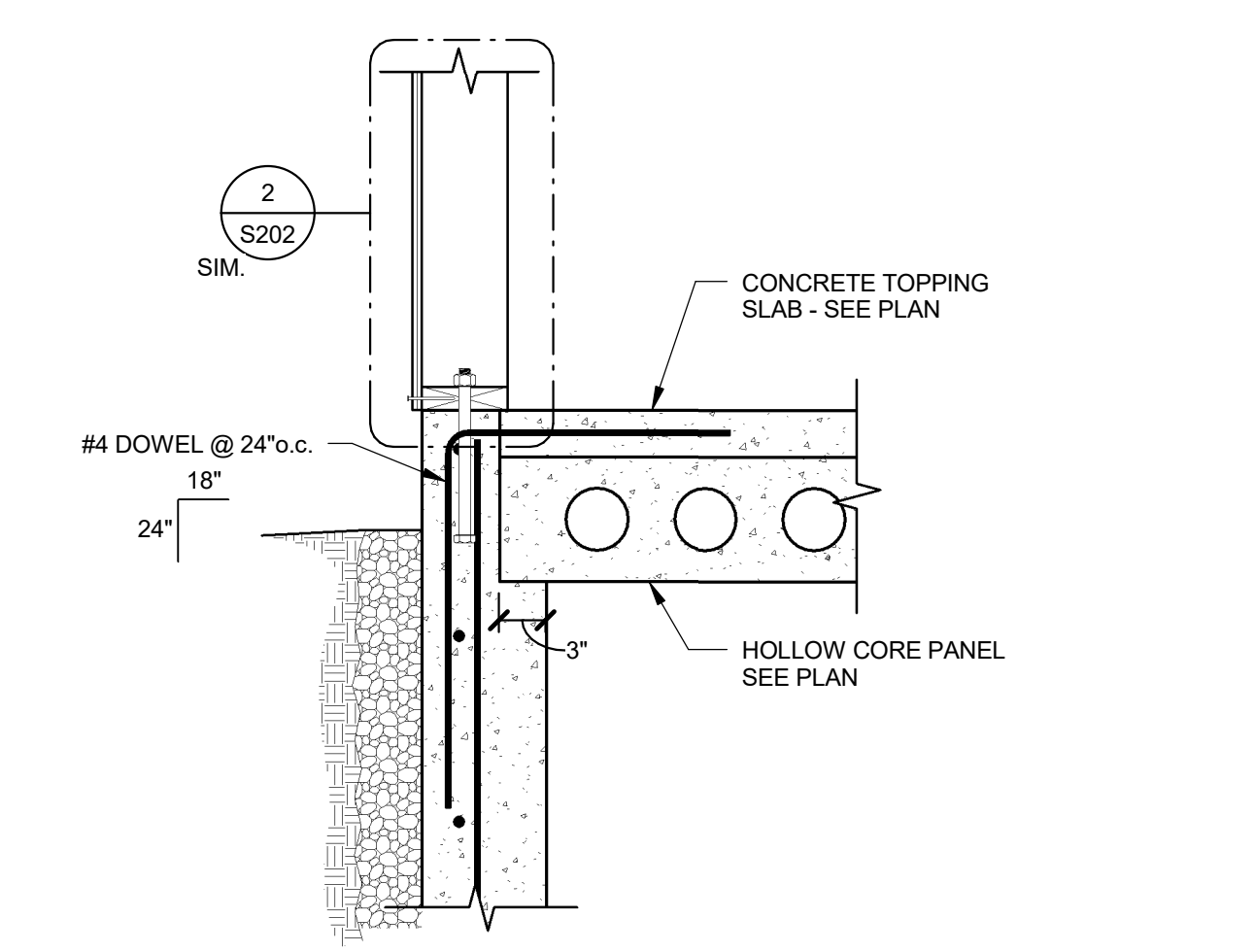
**13**  
DETAIL  
SCALE: NONE  
S202



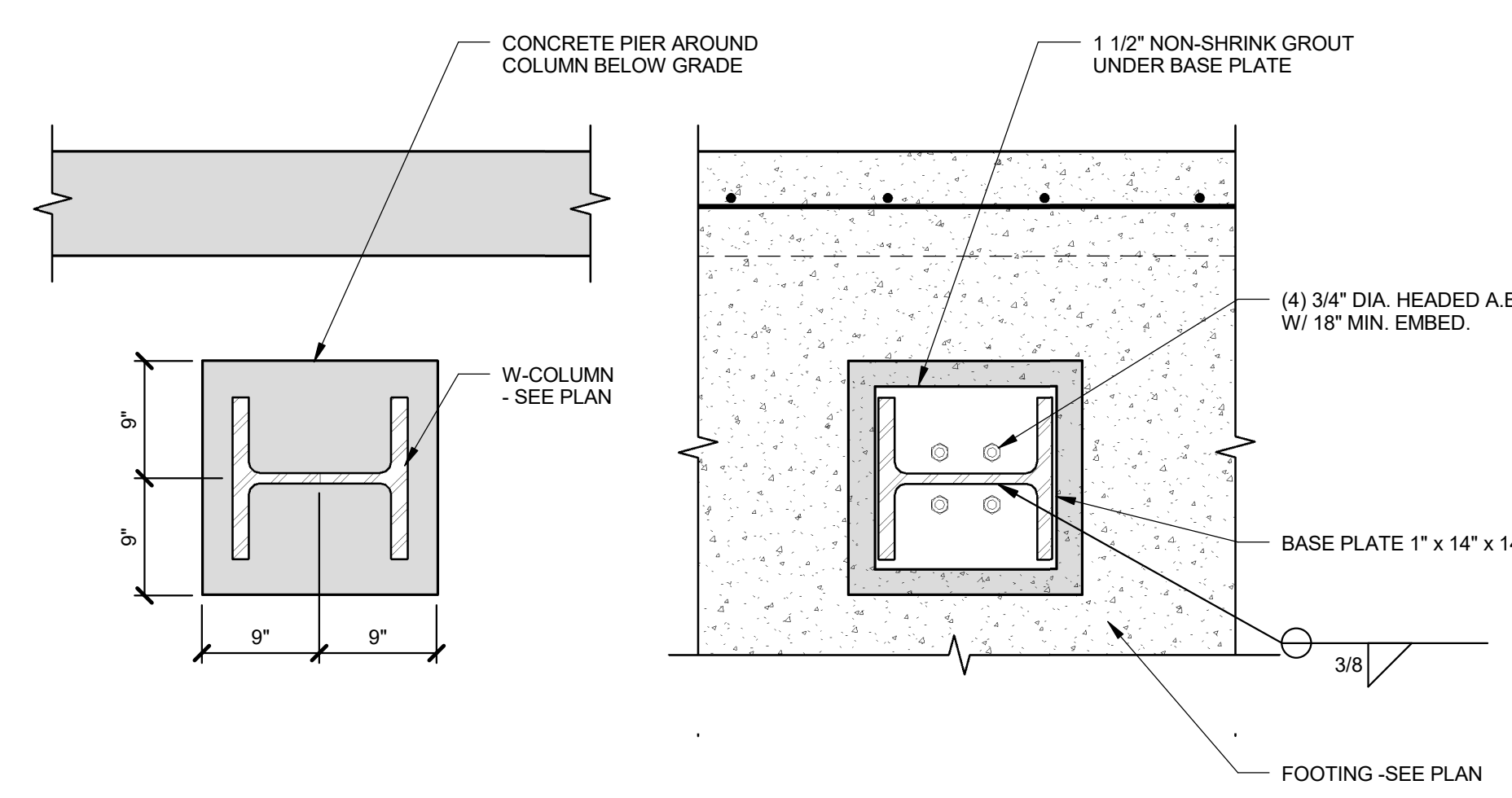
**14**  
DETAIL  
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S202



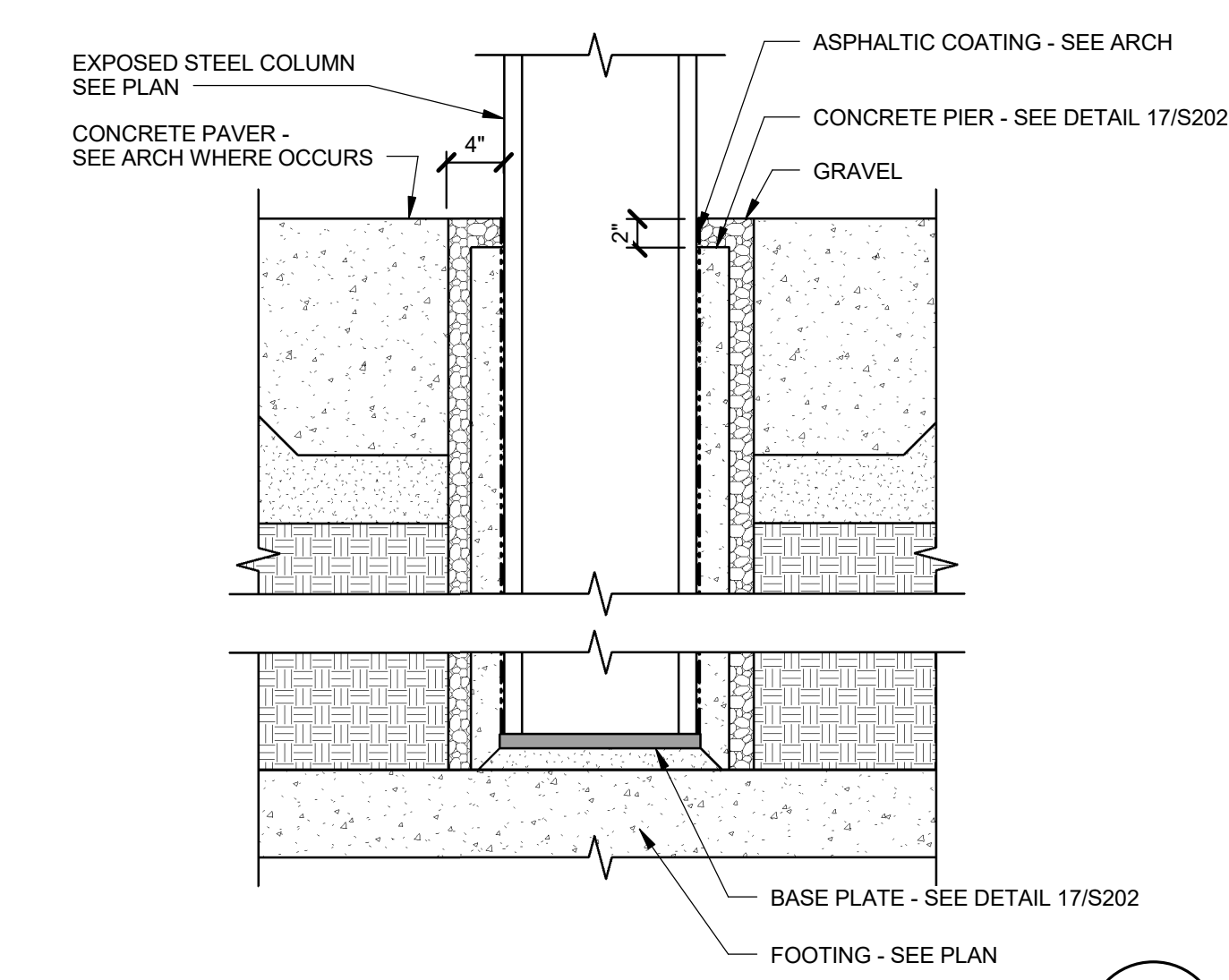
**15**  
DETAIL  
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S202



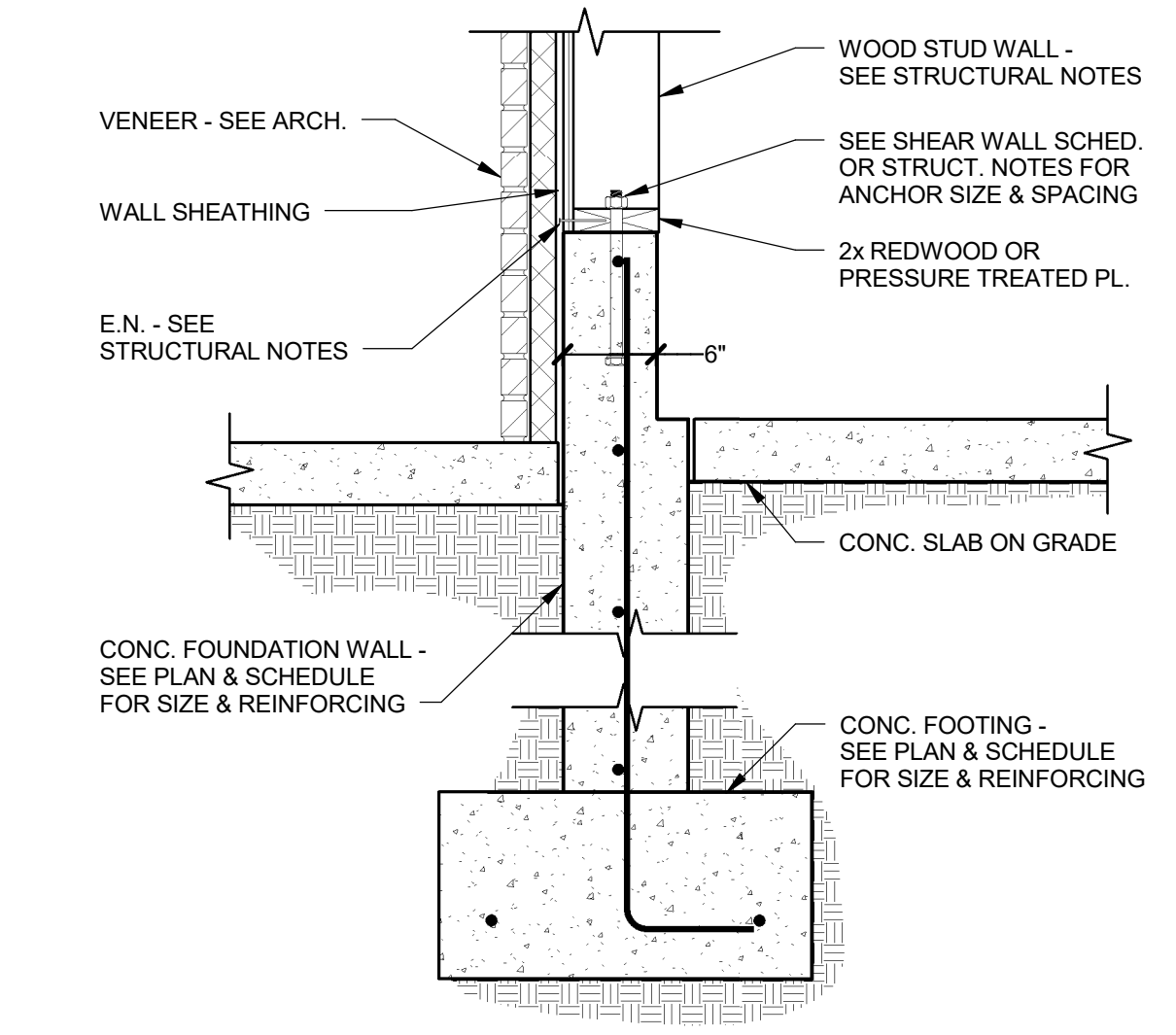
**16**  
DETAIL  
SCALE: NONE  
S202



**17**  
DETAIL  
SCALE: NONE  
S202

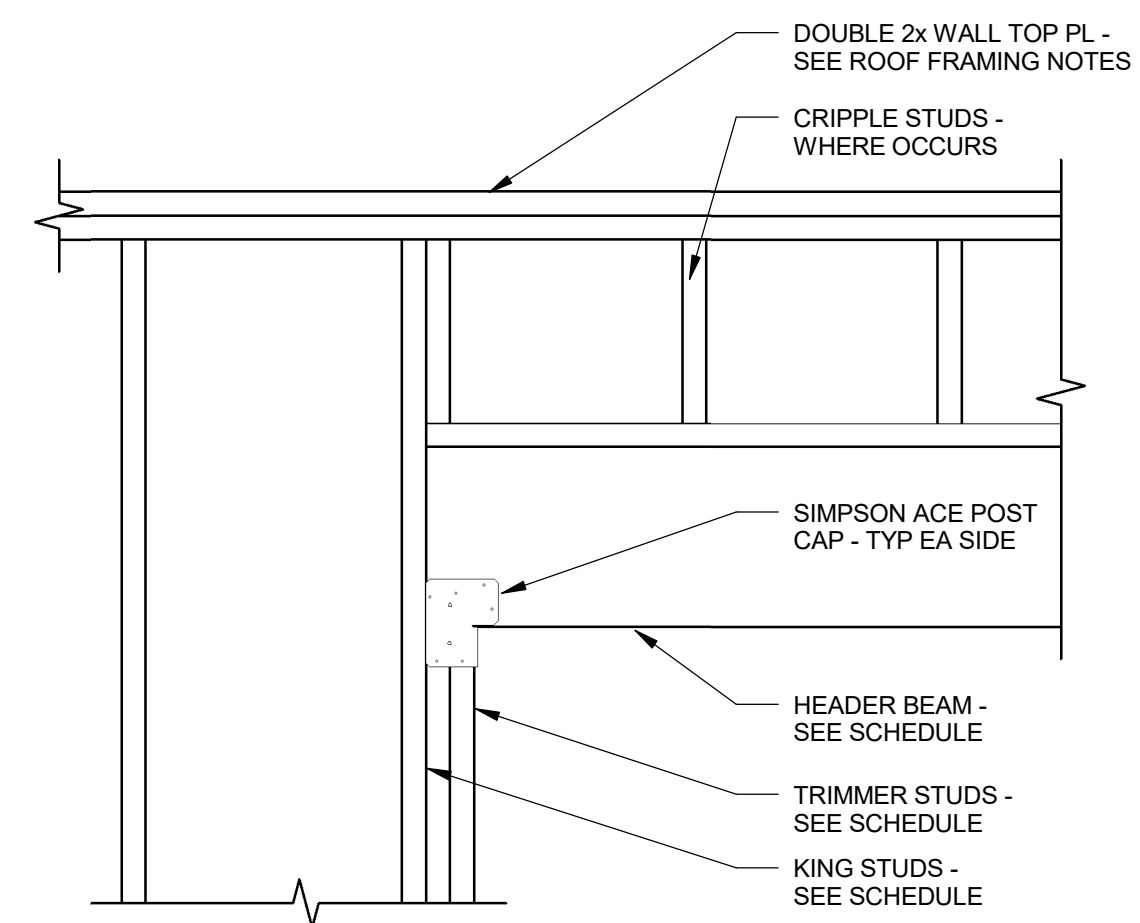


**18**  
DETAIL  
SCALE: NONE  
S202



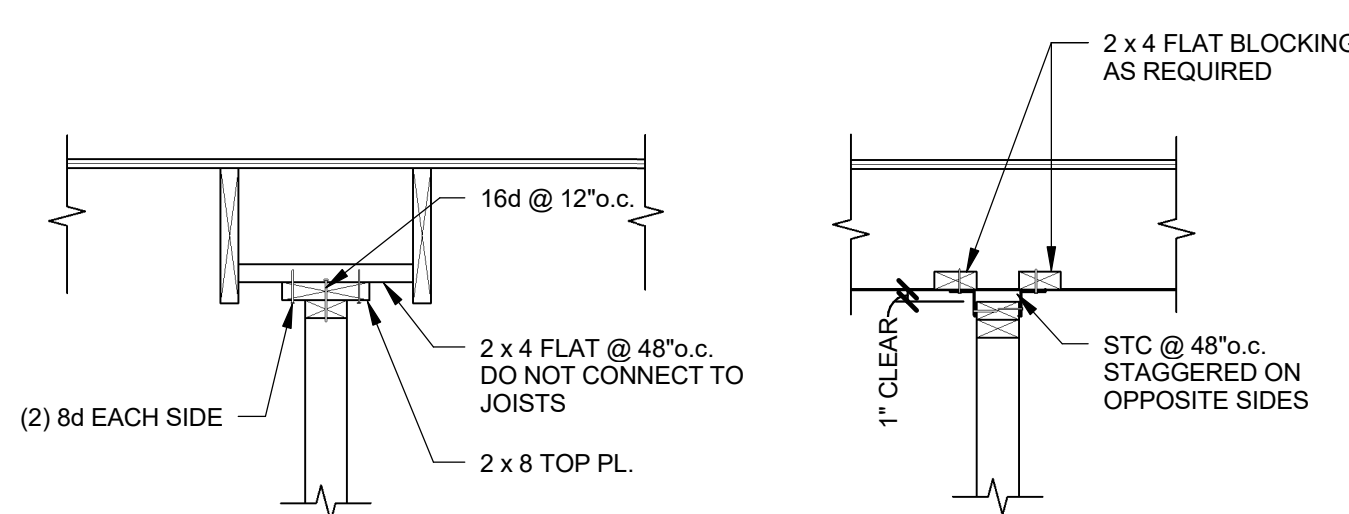
**19**  
DETAIL  
SCALE: NONE  
S202





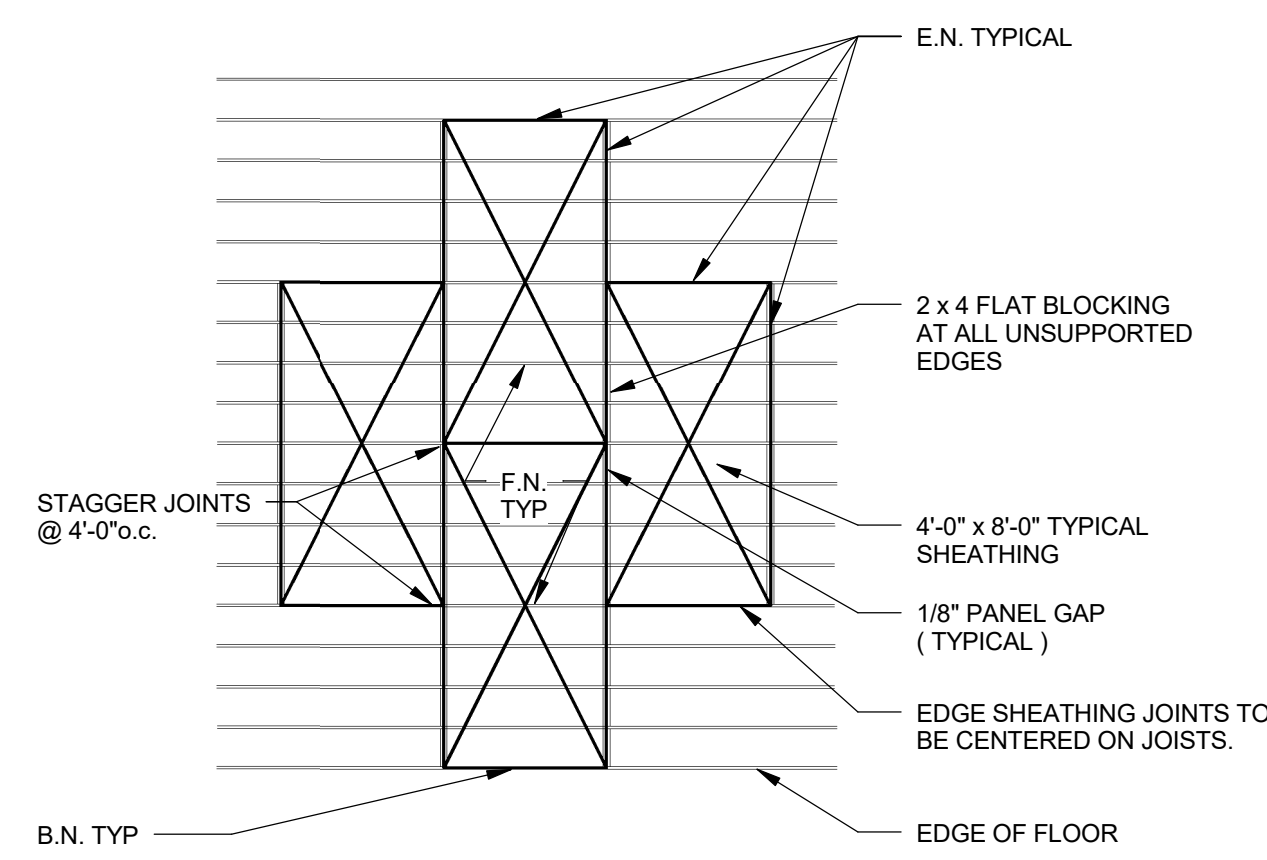
OPENING SIZE	KING STUDS	TRIMMER STUDS	HEADER
UP TO 6'-0"	(1) 2x6	(1) 2x6	(3) 2x10
6'-1" TO 10'-0"	(1) 2x6	(2) 2x6	(3) 1 3/4" x 11 7/8" LVL

