

STRUCTURAL STEEL SPECIAL INSPECTION SCHEDULE

ESTABLISHED PER 2012 IBC SECTION 1705.2.1

INSPECTION TASKS PRIOR TO WELDING (TABLE N5.4-1)	FABRICATOR QUALITY CONTROL		SPECIAL INSPECTOR QUALITY ASSURANCE		NOTES	INSPECTION TASKS PRIOR TO BOLTING (TABLE N5.6-1)				NOTES							
	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC								
WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	●		●		1. PERIODIC - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS. 2. CONTINUOUS - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER. 3. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR. 4. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), PURCHASER, OWNER, OR ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE, EXCEPT AS PERMITTED IN ACCORDANCE WITH SECTION N7. 5. QC AND QA INSPECTORS SHALL BE QUALIFIED IN ACCORDANCE WITH AISC 360-10 CHAPTER N4. 6. NONDESTRUCTIVE TESTING PERSONNEL SHALL BE QUALIFIED IN ACCORDANCE WITH AISC 360-10 CHAPTER N4.3. 7. NONDESTRUCTIVE TESTING OF WELDED JOINTS SHALL COMPLY WITH AISC 360-10 CHAPTER N5a AND b. 8. OBSERVATION OF WELDING OPERATIONS AND VISUAL INSPECTION OF IN-PROCESS AND COMPLETED WELDS SHALL BE THE PRIMARY METHOD TO CONFIRM THAT THE MATERIALS, PROCEDURES AND WORKMANSHIP ARE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS. FOR STRUCTURAL STEEL, ALL PROVISIONS OF AWS D1.1 / D1.1M STRUCTURAL WELDING CODE - STEEL FOR STATICALLY LOADED STRUCTURES SHALL APPLY. 9. THERMALLY CUT SURFACES OF ACCESS HOLES SHALL BE TESTED BY QA USING MT OR PT, WHEN THE FLANGE THICKNESS EXCEEDS 2 IN. (50mm) FOR ROLLED SHAPES, OR WHEN THE WEB THICKNESS EXCEEDS 2 IN. (50mm) FOR BUILT-UP SHAPES. ANY CRACK SHALL BE DEEMED UNACCEPTABLE REGARDLESS OF SIZE OR LOCATION. 10. WHEN REQUIRED BY APPENDIX 3, TABLE A-3.1, WELDED JOINTS REQUIRING WELD SOUNDNESS TO BE ESTABLISHED BY RADIOGRAPHICS OR ULTRASONIC INSPECTION SHALL BE TESTED BY QA AS PRESCRIBED. REDUCTION IN THE RATE OF UT IS PROHIBITED. 11. REDUCTION OF RATE OF ULTRASONIC TESTING - THE RATE OF UT IS ONLY PERMITTED TO BE REDUCED IF APPROVED BY THE EOR AND THE AHJ PER AISC 360-10 CHAPTER N5g. 12. FOR STRUCTURES IN RISK CATEGORY II, WHERE THE INITIAL RATE FOR UT IS 10%, THE NOT RATE FOR AN INDIVIDUAL WELDER OR WELDING OPERATOR SHALL BE INCREASED TO 100% SHOULD THE REJECT RATE, THE NUMBER OF WELDS CONTAINING UNACCEPTABLE DEFECTS DIVIDED BY THE NUMBER OF WELDS COMPLETED, EXCEEDS 5% OF THE WELDS TESTED FOR THE WELDER OR WELDING OPERATOR. A SAMPLING OF AT LEAST 20 COMPLETED WELDS FOR A JOB SHALL BE MADE PRIOR TO IMPLEMENTING SUCH AN INCREASE. WHEN THE REJECT RATE FOR THE WELDER OR WELDING OPERATOR, AFTER A SAMPLING OF AT LEAST 40 COMPLETED WELDS, HAS FALLEN TO 5% OR LESS, THE RATE OF UT SHALL BE RETURNED TO 10%. FOR EVALUATING THE REJECT RATE OF CONTINUOUS WELDS OVER 3 FT (1M) IN LENGTH WHERE THE EFFECTIVE THROAT IS 1 IN. (25mm) OR LESS, EACH 12 IN. (300mm) INCREMENT OR FRACTION THEREOF SHALL BE CONSIDERED AS ONE WELD. FOR EVALUATING THE REJECT RATE ON CONTINUOUS WELDS OVER 3 FT (1M) IN LENGTH WHERE THE EFFECTIVE THROAT IS GREATER THAN 1 IN. (25mm), EACH 6 IN. (150mm) OF LENGTH OR FRACTION THEREOF SHALL BE CONSIDERED ON WELD. 13. ALL NDT PERFORMED SHALL BE DOCUMENTED. FOR SHOP FABRICATION, THE NDT REPORT SHALL IDENTIFY THE TESTED WELD BY PIECE MARK AND LOCATION IN THE PIECE. FOR FIELD WORK, THE NDT REPORT SHALL IDENTIFY THE TESTED WELD BY LOCATION IN THE STRUCTURE, PIECE MARK, AND LOCATION IN THE PIECE. WHEN A WELD IS REJECTED ON THE BASIS OF NDT, THE NDT RECORD SHALL INDICATE THE LOCATION OF THE DEFECT AND THE BASIS OF REJECTION. 14. DEMAND CRITICAL WELDS SHALL MEET THE PROVISION FOUND IN AISC 341-10 AND WELDING METHODS, PROCEDURES AND QUALITY CONTROL SHALL COMPLY WITH AWS D1.1 AND THE FOLLOWING: a. ARC STRIKES, GOUGES AND OTHER IMPERFECTIONS WITHIN OR ADJACENT TO THE JOINT, SHALL BE REPAIRED OR REMOVED. b. PREHEAT AND INTER-PASS REQUIREMENTS AS OUTLINED IN SECTION 5.5. c. UNREPAIRED CRACKS, GOUGES, AND NOTCHES WILL NOT BE PERMITTED IN THE JOINT AREA. d. USE ELECTRODES WITH CHARPY V-NOTCH ABSORBED ENERGY EQUAL TO OR GREATER THAN 20 FT-LB AT -20 DEGREES FAHRENHEIT UNDER AWS AS CLASSIFICATION TEST METHODS, AND 40 FT-LBS AT 70 DEGREES FAHRENHEIT USING TEST PROCEDURES PRESCRIBED IN APPENDIX X OF AISC 358. ACCEPTABLE ELECTRODES INCLUDE E70T-K2, E71 T-1.	●	●	●	●	●	●	●	●				
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	●		●			MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	●	●	●	●	1. PERIODIC - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS. 2. CONTINUOUS - PERFORM THESE TASKS FOR EACH BOLTED CONNECTION. 3. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR. 4. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), APPLICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE, EXCEPT AS PERMITTED IN ACCORDANCE WITH SECTION N7. 5. FOR SNUG-TIGHT JOINTS, PRE-INSTALLATION VERIFICATION TESTING AS SPECIFIED IN TABLE N5.6-1 AND MONITORING OF THE INSTALLATION PROCEDURES AS SPECIFIED IN TABLE N5.6-2 ARE NOT APPLICABLE. THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS IN SNUG-TIGHT JOINTS. 6. FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE TURN-OF-NUT METHOD WITH MATCH-MARKING TECHNIQUES, THE DIRECT-TENSION INDICATOR METHOD, OR THE TWIST-OFF-TYPE TENSION CONTROL BOLT METHOD, MONITORING OF BOLT PRETENSIONING PROCEDURES SHALL BE AS SPECIFIED IN TABLE N5.6-2. THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS WHEN THESE METHODS ARE USED BY THE INSTALLER. 7. FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE CALIBRATED WRENCH METHOD OR THE TURN-OF-NUT METHOD WITHOUT MATCH-MARKING, MONITORING OF BOLT PRETENSIONING PROCEDURES SHALL BE AS SPECIFIED IN TABLE N5.6-2. THE QCI AND QAI SHALL BE ENGAGED IN THEIR ASSIGNED INSPECTION DUTIES DURING INSTALLATION OF FASTENERS WHEN THESE METHODS ARE USED BY THE INSTALLER. 8. OBSERVATION OF BOLTING OPERATIONS SHALL BE THE PRIMARY METHOD USED TO CONFIRM THAT THE MATERIALS, PROCEDURES AND WORKMANSHIP INCORPORATED IN CONSTRUCTION ARE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS AND THE PROVISIONS OF THE RCSC SPECIFICATION.						
MATERIAL IDENTIFICATION (TYPE / GRADE)	●		●			FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	●	●	●	●							
WELDER IDENTIFICATION SYSTEM ¹	●		●			PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	●	●	●	●							
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)	●		●			PROPER BOLTING PROCEDURES SELECTED FOR JOINT DETAIL	●	●	●	●							
* JOINT PREPARATION	●		●			CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FINISH SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	●	●	●	●							
* DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)	●		●			PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	●	●	●	●							
* CLEANLINESS (CONDITION OF STEEL SURFACES)	●		●			PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	●	●	●	●							
* TACKING (TACK WELD QUALITY AND LOCATION)	●		●				●	●	●	●							
* BACKING TYPE AND FIT (IF APPLICABLE)	●		●				●	●	●	●							
CONFIGURATION AND FINISH OF ACCESS HOLES	●		●				●	●	●	●							
FIT-UP OF FILLET WELDS	●		●				●	●	●	●							
* DIMENSIONS (ALIGNMENT, GAPS AT ROOT)	●		●				●	●	●	●							
* CLEANLINESS (CONDITION OF STEEL SURFACES)	●		●			●	●	●	●								
* TACKING (TACK WELD QUALITY AND LOCATION)	●		●			●	●	●	●								
CHECK WELDING EQUIPMENT	●		●			●	●	●	●								
¹ THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.																	
INSPECTION TASKS DURING WELDING (TABLE N5.4-2)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC								
USE OF QUALIFIED WELDERS	●		●			●		●									
CONTROL AND HANDLING OF WELDING CONSUMABLES	●		●			●		●									
* PACKAGING	●		●			●		●									
* EXPOSURE CONTROL	●		●			●		●									
NO WELDING OVER CRACKED TACK WELDS	●		●			●		●									
ENVIRONMENTAL CONDITIONS	●		●			●		●									
* WIND SPEED WITHIN LIMITS	●		●			●		●									
* PRECIPITATION AND TEMPERATURE	●		●			●		●									
WPS FOLLOWED	●		●			●		●									
* SETTINGS ON WELDING EQUIPMENT	●		●			●		●									
* TRAVEL SPEED	●		●			●		●									
* SELECTED WELDING MATERIALS	●		●			●		●									
* SHIELDING GAS TYPE / FLOW RATE	●		●			●		●									
* PREHEAT APPLIED	●		●			●		●									
* INTERPASS TEMPERATURE MAINTAINED (MIN. / MAX)	●		●			●		●									
* PROPER POSITION (F, V, H, OH)	●		●			●		●									
WELDING TECHNIQUES	●		●			●		●									
* INTERPASS AND FINAL CLEANING	●		●			●		●									
* EACH PASS WITHIN PROFILE LIMITATIONS	●		●			●		●									
* EACH PASS MEETS QUALITY REQUIREMENTS	●		●			●		●									
INSPECTION TASKS AFTER WELDING (TABLE N5.4-3)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC								
WELDS CLEANED	●		●			●		●									
SIZE, LENGTH AND LOCATION OF WELDS	●		●			●		●									
WELDS MEET VISUAL ACCEPTANCE CRITERIA	●		●			●		●									
* CRACK PROHIBITION	●		●			●		●									
* WELD / BASE-METAL FUSION	●		●			●		●									
* CRATER CROSS SECTION	●		●			●		●									
* WELD PROFILES	●		●			●		●									
* WELD SIZE	●		●			●		●									
* UNDERCUT	●		●			●		●									
* POROSITY	●		●			●		●									
ARC STRIKES	●		●			●		●									
K-AREA ¹	●		●			●		●									
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	●		●			●		●									
REPAIR ACTIVITIES	●		●			●		●									
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	●		●			●		●									
¹ WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. (75mm) OF THE WELD.																	
						INSPECTION TASKS DURING BOLTING (TABLE N5.6-2)											
						CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC								
						●	●	●	●								
						INSPECTION TASKS AFTER BOLTING (TABLE N5.6-3)											
						CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC								
						●	●	●	●								
						INSPECTION OF STEEL ELEMENTS OF COMPOSITE CONSTRUCTION PRIOR TO CONCRETE PLACEMENT (TABLE N6.1)											
						CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC								
						●	●	●	●								
						GENERAL STEEL SPECIAL INSPECTION NOTES :											
						1. QUALITY ASSURANCE (QA) INSPECTION OF FABRICATED ITEMS SHALL BE MADE AT THE FABRICATOR'S PLANT. THE QUALITY ASSURANCE INSPECTOR (QAI) SHALL SCHEDULE THIS WORK TO MINIMIZE INTERRUPTION TO THE WORK OF THE FABRICATOR. 2. QA INSPECTION OF THE ERRECTED STEEL SYSTEM SHALL BE MADE AT THE PROJECT SITE. THE QAI SHALL SCHEDULE THIS WORK TO MINIMIZE INTERRUPTION TO THE WORK OF THE ERECTOR. 3. WHERE A TASK IS NOTED TO BE PERFORMED BY BOTH QC AND QA, IT IS PERMITTED TO COORDINATE THE INSPECTION FUNCTION BETWEEN THE QCI AND QAI SO THAT THE INSPECTION FUNCTIONS ARE PERFORMED BY ONLY ONE PARTY. WHERE QA RELIES UPON INSPECTION FUNCTIONS PERFORMED BY QC, THE APPROVAL OF THE ENGINEER OF RECORD AND THE AUTHORITY HAVING JURISDICTION IS REQUIRED. 4. THE FABRICATOR'S QCI SHALL INSPECT THE FABRICATED STEEL TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE SHOP DRAWINGS, SUCH AS PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. THE ERECTOR'S QCI SHALL INSPECT THE ERRECTED STEEL FRAME TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE ERECTION DRAWINGS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. 5. THE QAI SHALL BE ON THE PREMISES FOR INSPECTION DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. AS A MINIMUM, THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE VERIFIED PRIOR TO PLACEMENT OF THE CONCRETE. 6. THE QAI SHALL INSPECT THE FABRICATED STEEL OR ERRECTED STEEL FRAME, AS APPROPRIATE, TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. 7. QUALITY ASSURANCE (QA) INSPECTIONS, EXCEPT NONDESTRUCTIVE TESTING (NDT), MAY BE WAIVED WHEN THE WORK IS PERFORMED IN A FABRICATING SHOP OR BY AN ERECTOR APPROVED BY THE AUTHORITY HAVING JURISDICTION (AHJ) TO PERFORM THE WORK WITHOUT QA. NOT OF WELDS COMPLETED IN AN APPROVED FABRICATOR'S SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE AHJ. WHEN THE FABRICATOR PERFORMS THE NDT, THE QA AGENCY SHALL REVIEW THE FABRICATOR'S NDT REPORTS. 8. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE FABRICATOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS. AT COMPLETION OF ERECTION, THE APPROVED ERECTOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE ERECTOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS. 9. IDENTIFICATION AND REJECTION OF MATERIAL OR WORKMANSHIP THAT IS NOT IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS, SHALL BE PERMITTED AT ANY TIME DURING THE PROGRESS OF THE WORK. HOWEVER, THIS PROVISION SHALL NOT RELIEVE THE OWNER OR THE INSPECTOR OF THE OBLIGATION FOR TIMELY, IN-SEQUENCE INSPECTIONS. NONCONFORMING MATERIAL AND WORKMANSHIP SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE FABRICATOR OR ERECTOR, AS APPLICABLE. 10. NONCONFORMING MATERIAL OR WORKMANSHIP SHALL BE BROUGHT INTO CONFORMANCE, OR MADE SUITABLE FOR ITS INTENDED PURPOSE AS DETERMINED BY THE ENGINEER OF RECORD. 11. CONCURRENT WITH THE SUBMITTAL OF SUCH REPORTS TO THE AHJ, EOR OR OWNER, THE QA AGENCY SHALL SUBMIT TO THE FABRICATOR AND ERECTOR: (1) NONCONFORMANCE REPORTS (2) REPORTS OF REPAIR, REPLACEMENT OR ACCEPTANCE OF NONCONFORMING ITEMS.											

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SCHEDULES

June 26, 2015
November 2, 2015
June 27, 2017
September 22, 2017
October 25, 2017



2014168

S002

FOR CONSTRUCTION

SPECIAL INSPECTION SCHEDULE 1,2				
ESTABLISHED PER 2012 IBC SECTION 110 AND CHAPTER 17				
ITEM	CONTINUOUS ³	PERIODIC ³	REFERENCE	COMMENTS
PRE-FAB CONSTRUCTION (IBC 1704.2)			REFERENCE NOTES P1 & P2	P1. SPECIAL INSPECTION IS NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION, PROVIDED THE FABRICATOR COMPLETES WITH IBC INSPECTION FOR PREFABRICATED CONSTRUCTION SHALL BE THE SAME AS IF THE MATERIAL USED IN THE CONSTRUCTION TOOK PLACE ON SITE. SPECIAL INSPECTION WILL NOT BE REQUIRED DURING PREFABRICATION IF THE APPROVED SPECIALIST CERTIFIES THE CONSTRUCTION AND FURNISHES EVIDENCE OF COMPLIANCE. (SEE NOTE 2).
CONCRETE CONSTRUCTION (IBC 1705.3)			SEE IBC TABLE 1705.3 - REF. NOTE C1	C1. SPECIAL INSPECTION IS NOT REQUIRED FOR CONC. ISOLATED SPREAD FOOTINGS, CONTINUOUS FOOTINGS, NON-STRUCTURAL SLABS, FOUNDATION WALLS, PATIOS, DRIVEWAYS, AND SIDEWALKS PROVIDED THE REQUIREMENTS OF IBC 1705.3 ARE MET.
REINFORCING STEEL PLACEMENT		●	REFERENCE NOTE C2	C2. PERIODIC SPECIAL INSPECTION IS ALLOWED FOR VERIFICATION OF THE WELDABILITY OF REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES. BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR WALLS AND SHEAR REINFORCEMENT. PERIODIC SPECIAL INSPECTION IS ALLOWED FOR WELDING OF ASTM A 709 REINFORCING STEEL NOT INCLUDED IN THE CONTINUOUS SPECIAL INSPECTION REQUIREMENTS NOTED ABOVE.
WELDING OF REINFORCING STEEL	●	●		C3. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR VERIFICATION OF IN-SITU CONCRETE STRENGTH FOR POST-TENSIONED CONCRETE PRIOR TO TENSIONING TENDONS OR REMOVING SHORING OR FORMS.
EMBEDDED BOLTS & PLATES	●	●	REFERENCE NOTE C3	C4. PERFORM AIR SLUMP AND TEMP. TESTS WHEN CONCRETE SAMPLES ARE CAST.
VERIFYING REQUIRED DESIGN MIX	●	●	REFERENCE NOTE C4	C5. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR VERIFICATION OF IN-SITU CONCRETE STRENGTH FOR POST-TENSIONED CONCRETE PRIOR TO TENSIONING TENDONS OR REMOVING SHORING OR FORMS.
CONCRETE PLACEMENT / SAMPLING	●	●	REFERENCE NOTE C5	EPOXY AND EXPANSION ANCHORS TO MASONRY OR CONCRETE MAY BE USED ONLY WHEN APPROVED BY ARCHITECT AND/OR ENGINEER USING AN APPROVED PRODUCT WITH CURRENT PUBLISHED ICC RESEARCH REPORT NUMBERS. COORDINATE CONTINUOUS/PERIODIC SPECIAL INSPECTION REQUIREMENTS WITH ICC REPORT.
CURING TEMPERATURE / TECHNIQUES	●	●		
VERIFICATION OF IN-SITU STRENGTH	●	●		
EPOXY / EXPANSION ANCHOR PLACEMENT	●	●		
WOOD (IBC 1705.5)				W1. WOOD STRUCTURAL PANEL SHEATHING SHALL BE INSPECTED TO ASCERTAIN THAT GRADE AND THICKNESS ARE IN COMPLIANCE WITH APPROVED BUILDING PLANS. NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES, THE NAIL OR STAPLE DIAMETER AND LENGTH, THE NUMBER OF FASTENER LINES, AND SPACING BETWEEN FASTENERS IN EACH LINE AND AT EDGE MARGINS SHALL ALSO BE INSPECTED AND VERIFIED FOR COMPLIANCE WITH APPROVED BUILDING PLANS.
HIGH LOAD DIAPHRAGMS (ROOF / FLOOR)		●	REFERENCE NOTE W1	W2. SPECIAL INSPECTION IS NOT REQUIRED FOR WOOD SHEAR WALLS, WOOD DIAPHRAGMS, INCLUDING NAILING, BOLTING, AND OTHER FASTENING TO OTHER COMPONENTS WHERE THE SPACING OF THE SHEATHING FASTENERS IS GREATER THAN 4"x.c.
SITE-BUILT ASSEMBLIES		●		
SHEAR WALL & DIAPHRAGM NAILING		●	REFERENCE NOTE W2	
DRAG STRUTS		●		
BRACES & SHEAR PANELS		●		
HOLD-DOWNS		●		
GLUING OPERATIONS	●	●		
SOILS (IBC 1705.6)			REFERENCE NOTE F1	F1. SPECIAL INSPECTION OF SOILS SHALL REFERENCE THE APPROVED SOILS REPORT TO DETERMINE COMPLIANCE.
VERIFY ADEQUATE MATERIALS BELOW FOOTINGS		●	REFERENCE NOTE F1	F2. WHERE SOILS REPORT IS NOT PROVIDED SPECIAL INSPECTIONS ARE REQUIRED TO VERIFY THAT THE IN-PLACE DRY DENSITY OF THE COMPACTED FILL IS NOT LESS THAN 90 PERCENT OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT DETERMINED IN ACCORDANCE WITH ASTM D 1557.
EXCAVATIONS EXTEND TO PROPER DEPTH AND REACH PROPER MATERIAL		●	REFERENCE NOTE F2	
CLASSIFY & TEST CONTROLLED FILL MATERIALS		●	REFERENCE NOTE F2	
PERFORM MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL	●	●	REFERENCE NOTE F1	
PROPERLY PREPARED SITE AND SUB-GRADE PRIOR TO FILL		●	REFERENCE NOTE F1	

- GENERAL SPECIAL INSPECTION NOTES :**
- THE ITEMS MARKED WITH "●" IN THE SPECIAL INSPECTION SCHEDULE SHALL BE INSPECTED IN ACCORDANCE WITH IBC CHAPTER 17 BY A CERTIFIED SPECIAL INSPECTOR FROM AN ESTABLISHED TESTING AGENCY. FOR MATERIAL SAMPLING AND TESTING REQUIREMENTS, REFER TO THE MATERIAL SAMPLING AND TESTING SECTION, THE PROJECT SPECIFICATIONS, AND THE SPECIFIC GENERAL NOTES SECTIONS. THE TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE ARCHITECT, ENGINEER, CONTRACTOR, AND BUILDING OFFICIAL. ANY ITEMS WHICH FAIL TO COMPLY WITH THE APPROVED CONSTRUCTION DOCUMENTS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF DISCREPANCIES ARE NOT CORRECTED, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL, ARCHITECT, AND ENGINEER PRIOR TO COMPLETION OF THAT PHASE OF WORK. SPECIAL INSPECTION TESTING REQUIREMENTS APPLY EQUALLY TO ALL BIDDER DESIGNED COMPONENTS.
 - ANY CONSTRUCTION OR MATERIAL THAT HAS FAILED INSPECTION SHALL BE SUBJECT TO REMOVAL AND REPLACEMENT.
 - CONTINUOUS SPECIAL INSPECTION MEANS THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. PERIODIC SPECIAL INSPECTION MEANS THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK. (IBC SECTION 1702)

BEAM CONNECTIONS SCHEDULE						
CONNECTION SCHEDULE						
BEAM DEPTH	SHEAR PLATE INFORMATION		BOLTS W/ STANDARD WASHERS OVER SLOTS		WELD 'A'	COMMENTS
	PL. DIMENSIONS W/ SHORT-SLOTTED HOLES	Lev	No.	SIZE		
W8x, W10x	PL. 1/4" x 4"	1 1/2"	2"	2	3/4" Ø	3/16"
W12x	PL. 5/16" x 4"	1 1/2"	2"	3	3/4" Ø	1/4"
W14x 90 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	3	3/4" Ø	1/4"
W16x 77 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	4	3/4" Ø	1/4"
W18x 105 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	5	3/4" Ø	1/4"
W21x 75 & LIGHTER	PL. 5/16" x 4"	1 1/2"	2"	6	3/4" Ø	1/4"
W24x 94 & LIGHTER	PL. 3/8" x 4"	1 1/2"	2"	7	7/8" Ø	1/4"
W27x 114 & LIGHTER	PL. 3/8" x 4"	1 1/2"	2"	7	7/8" Ø	1/4"
W30x 124 & LIGHTER	PL. 1/2" x 4"	1 3/4"	2"	8	1" Ø	5/16"
W33x 130 & LIGHTER	PL. 1/2" x 4"	1 3/4"	2"	9	1" Ø	5/16"
W36x 160 & LIGHTER	PL. 1/2" x 4 1/2"	2"	2 1/4"	10	1-1/8" Ø	5/16"

STANDARD HOOK & BEND SCHEDULE						
BAR SIZE	DIMENSION OF STANDARD 180-DEG HOOKS, ALL GRADES			DIMENSION OF STANDARD 90-DEG HOOKS, ALL GRADES		
	A or G	J	D	A or G	J	D
#3	5"	3"	2 1/4"	6"	6"	2 1/4"
#4	6"	4"	3"	8"	8"	3"
#5	7"	5"	3 3/4"	10"	10"	3 3/4"
#6	8"	6"	4 1/2"	12"	12"	4 1/2"
#7	10"	7"	5 1/4"	15"	15"	5 1/4"
#8	11"	8"	6"	16"	16"	6"
#9	13"	11 3/4"	9 1/2"	17"	17"	9 1/2"
#10	15"	13 1/4"	10 3/4"	19"	19"	10 3/4"
#11	17"	15 1/2"	12"	21"	21"	12"

BEAM FRAMING SCHEDULE						
BEAM SIZE	ANGLE SIZE (EA. SIDE)	WELD 'A'	BOLTS	EMBED PLATE	# OF H.S.A.	H.S.A. PATTERN
W8x, C8x	3 x 5 x 1/4	3/16"	(2) 3/4" Ø	8" x 8"	4	••
W10x	3 x 5 x 5/16	1/4"	(2) 3/4" Ø	12" x 1'-4"	6	•••
W12x, C12x	3 x 5 x 5/16	1/4"	(3) 3/4" Ø	12" x 1'-4"	6	••••
W14x	3 x 5 x 5/16	1/4"	(3) 3/4" Ø	15" x 1'-4"	9	•••••
W16x	3 x 5 x 5/16	1/4"	(4) 3/4" Ø	15" x 2'-0"	12	••••••
W18x	3 x 5 x 5/16	1/4"	(5) 3/4" Ø	15" x 2'-0"	12	•••••••
W21x	3 x 5 x 3/8	1/4"	(6) 7/8" Ø	15" x 2'-0"	12	••••••••
W24x	3 x 5 x 3/8	1/4"	(7) 7/8" Ø	15" x 2'-0"	12	•••••••••
W27x	3 x 5 x 3/8	3/8"	(8) 7/8" Ø	15" x 2'-6"	15	••••••••••
W30x	3 x 5 x 3/8	3/8"	(9) 7/8" Ø	15" x 2'-6"	15	•••••••••••

FOOTING SCHEDULE						
MARK	WIDTH	LENGTH	THICK	LENGTHWISE REINF. NO.	CROSSWISE REINF. SIZE	REMARKS
FT2	2'-0"	CONT.	12"	(2)	#5	-- --
FC2	2'-0"	CONT.	12"	(2)	#5	-- --
FC3	3'-0"	CONT.	12"	(3)	#5	-- --
FC4	4'-0"	CONT.	12"	(4)	#5	-- #5 12"
FC6.5	6'-6"	CONT.	16"	(7)	#5	-- #5 12"
FC7.5	7'-6"	CONT.	16"	(7)	#6	-- #6 12"
F3	3'-0"	3'-0"	12"	(3)	#5	(3) #5 --
F4	4'-0"	4'-0"	12"	(4)	#5	(4) #5 --
F5	5'-0"	5'-0"	12"	(5)	#5	(5) #5 --
F5.5	5'-6"	5'-6"	12"	(5)	#5	(5) #5 --
F6	6'-0"	6'-0"	12"	(6)	#5	(6) #5 --
F6.5	6'-6"	6'-6"	14"	(7)	#5	(7) #5 --
F7	7'-0"	7'-0"	14"	(7)	#5	(7) #5 --
F7.5	7'-6"	7'-6"	14"	(7)	#6	(7) #6 --
F8	8'-0"	8'-0"	16"	(8)	#6	(8) #6 --

2012 IBC CONC. REBAR LAP SPlice SCHEDULE																										
FOR CONCRETE APPLICATIONS (ACI 318 - 11)																										
BAR LOCATION	CONCRETE TYPE	STRENGTH	CONCRETE REINFORCING & SPLICE LENGTHS (IN)																							
			BAR SIZE																							
#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#14	#15														
VERT. WALL BARS, FILL ON METAL DECK	NWC	3000 PSI	17	22	8	22	29	8	28	36	10	33	43	12	48	62	13	55	72	15	62	17	69	19	76	30
HORIZ. WALL BARS, FOOTING TOP BARS	NWC	3000 PSI	17	22	8	22	29	8	28	36	10	33	43	12	48	62	13	55	72	15	62	17	69	19	76	30
BEAM BOTTOM BARS, COLUMN BARS	NWC	3000 PSI	17	22	8	22	29	11	28	36	14	33	43	16	48	62	19	55	72	22	62	25	69	27	76	30
FOOTING BOTTOM BARS	NWC	3000 PSI	12	16	8	14	18	8	17	22	10	20	26	12	29	38	13	33	43	15	37	17	42	19	46	30
BEAM TOP BARS	NWC	3000 PSI	22	29	8	29	38	11	36	47	14	43	56	16	63	82	19	72	94	22	81	25	90	27	98	30
SLAB ON GRADE	NWC	3000 PSI	12	16	8	14	18	8	17	22	10	20	26	12	32	42	13	42	55	15	53	17	69	19	76	30

BAR LOCATION	CONCRETE TYPE	STRENGTH	CONCRETE REINFORCING & SPLICE LENGTHS (IN)																							
			BAR SIZE																							
#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	#14	#15														
VERT. WALL BARS, FILL ON METAL DECK	NWC	4500 PSI	14	18	7	18	23	6	23	30	8	27	35	9	40	52	11	45	59	13	51	14	56	16	62	25
HORIZ. WALL BARS, FOOTING TOP BARS	NWC	4500 PSI	14	18	7	18	23	6	23	30	8	27	35	9	40	52	11	45	59	13	51	14	56	16	62	25
BEAM BOTTOM BARS, COLUMN BARS	NWC	4500 PSI	14	18	7	18	23	9	23	30	11	27	35	13	40	52	16	45	59	18	51	20	56	22	62	25
FOOTING BOTTOM BARS	NWC	4500 PSI	12	16	7	12	16	6	14	18	8	17	22	9	24	31	11	27	35	13	31	14	34	16	37	25
BEAM TOP BARS	NWC	4500 PSI	18	23	7	24	31	9	30	39	11	35	46	13	51	66	16	59	77	18	60	20	73	22	80	25
SLAB ON GRADE	NWC	4500 PSI	12	16	7	12	16	6	14	18	8	17	22	9	27	35	11	34	44	13	44	14	56	16	62	25

- NOTES:**
- MECHANICAL COUPLERS MAY BE USED IN LIEU OF LAP SPLICES SHOWN. SEE STRUCTURAL NOTES FOR MINIMUM COUPLER CAPACITY.
 - WHERE MECHANICAL COUPLERS ARE USED, STAGGER ADJACENT SPLICES A MINIMUM OF 24" AS INDICATED ABOVE.
 - DEVELOPMENT LENGTHS SHALL BE INCREASED BY 50% FOR STRAIGHT BAR DEVELOPMENT AND 20% FOR HOOKED BARS WHERE EPOXY COATING IS USED.
 - SPLICE BARS LARGER THAN #11 USING MECHANICAL COUPLERS.

FOR CONSTRUCTION

ENGINEERS

structural consultants
1584 W. Park, Ok. Ogden, Utah 84403
PH: 801.726.6028 FAX: 801.726.4856