DETAIL SCALE: NONE **1** S203

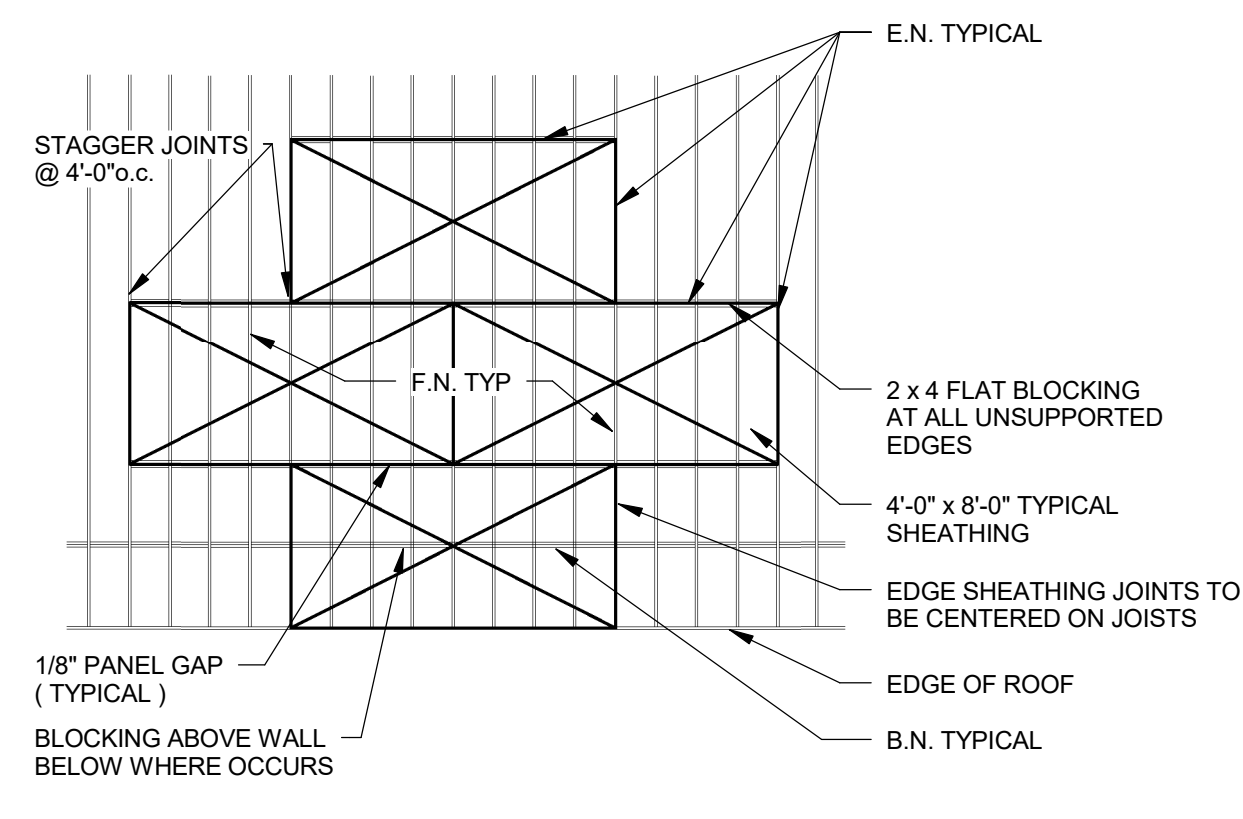


WALL PARALLEL TO JOISTS WALL PERPENDICULAR TO JOISTS

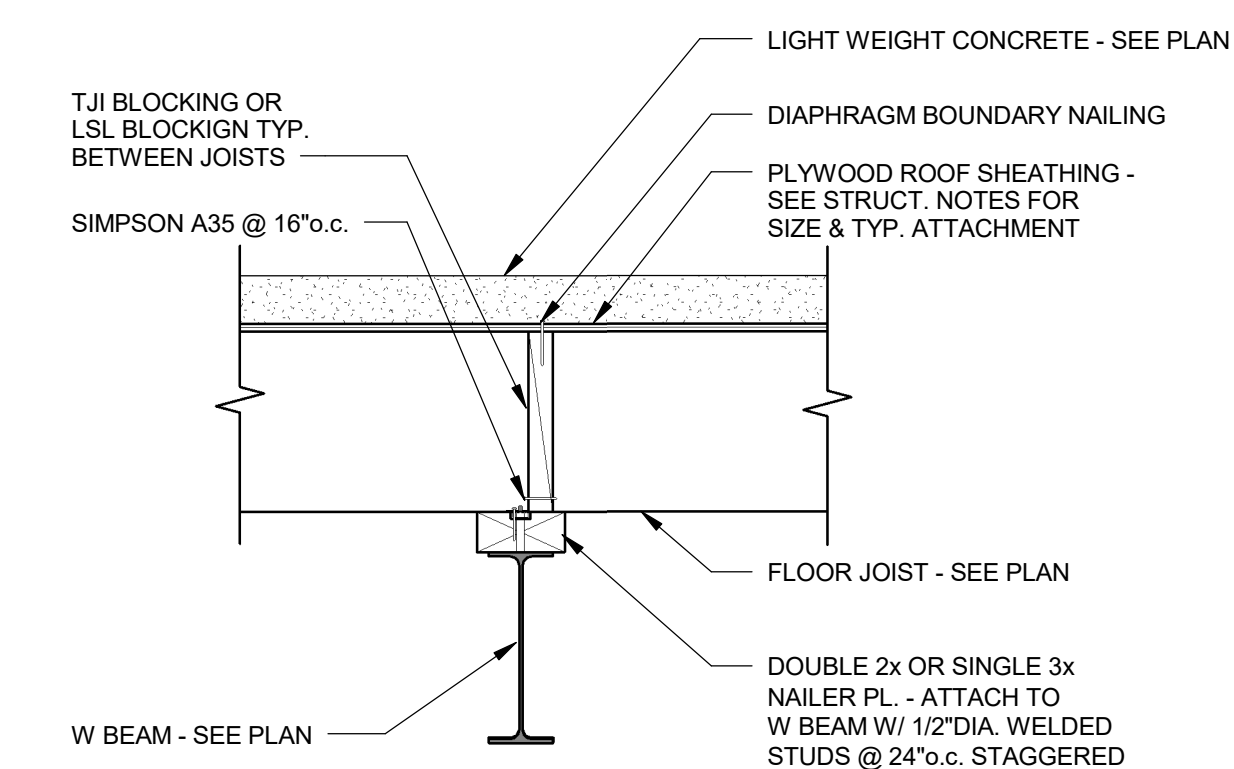
DETAIL SCALE: NONE **2** S203



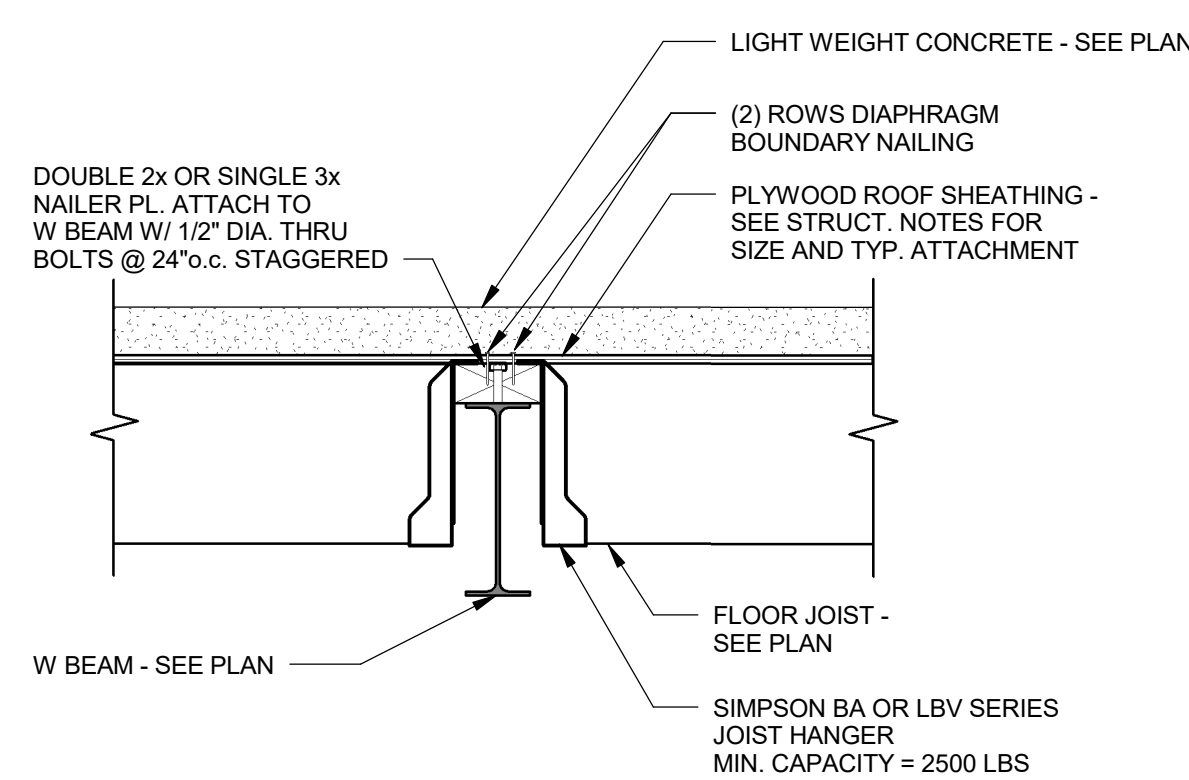
TYPICAL SHEATHING LAYOUT SCALE: NONE



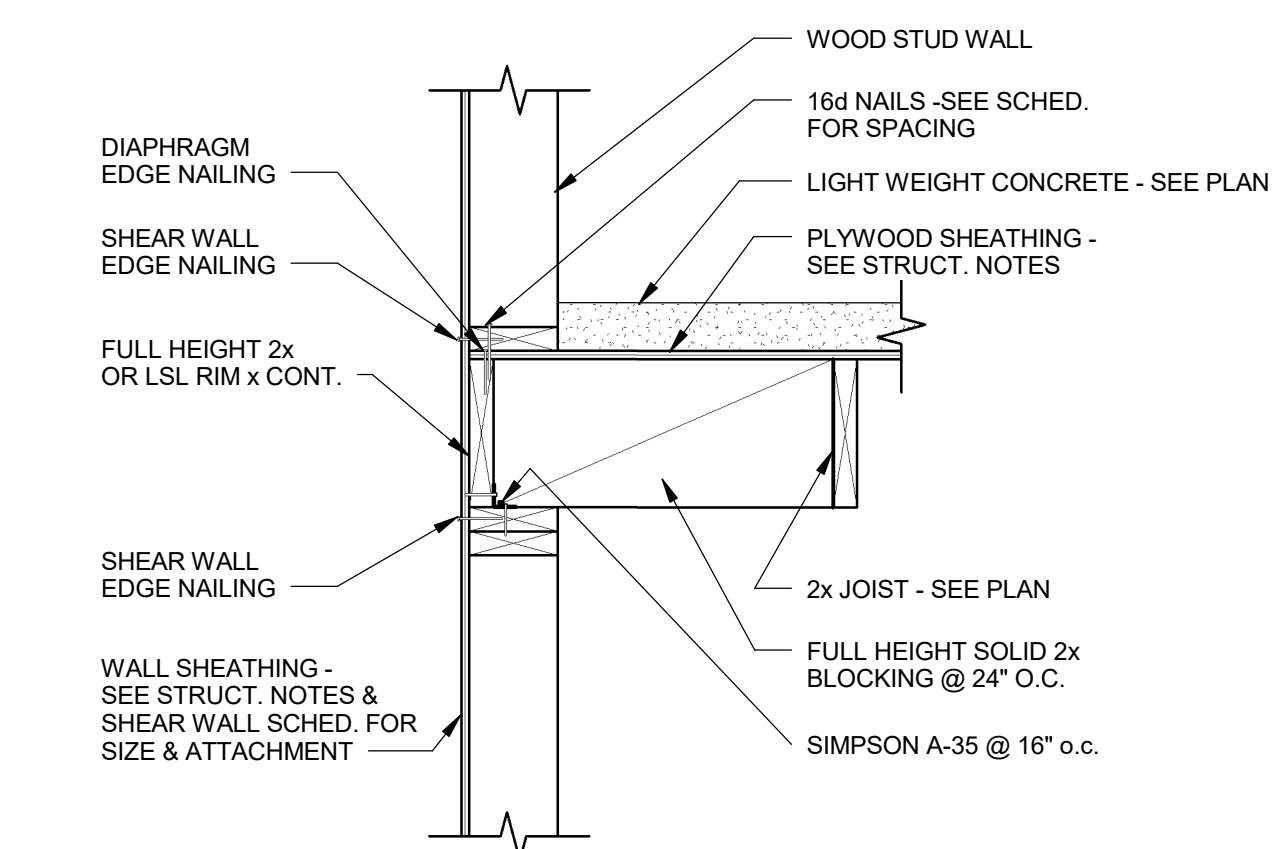
**3** S203



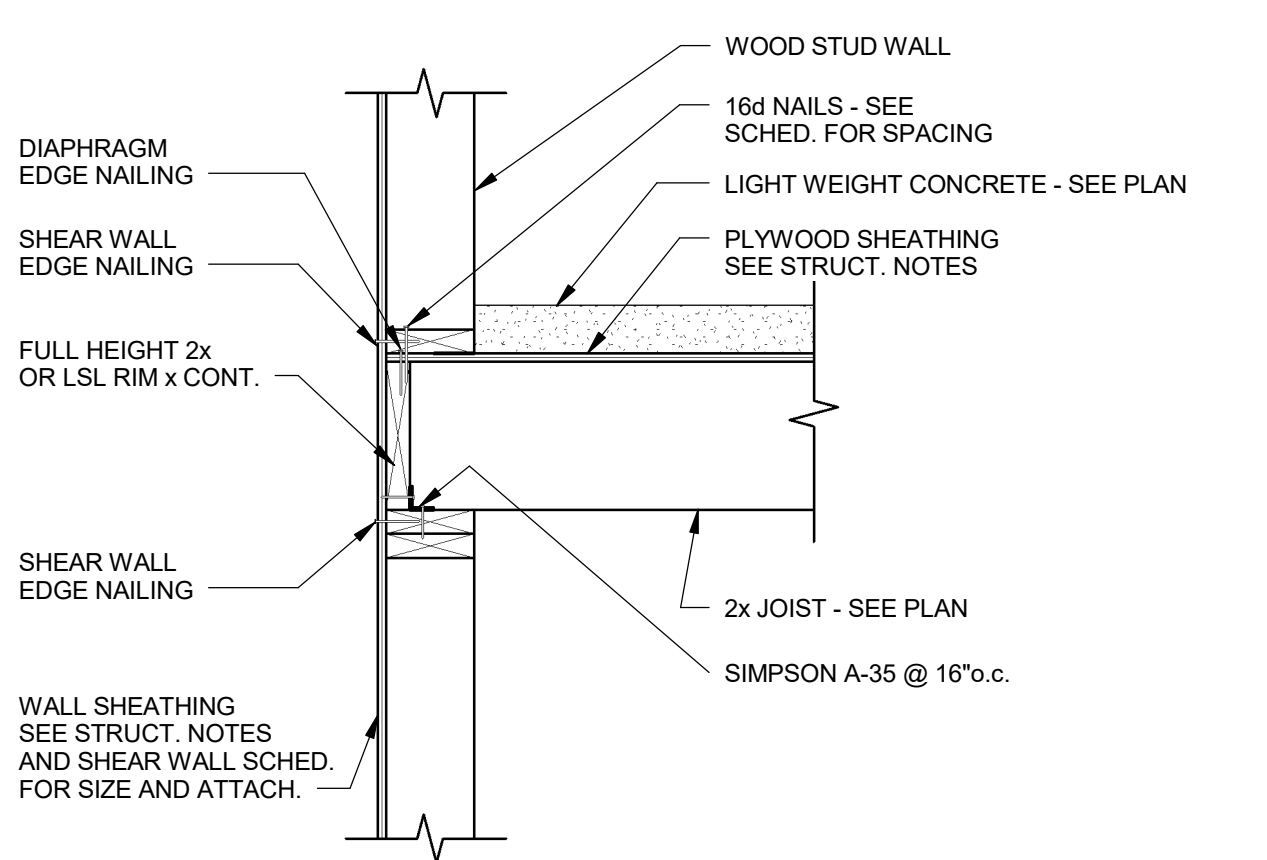
DETAIL SCALE: NONE **4** S203



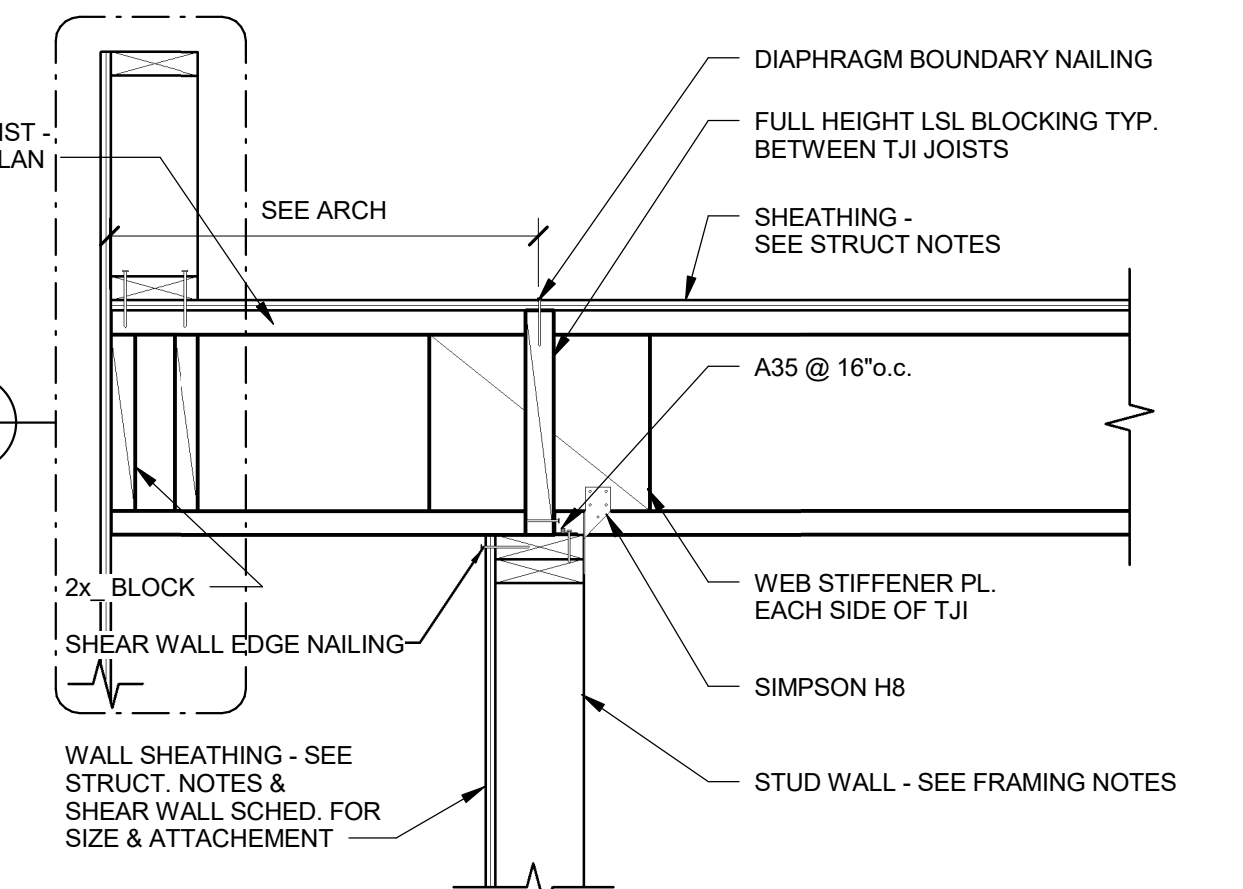
DETAIL SCALE: NONE **5** S203



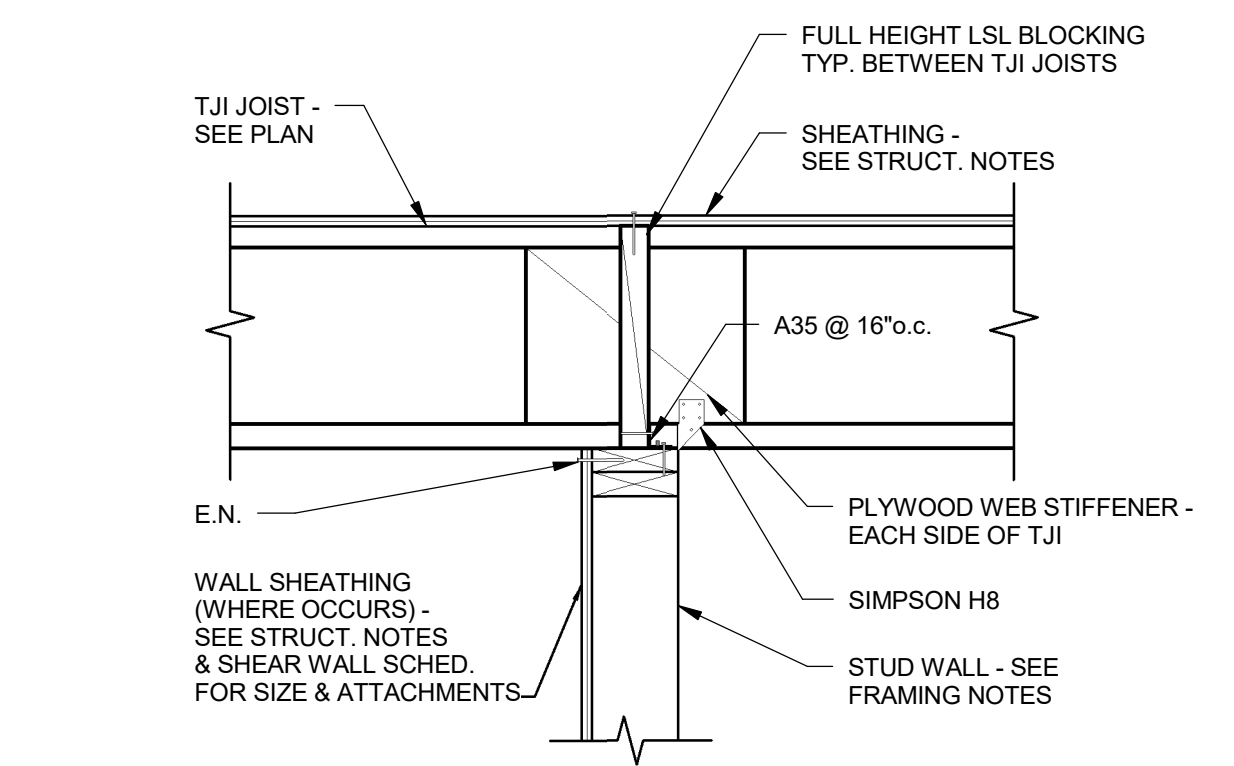
DETAIL SCALE: NONE **6** S203



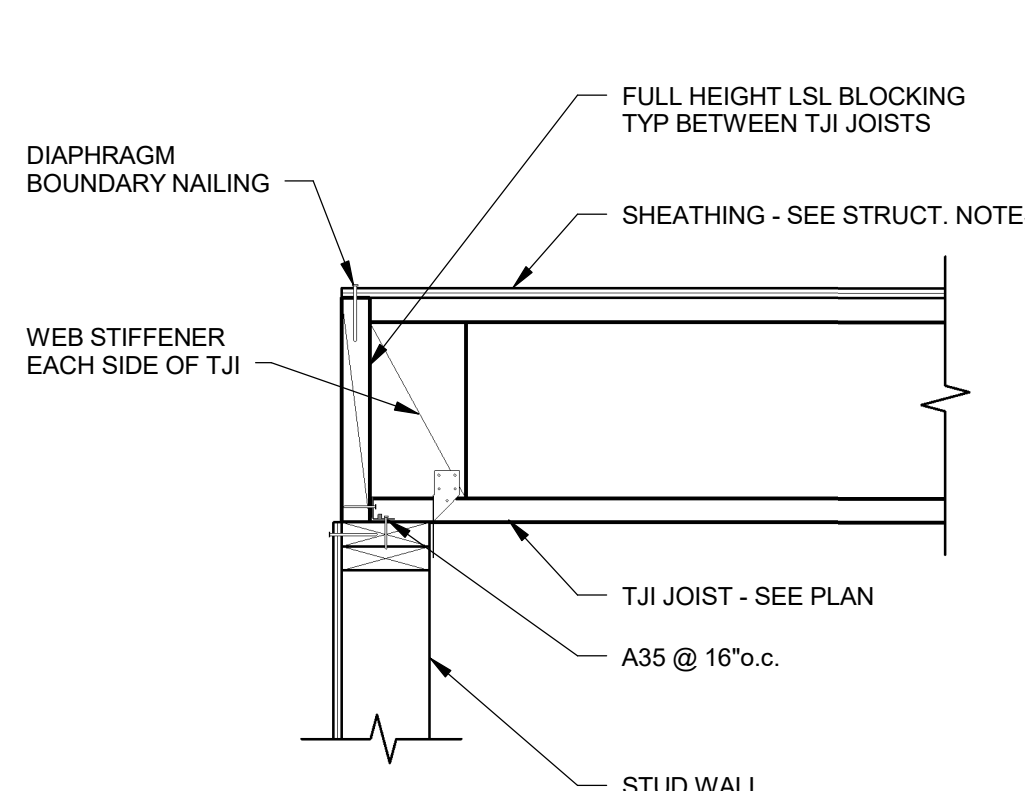
DETAIL SCALE: NONE **7** S203



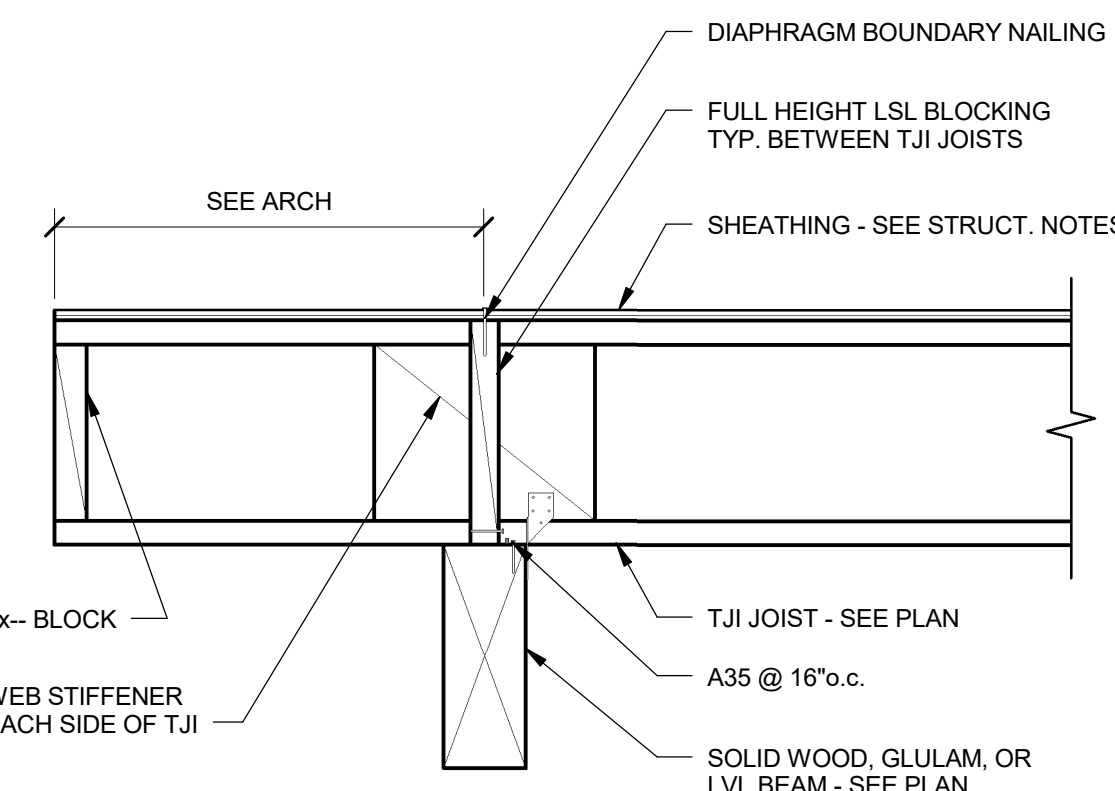
DETAIL SCALE: NONE **8** S203



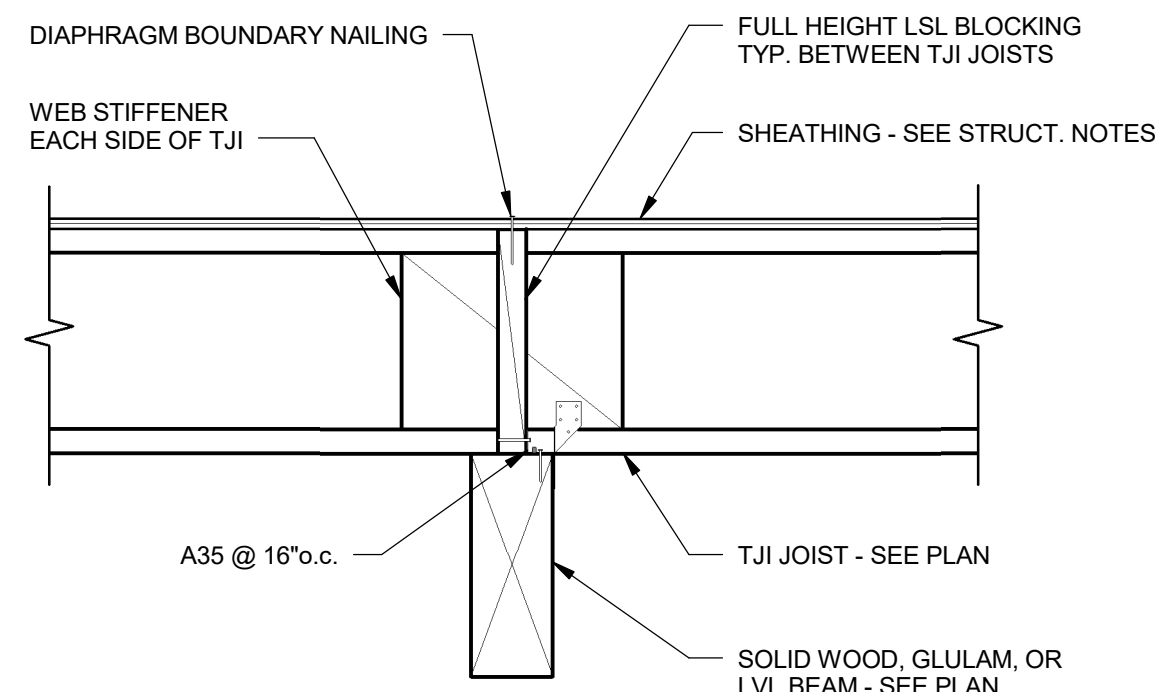
DETAIL SCALE: NONE **9** S203