39 SUMMIT, LLC

Summit at Powder Mountain Lot 39
8365 E. SUMMIT PASS

SCHEDULES

June 26, 2015
November 2, 2015
June 27, 2017
September 22, 2017
October 25, 2017

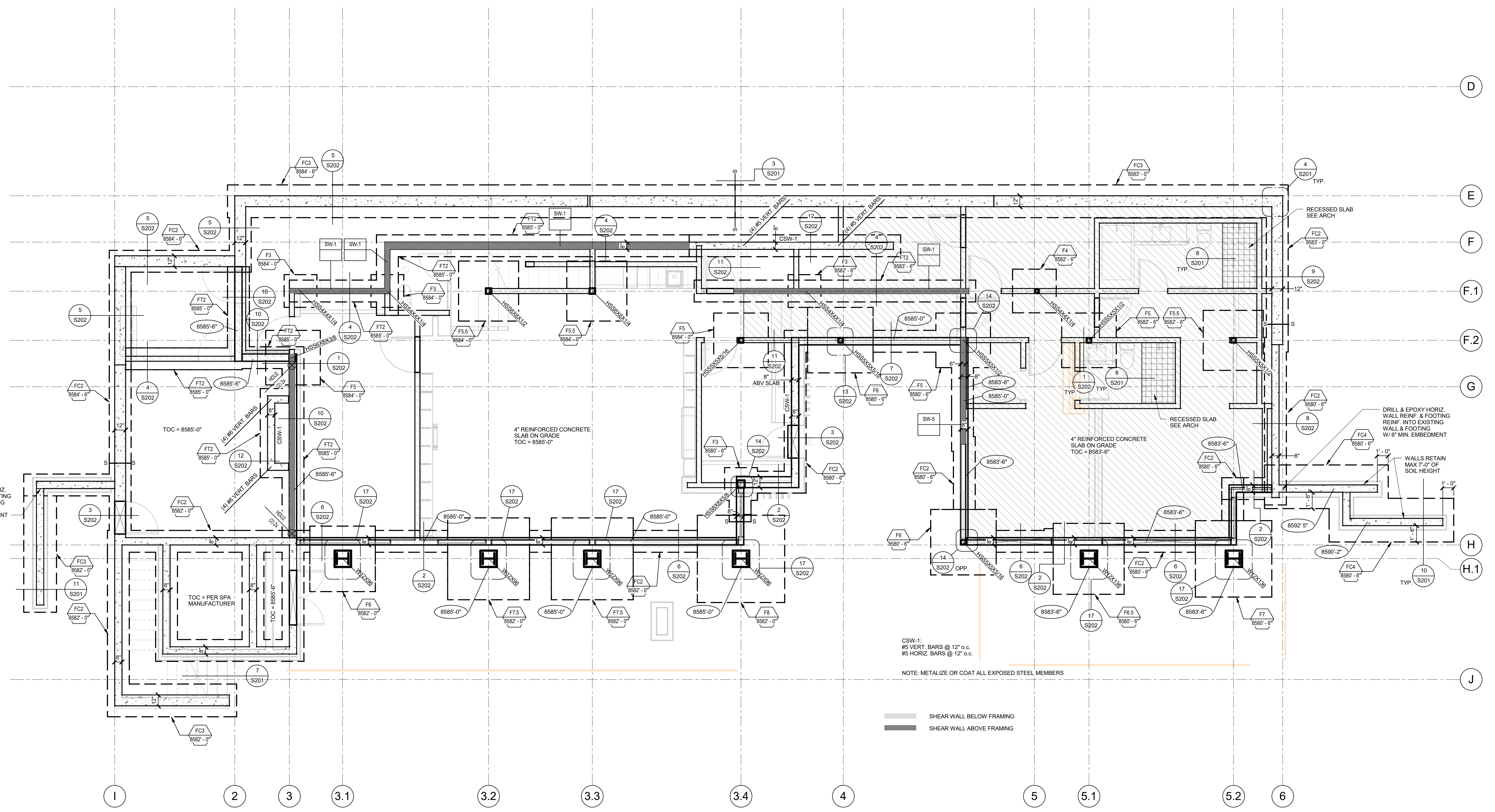
FOR CONSTRUCTION

No. 37896
Typ. A-E-Div.
07/15/17

S003

2014168

FOR CONSTRUCTION

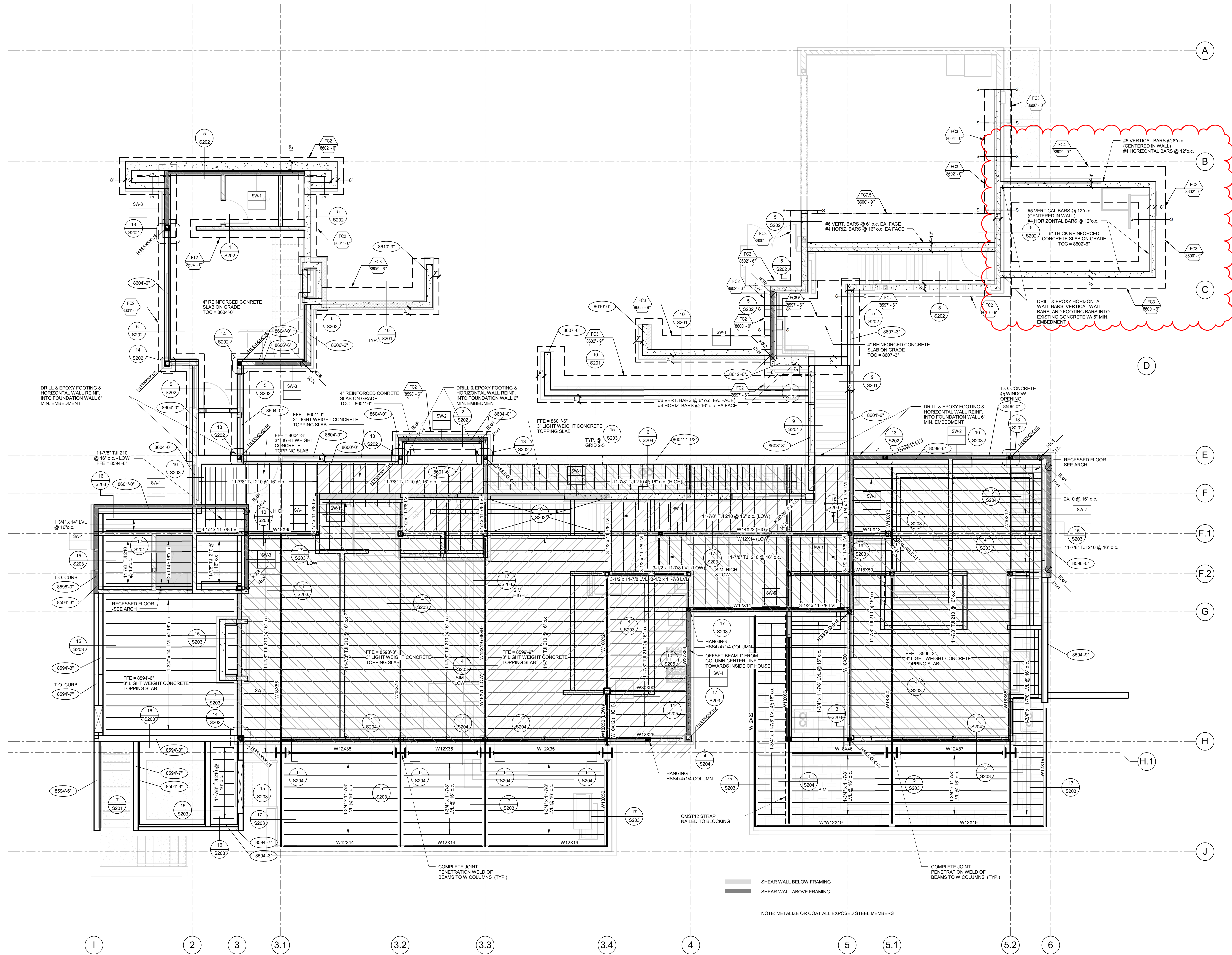


T.O. SUB-SLAB @ BASEMENT LEVEL
 SCALE: 1/4" = 1'-0"

A
 S101

— SHEAR WALL BELOW FRAMING
 — SHEAR WALL ABOVE FRAMING

CSW-1:
 #5 VERT. BARS @ 12" o.c.
 #5 HORIZ. BARS @ 12" o.c.
 NOTE: METALIZE OR COAT ALL EXPOSED STEEL MEMBERS

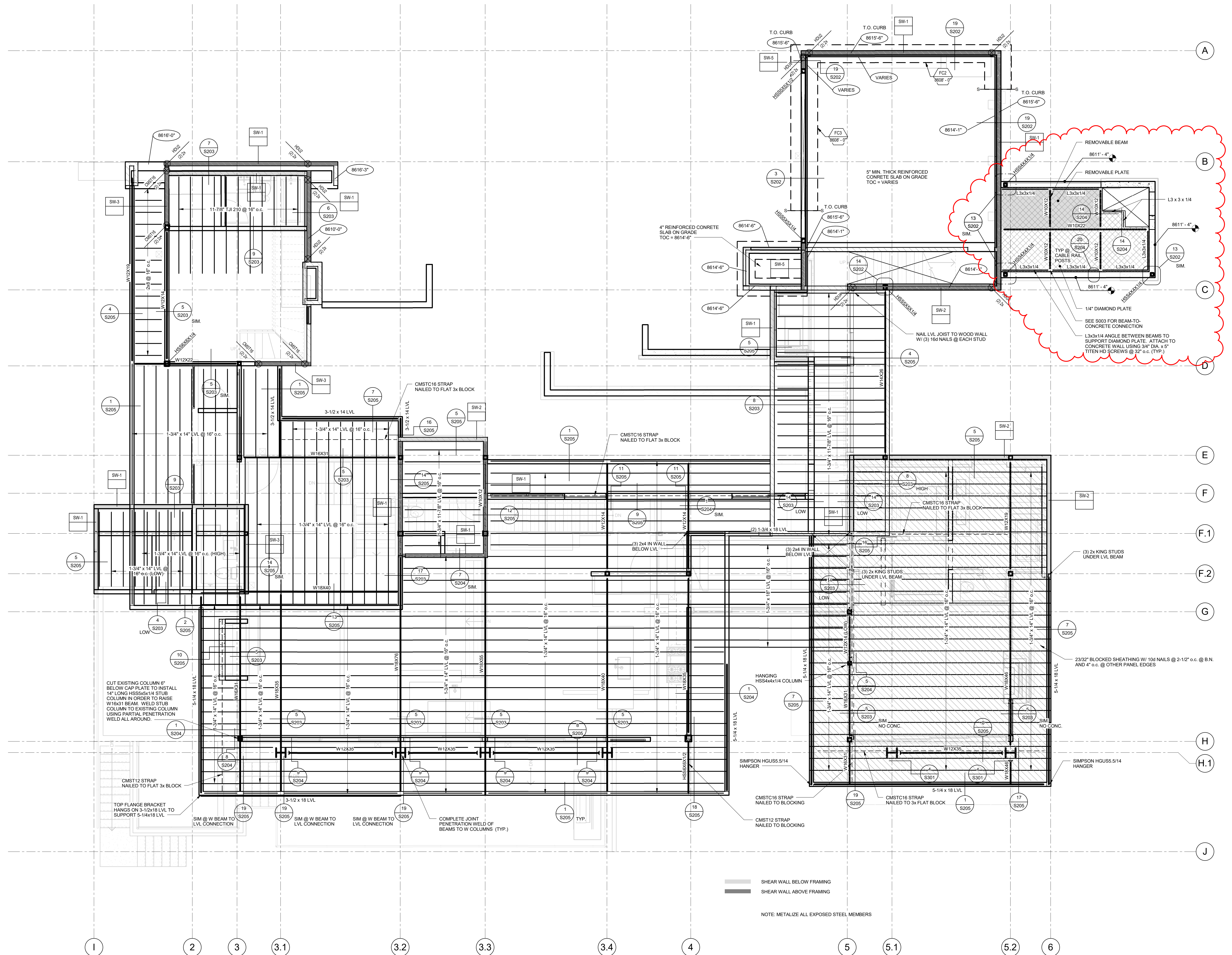


T.O. SUB-FLOOR @ KITCHEN & DINING
SCALE: 1/4" = 1'-0"

A
S102

SHEAR WALL BELOW FRAMING
 SHEAR WALL ABOVE FRAMING
 NOTE: METALIZE OR COAT ALL EXPOSED STEEL MEMBERS

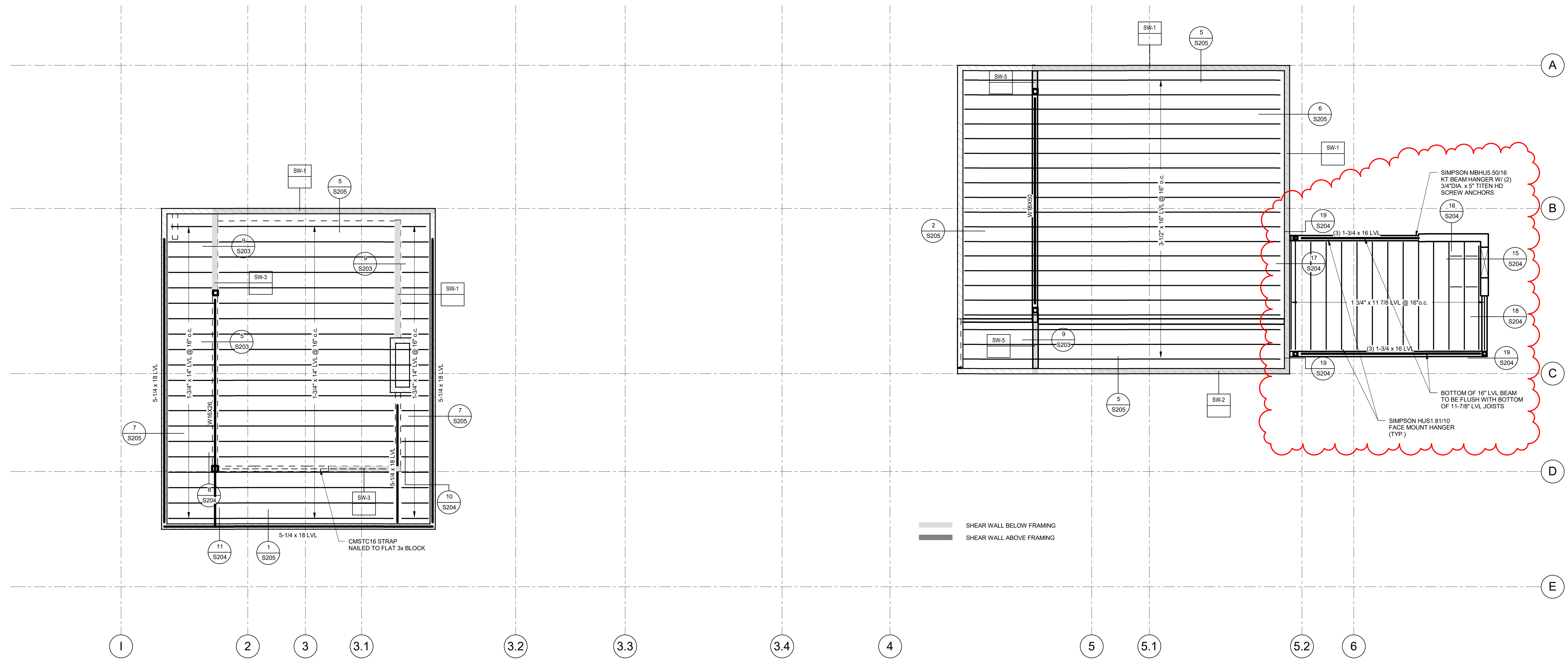
FOR CONSTRUCTION



ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"

A
S103

SHEAR WALL BELOW FRAMING
 SHEAR WALL ABOVE FRAMING
 NOTE: METALIZE ALL EXPOSED STEEL MEMBERS



HIGH ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"

A
S104

— SHEAR WALL BELOW FRAMING
— SHEAR WALL ABOVE FRAMING

SIMPSON MBHU5.50/16
KT BEAM HANGER W/ (2)
3/4" DIA. x 5" TITEN HD
SCREW ANCHORS

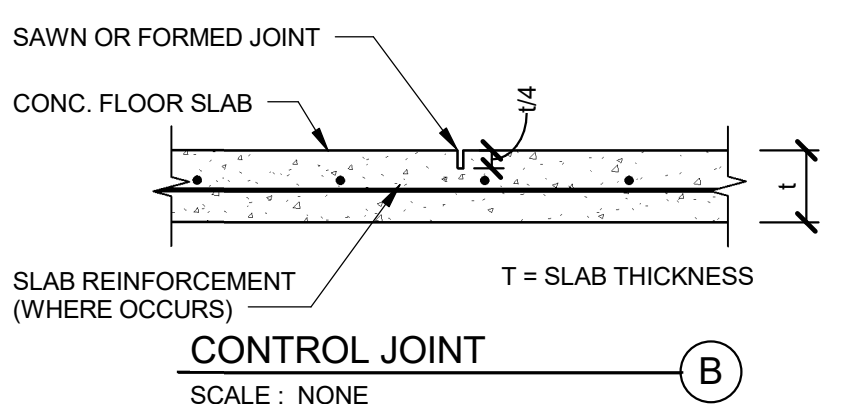
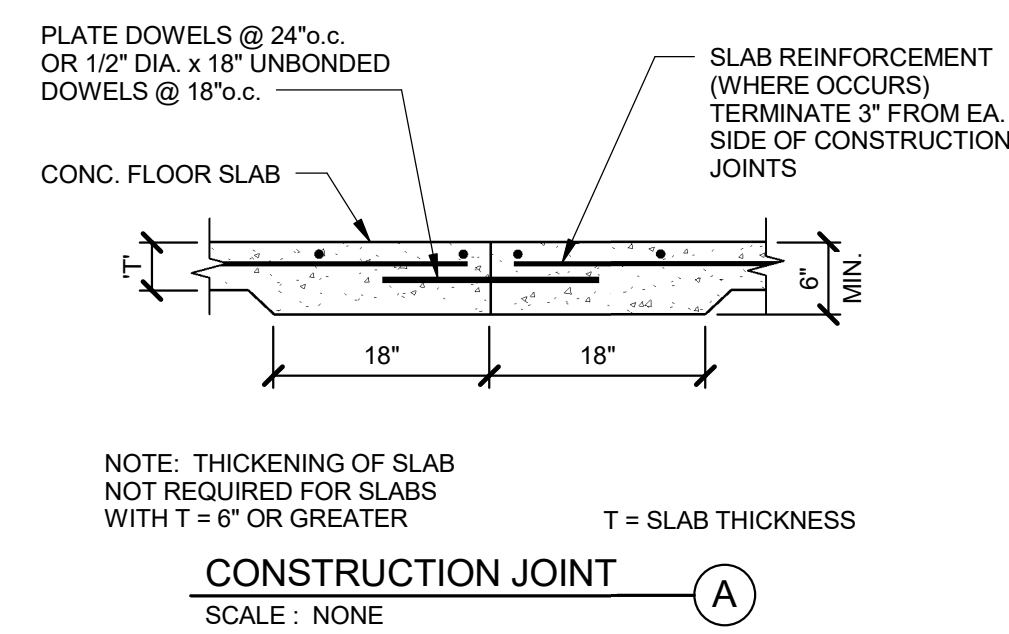
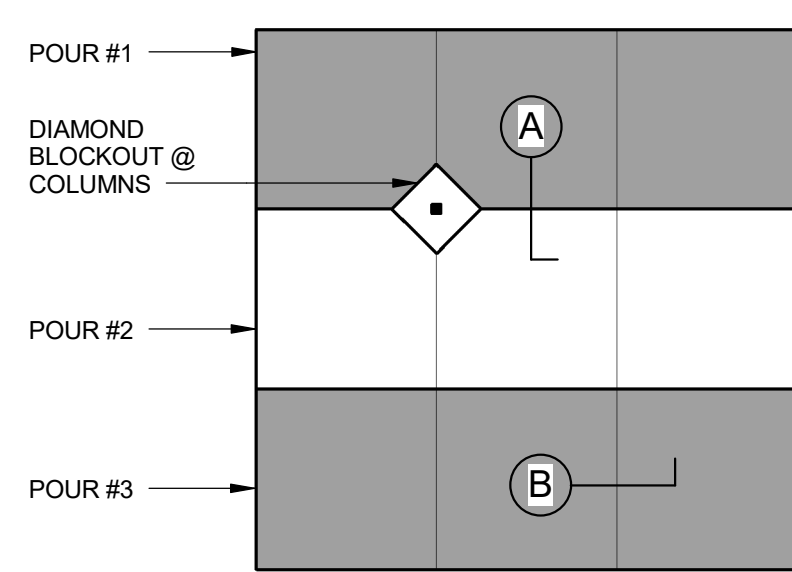
(3) 1-3/4 x 16 LVL

1-3/4 x 11 7/8 LVL @ 16" c.c.

— BOTTOM OF 16" LVL BEAM
TO BE FLUSH WITH BOTTOM
OF 11-7/8" LVL JOISTS

SIMPSON HUS1.81/10
FACE MOUNT HANGER
(TYP)

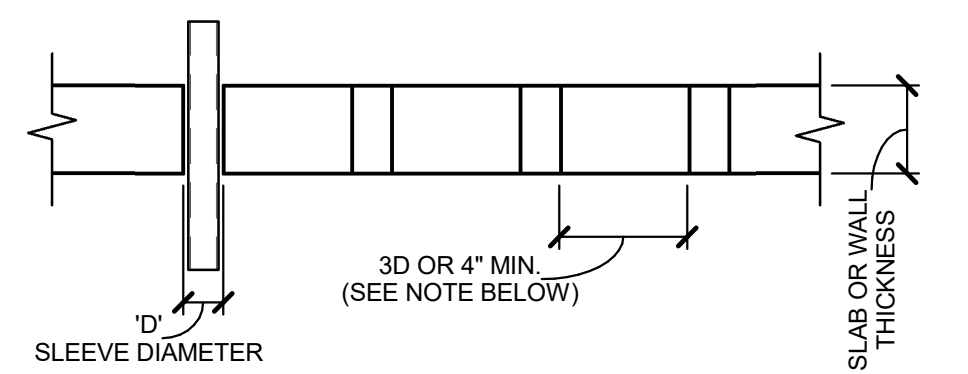
FOR CONSTRUCTION



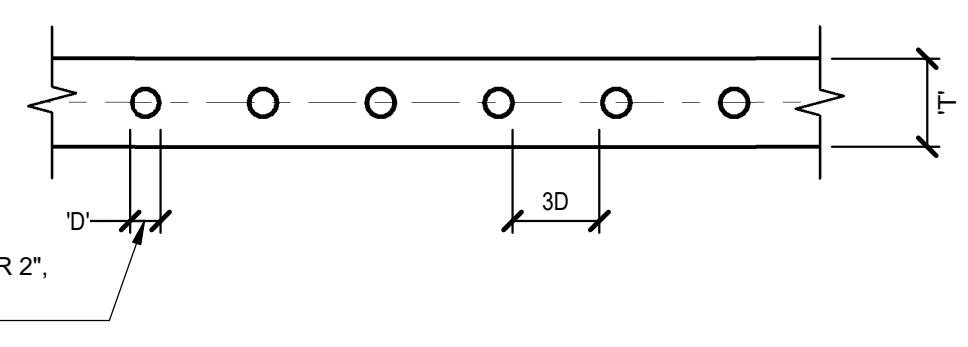
NOTES:
 1. JOINTS SHALL OCCUR AT MAIN COLUMN/GRID LINES W/ 10'-0" MAX. SPACING BETWEEN JOINTS @ 4' SLABS; 12'-0" MAX. @ 5' SLABS; & 15'-0" MAX. @ 8' SLABS.
 2. SEE PLAN FOR SLAB THICKNESS 'T' AND REINFORCING SIZE AND SPACING.
 3. SEE ARCHITECT FOR CONTROL JOINT LAYOUT IN TOPPING SLABS

TYPICAL CONCRETE SLAB JOINTS
 SCALE: NONE

1
S201



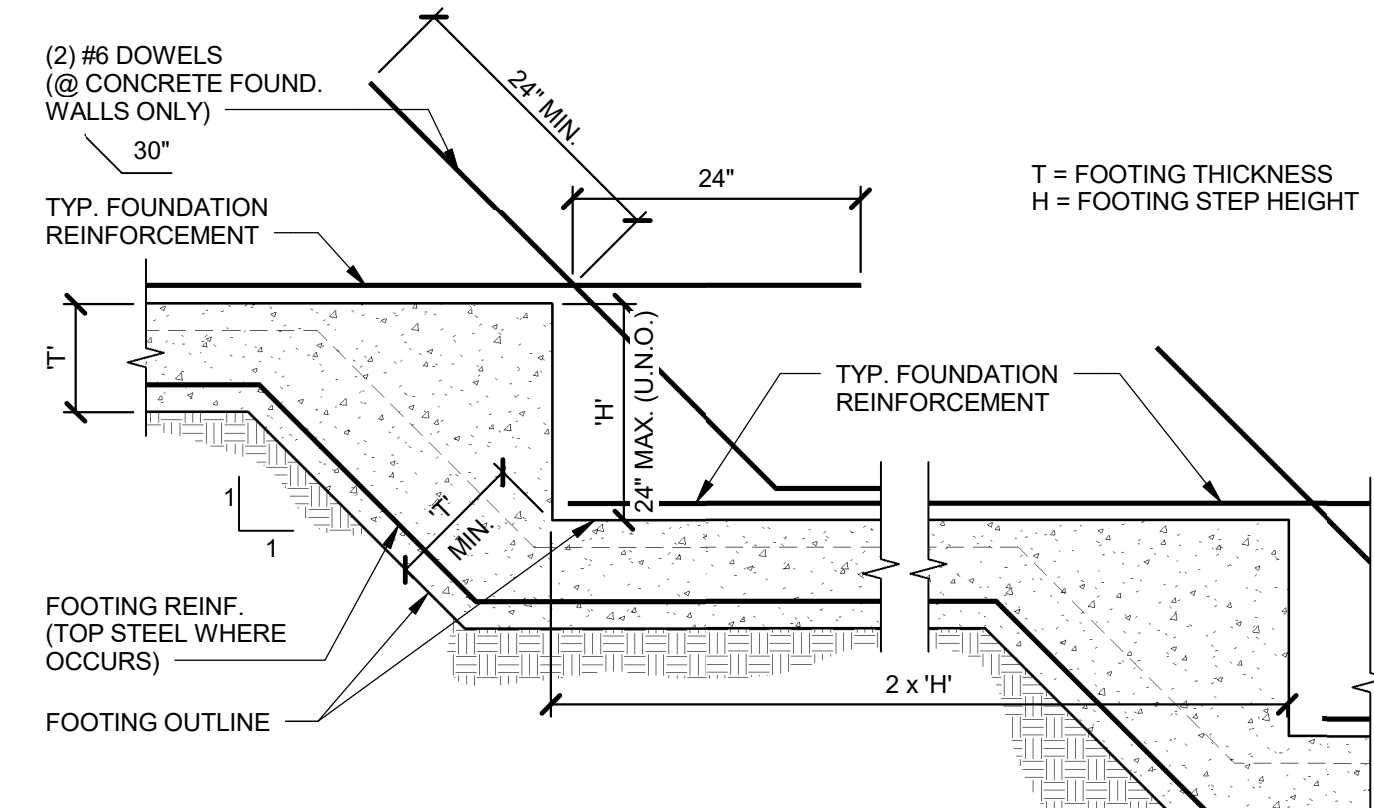
PIPING/CONDUIT THRU SLAB OR WALL



PIPING/CONDUIT IN SLAB OR WALL

TYP. PIPING/CONDUIT AT SLAB OR WALL
 SCALE: NONE

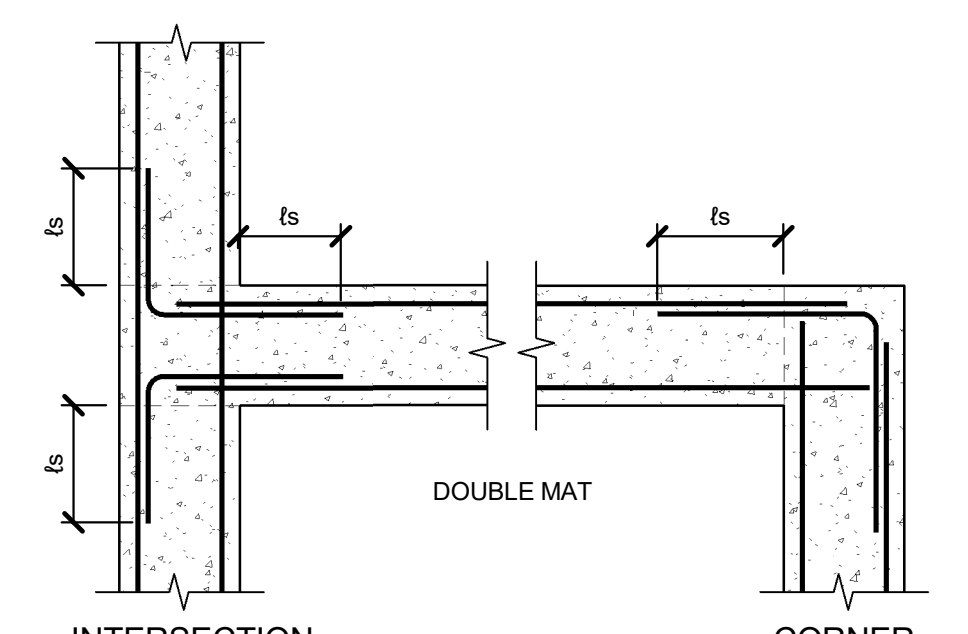
2
S201



TYPICAL STEPPED FOOTING

TYPICAL STEPPED FOOTING
 SCALE: NONE

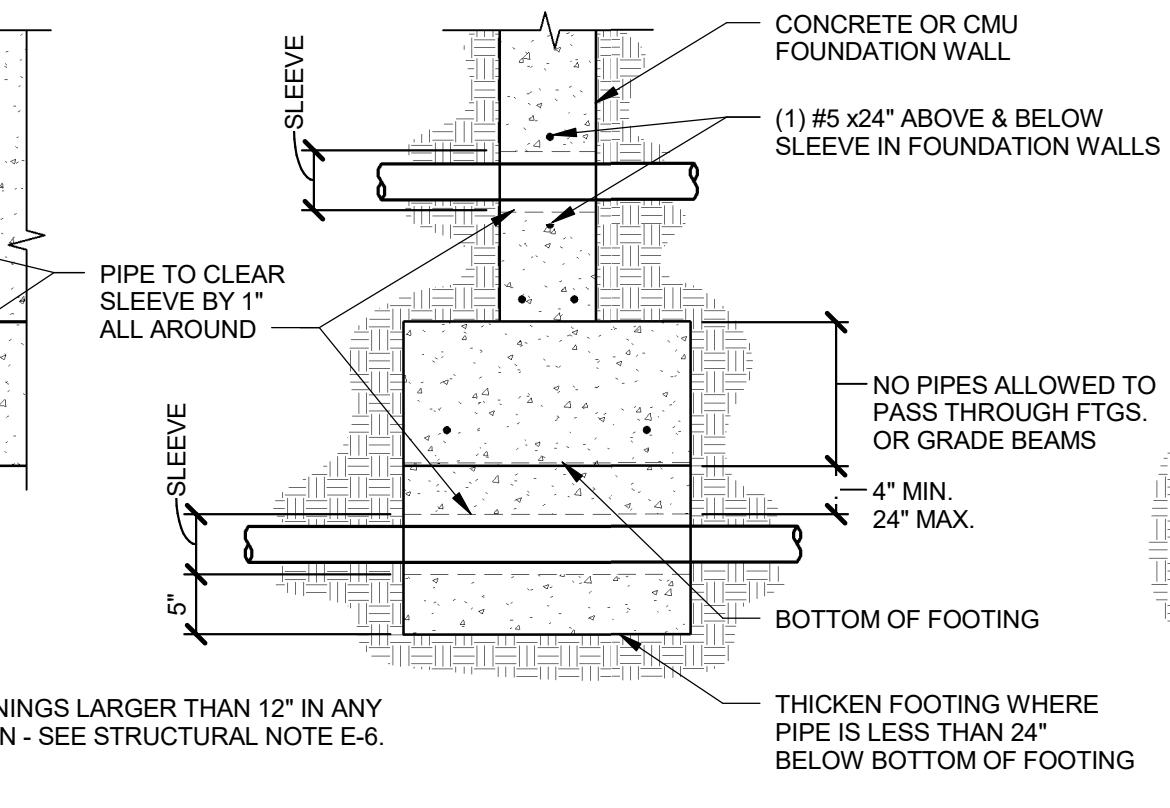
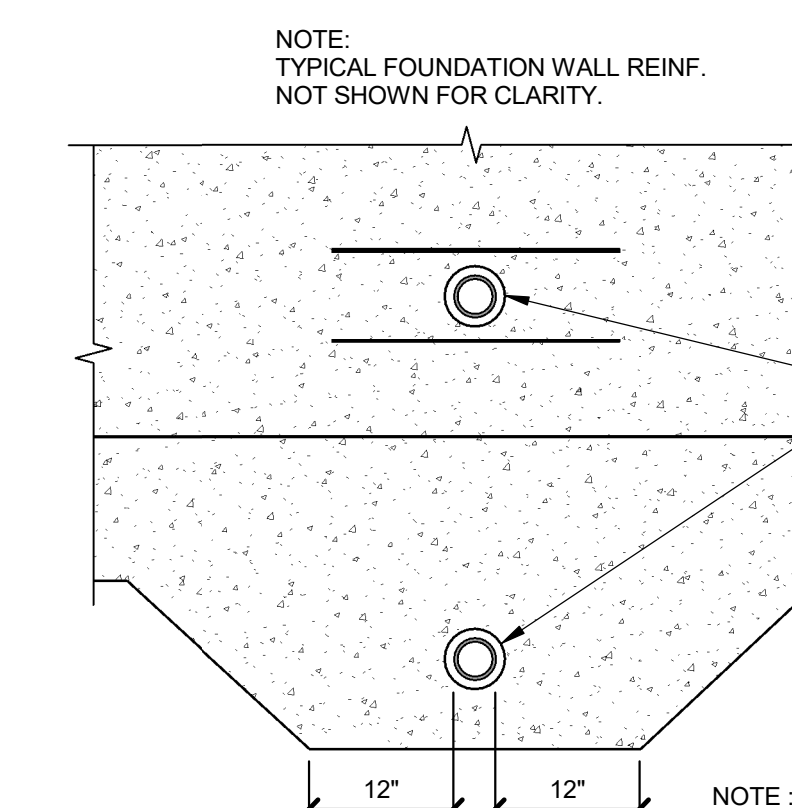
3
S201