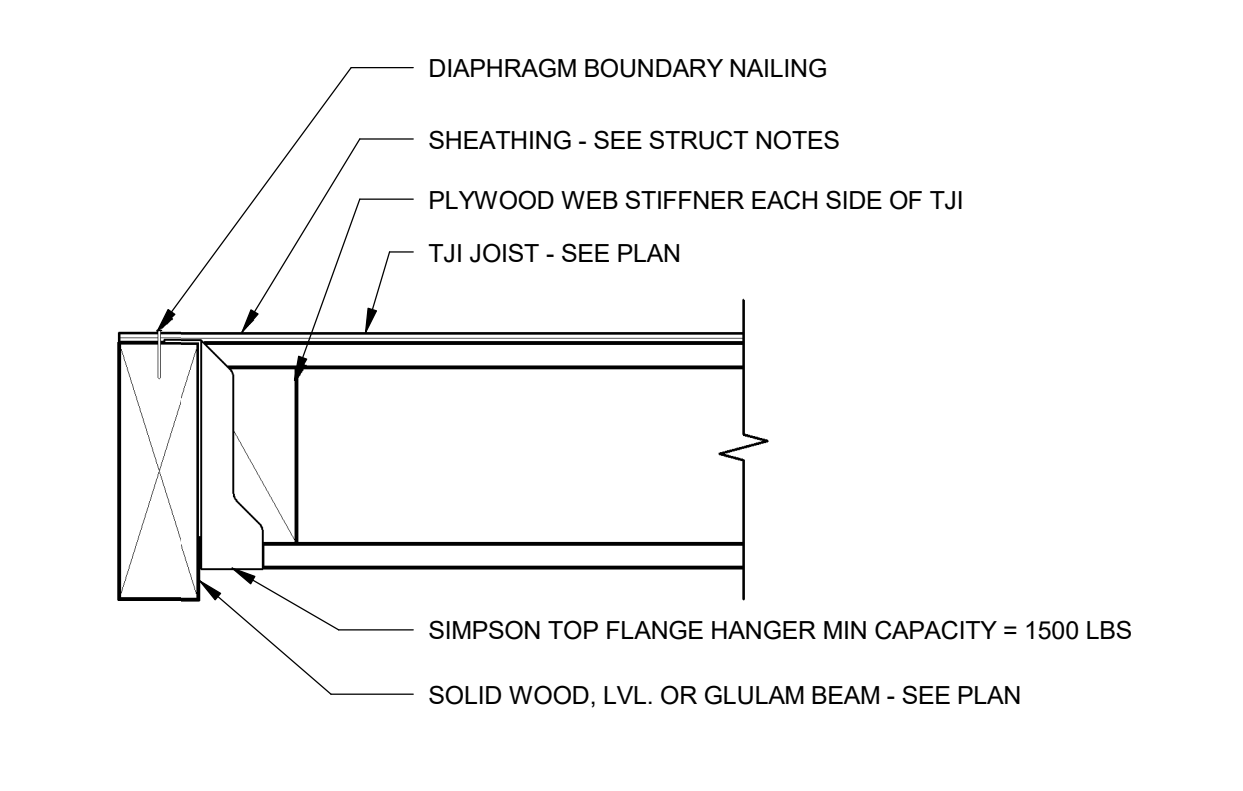
DETAIL NOT USED SCALE: NONE **10** S203



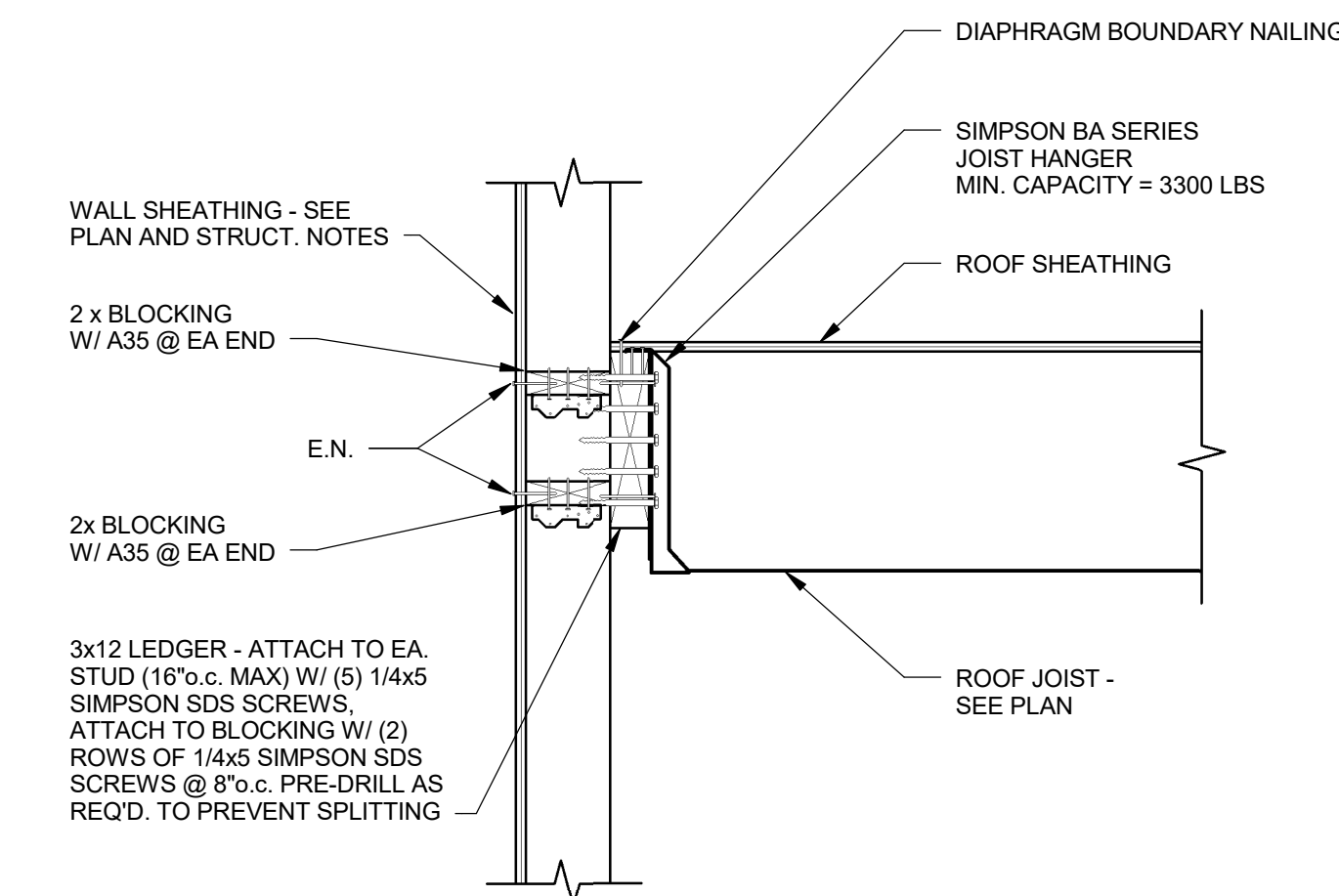
DETAIL NOT USED SCALE: NONE **11** S203



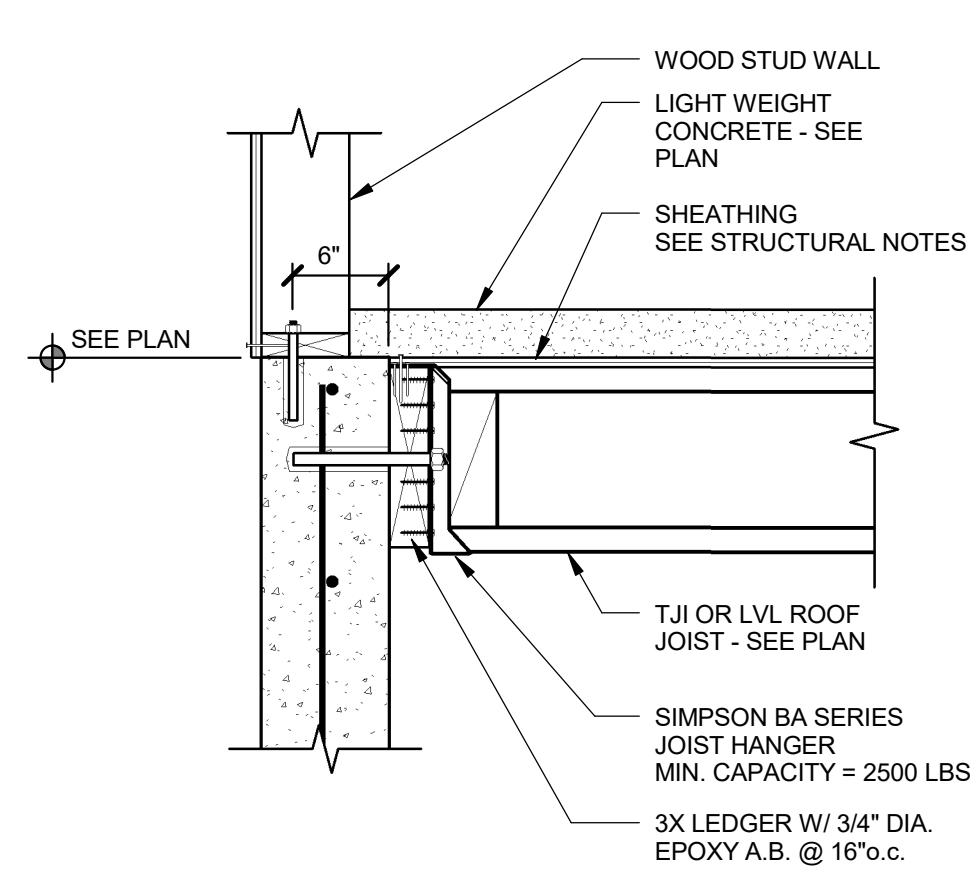
DETAIL NOT USED SCALE: NONE **12** S203



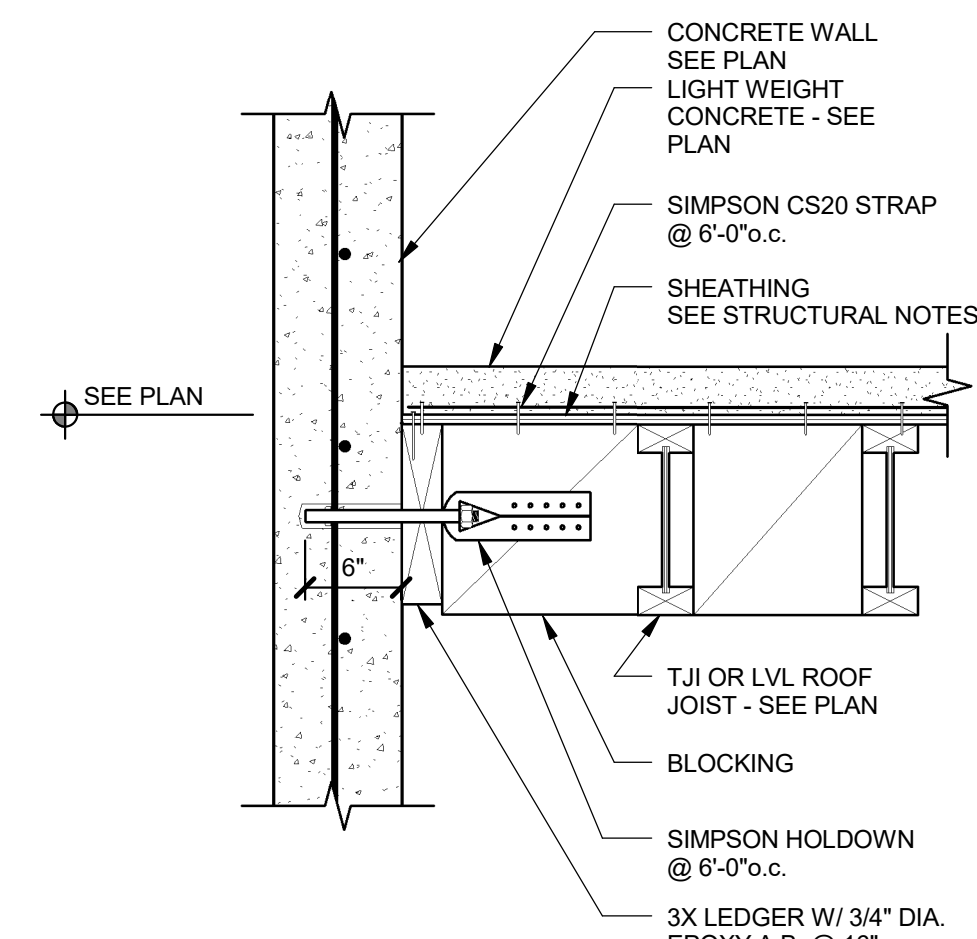
DETAIL NOT USED SCALE: NONE **13** S203



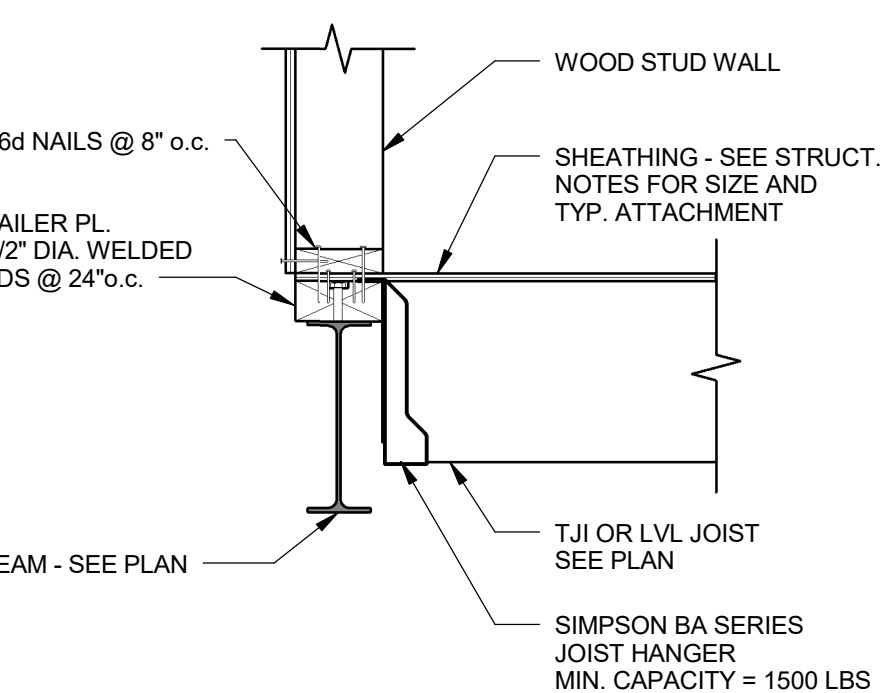
DETAIL SCALE: NONE **14** S203



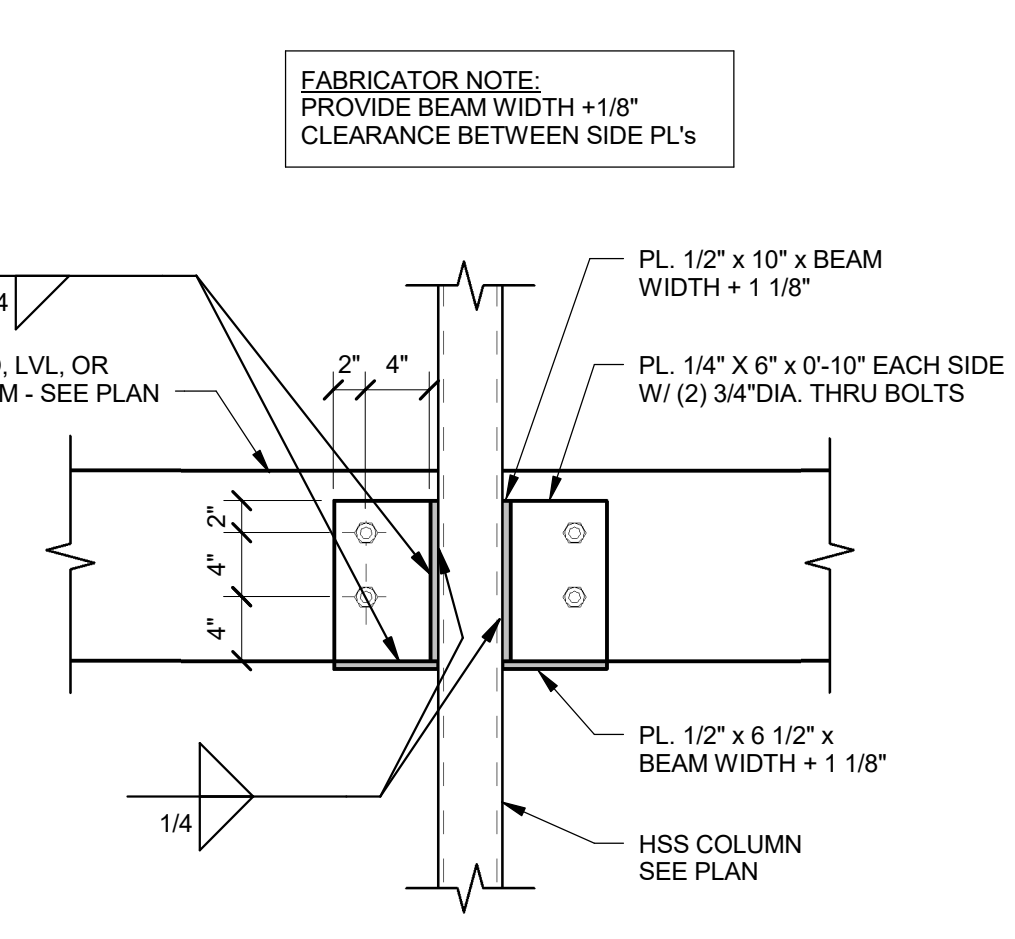
DETAIL SCALE: NONE **15** S203



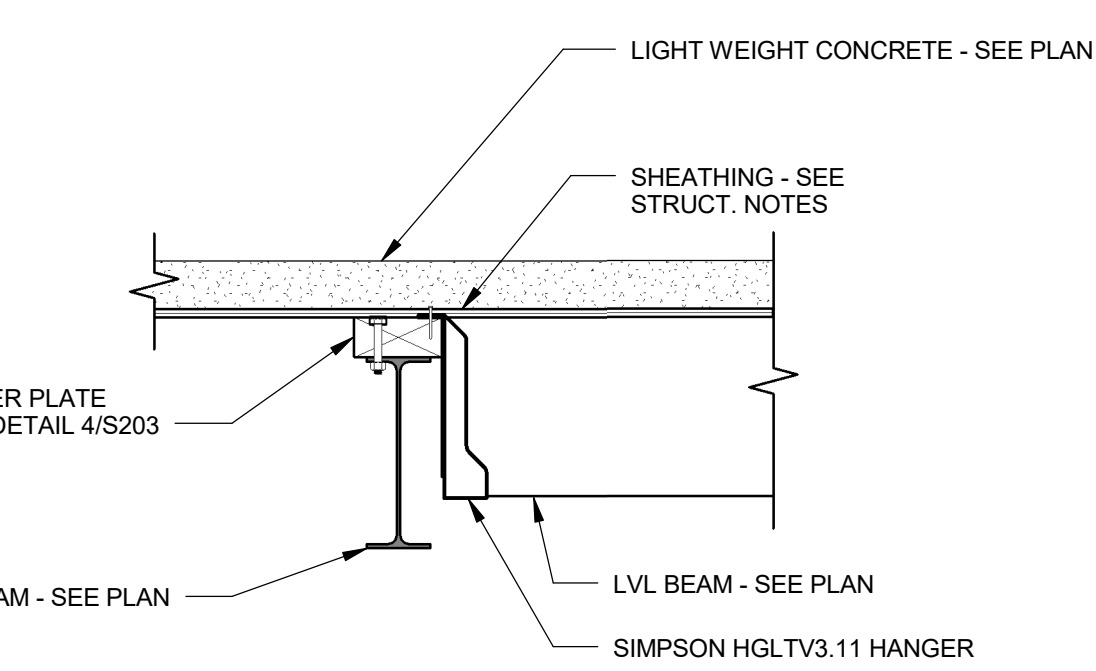
DETAIL SCALE: NONE **16** S203



DETAIL SCALE: NONE **17** S203

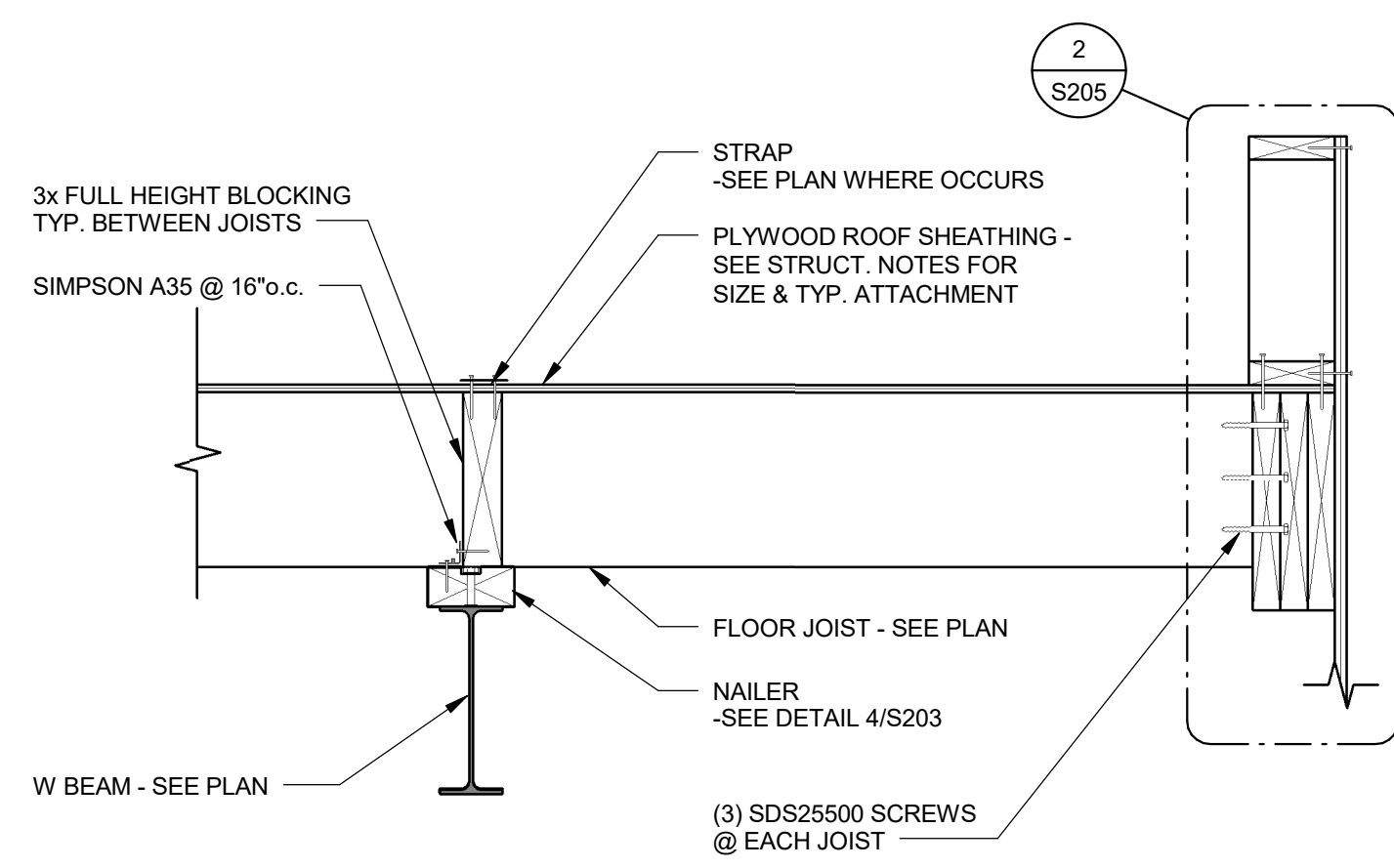


DETAIL SCALE: NONE **18** S203

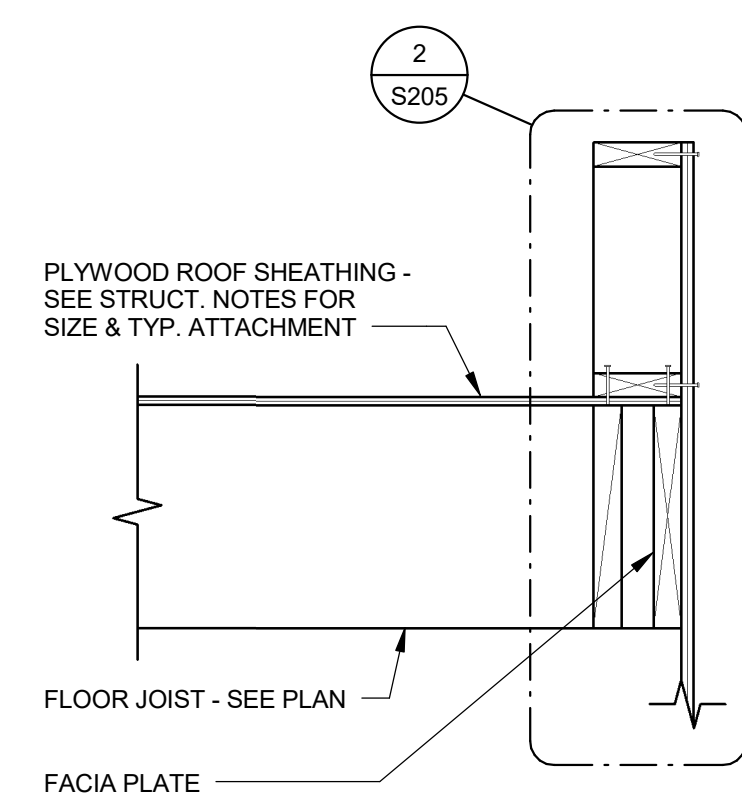


DETAIL SCALE: NONE **19** S203

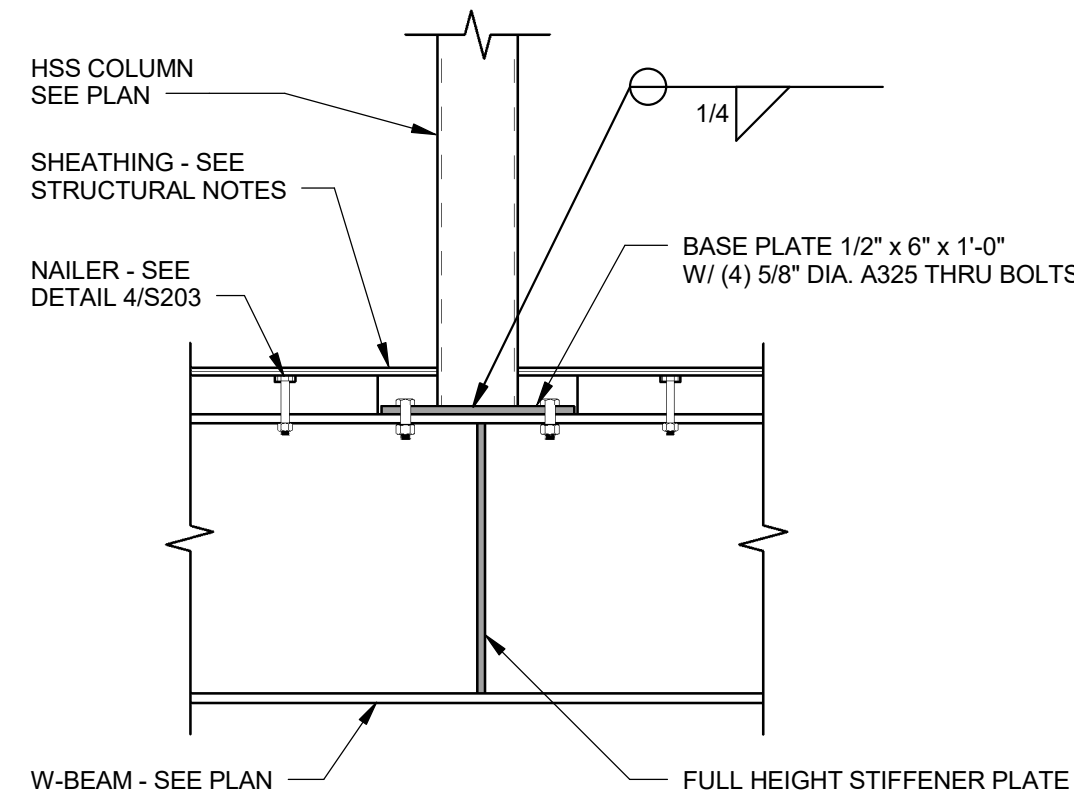