TYP. REINF. @ INTERSECTIONS IN CONC. DETAIL

TYP. REINF. @ INTERSECTIONS IN CONC. DETAIL
 SCALE: NONE

4
S201

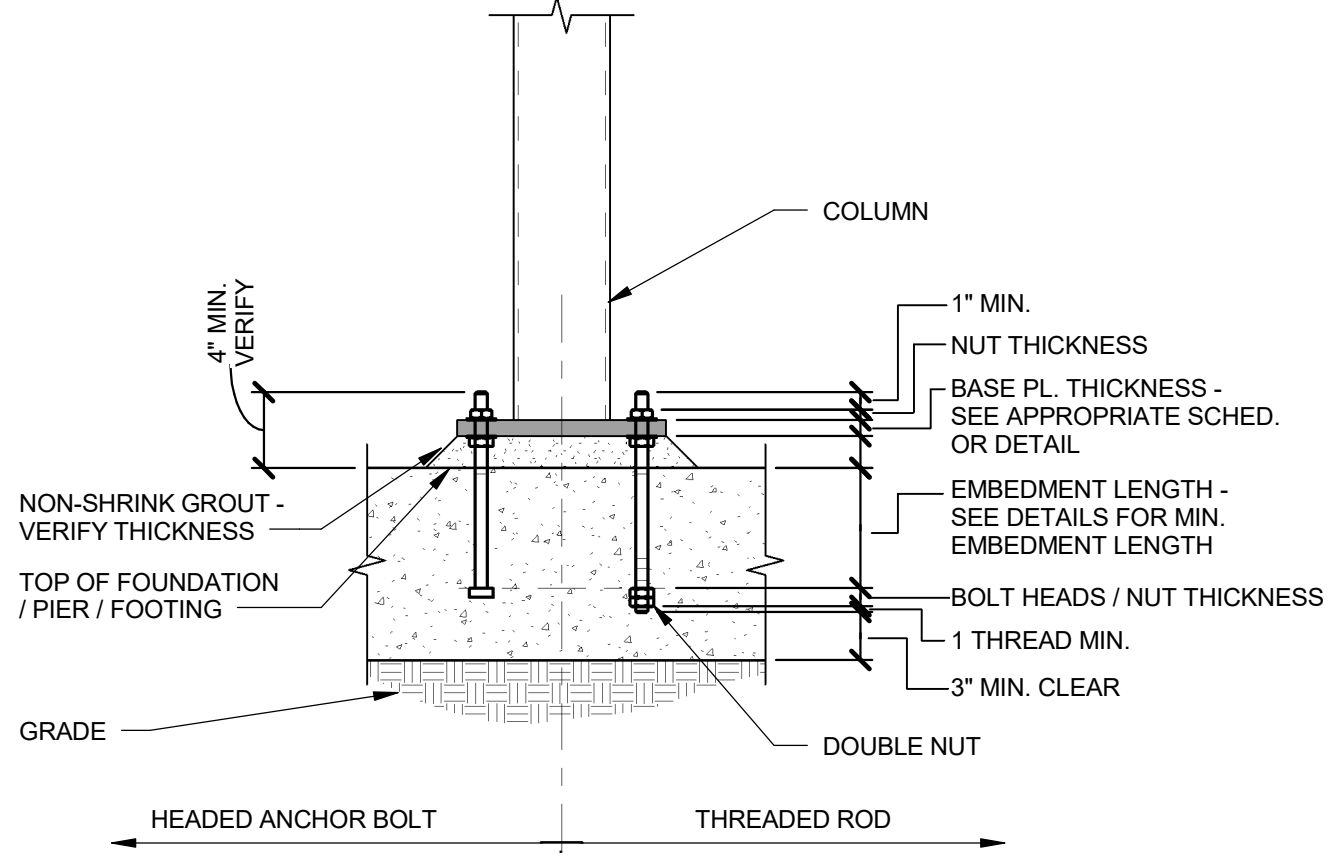


PIPE CROSSING FOOTING / FOUNDATION WALL

PIPE PARALLEL TO FOOTING

ALLOWABLE PIPING LOCATIONS @ FOOTING DETAIL
 SCALE: NONE

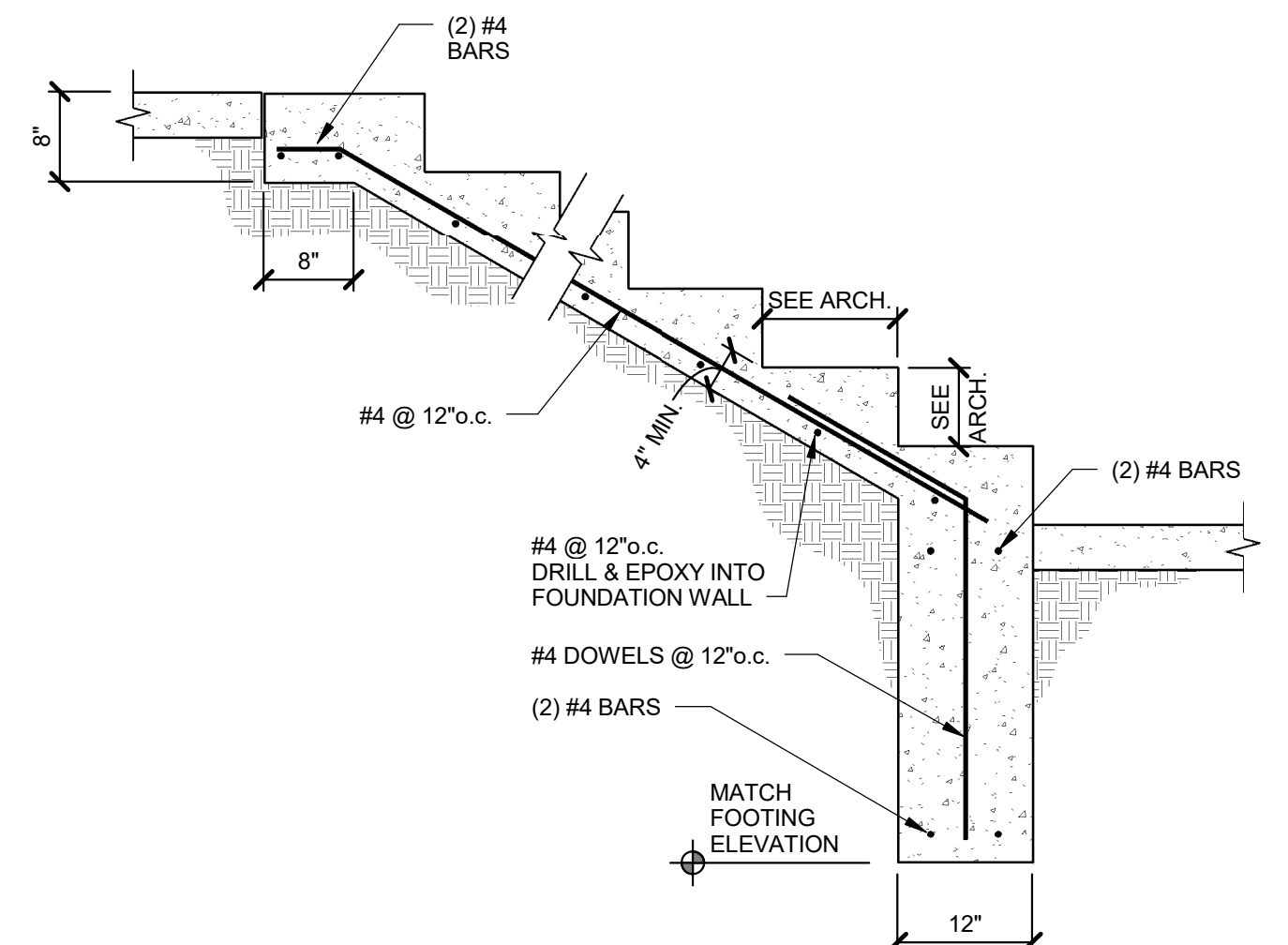
5
S201



TYPICAL ANCHOR BOLT EMBEDMENT DETAIL

TYPICAL ANCHOR BOLT EMBEDMENT DETAIL
 SCALE: NONE

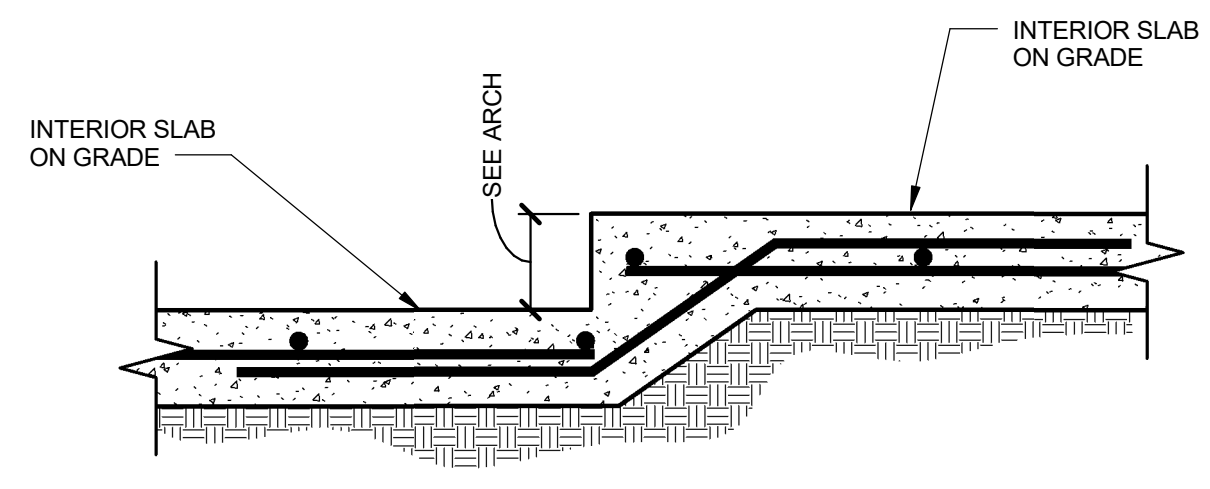
6
S201



TYPICAL STEPPED STAIR FOOTING

TYPICAL STEPPED STAIR FOOTING
 SCALE: NONE

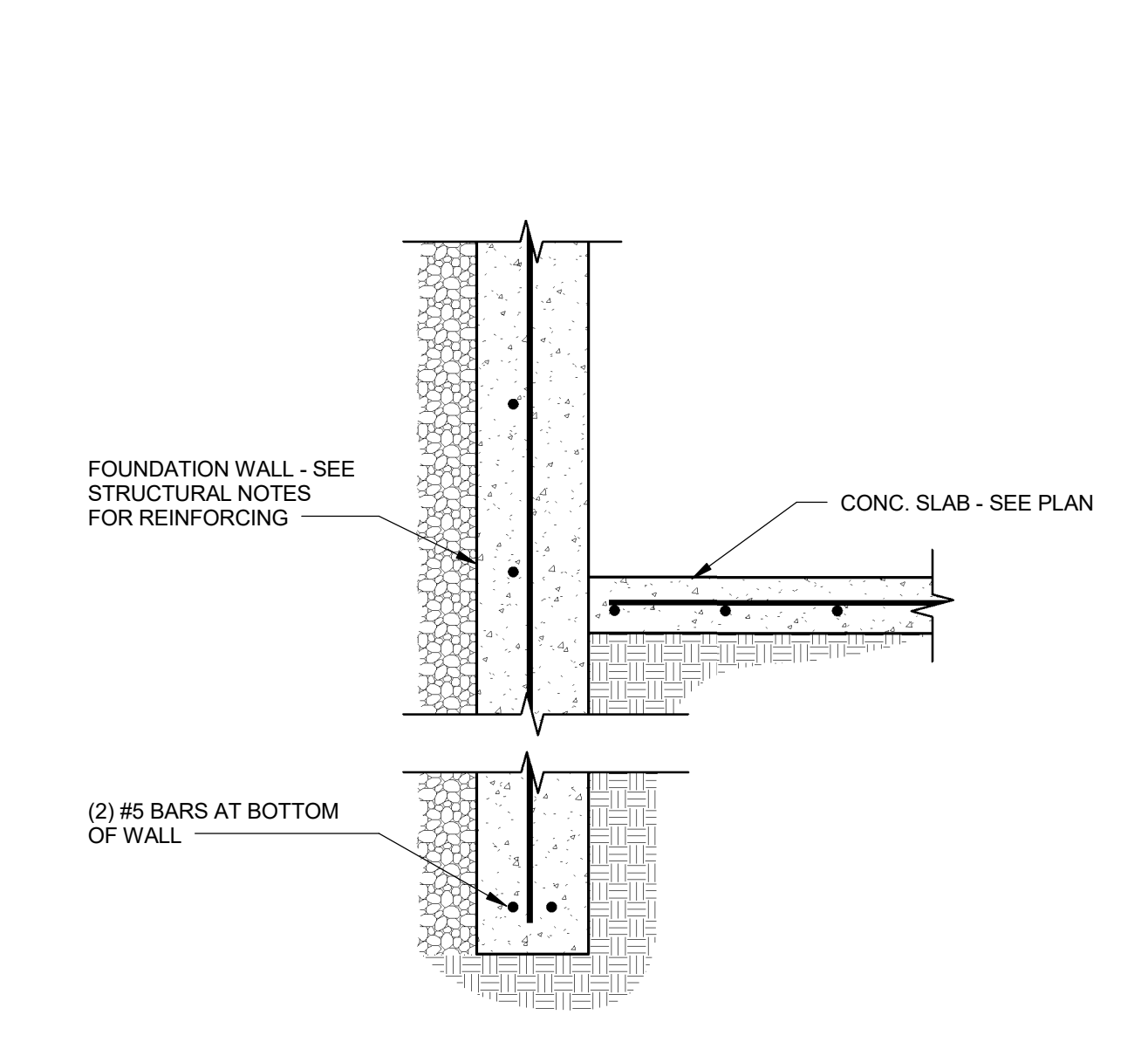
7
S201



TYPICAL RECESSED SLAB

TYPICAL RECESSED SLAB
 SCALE: NONE

8
S201



DETAIL
 SCALE: NONE

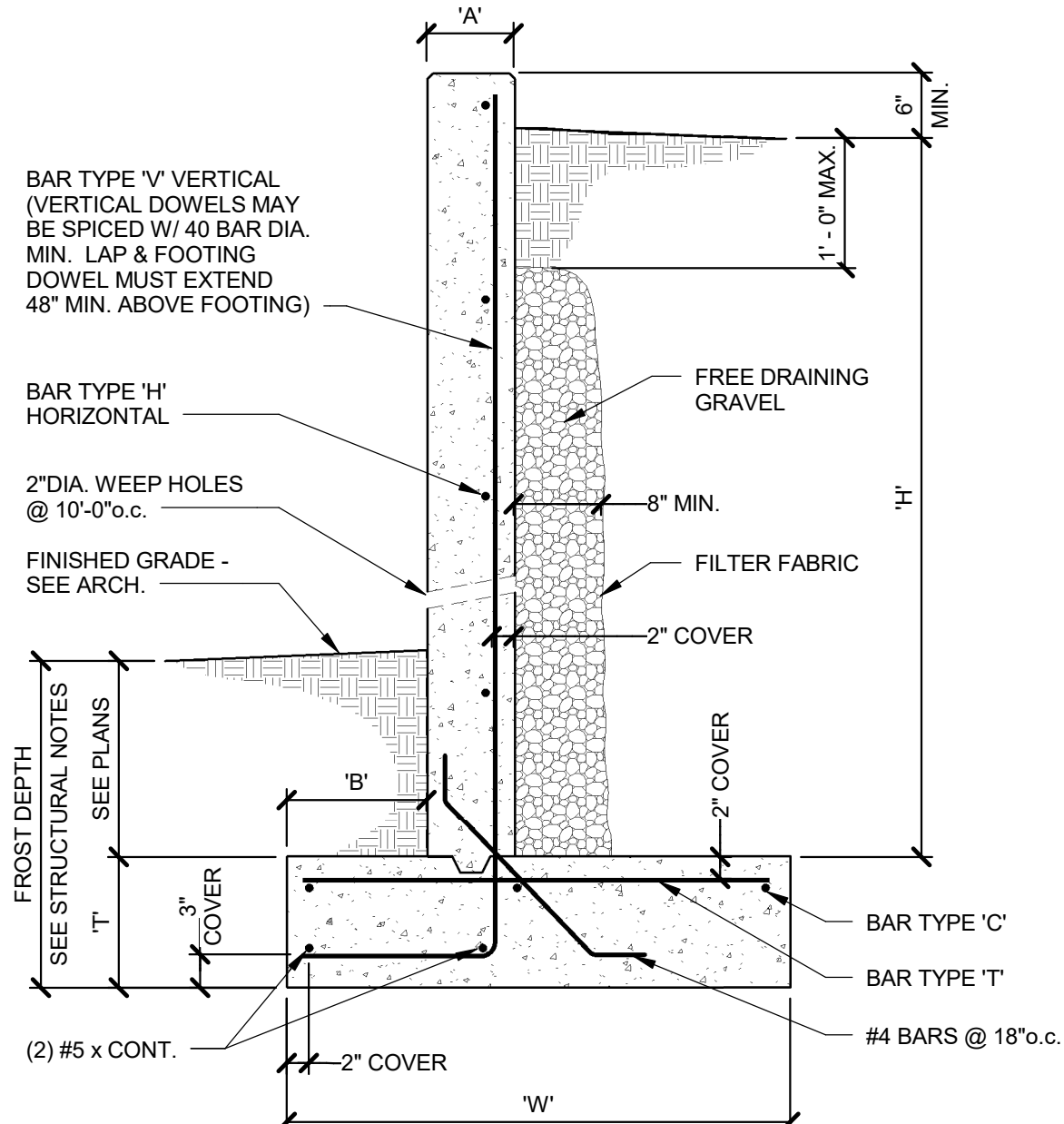
9
S201

NOTES:
 1. PROVIDE 3/4" DEEP VERTICAL CONTROL JOINT AT 15'-0" MAXIMUM SPACING. CONTINUE ALL HORIZONTAL REINFORCING THROUGH JOINT. PROVIDE MATCHING MASONRY (WHERE OCCURS) CONTROL JOINT.
 2. PROVIDE EXPANSION JOINTS @ 48'-0" MAX. SPACING. 3/4" PRE-MOLDED JOINT MATERIAL.
 3. EXTEND HORIZONTAL REINFORCING AROUND CORNERS OR ADD CORNER BARS AND LAP EACH WAY. SEE REBAR LAP SCHEDULE.
 4. ALLOW CONCRETE TO REACH 100% OF DESIGN STRENGTH (f_c) PRIOR TO BACKFILLING.
 5. PROVIDE 2" DIA. PVC PIPE WEEP HOLES @ 10'-0" WITH NON-FERROUS SCREEN AND GRAVEL BACKING. CONTINUOUS PERFORATED FOUNDATION DRAIN LINE TIED TO STORM DRAIN SYSTEM IS AN ACCEPTABLE ALTERNATE TO WEEP HOLES.
 6. PROVIDE FILTER FABRIC BETWEEN GRANULAR BACKFILL AND BACK SIDE OF WALL. FILTER FABRIC TO BE FREE DRAINING WITHOUT ALLOWING INFILTRATION OF FINE SOILS.
 7. EVERY OTHER VERTICAL BAR MAY BE DISCONTINUED AT 1/2 ABOVE FOOTING FOR 8'-0" AND 10'-0" HIGH WALLS. 1/3 OF VERTICAL BARS MAY BE DISCONTINUED AT 0.2H + 30 BAR DIAMETERS AND 0.4H + 30 BAR DIAMETERS ABOVE FOOTING. SPACING OF VERTICAL BARS SHALL BE 18" MAX. FOR 11'-0" THRU 14'-0" HIGH WALLS.

DIMENSION 'H'	DIMENSION			BAR TYPE 'H'		BAR TYPE 'V'		BAR TYPE 'C'		BAR TYPE 'T'	
	T	A	B	SIZE	SPACING	SIZE	SPACING	SIZE	NUMBER	SIZE	SPACING
H ≤ 3'-0"	12"	8"	6"	#4	12" o.c.	#4	16" o.c.	#5	---	---	---
3'-1" - 5'-0"	12"	8"	9"	#4	12" o.c.	#4	16" o.c.	#4	(3)	#4	16" o.c.
5'-1" - 7'-0"	12"	8"	18"	#4	12" o.c.	#5	16" o.c.	#4	(5)	#5	16" o.c.

TYPICAL UNRESTRAINED RETAINING WALL DETAIL
 SCALE: NONE

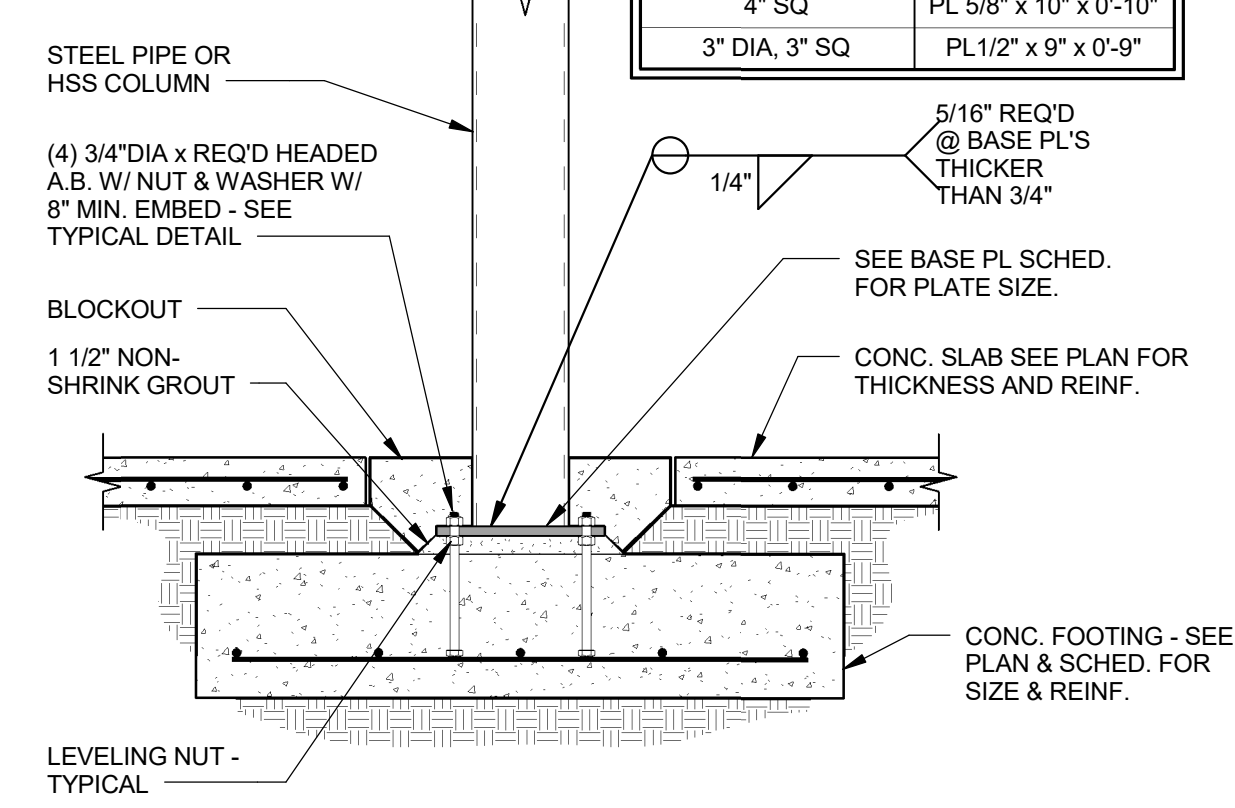
10
S201



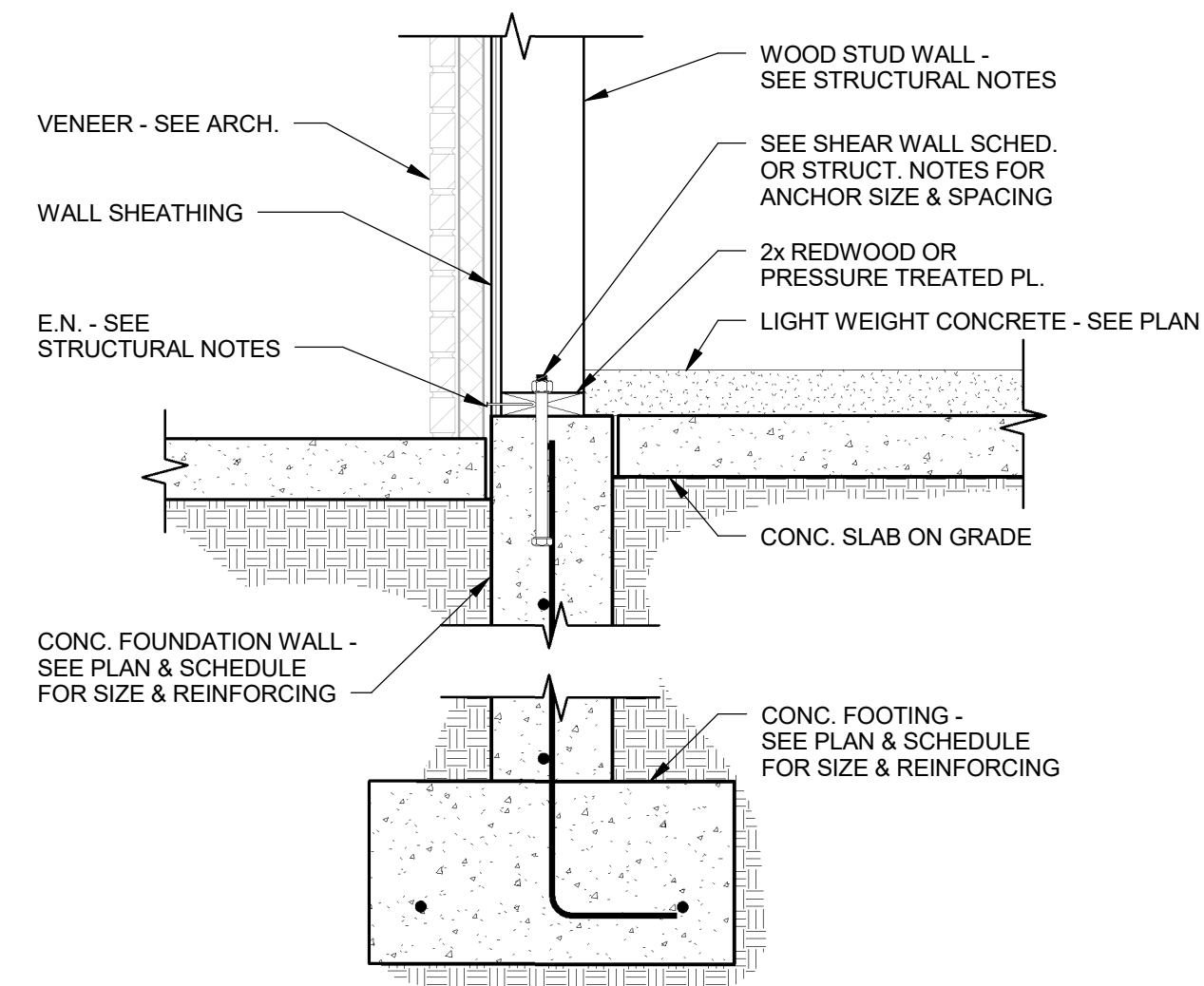
DETAIL
 SCALE: NONE

11
S201

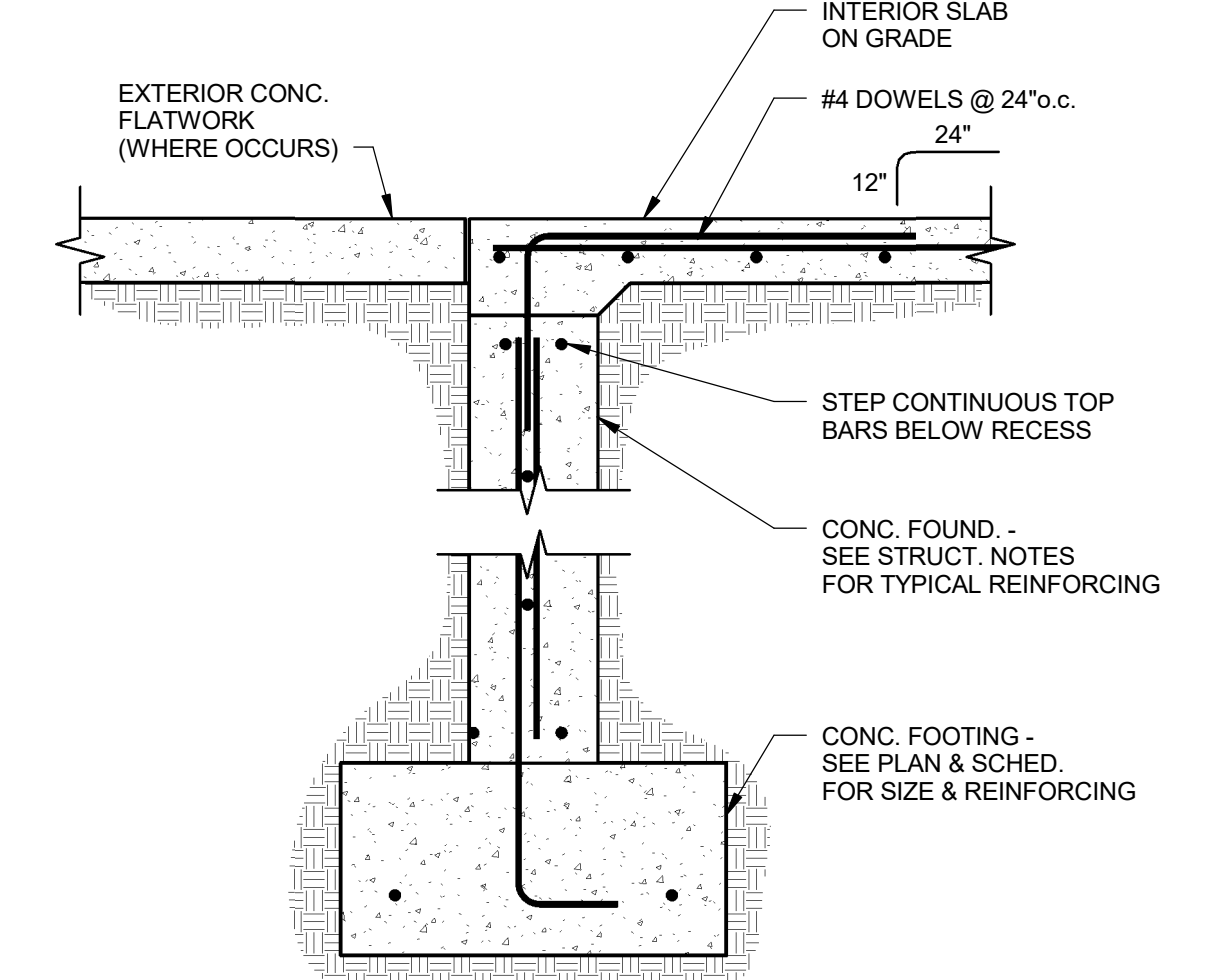
BASE PLATE SCHEDULE	
COLUMN SIZE	BASE PLATE SIZE
8" SQ, 10" DIA	PL 3/4" x 14" x 1'-2"
7" SQ	PL 3/4" x 13" x 1'-1"
6" SQ, 8" DIA	PL 3/4" x 12" x 1'-0"
5" SQ	PL 3/4" x 12" x 1'-0"
4" SQ	PL 5/8" x 10" x 0'-10"
3" DIA, 3" SQ	PL 1/2" x 9" x 0'-9"