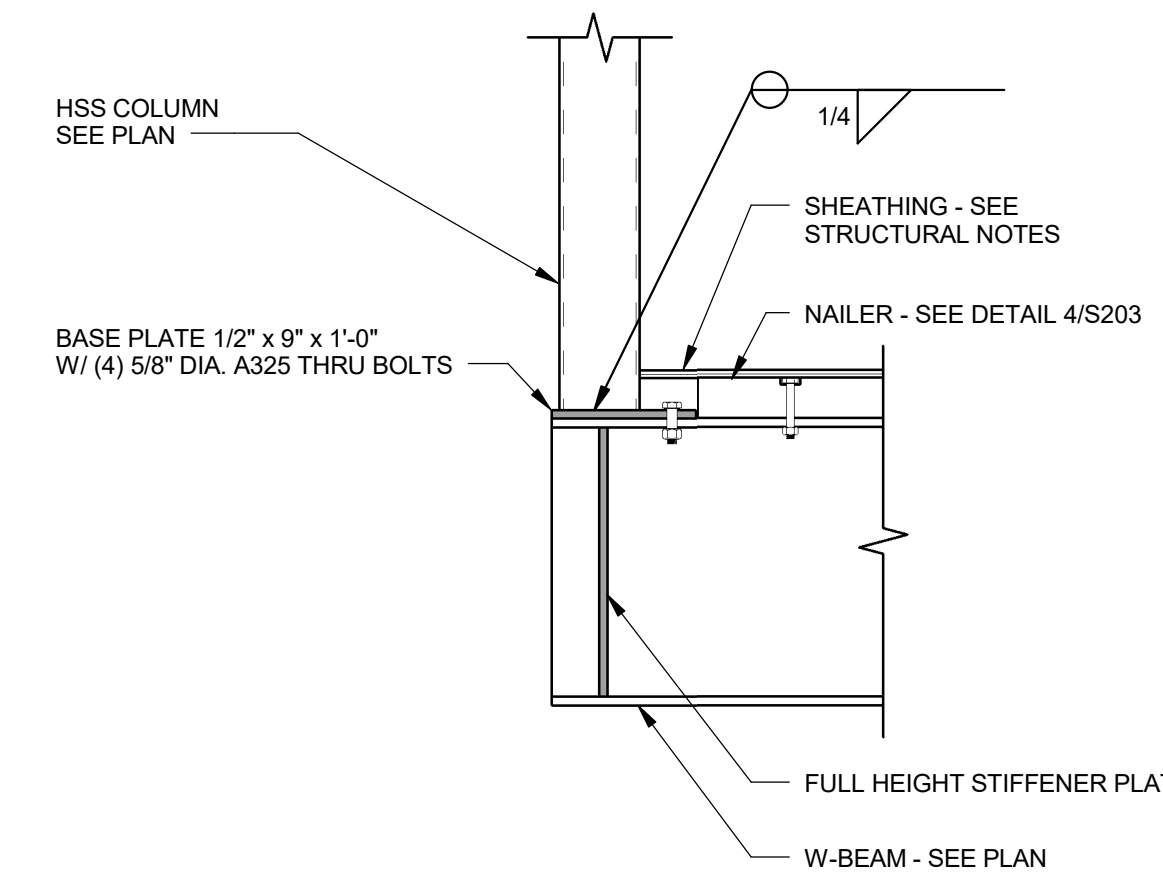
DETAIL  
SCALE: NONE  
1  
S204



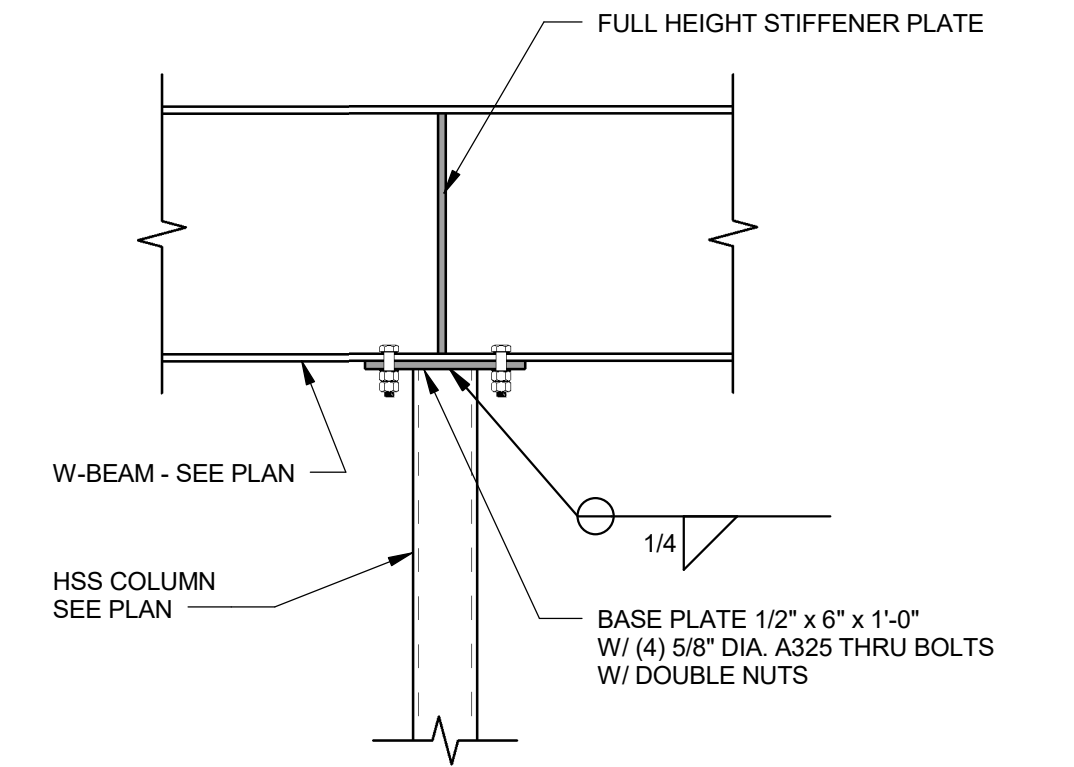
DETAIL  
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2  
S204



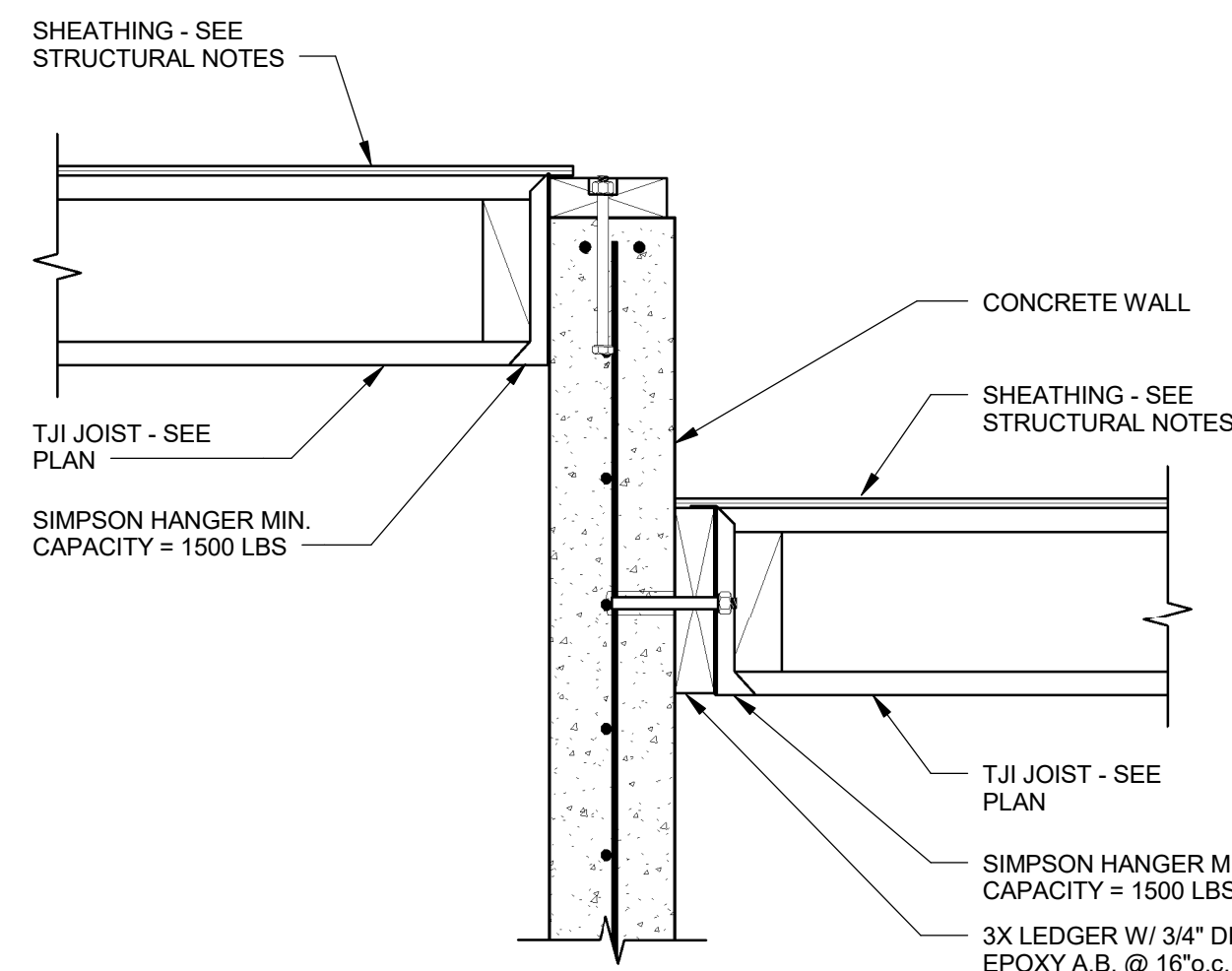
DETAIL  
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3  
S204



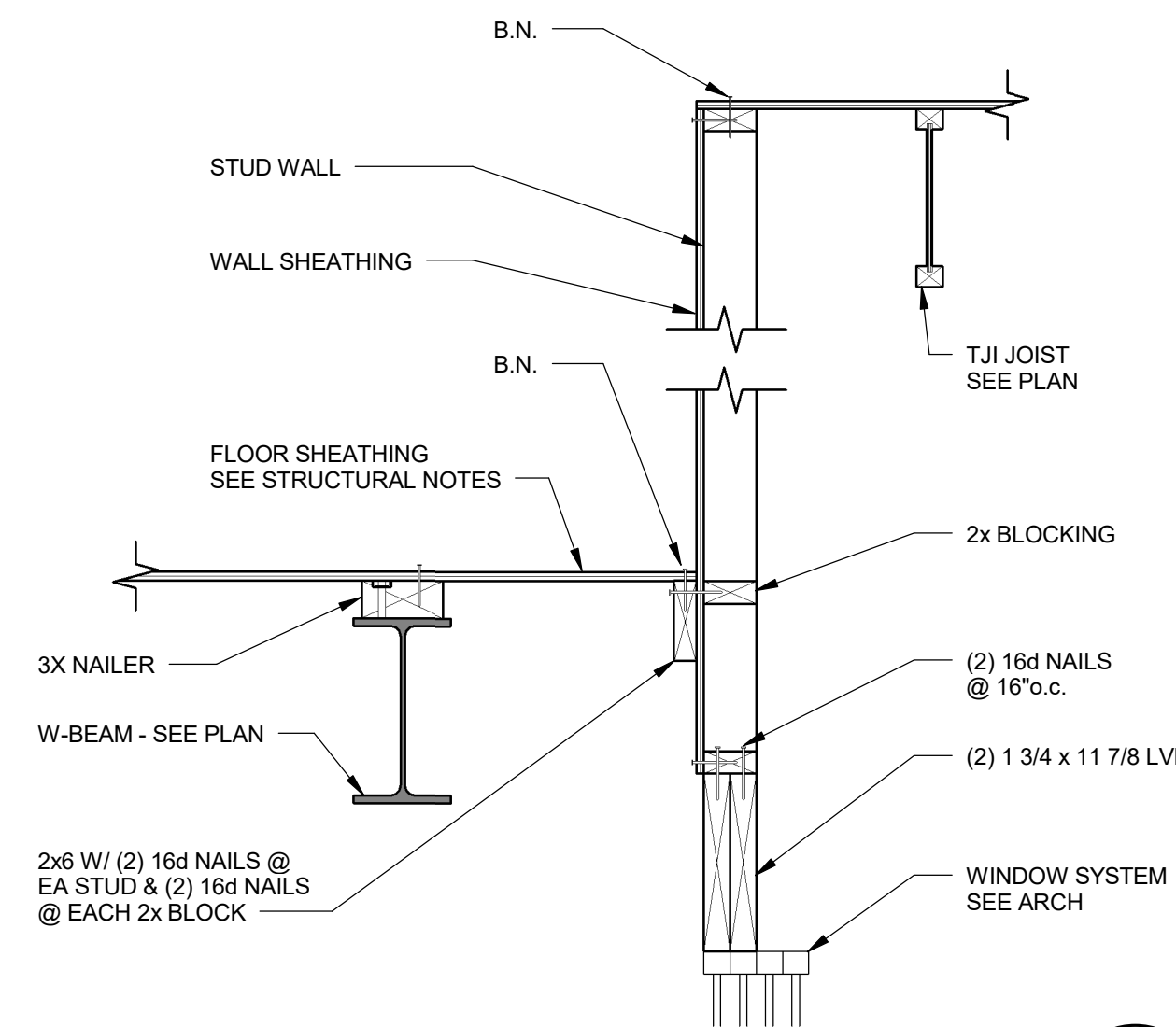
DETAIL  
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4  
S204



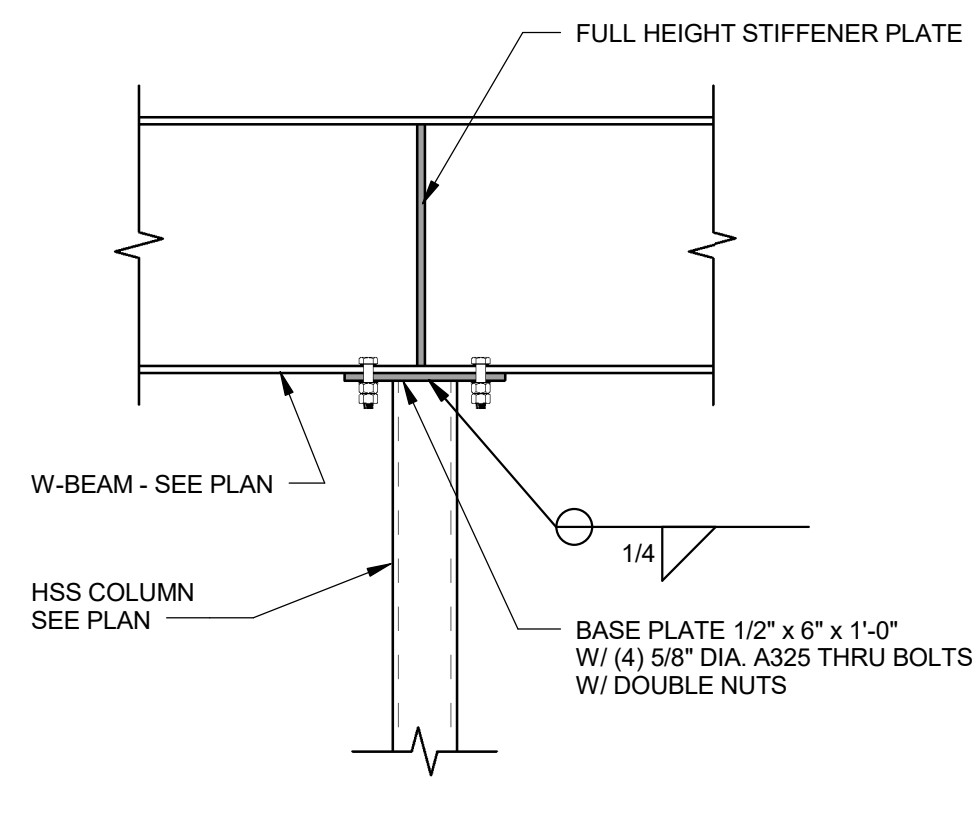
DETAIL  
SCALE: NONE  
5  
S204



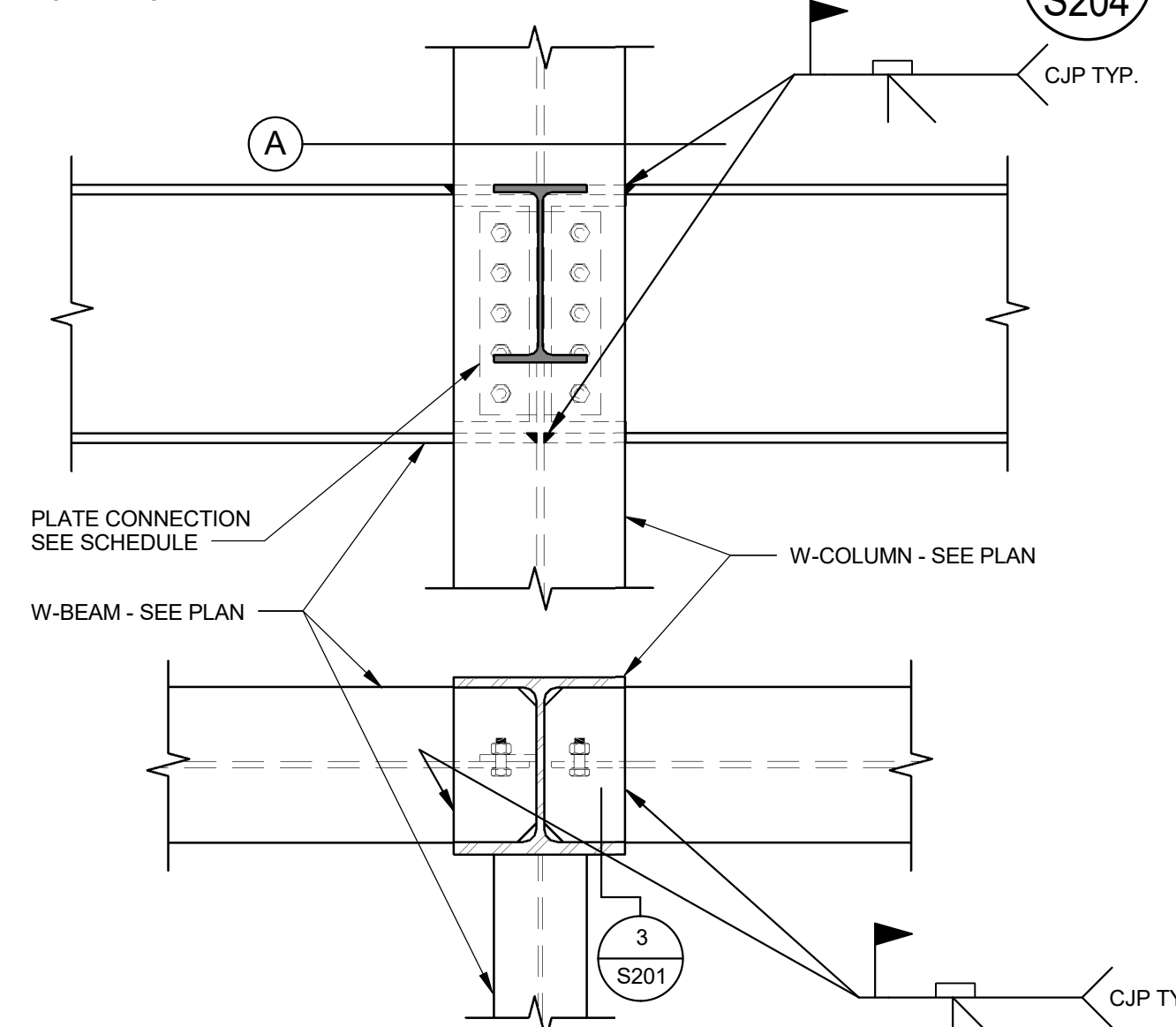
DETAIL  
SCALE: NONE  
6  
S204



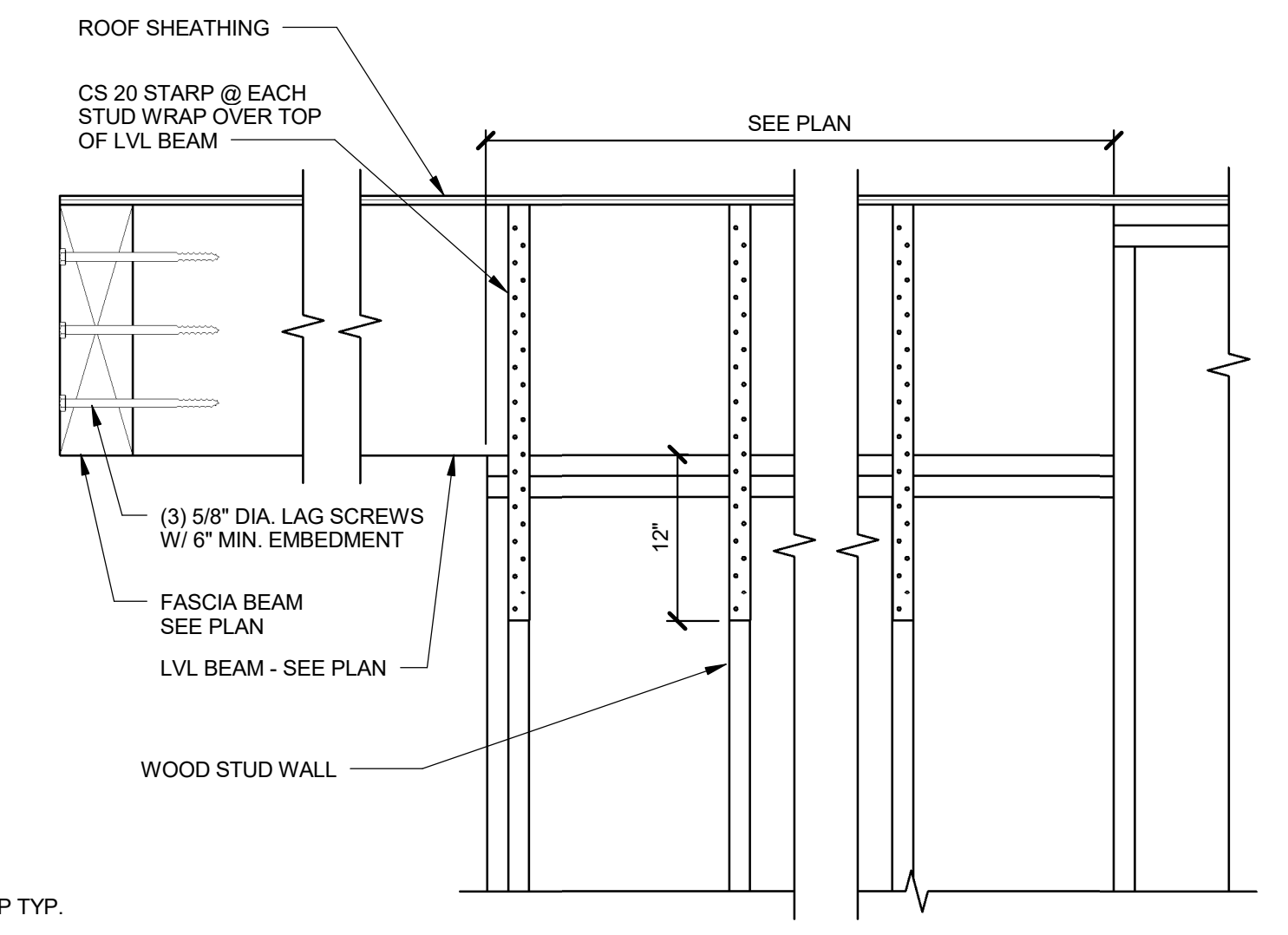
DETAIL  
SCALE: NONE  
7  
S204



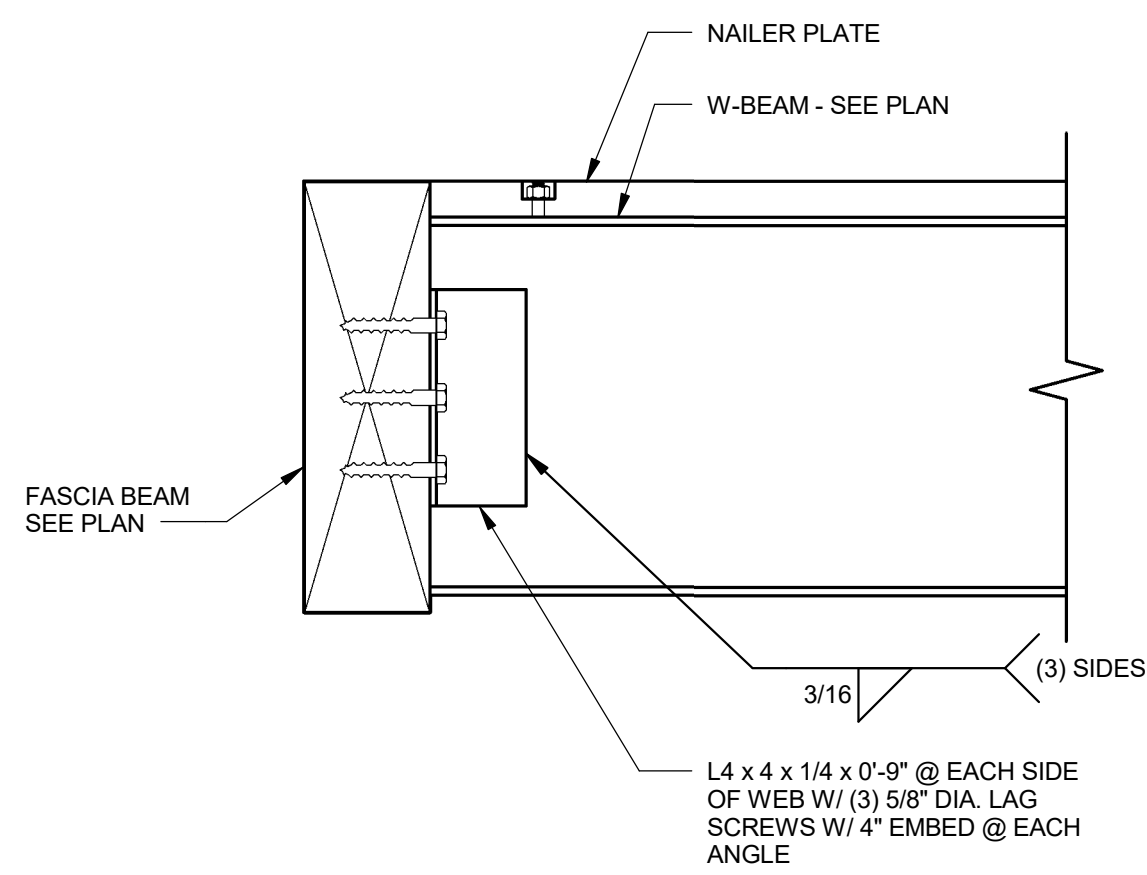
DETAIL  
SCALE: NONE  
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S204