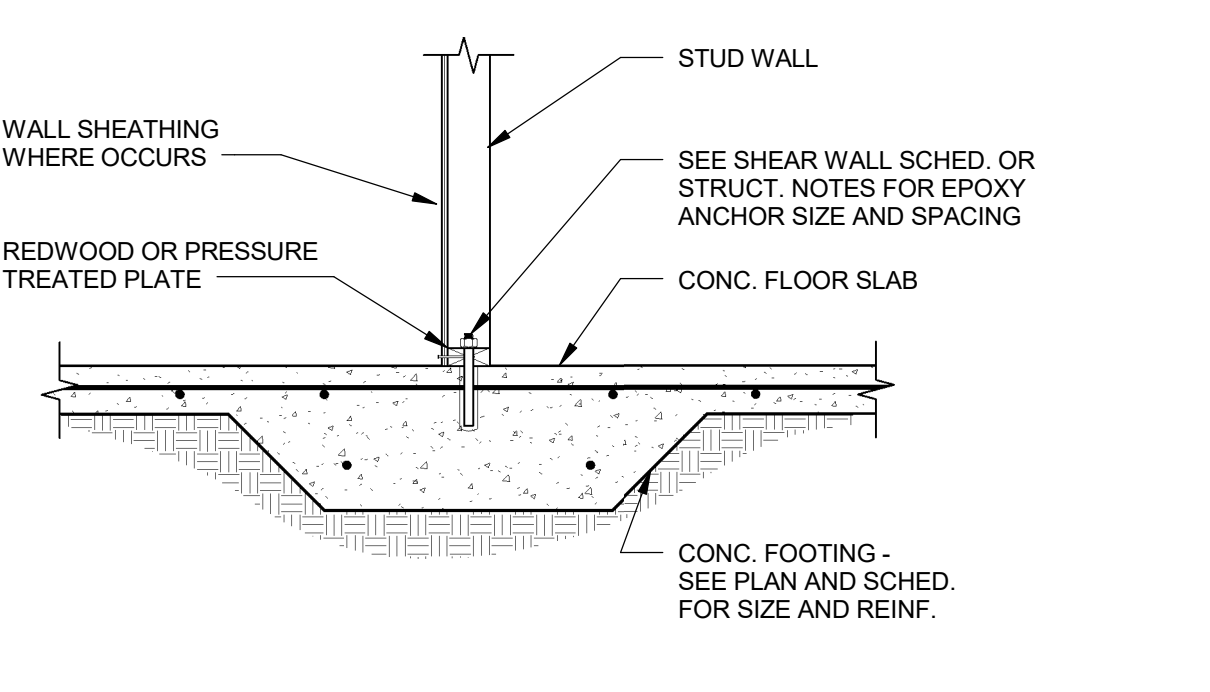
1
COLUMN TO SPOT FOOTING
SCALE: NONE
S202



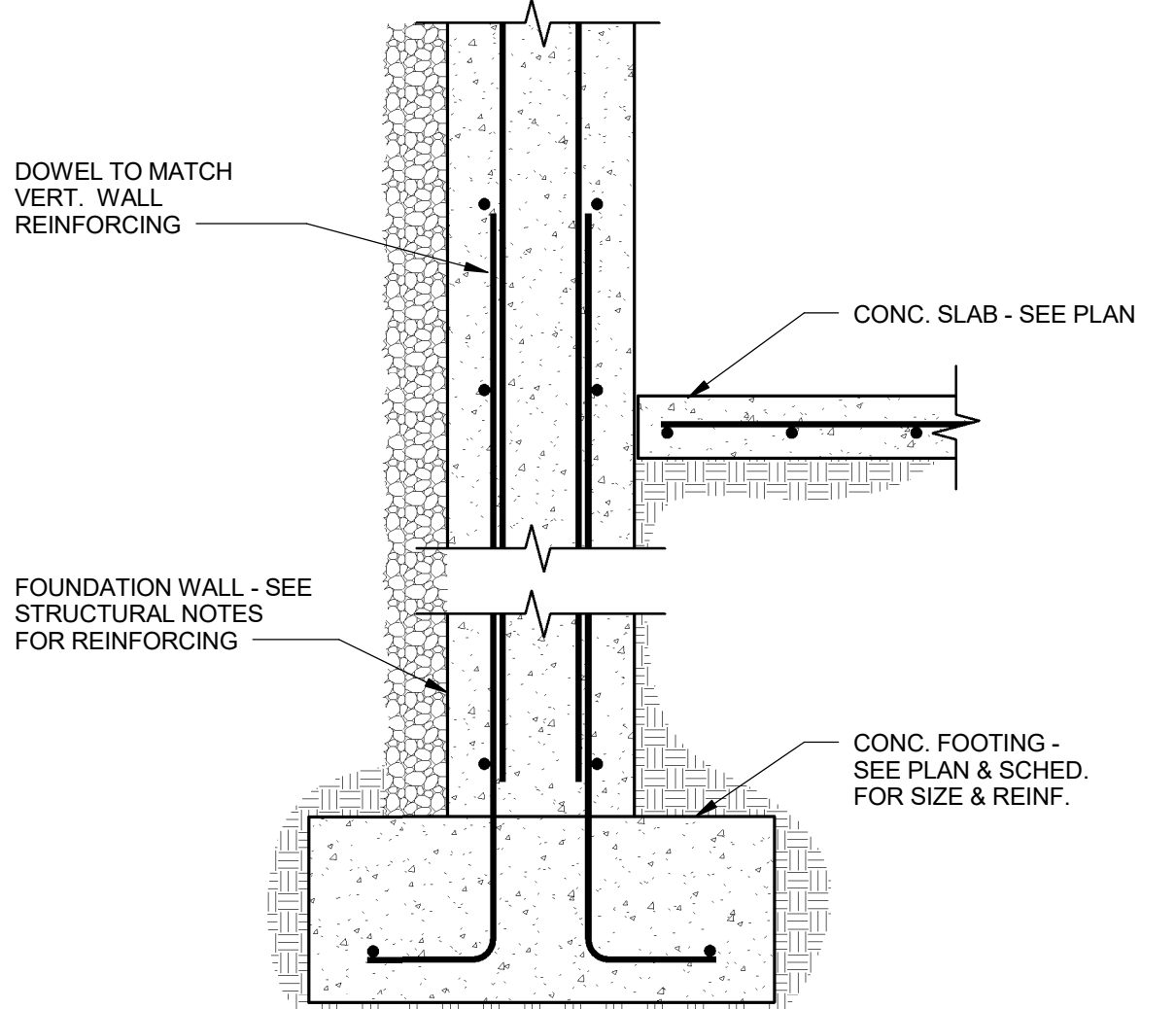
2
SECTION @ EXTERIOR TIMBER
SCALE: NONE
S202



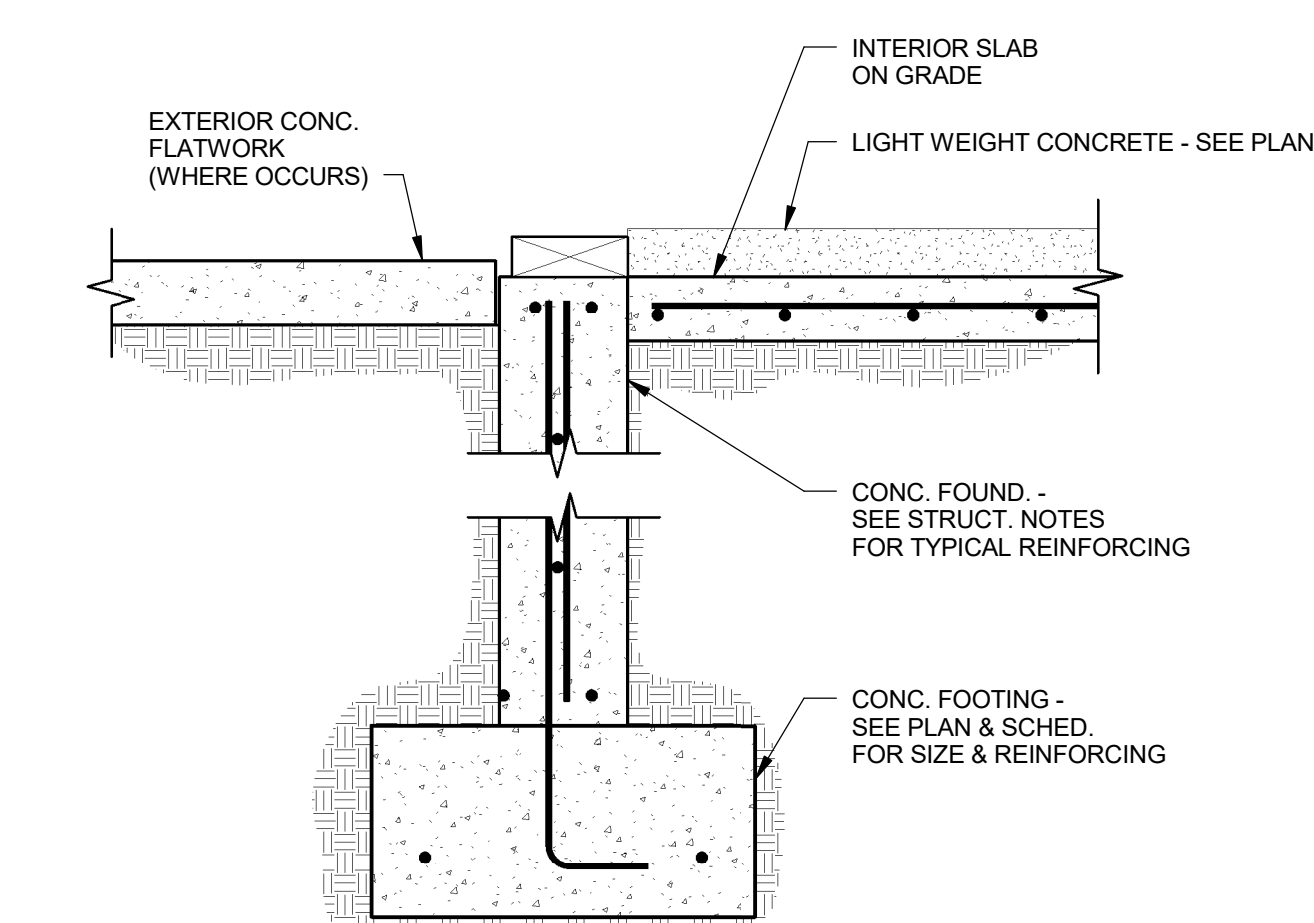
3
CONCRETE FOUNDATION @ OPENING
DETAIL
SCALE: NONE
S202



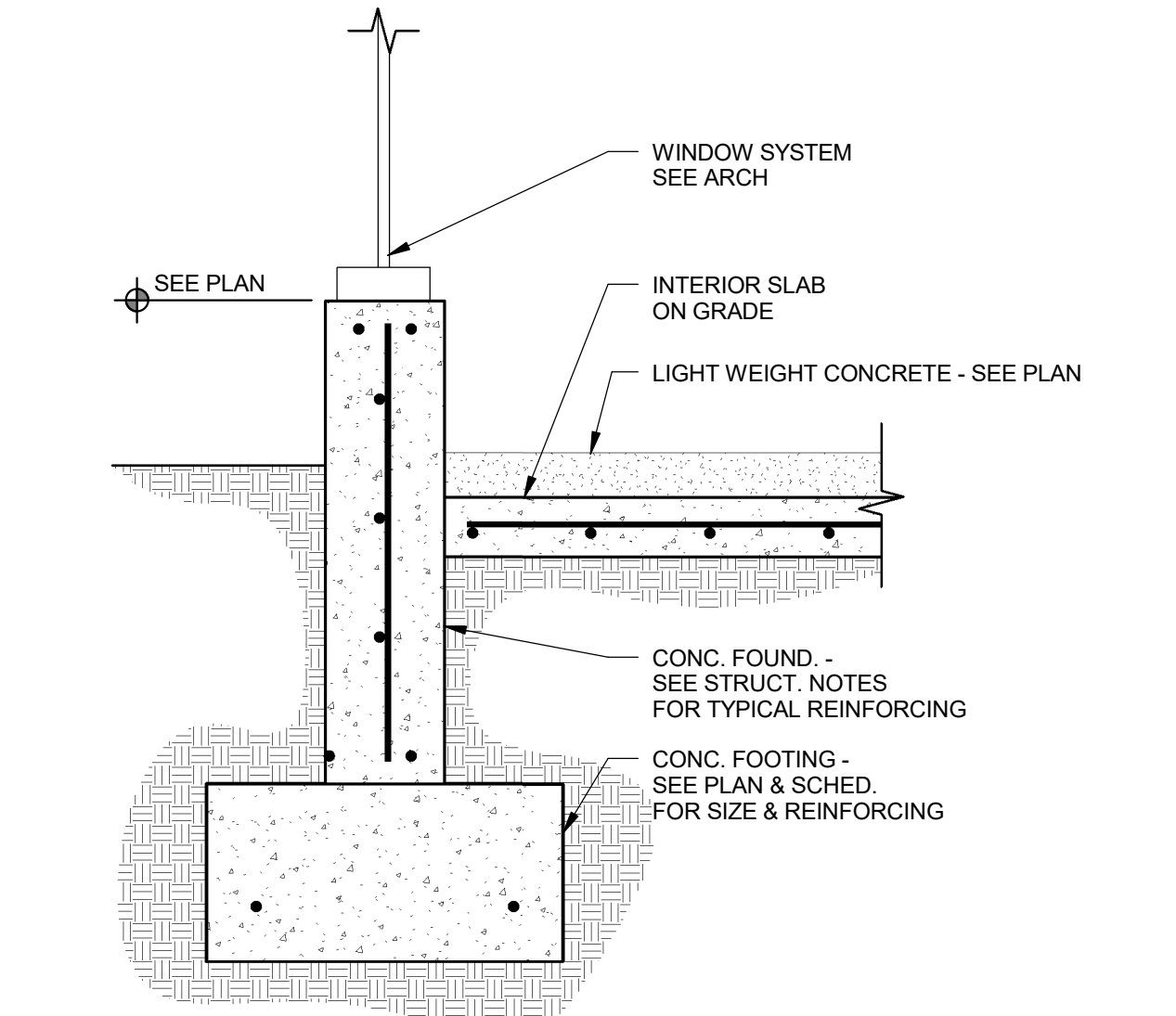
4
STUD WALL ON THICKENED SLAB
SCALE: NONE
S202



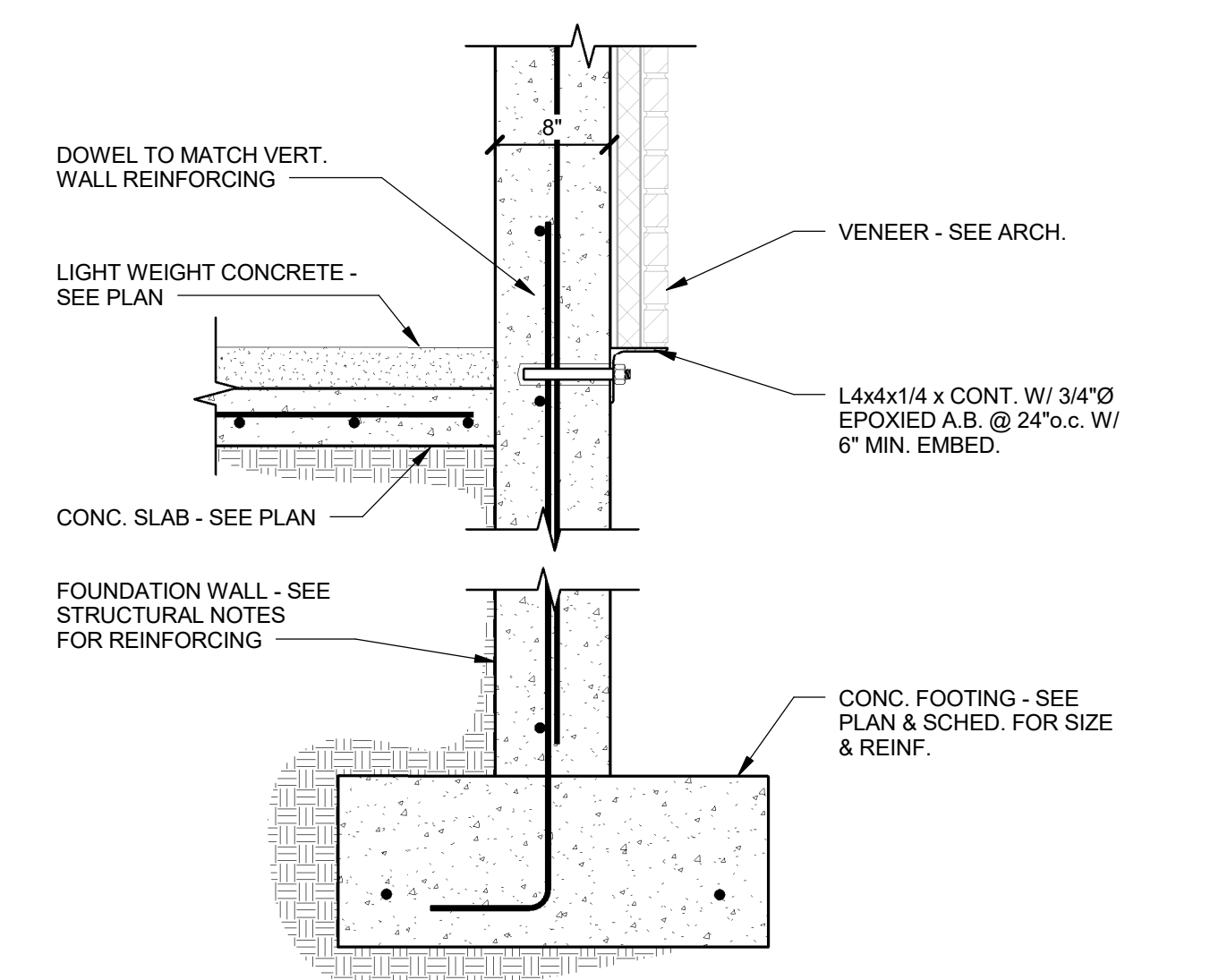
5
DETAIL
SCALE: NONE
S202



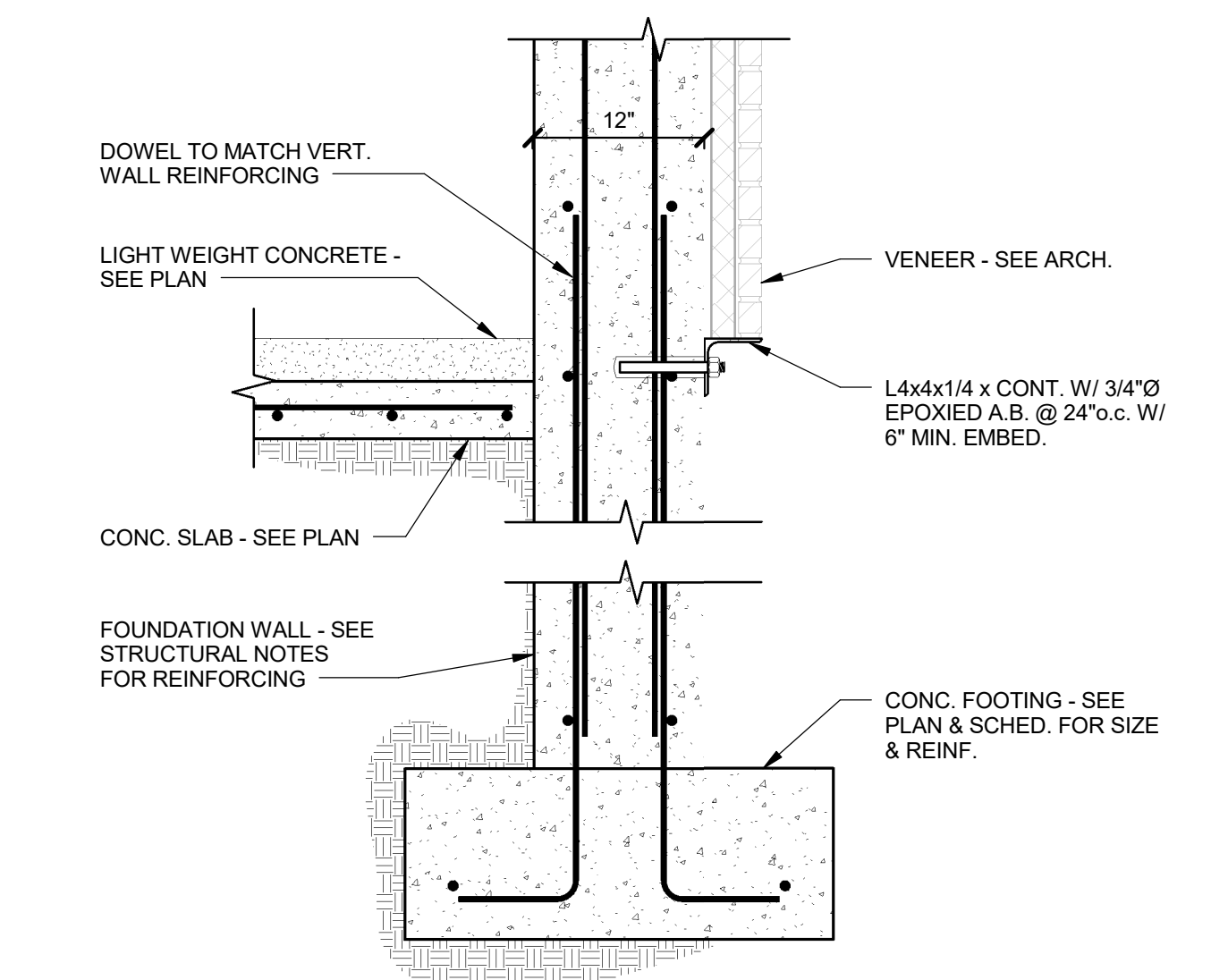
6
DETAIL
SCALE: NONE
S202



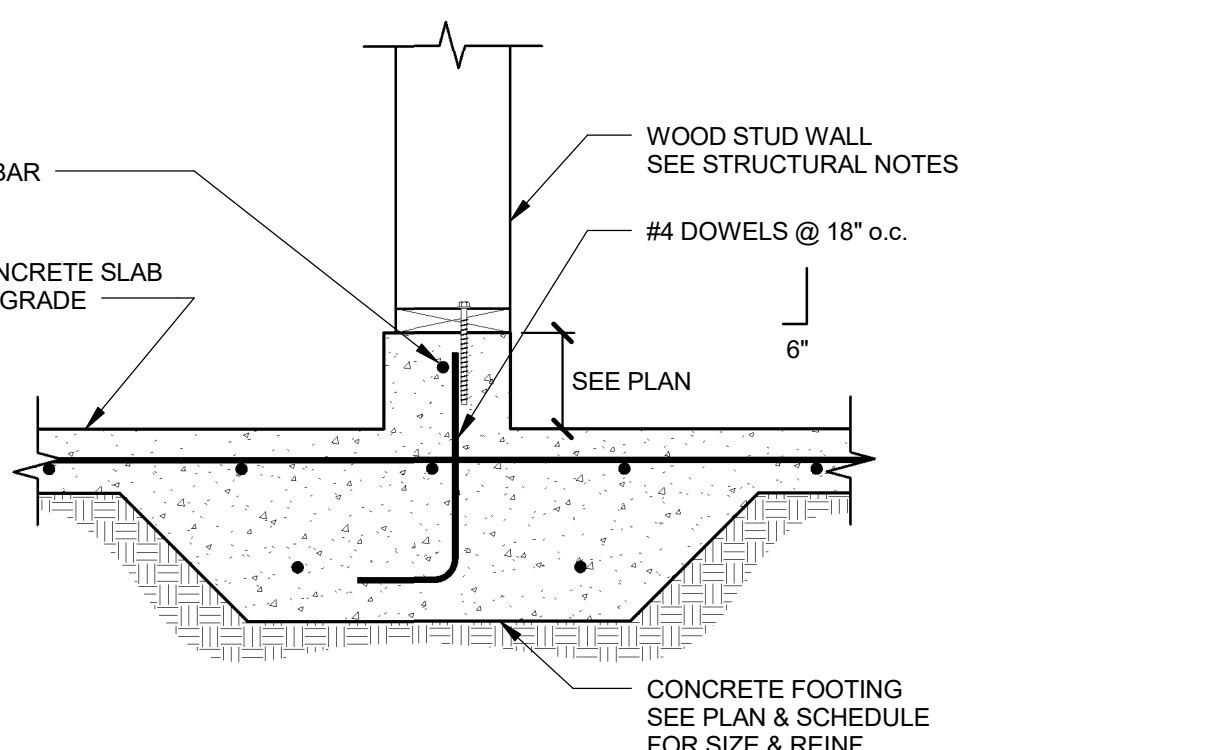