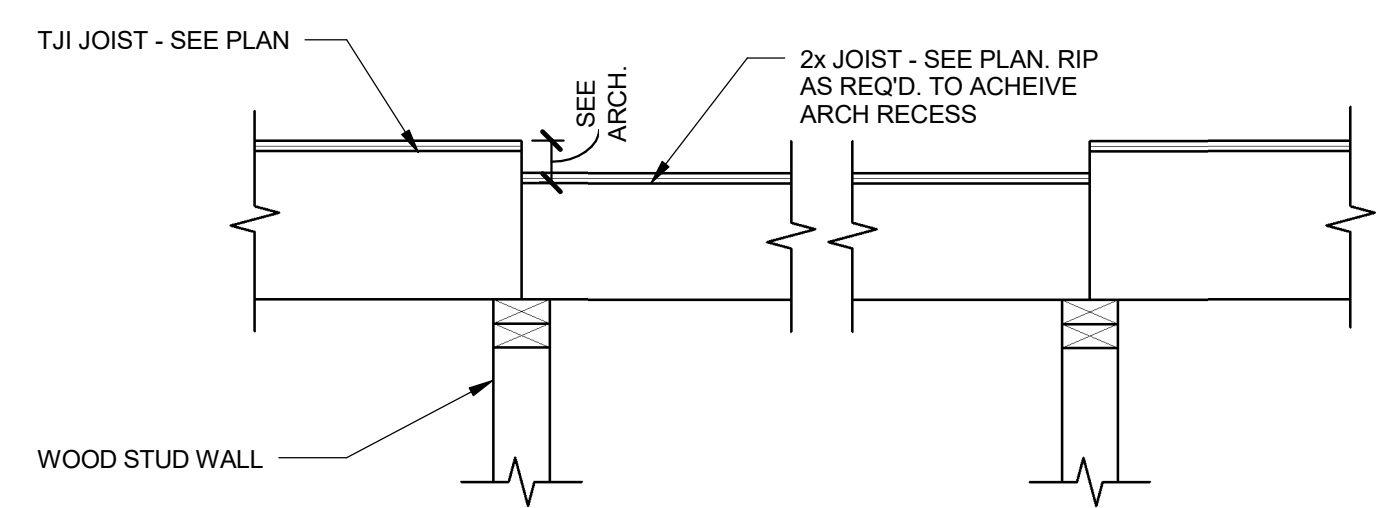
DETAIL  
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S204



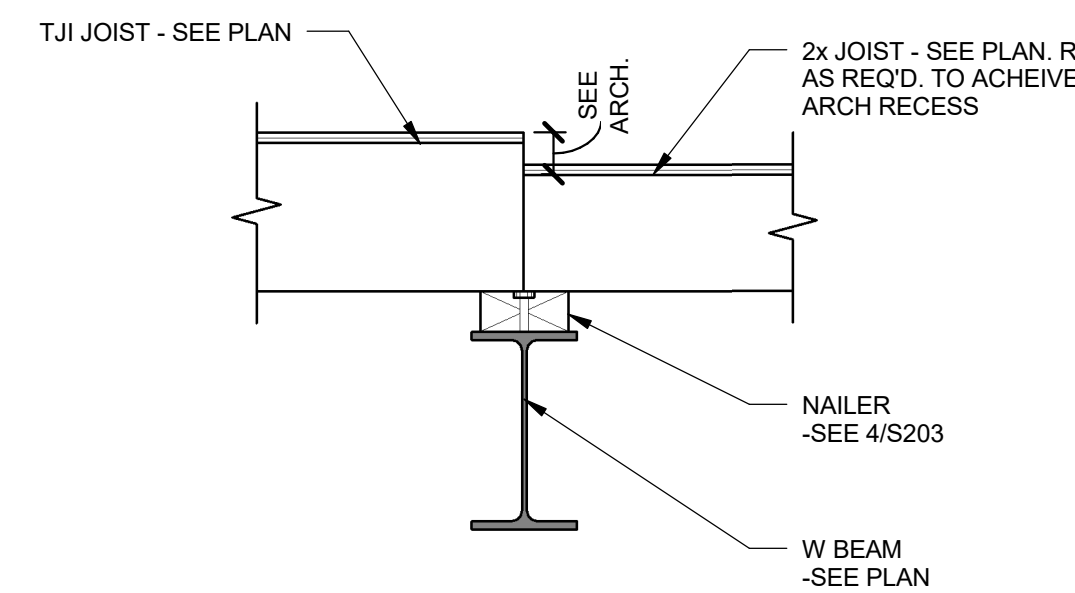
DETAIL  
SCALE: NONE  
10  
S204



DETAIL  
SCALE: NONE  
11  
S204

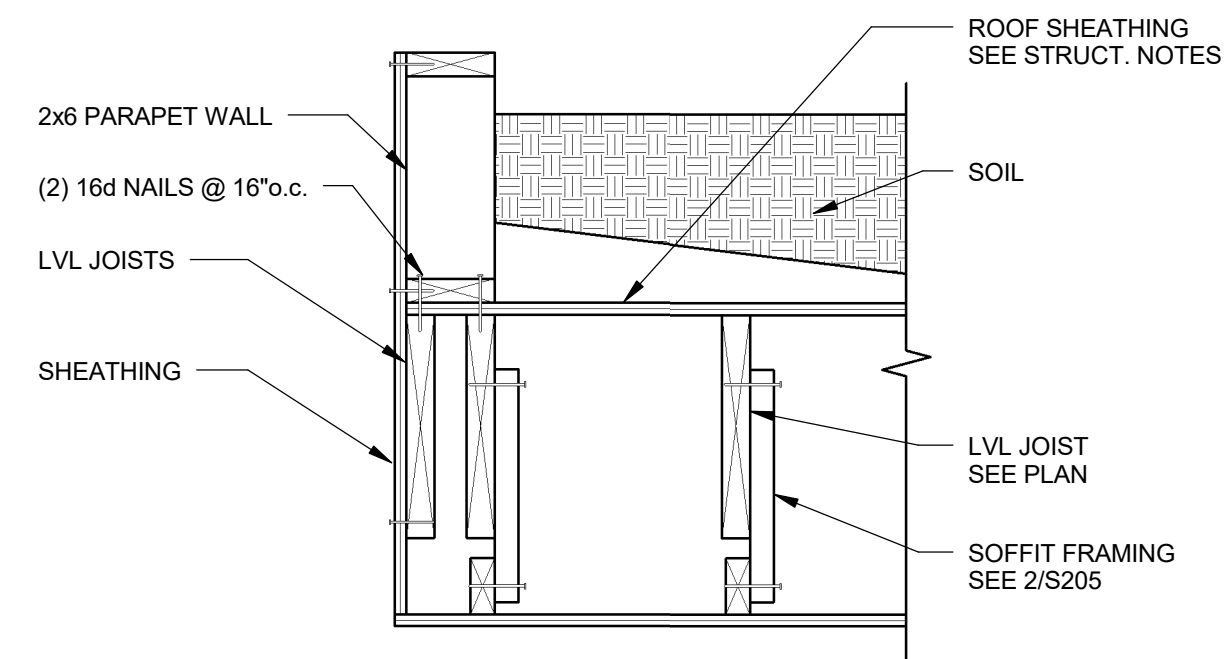


DETAIL  
SCALE: NONE  
12  
S204



DETAIL  
SCALE: NONE  
13  
S204

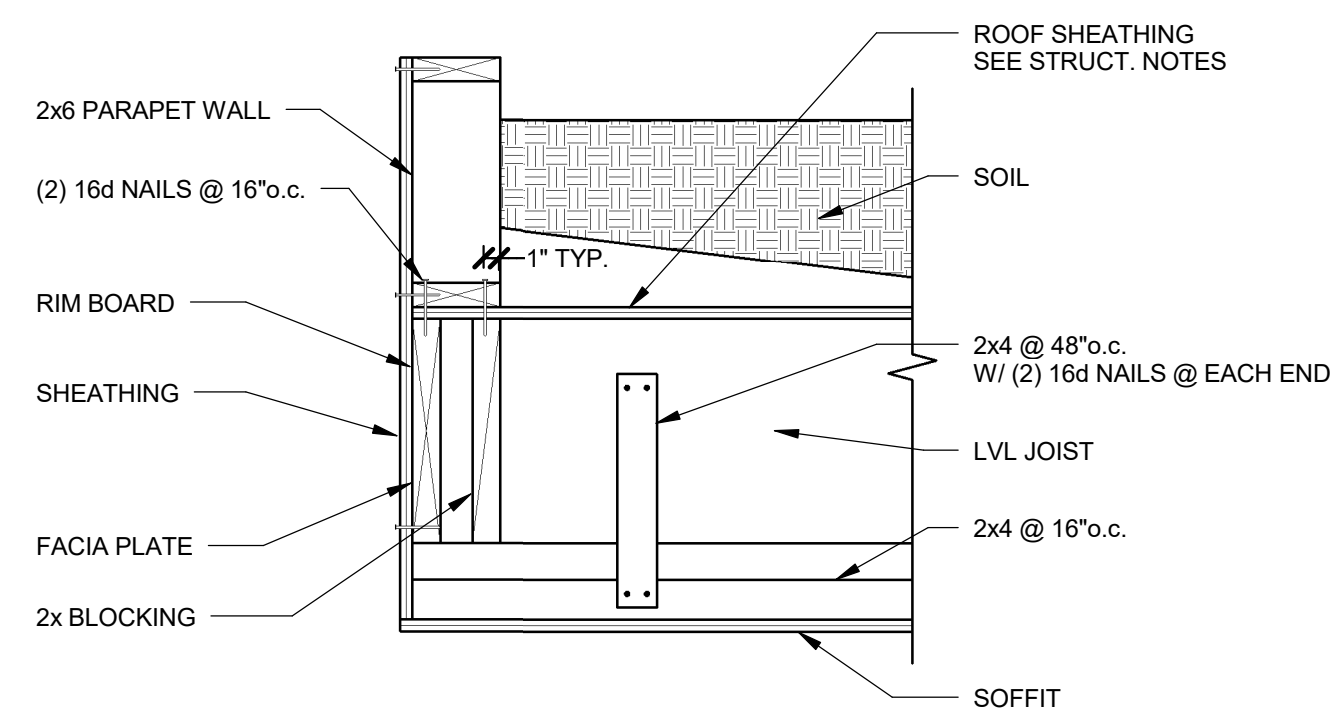




1 HIGH ROOF PARALLEL TO JOIST

SCALE: NONE

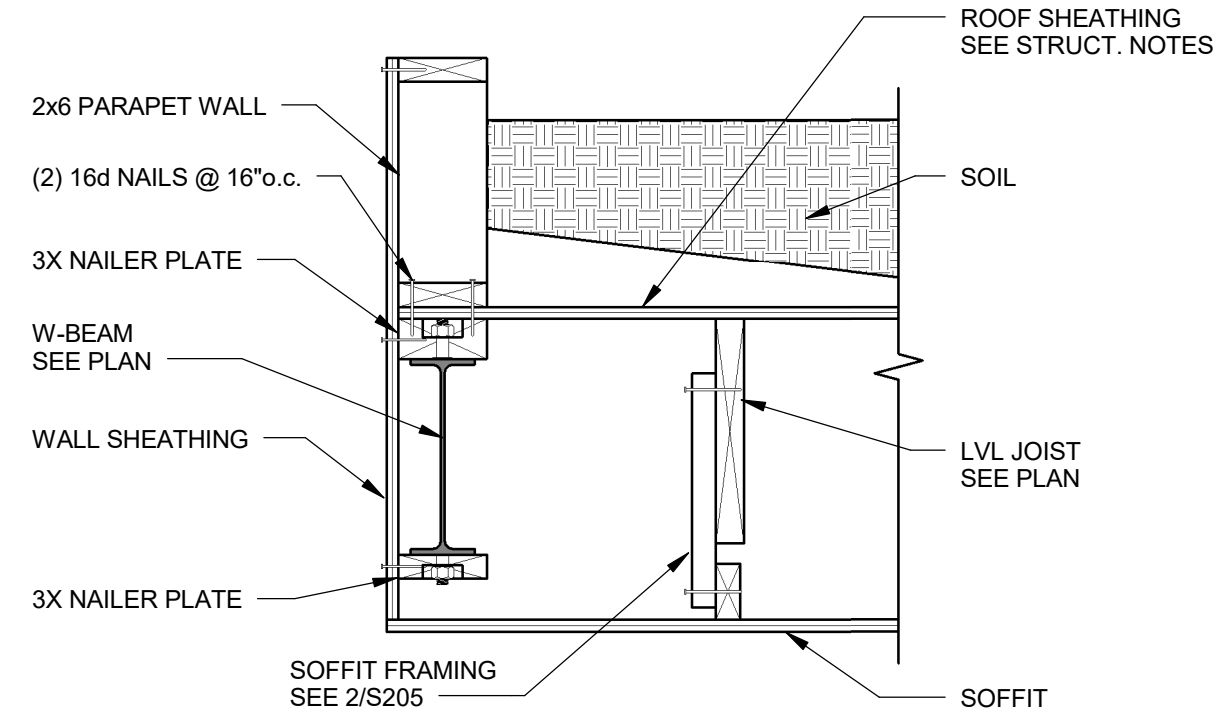
1 S205



2 HIGH ROOF PERPENDICULAR TO JOIST

SCALE: NONE

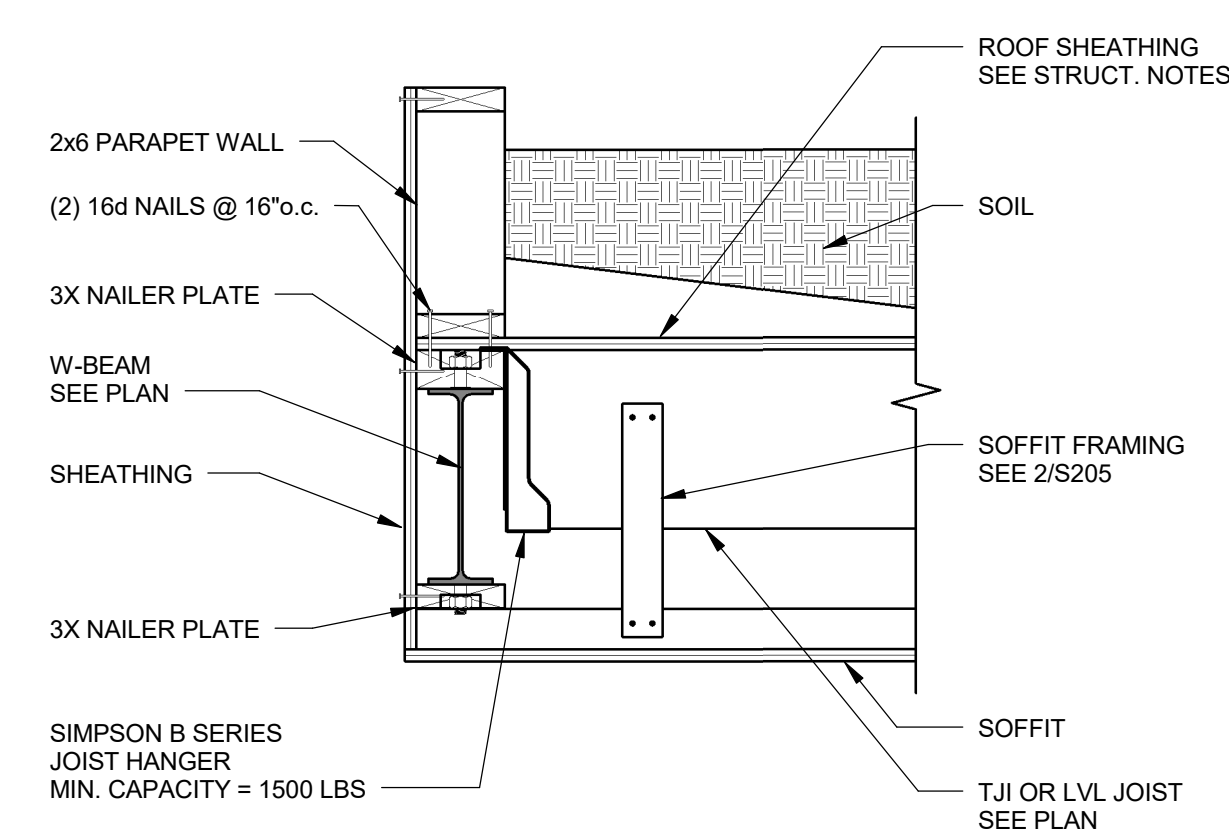
2 S205



3 HIGH ROOF PERPENDICULAR TO JOIST

SCALE: NONE

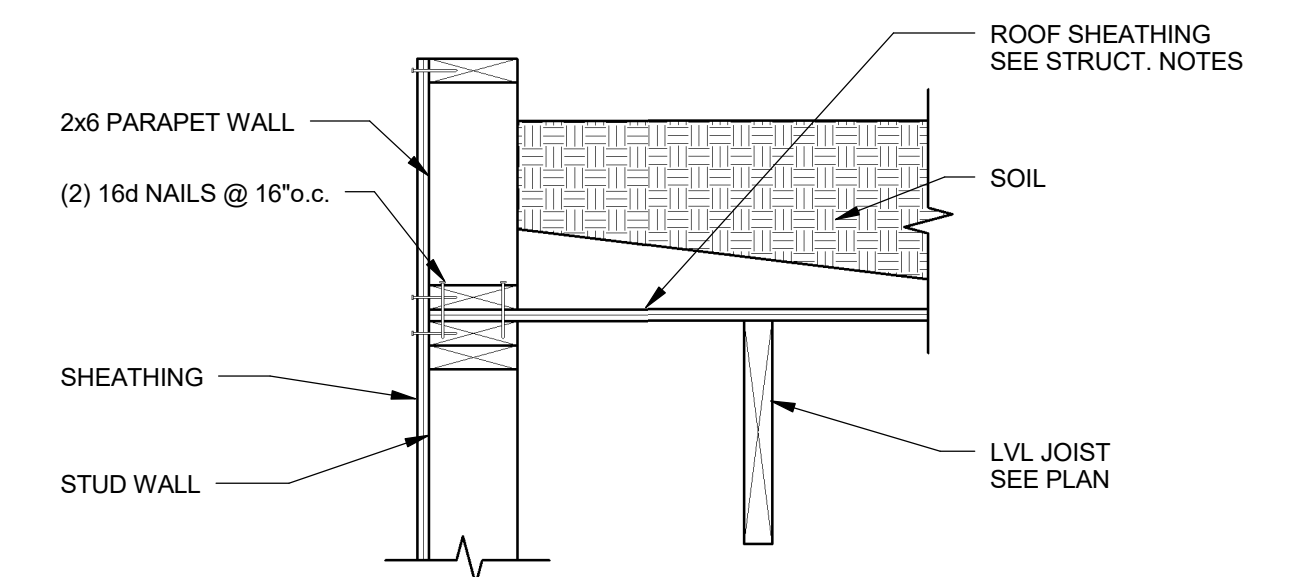
3 S205



4 DETAIL

SCALE: NONE

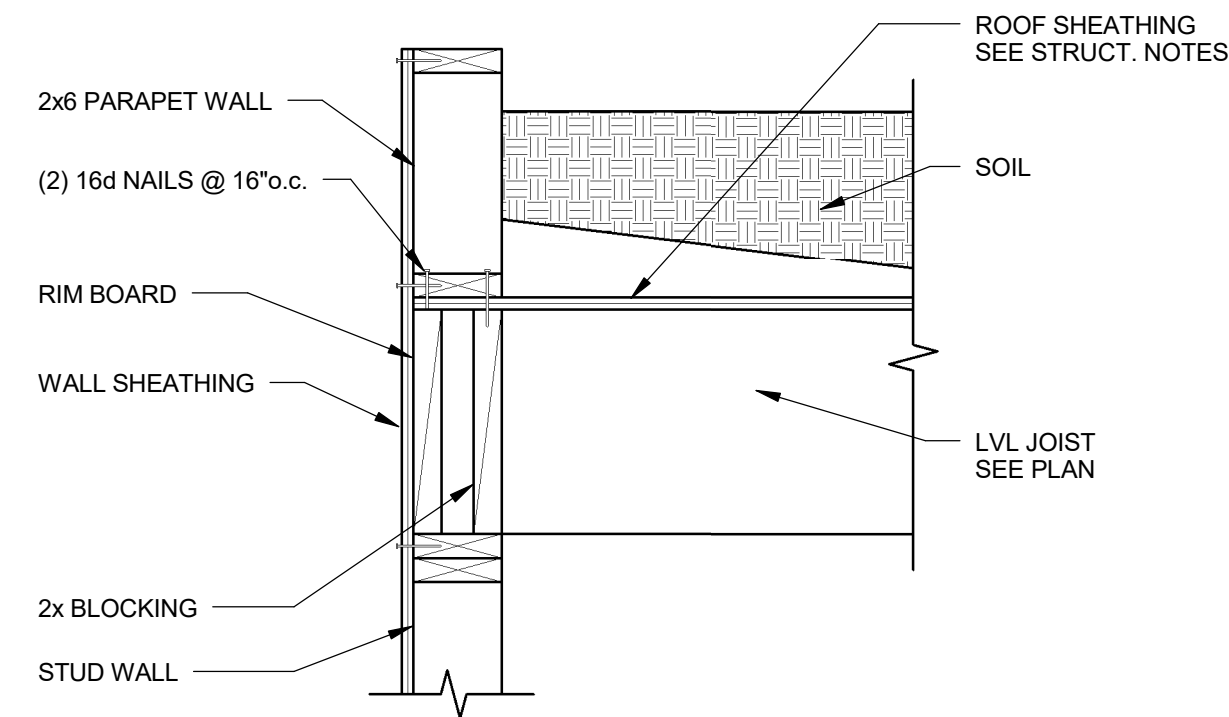
4 S205



5 DETAIL

SCALE: NONE

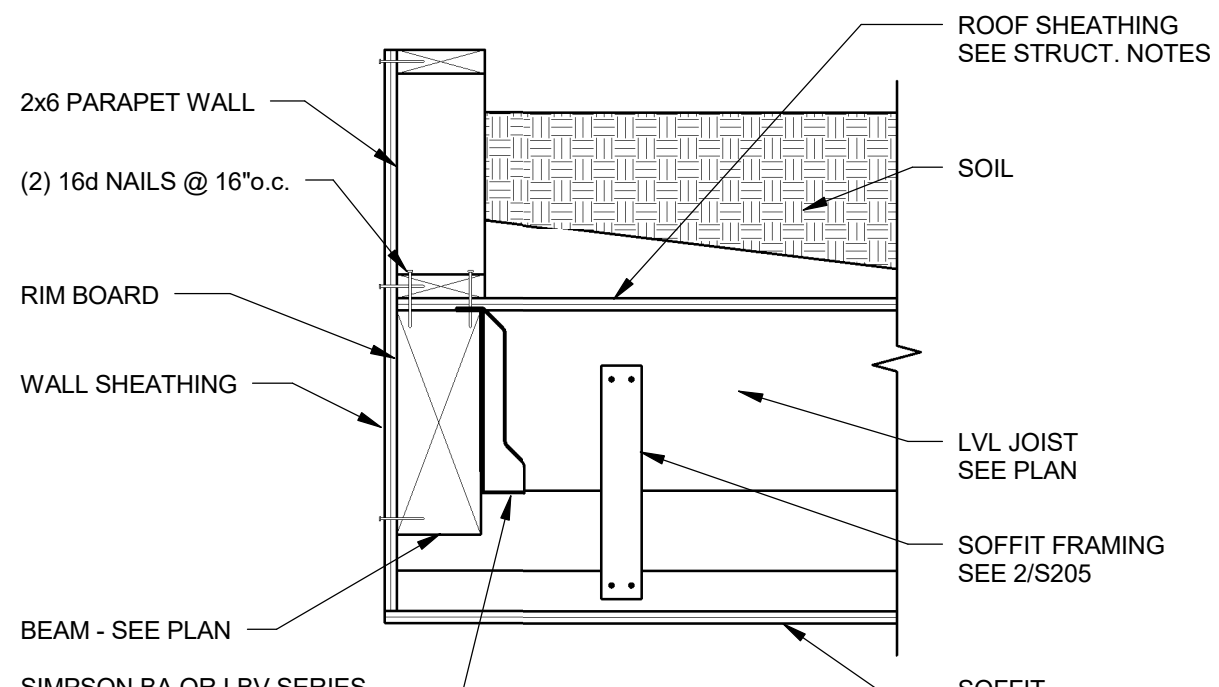
5 S205



6 DETAIL

SCALE: NONE

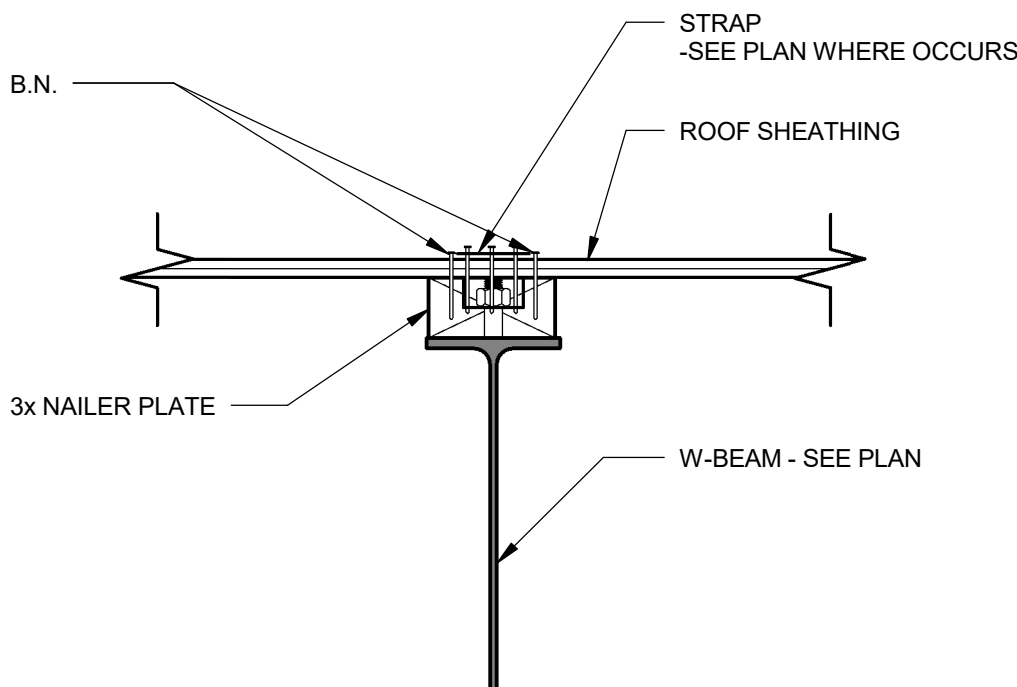
6 S205



7 DETAIL

SCALE: NONE

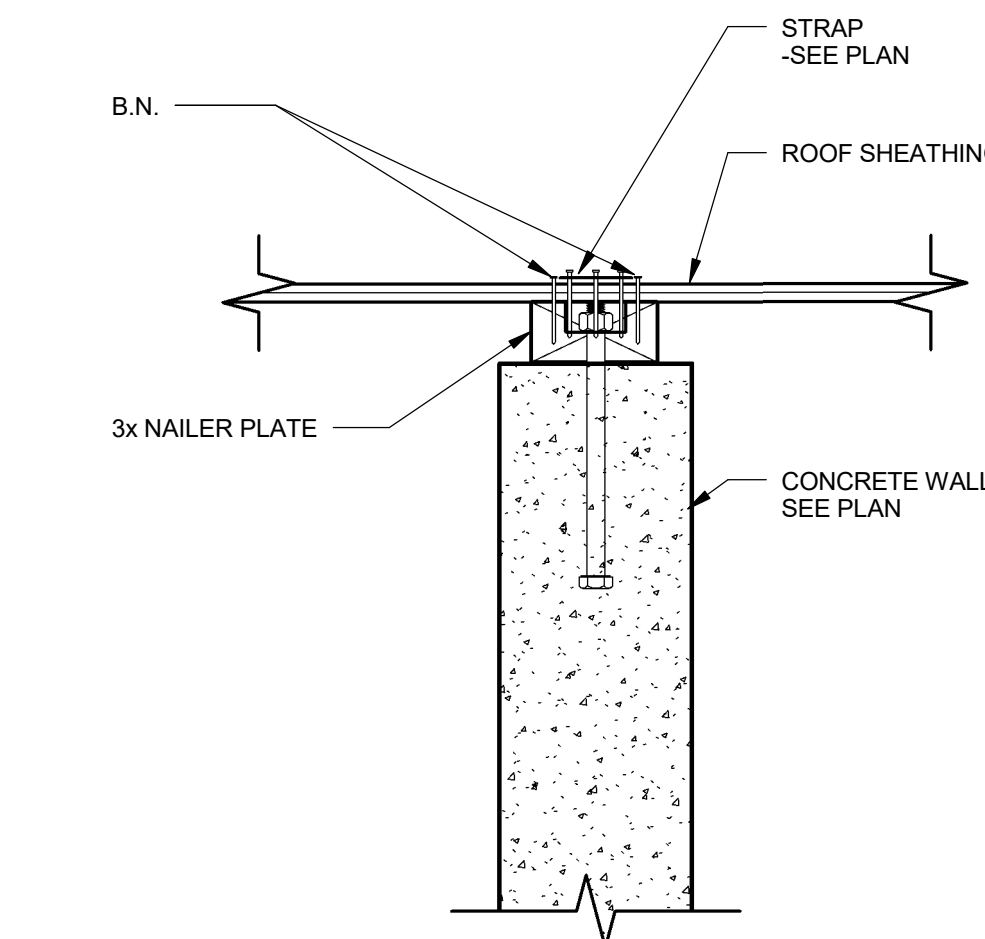
7 S205



8 DETAIL

SCALE: NONE

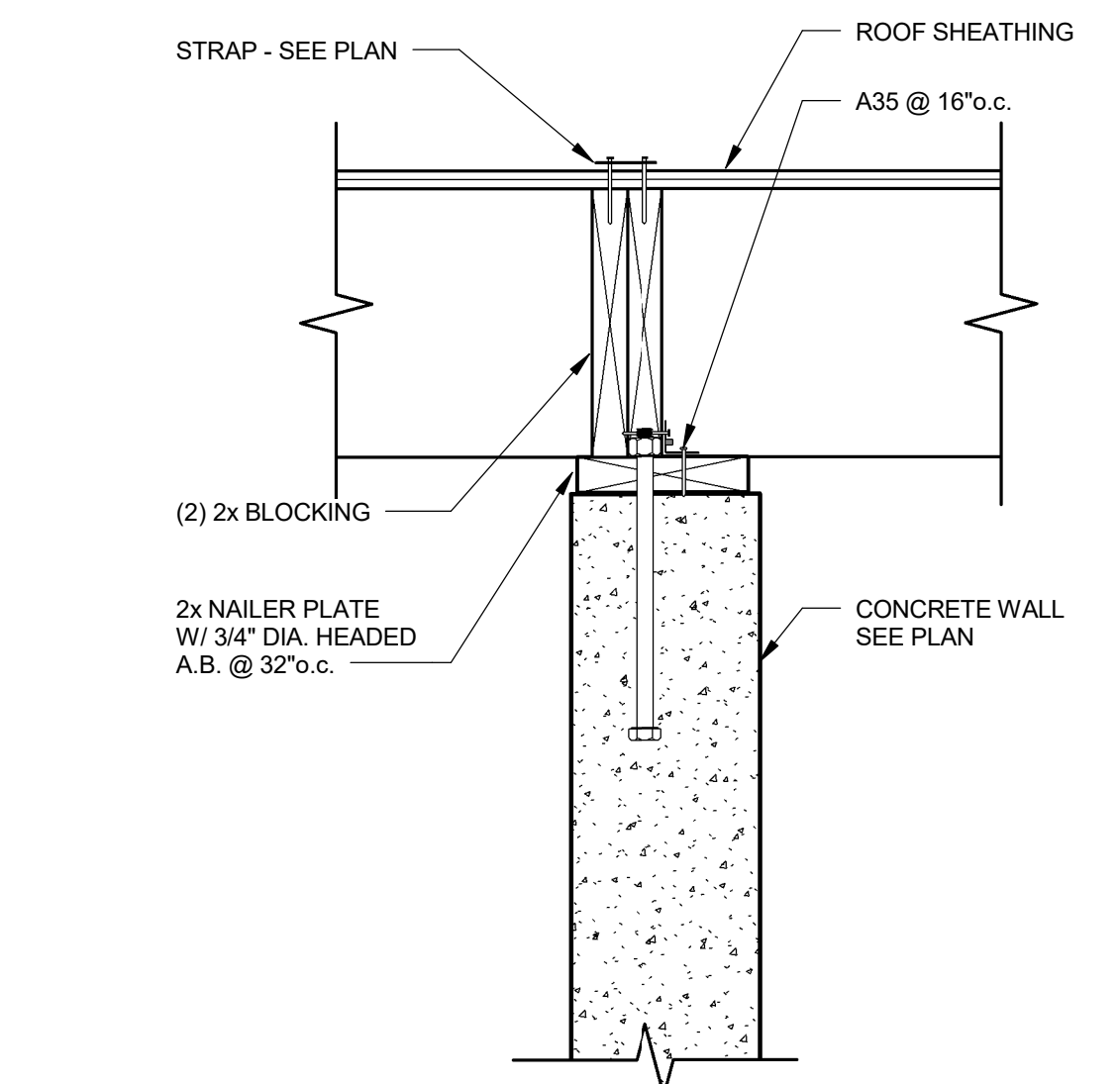
8 S205



9 DETAIL

SCALE: NONE

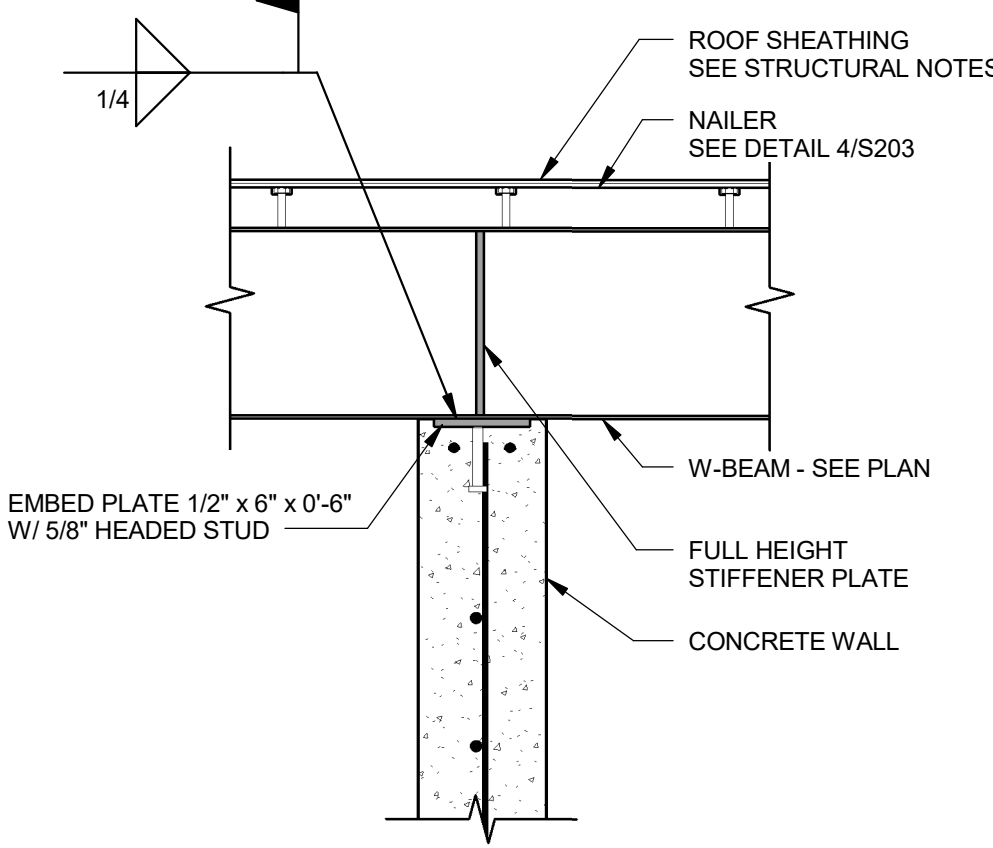
9 S205



10 DETAIL

SCALE: NONE

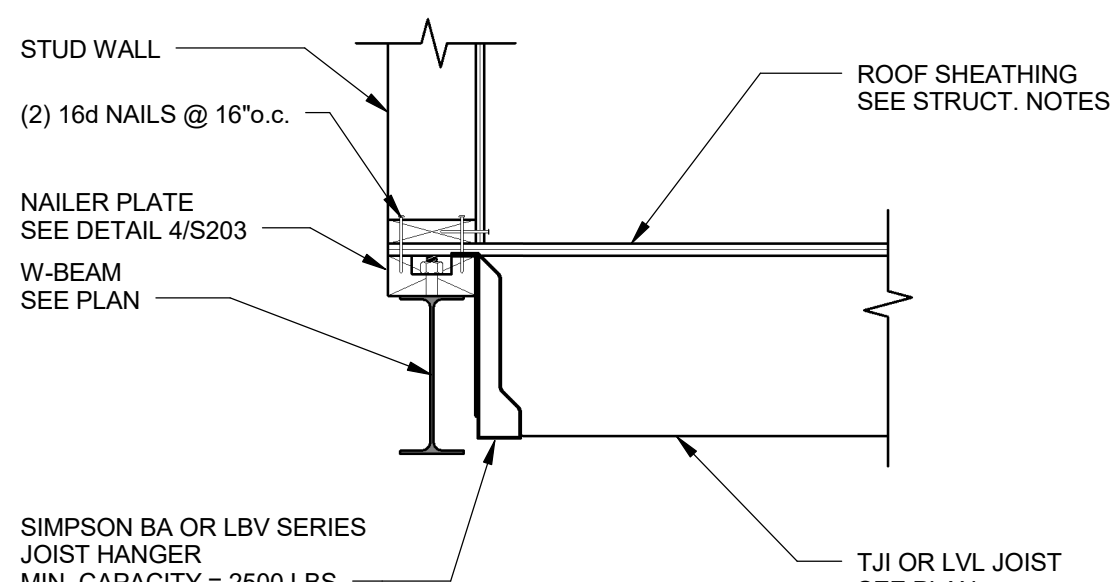
10 S205



11 DETAIL

SCALE: NONE

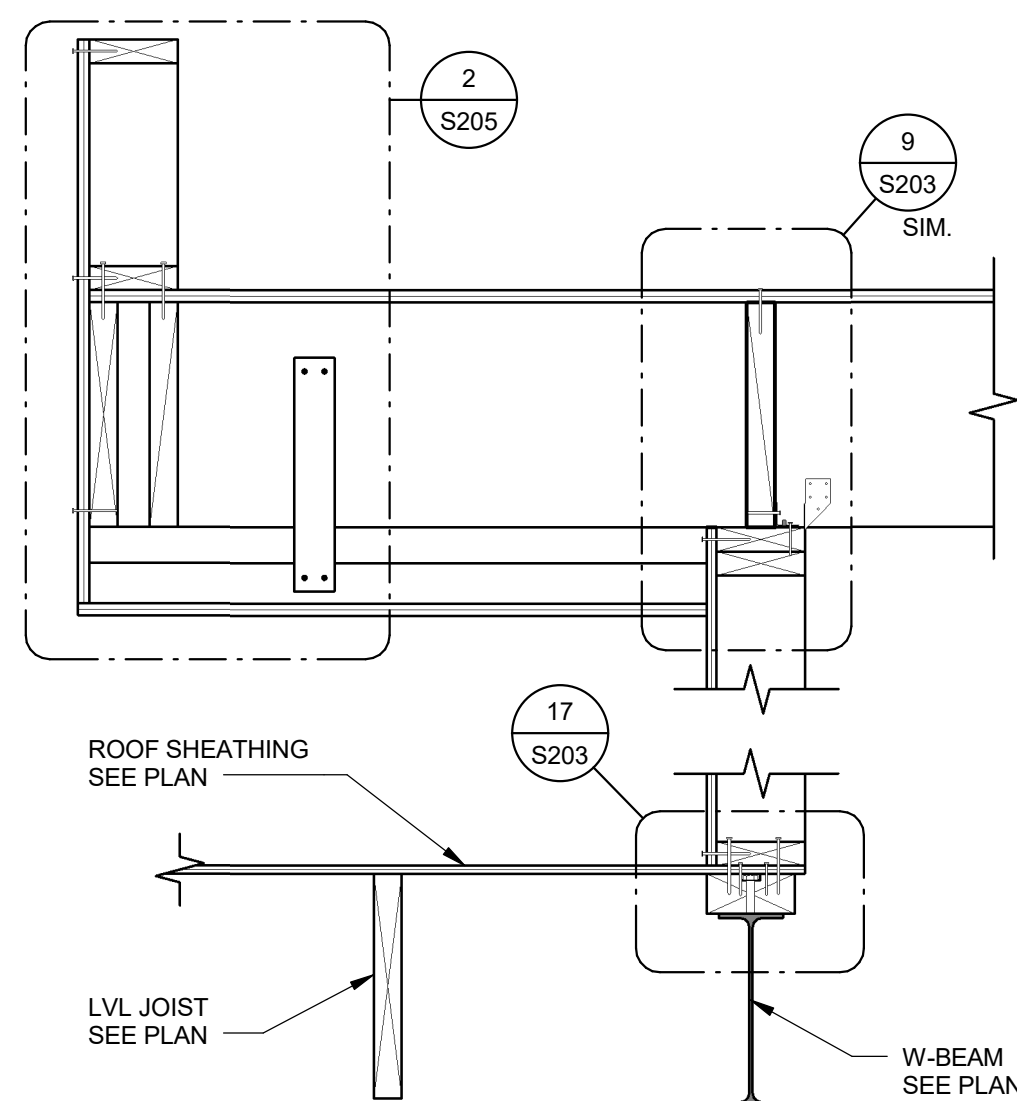
11 S205



12 DETAIL

SCALE: NONE

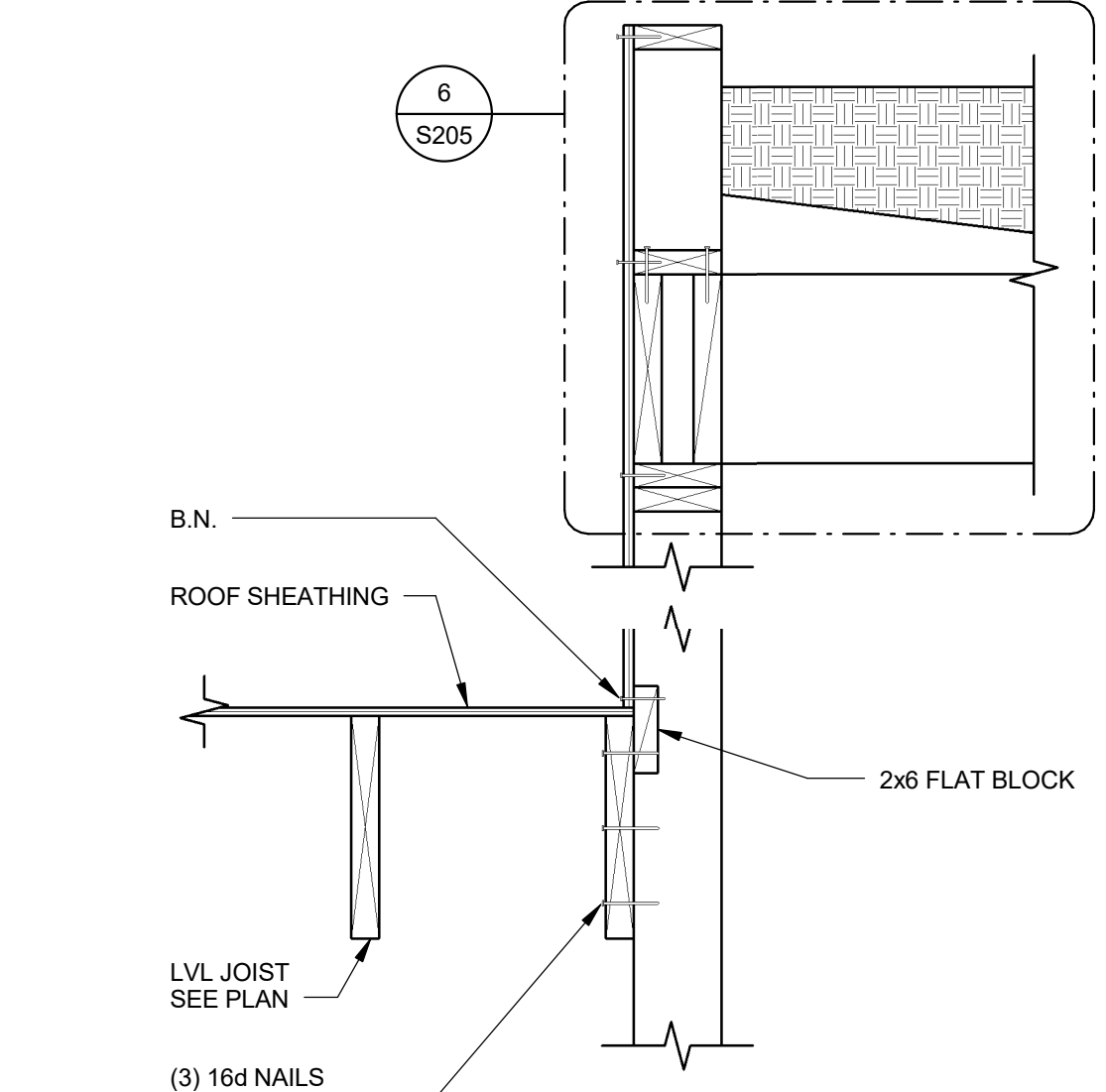
12 S205



13 DETAIL

SCALE: NONE

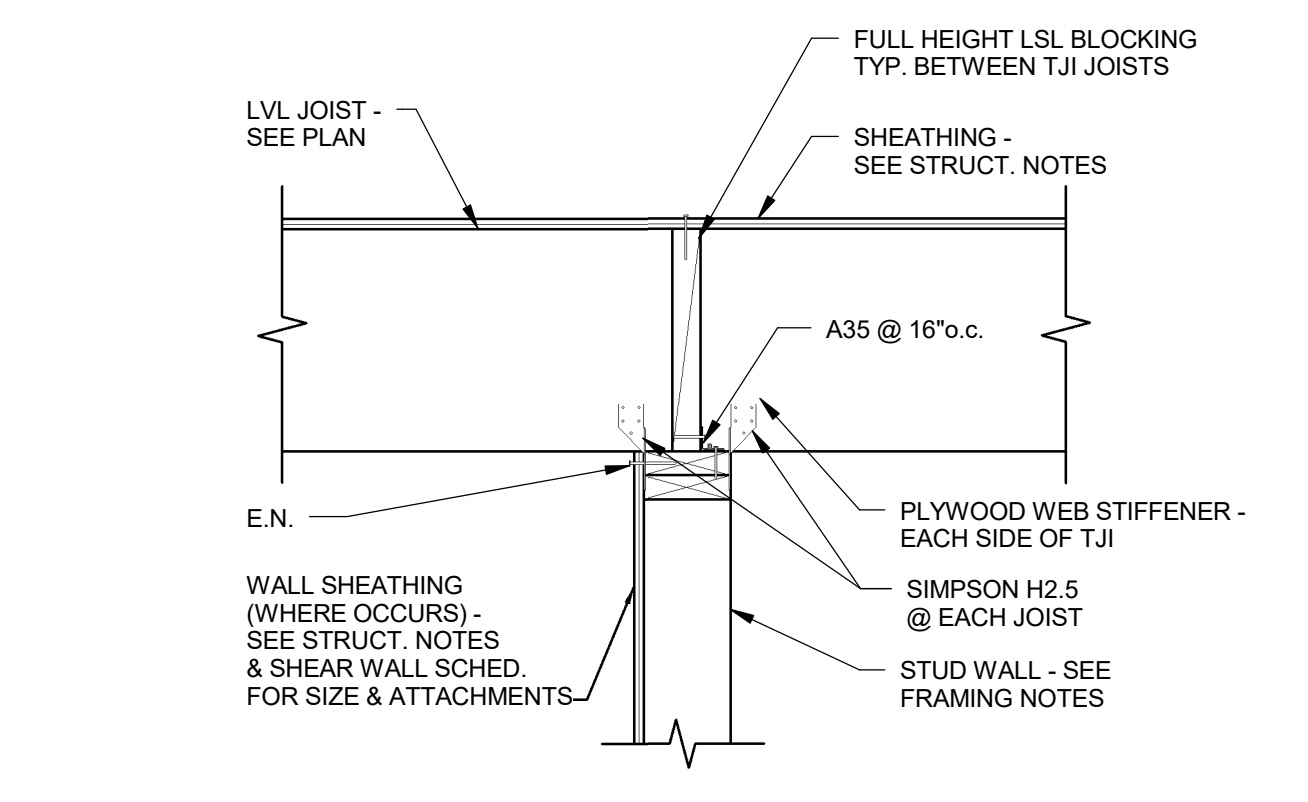
13 S205



14 DETAIL

SCALE: NONE

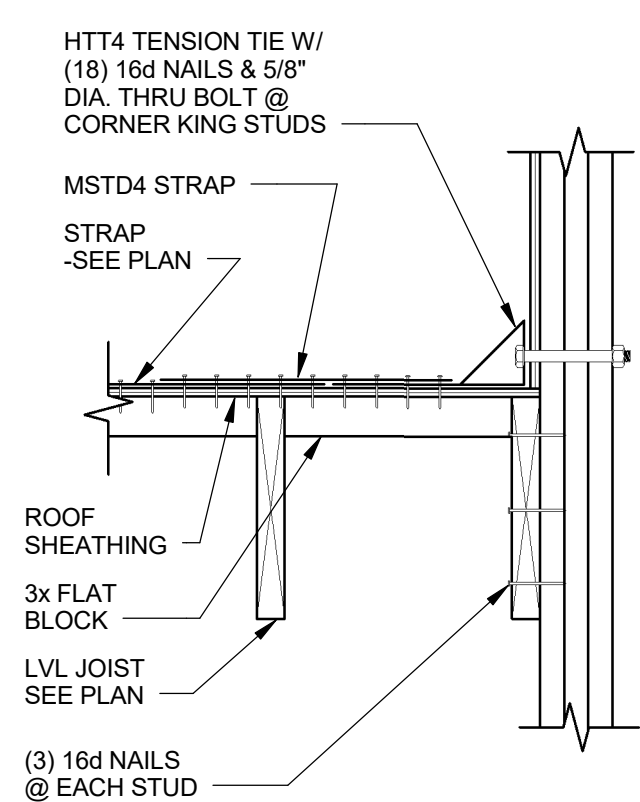
14 S205



15 DETAIL

SCALE: NONE

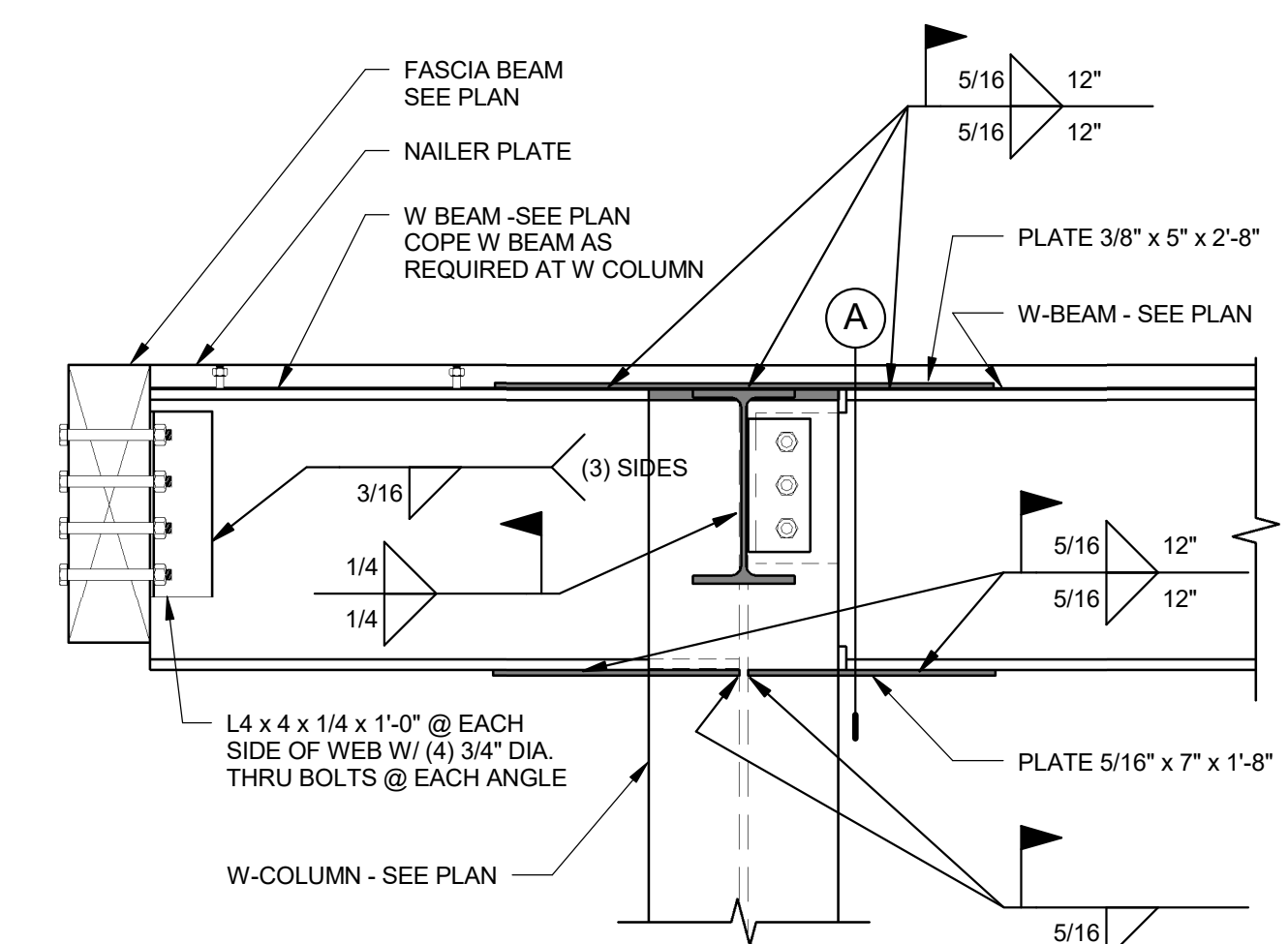
15 S205



16 DETAIL

SCALE: NONE

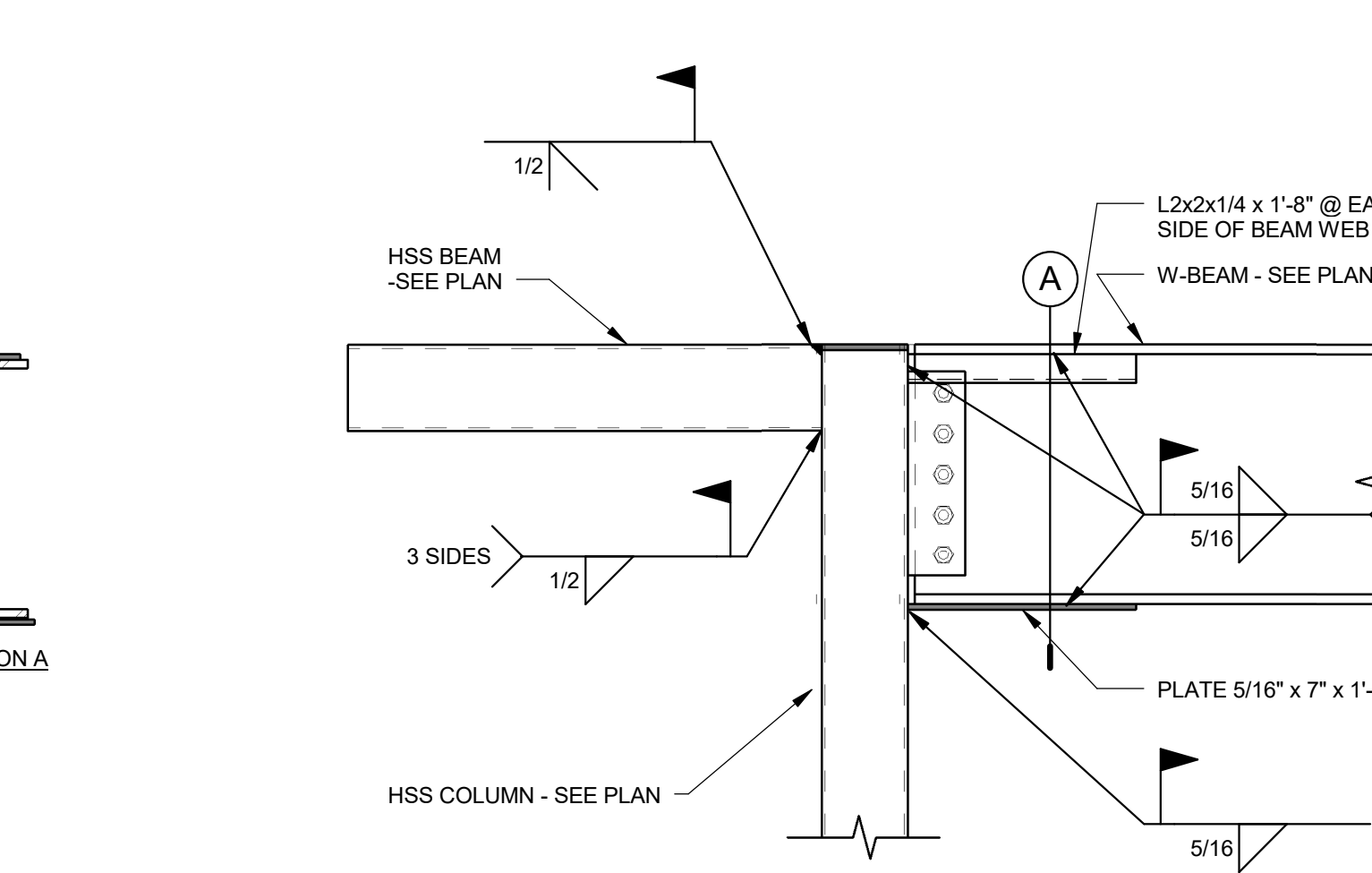
16 S205



17 DETAIL

SCALE: NONE

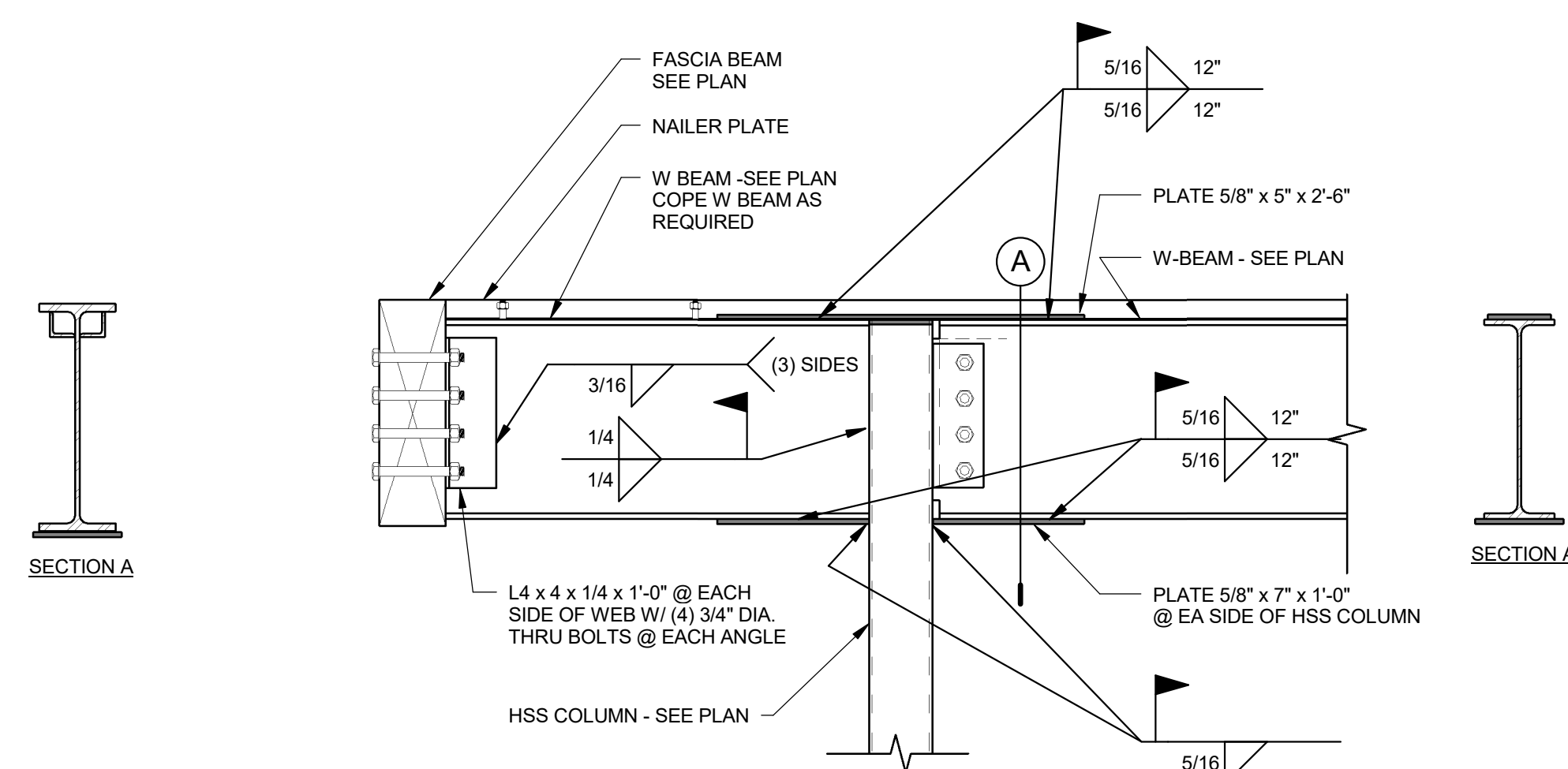
17 S205



18 DETAIL

SCALE: NONE

18 S205

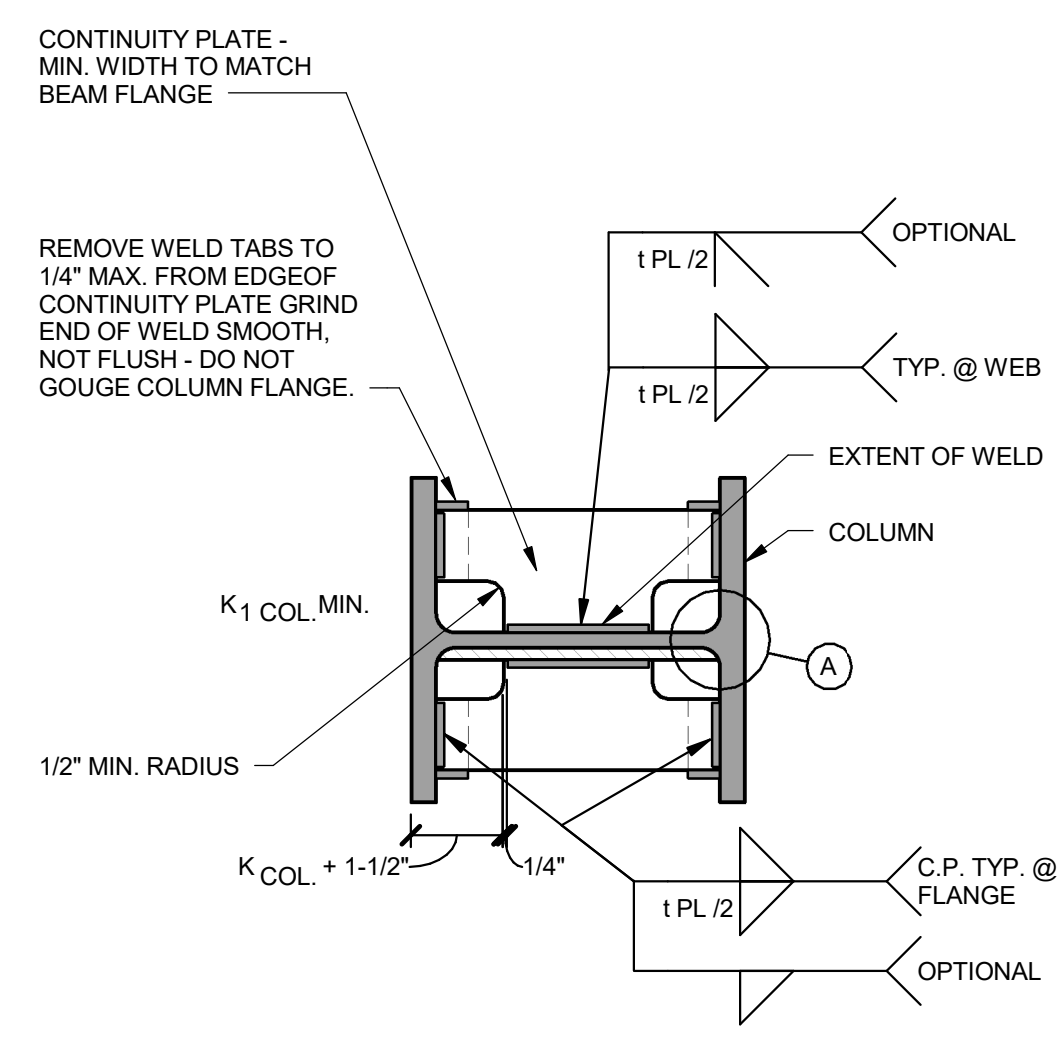


19 DETAIL

SCALE: NONE

19 S205

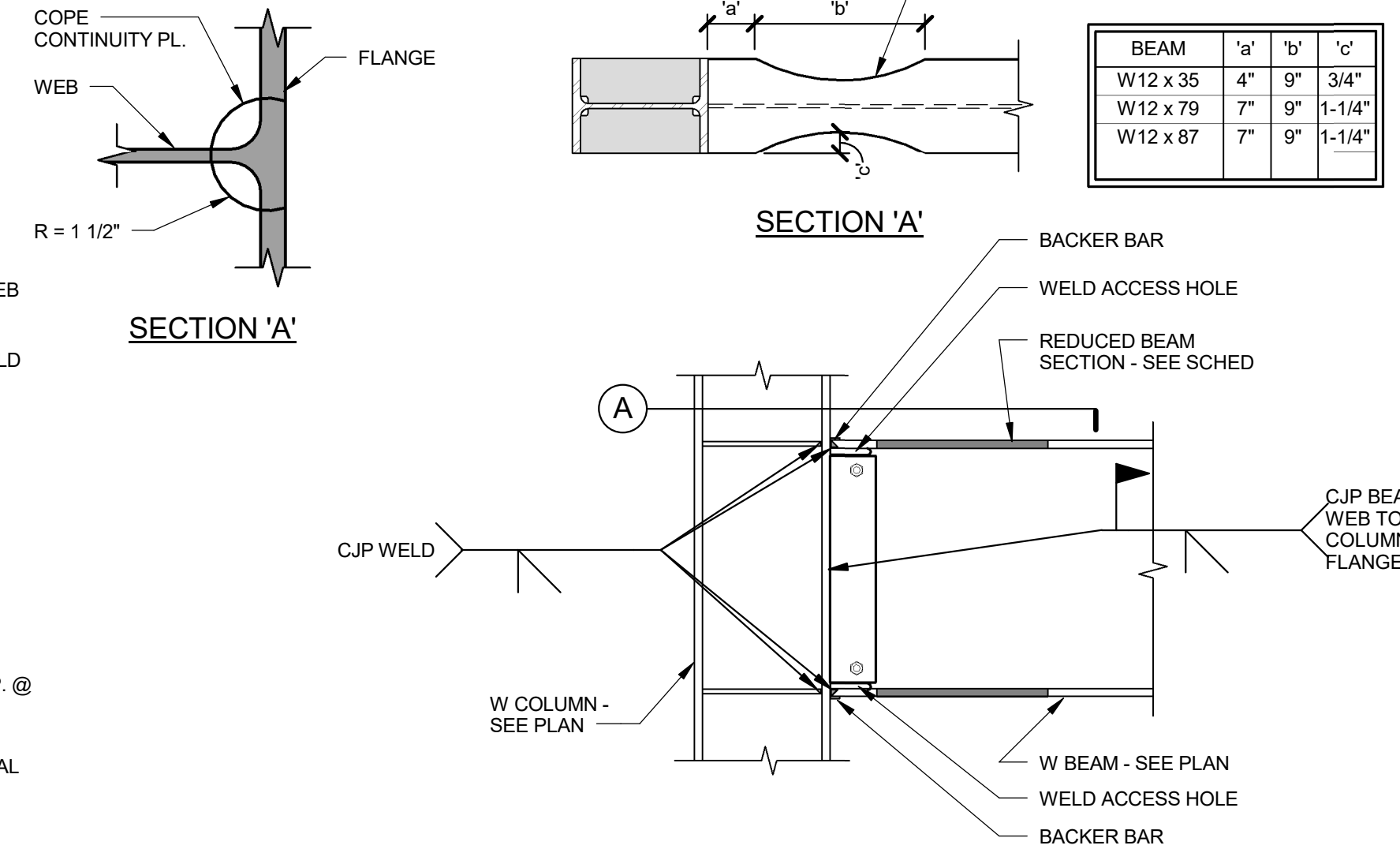




**TYPICAL MOMENT FRAME CONNECTION DETAIL**

SCALE: NONE

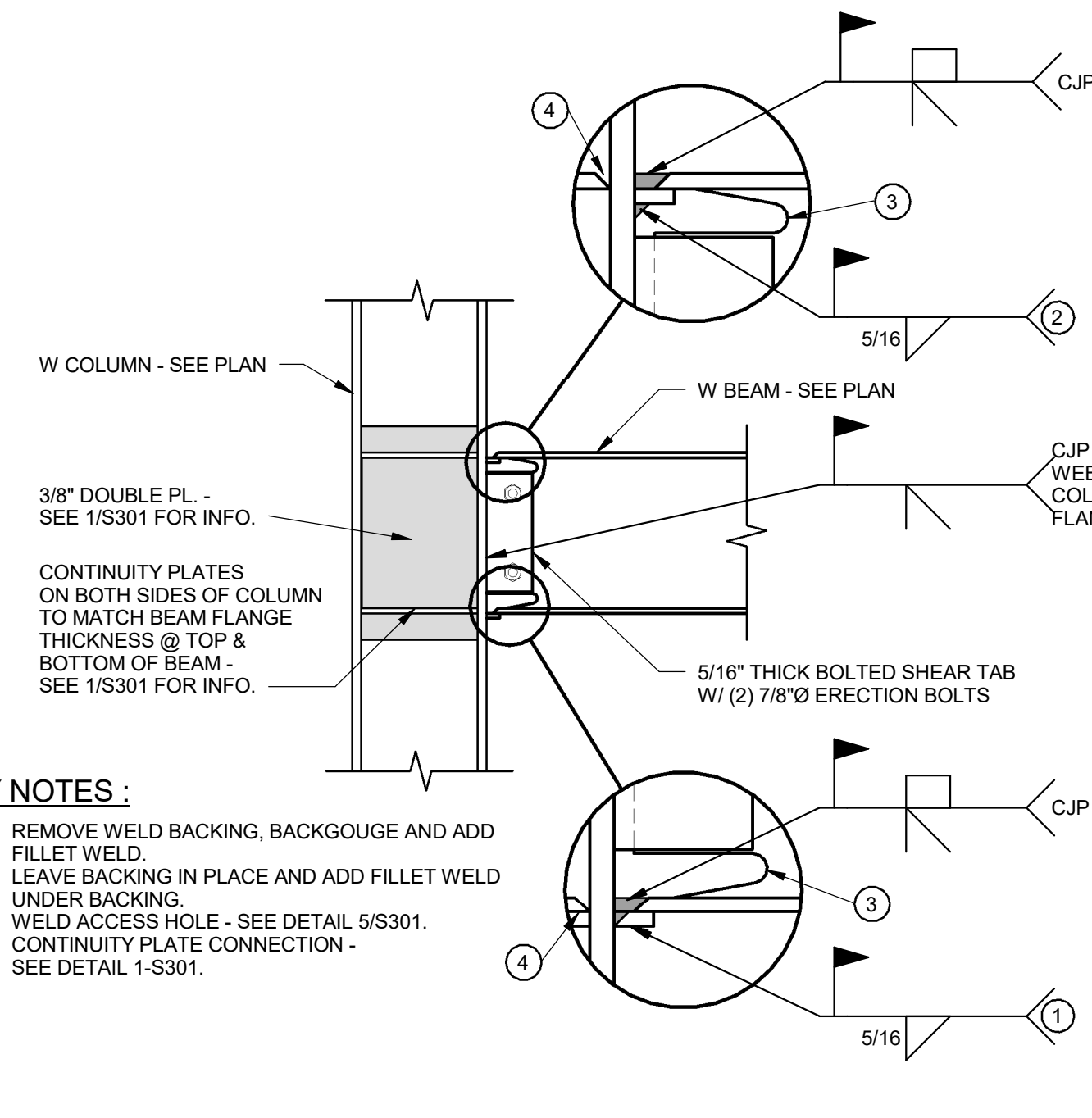
1  
S301



**TYPICAL MOMENT FRAME CONNECTION DETAIL**

SCALE: NONE

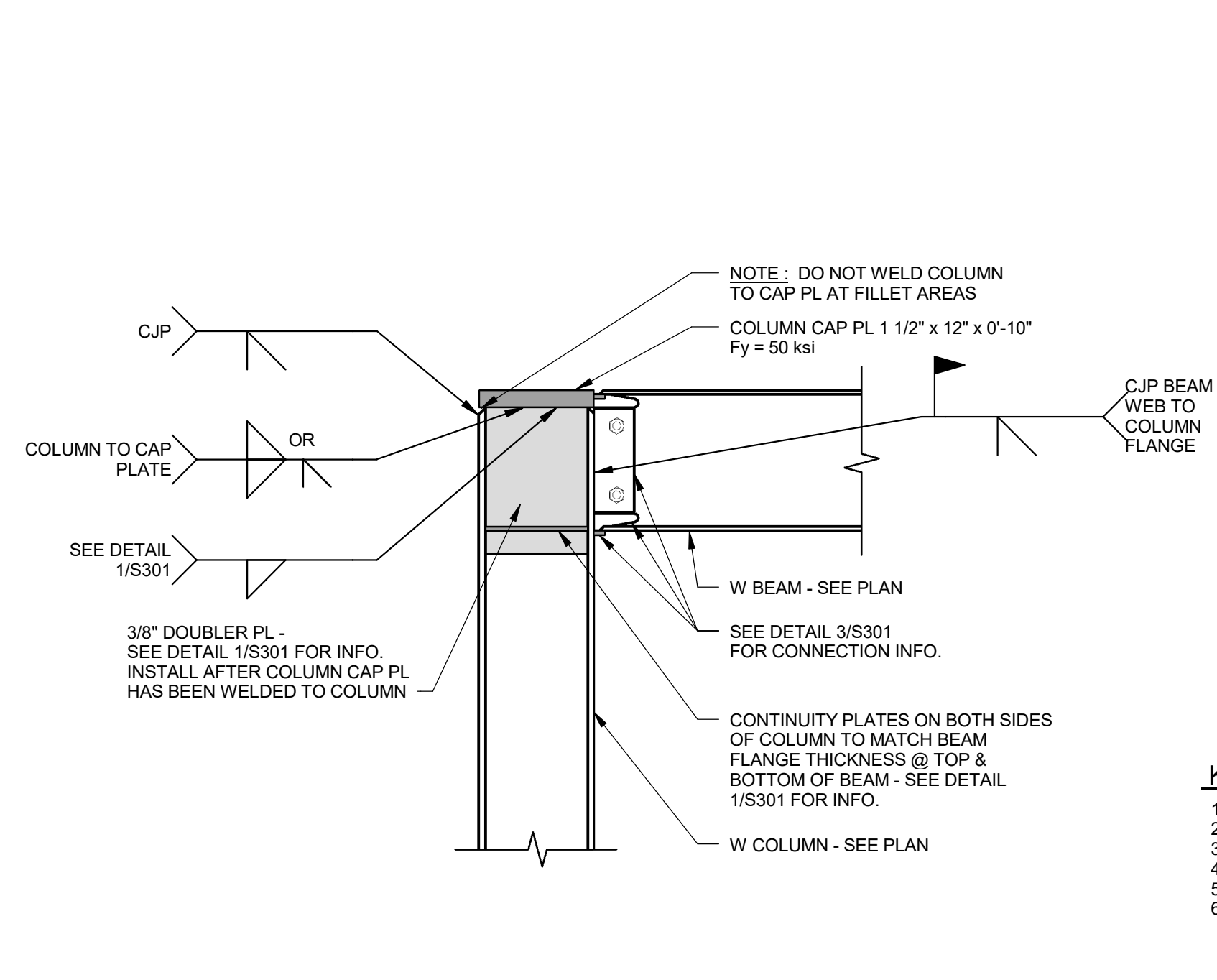
2  
S301



**DETAIL**

SCALE: NONE

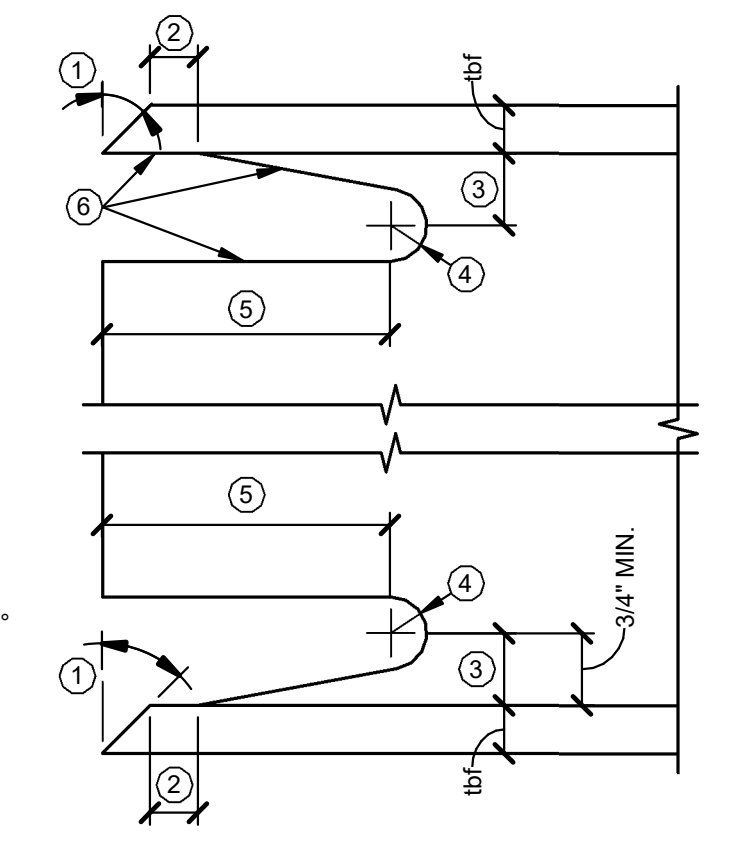
3  
S301



**MOMENT FRAME CONNECTION DETAIL**

SCALE: NONE

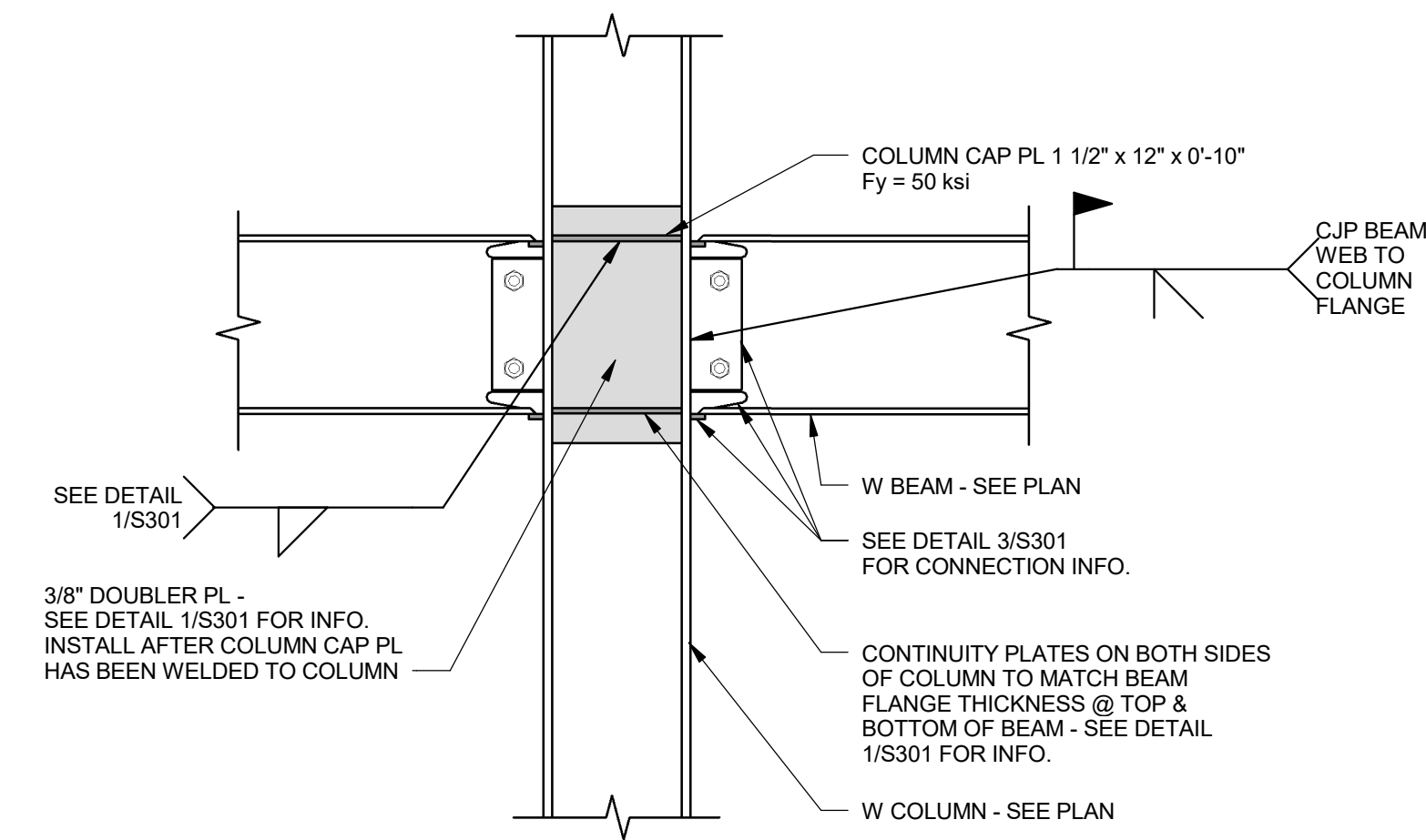
4  
S301



**DETAIL**

SCALE: NONE

5  
S301



**MOMENT FRAME CONNECTION DETAIL**

SCALE: NONE

6  
S301

BEAM	'd'	'b'	'c'
W12 x 35	4"	9"	3/4"
W12 x 79	7"	9"	1-1/4"
W12 x 87	7"	9"	1-1/4"

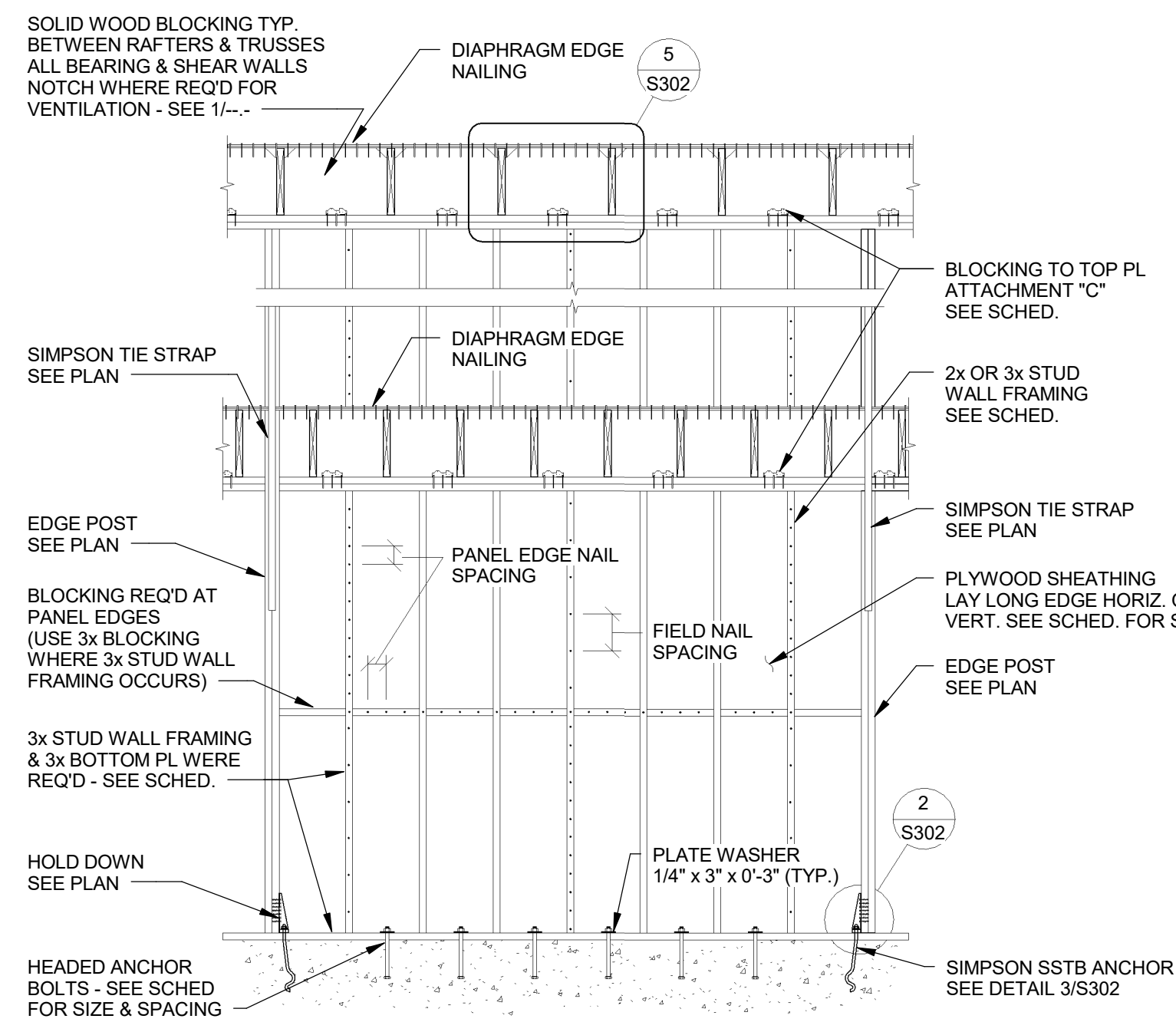
**KEY NOTES:**

1. REMOVE WELD BACKING, BACKGOUGE AND ADD FILLET WELD.
2. LEAVE BACKING IN PLACE AND ADD FILLET WELD UNDER BACKING.
3. WELD ACCESS HOLE - SEE DETAIL 5/S301.
4. CONTINUITY PLATE CONNECTION - SEE DETAIL 1-S301.

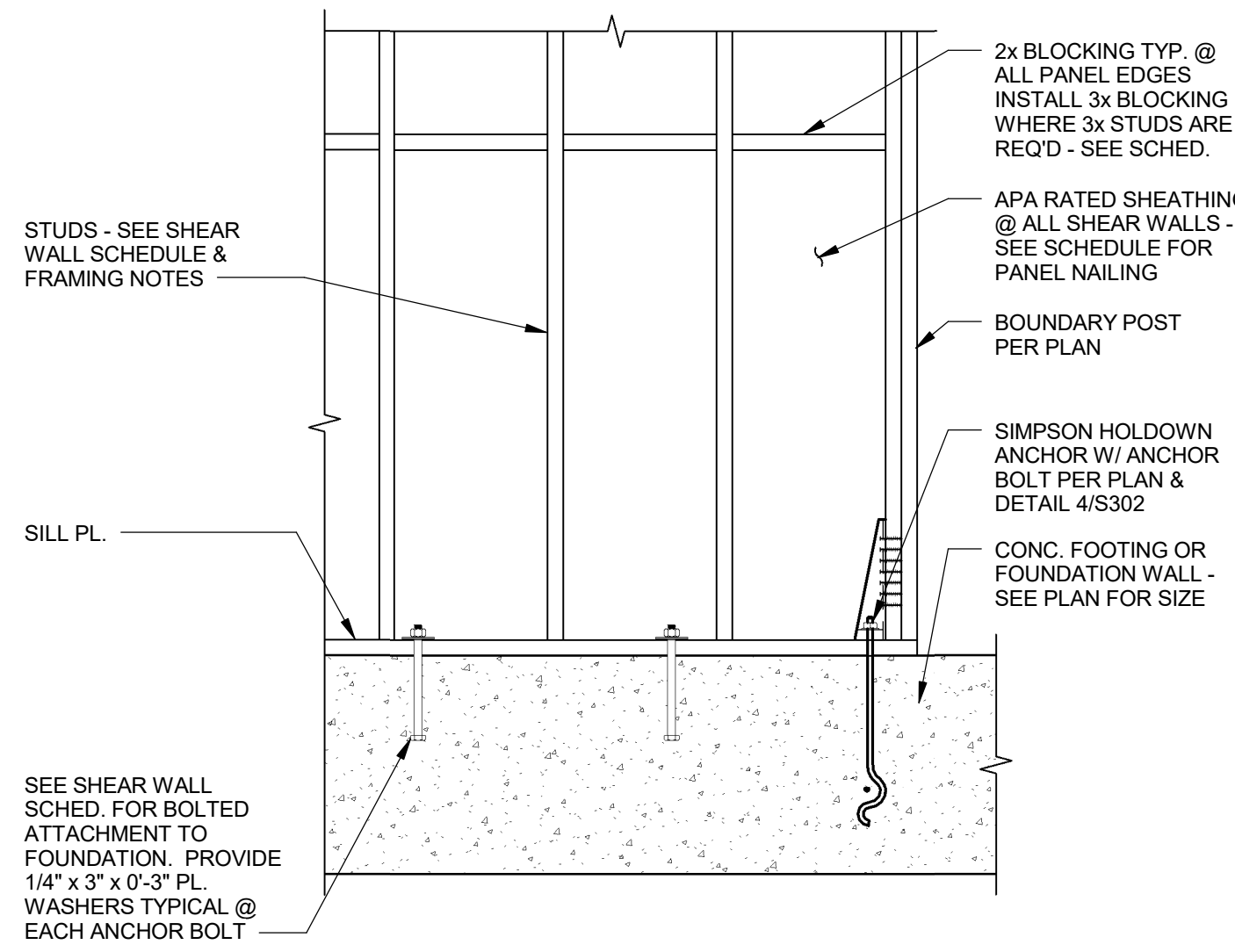
**KEY NOTES:**

1. BEVEL AS REQUIRED BY AWS D1.1 FOR SELECTED GROOVE WELD PROCEDURE.
2. LARGER OF  $1/4t_f$  OR 1/2 INCH (PLUS  $1/2t_f$ , OR MINUS  $1/4t_f$ ).
3.  $3/4t_f$  TO  $1t_f$ , .34" MINIMUM (+.14 INCH).
4. 3/8" MINIMUM RADIUS (PLUS NOT LIMITED, OR MINUS 0).
5.  $3t_f$  (+.12 INCH).
6. SEE FEMA-353, RECOMMENDED SPECIFICATIONS AND QUALITY ASSURANCE GUIDELINES FOR STEEL MOMENT-FRAME CONSTRUCTION FOR SEISMIC APPLICATIONS, FOR FABRICATION DETAILS INCLUDING CUTTING METHODS AND SMOOTHNESS REQUIREMENTS.





**DETAIL 1**  
SCALE: NONE  
S302



**TYP. HOLD DOWN DETAIL 2**  
SCALE: NONE  
S302

**HOLD DOWN EMBED SCHEDULE**

HOLD DOWN SIZE	ANCHOR SIZE	MIN. EMBEDMENT DEPTH "L <sub>e</sub> "
HDU2	SSTB16	12-5/8"
HDU4	SSTB20	16-5/8"
HDU5	SSTB24	24-7/8"
HDU8	SSTB28	24-7/8"

**SECTION A**

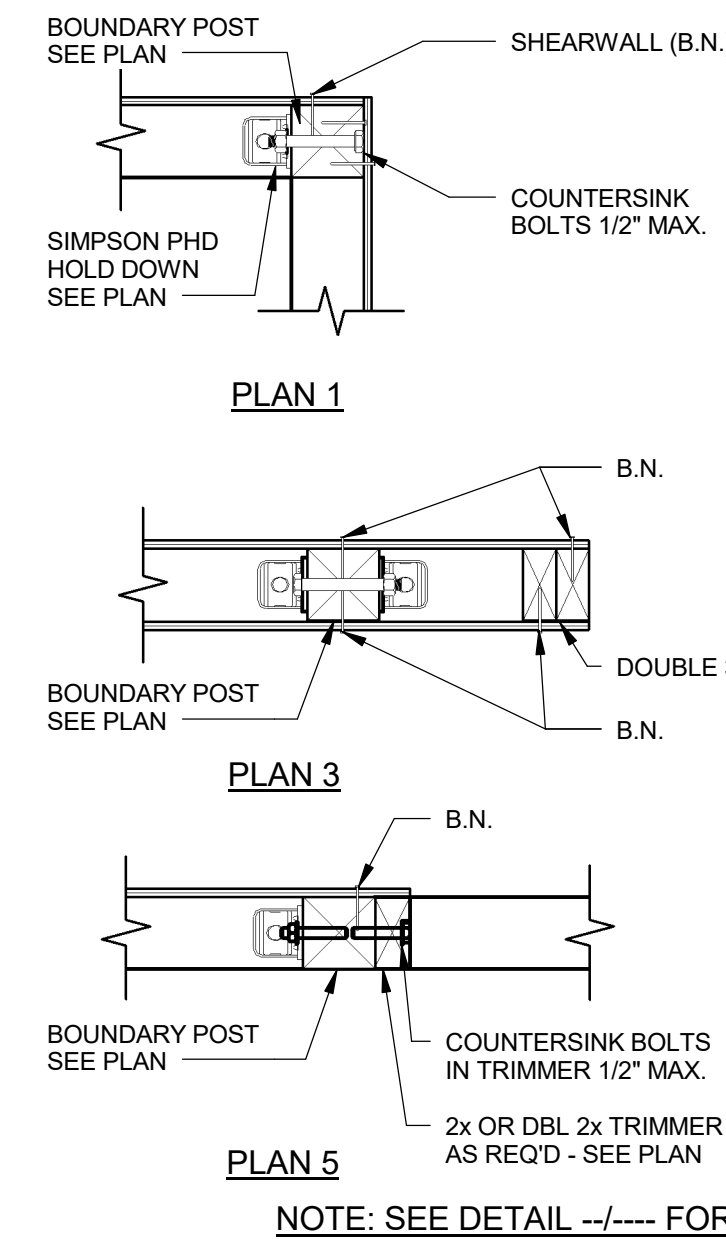
LOCATE APPROX. 45 DEG.

PLACE SSTB ANCHOR DIAGONAL IN CORNER @ END WALL APPLICATION

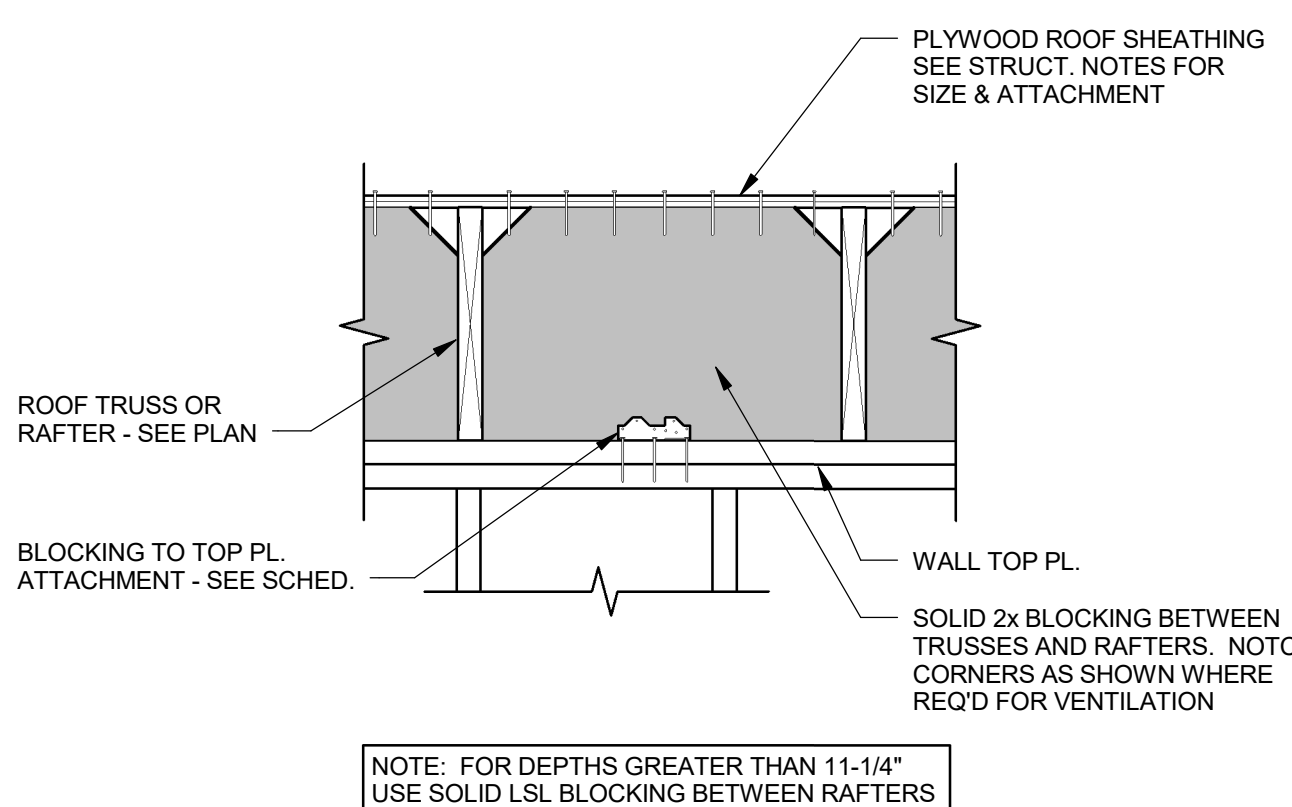
**CONTINUOUS WALL INSTALLATION**

NOTE: AT INTERIOR HOLD DOWNS ON CONC. SLAB ON METAL DECK, WELD THREADED ROD EQUIVALENT SIZE AND STRENGTH TO TOP OF W-BEAM AT INTERIOR HOLD DOWNS ON THICKENED SLAB FOOTINGS. PROVIDE HEADED ANCHOR BOLT OF EQUIVALENT SIZE W/ 8" MIN. EMBEDMENT.

**HOLD DOWN EMBED SCHEDULE 3**  
SCALE: NONE  
S302



**DETAIL 4**  
SCALE: NONE  
S302

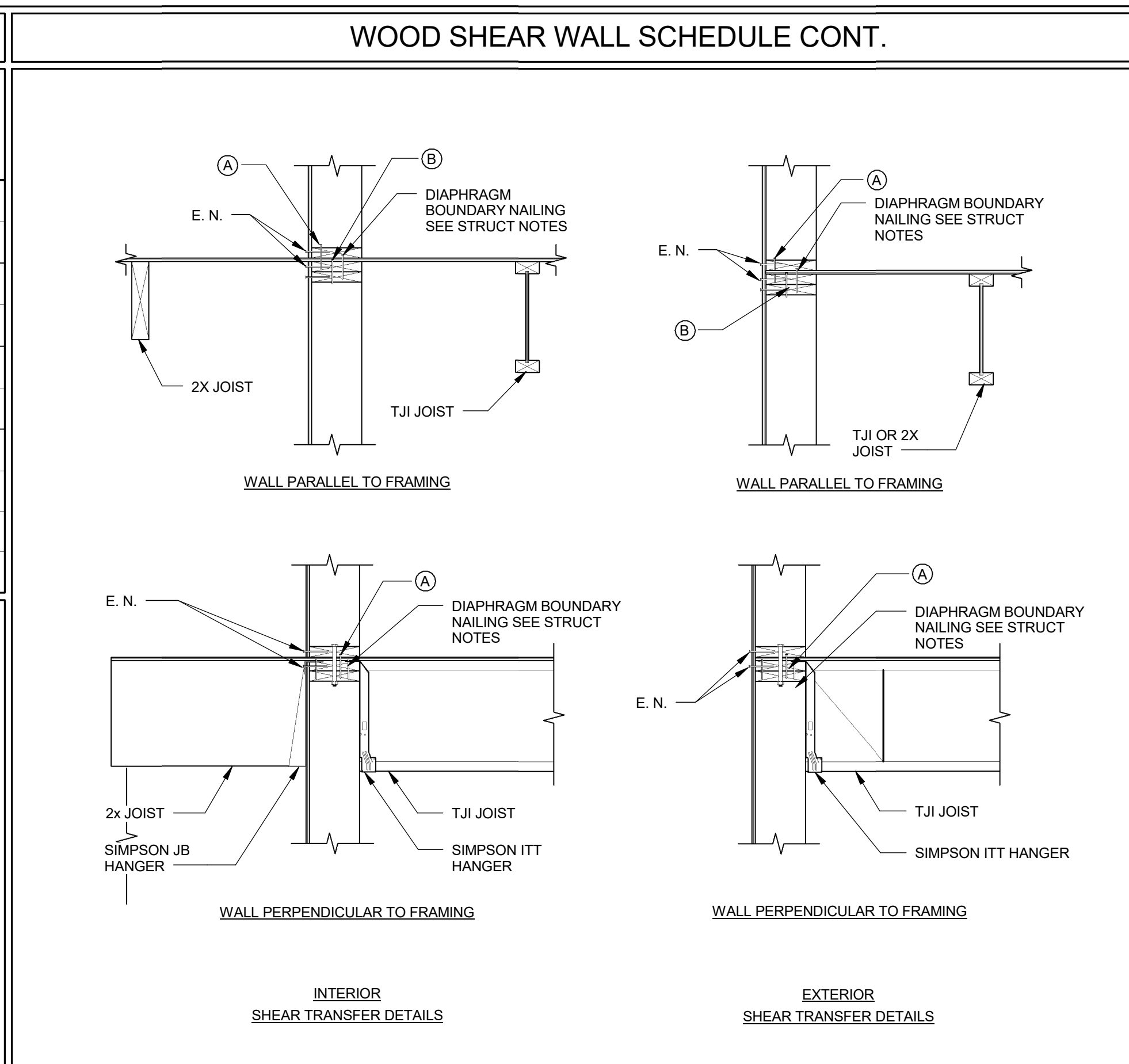


**DETAIL 5**  
SCALE: NONE  
S302

WOOD SHEAR WALL SCHEDULE											
WALL MARK	LEVEL	(NOTE 6) PLYWOOD SHEATHING (CDX U.N.O.)	EDGE NAILING (E.N.) (SEE NOTES 2 & 3)	NOMINAL BOTTOM PLATE SIZE	(NOTE 7) NOM. STUD SIZE (MIN.)	CONNECTION NAILING			A.B. @ FOUND.		COMMENTS
						BOTTOM PL. (A) (SEE NOTE 4) (L-LAG (ST)-STAGGER)	NAILING TOP PL. TOGETHER (B)	BLKG. TO TOP PL. (C)	DIA.	SPA.	
SW-1	1ST TO 2ND	15/32"	6"	2x	2x	--	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
	2ND TO ROOF	15/32"	6"	2x	2x	(2) 16d @ 16" o.c.	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
SW-2	1ST TO 2ND	15/32"	4"	2x	2x	--	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
	2ND TO ROOF	15/32"	4"	2x	2x	(2) 16d @ 16" o.c.	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
SW-3	1ST TO 2ND	15/32"	3"	3x	3x	--	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
	2ND TO ROOF	15/32"	3"	3x	3x	(2) 16d @ 8" o.c.	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
SW-4	1ST TO 2ND	15/32"	2"	3x	3x	--	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	16" o.c.	
	2ND TO ROOF	15/32"	2"	3x	3x	(2) 16d @ 8" o.c.	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	16" o.c.	
SW-5	1ST TO 2ND	15/32" BOTH FACES	2"	3x	3x	--	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	8" o.c.	
	2ND TO ROOF	15/32" BOTH FACES	2"	3x	3x	(2) 16d @ 8" o.c.	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	8" o.c.	

**NOTES:**

- ALL SHEATHING PANEL EDGES TO BE BLOCKED. USE 3x BLOCKING WHERE 3x STUDS ARE REQ'D.
- ALL NAILS TO BE COMMON OR GALV. BOX.
- FIELD NAILING TO BE SAME NAILS @ 12" o.c.
- (A) CONNECTION IS FOR 2ND FLOOR AND ABOVE.
- AT SHEAR WALLS W/ SHEATHING ON BOTH SIDES, BOTH VERTICAL AND HORIZONTAL JOINTS ON OPPOSITE SIDES OF THE WALL SHALL BE STAGGERED.
- STAGGER E.N. AT DOUBLE TOP PLATES.
- 3x NOMINAL FRAMING MEMBERS TO OCCUR AT ABUTTING PANEL EDGES. 2x NOMINAL FRAMING MEMBERS MAY BE USED AT INTERIOR OF PANEL, UNLESS NOTED OTHERWISE IN FLOOR FRAMING NOTES. (2) 2x NAILED TOGETHER W/ (2) 16d NAILS @ 6" o.c. OR 4x NOMINAL FRAMING MEMBERS OF THE SAME DEPTH AND LUMBER GRADE MAY BE USED IN LIEU OF 3x MEMBERS AT CONTRACTOR OPTION.
- SHEATHING SHALL BE STAMPED W/ APA STAMP. O.S.B. OF EQUIVALENT THICKNESS, GRADE, AND RATING MAY BE USED IN LIEU OF PLYWOOD.
- SEE THIS SHEET FOR TYPICAL SHEAR TRANSFER DETAILS.



FOR CONSTRUCTION

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1584 W. Park, Ok. Ogden, Utah 84403  
PH: 801.752.6028 FAX: 801.752.4856

**39 SUMMIT, LLC**  
Summit at Powder Mountain Lot 39  
8365 E. SUMMIT PASS

DETAILS

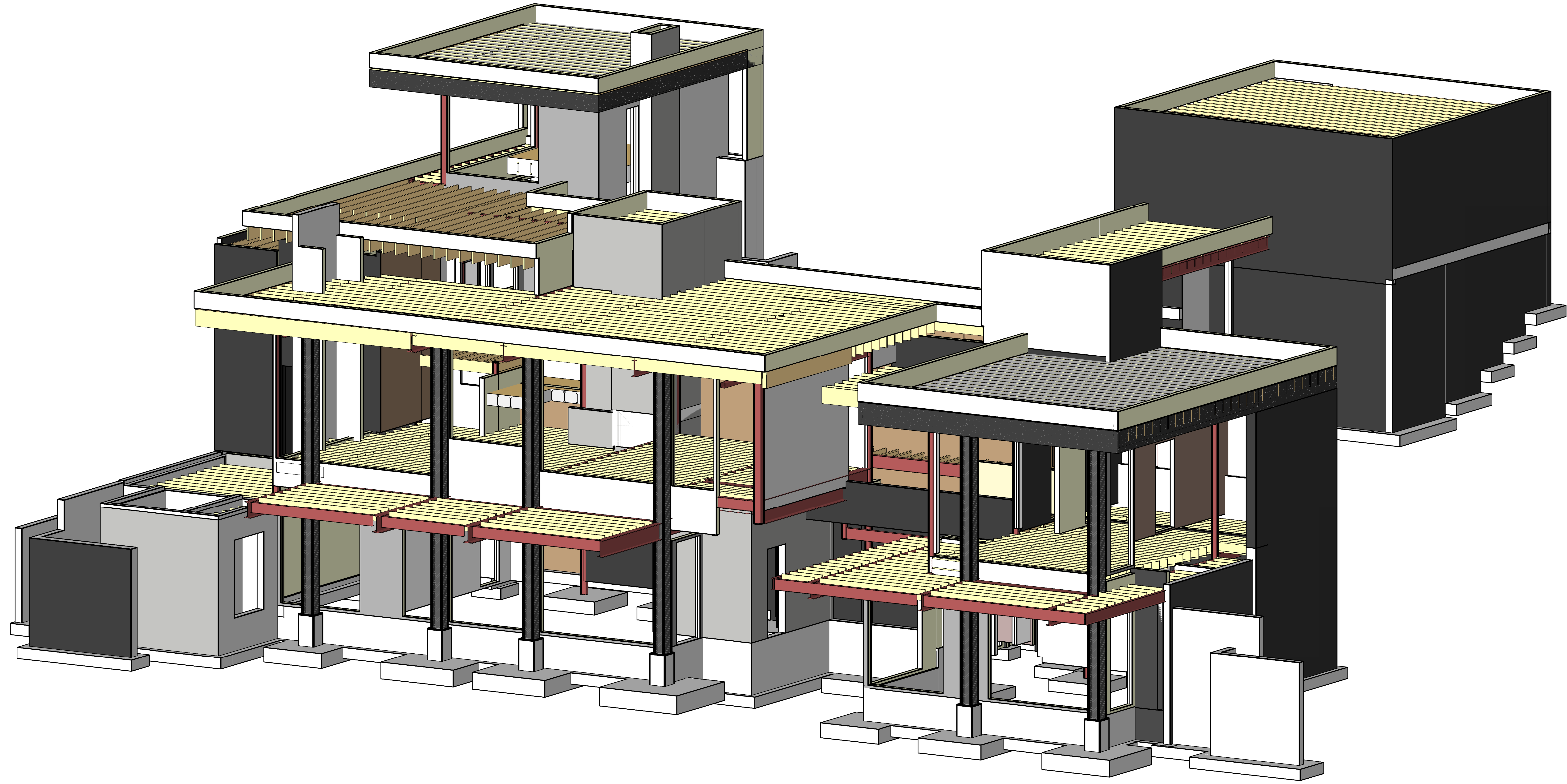
June 26, 2015  
November 2, 2015  
June 27, 2017  
September 22, 2017



201468

**S302**





NOTE: THIS VIEW REPRESENTS A SCHEMATIC RENDERING ONLY AND IS NOT INTENDED TO CONVEY CONSTRUCTION INFORMATION. ALL CONSTRUCTION SHALL COMPLY WITH SPECIFIC NOTES AND DETAILS WITHIN THE STRUCTURAL DRAWINGS.

FOR CONSTRUCTION