7
DETAIL
SCALE: NONE
S202



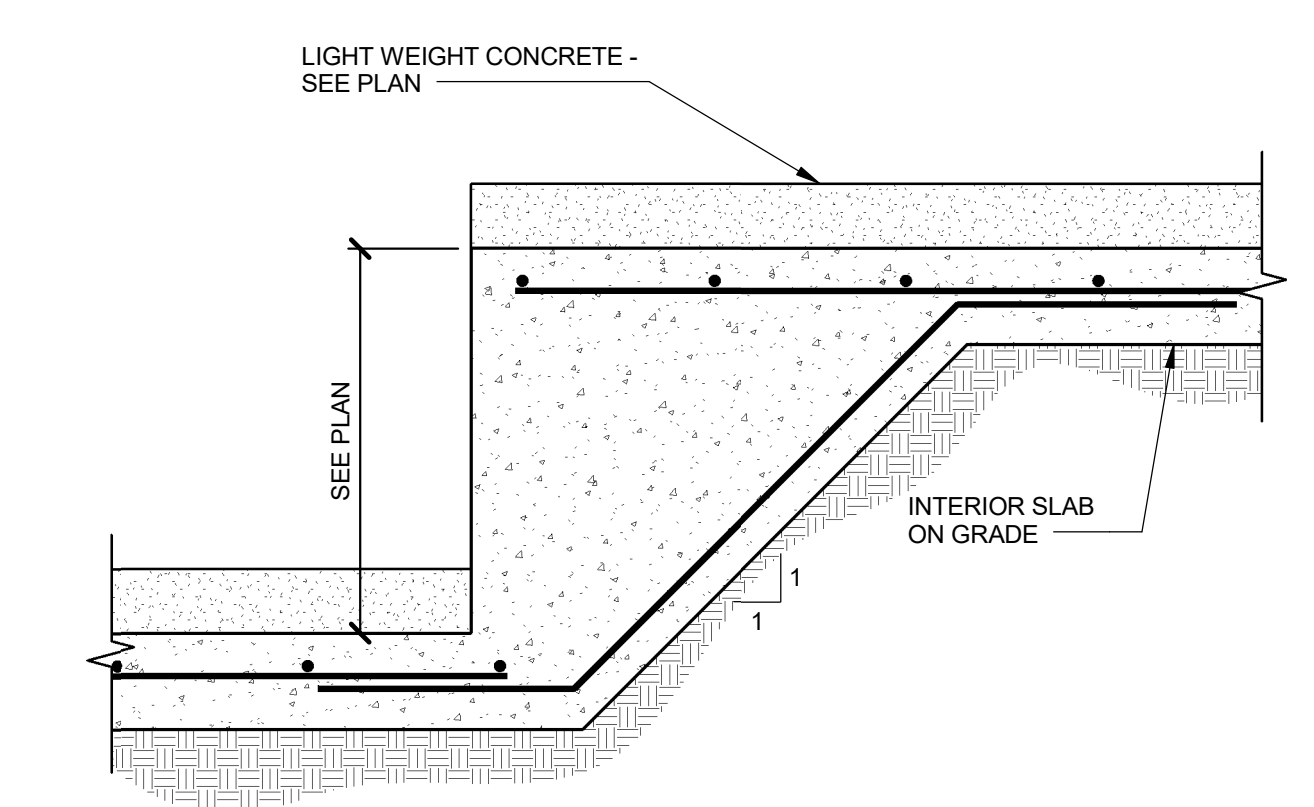
8
DETAIL
SCALE: NONE
S202



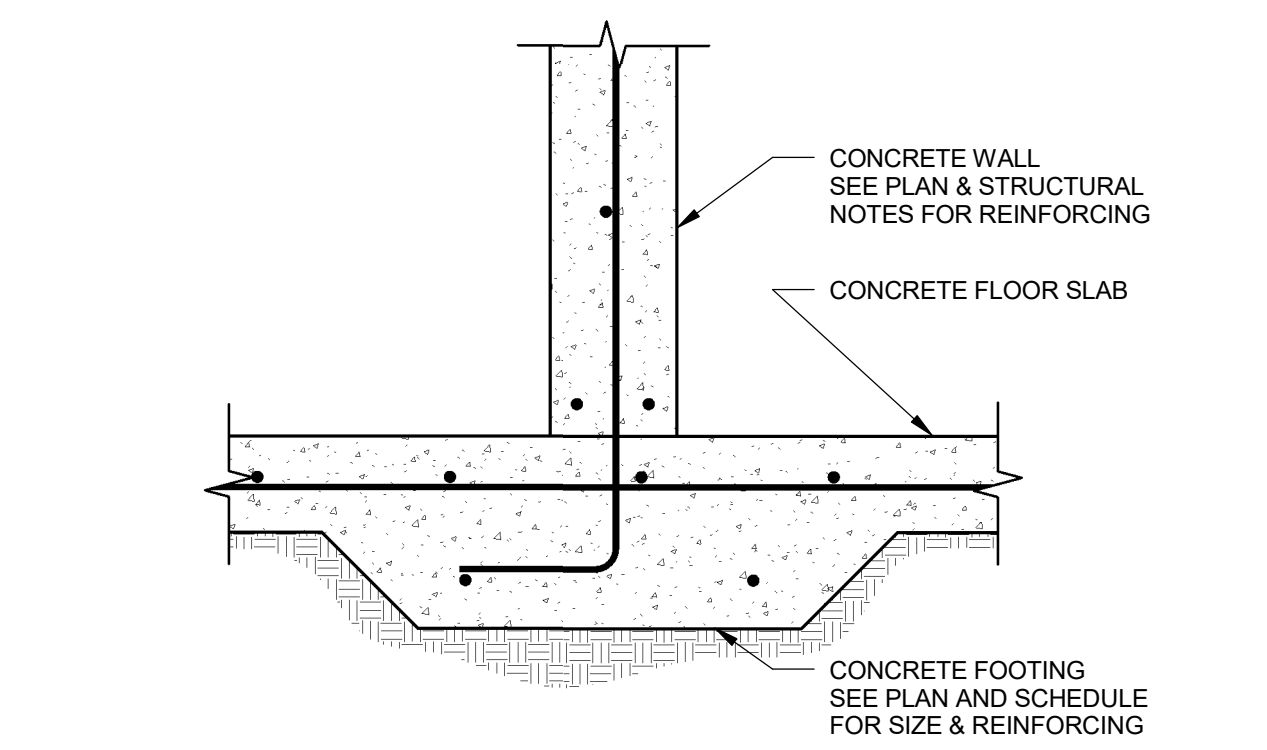
9
DETAIL
SCALE: NONE
S202



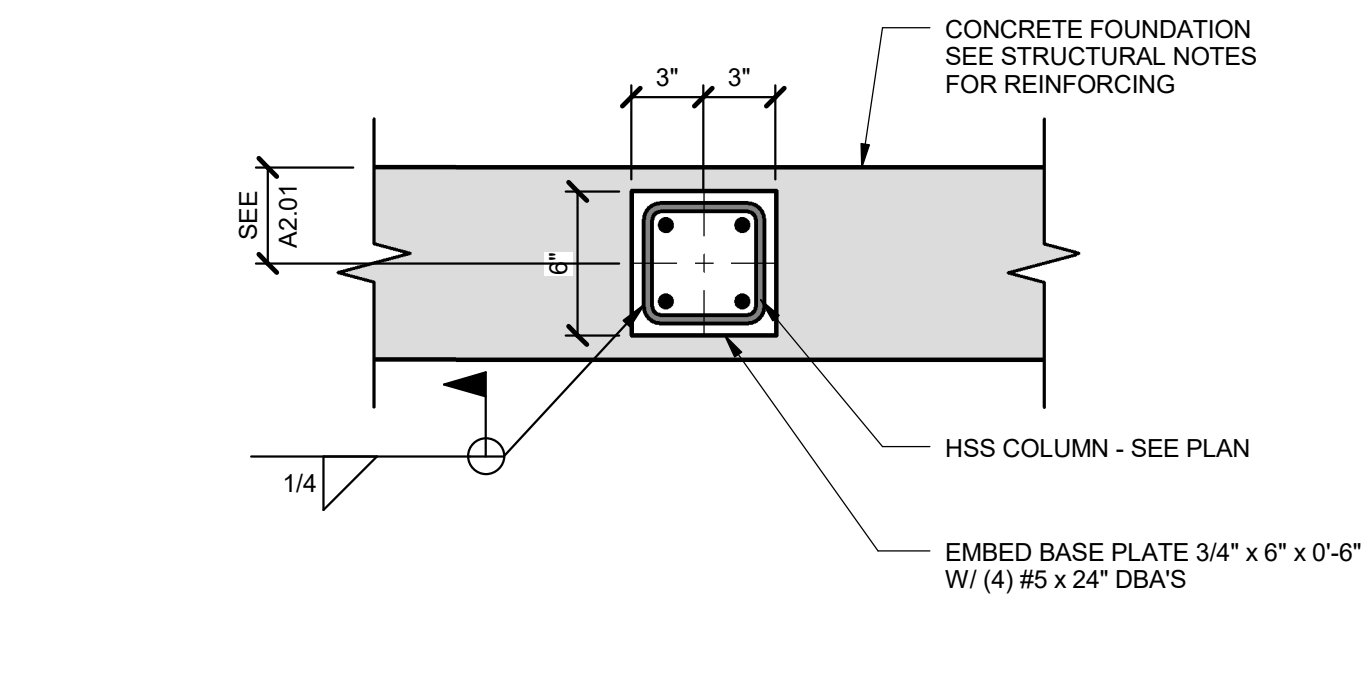
10
DETAIL
SCALE: NONE
S202



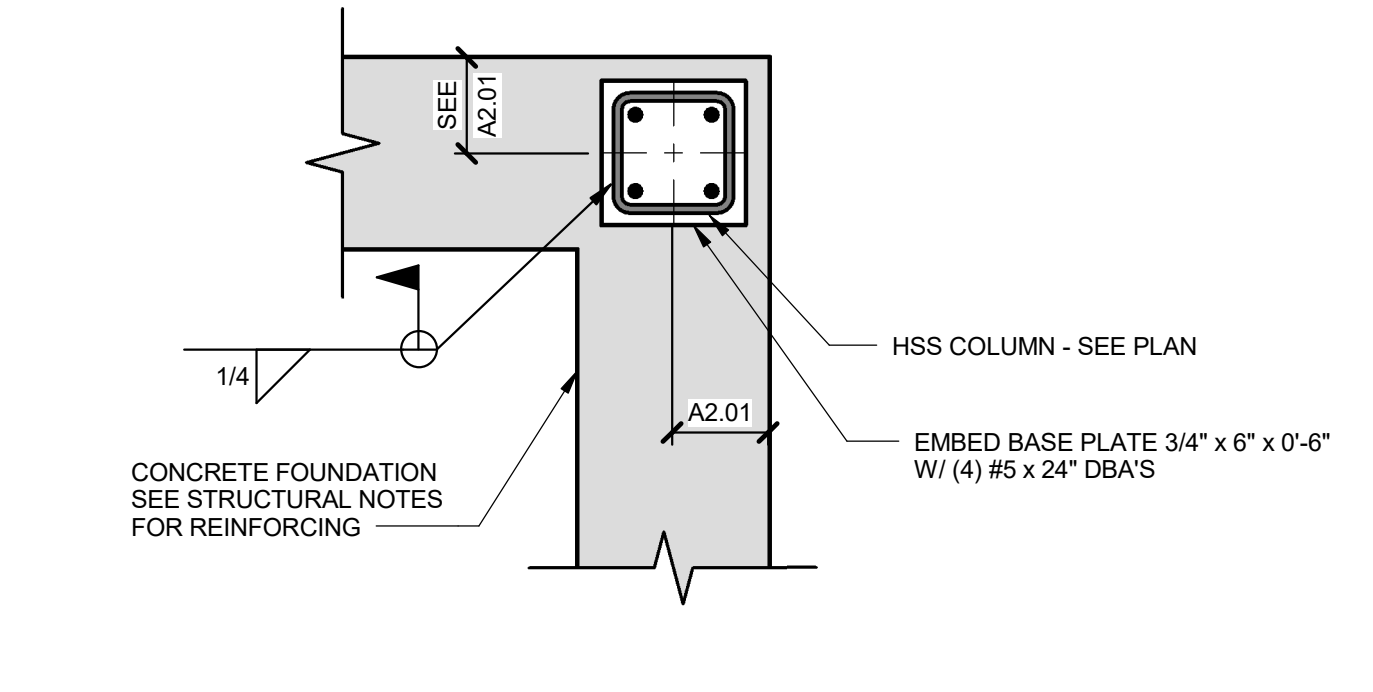
11
DETAIL
SCALE: NONE
S202



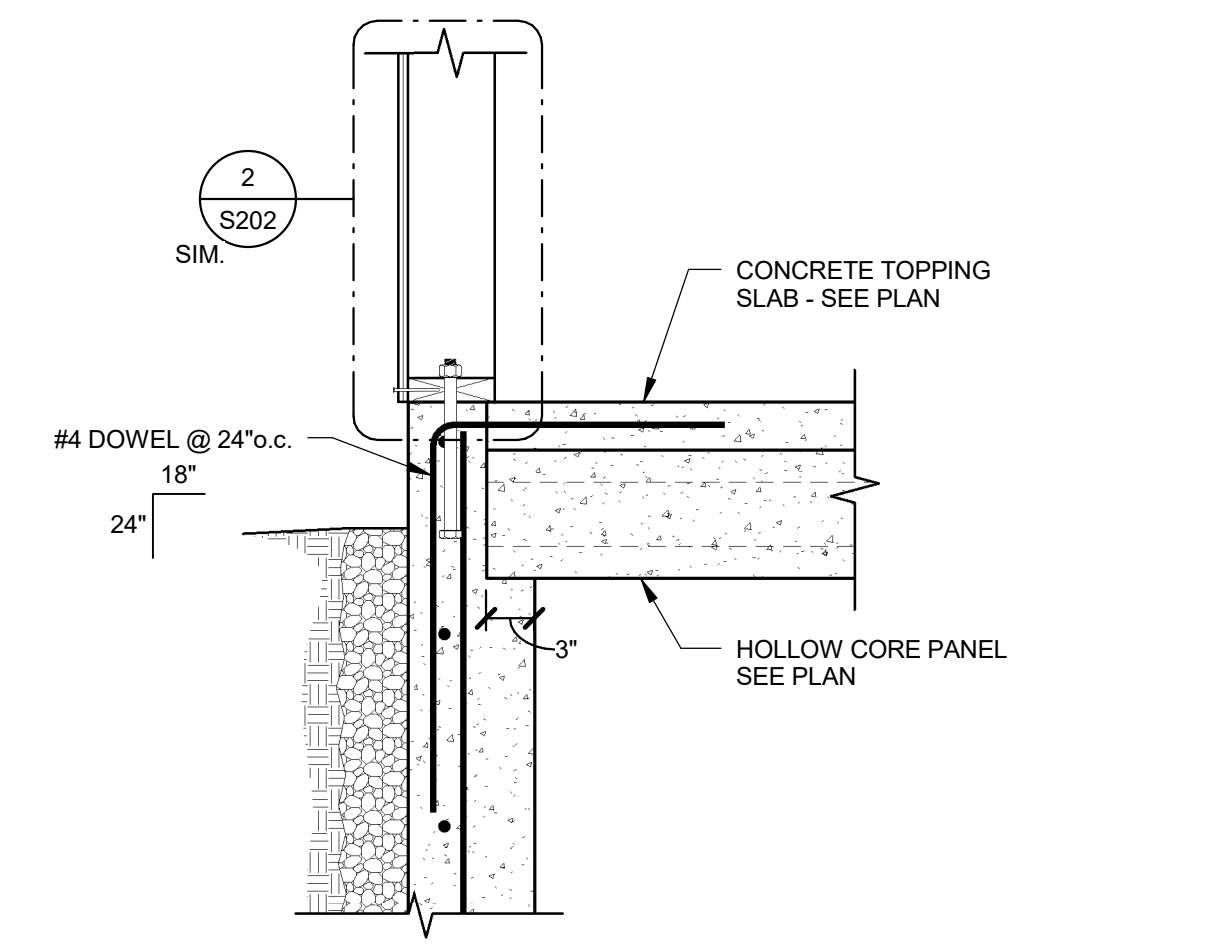
12
DETAIL
SCALE: NONE
S202



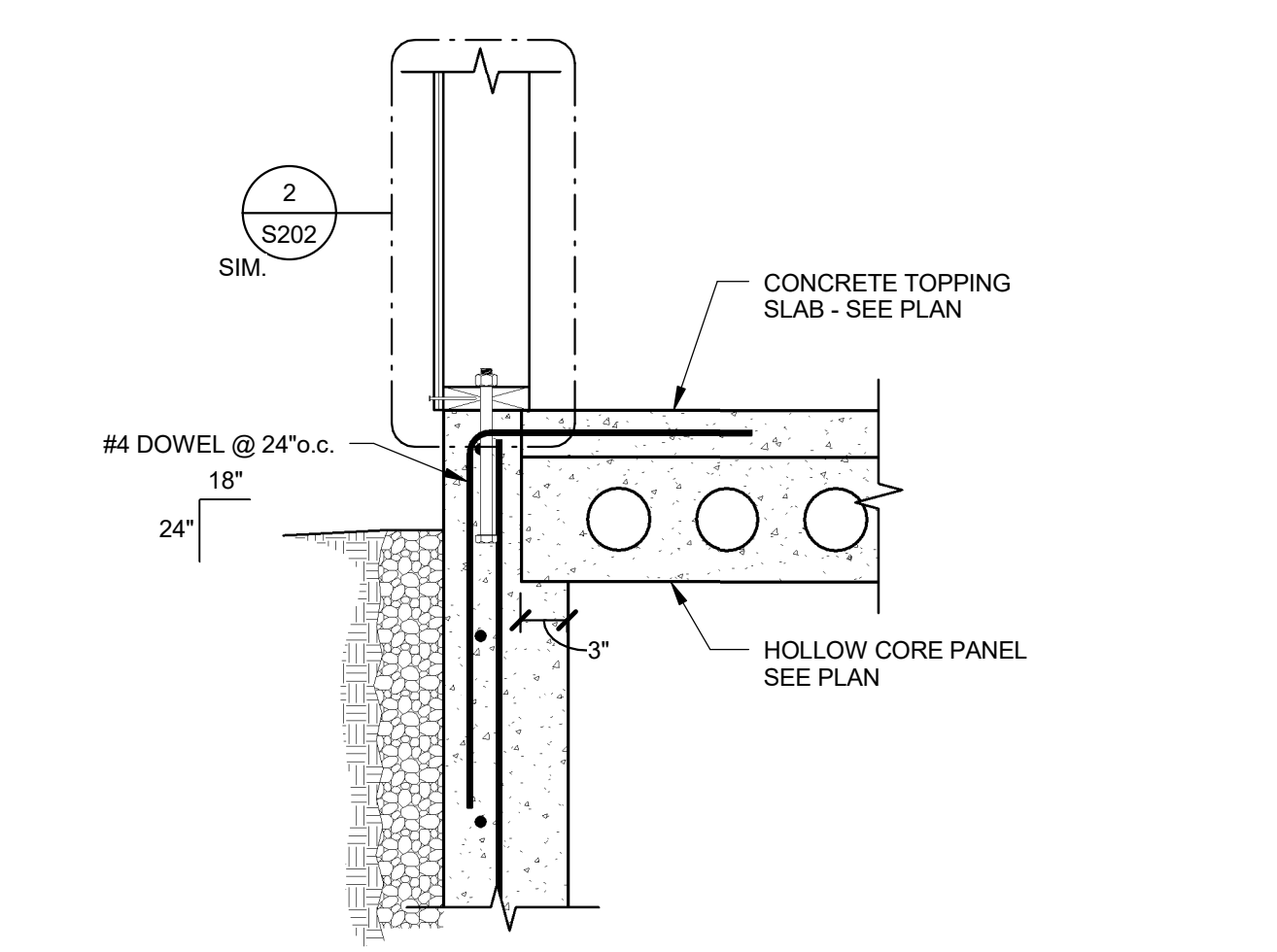
13
DETAIL
SCALE: NONE
S202



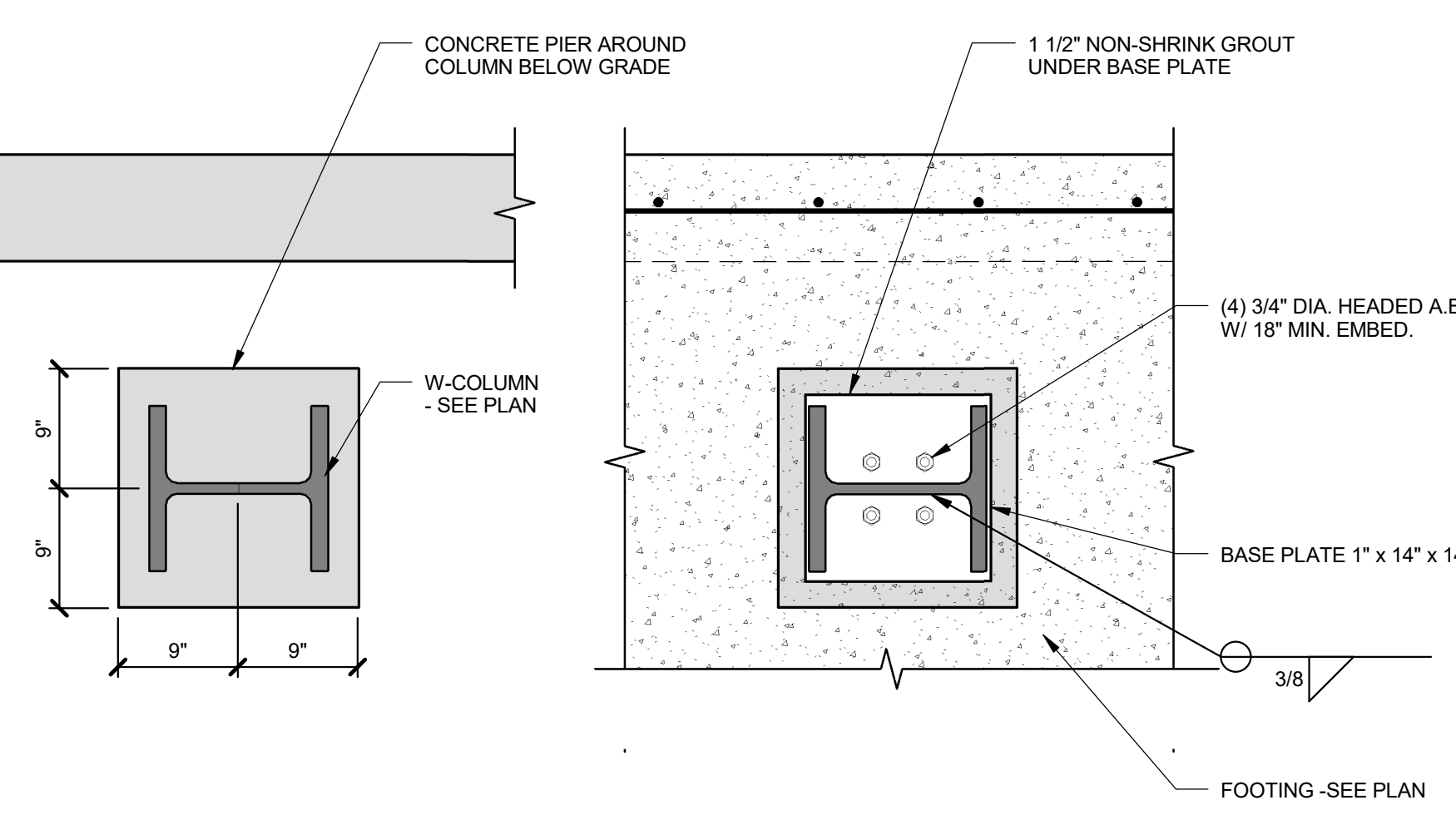
14
DETAIL
SCALE: NONE
S202



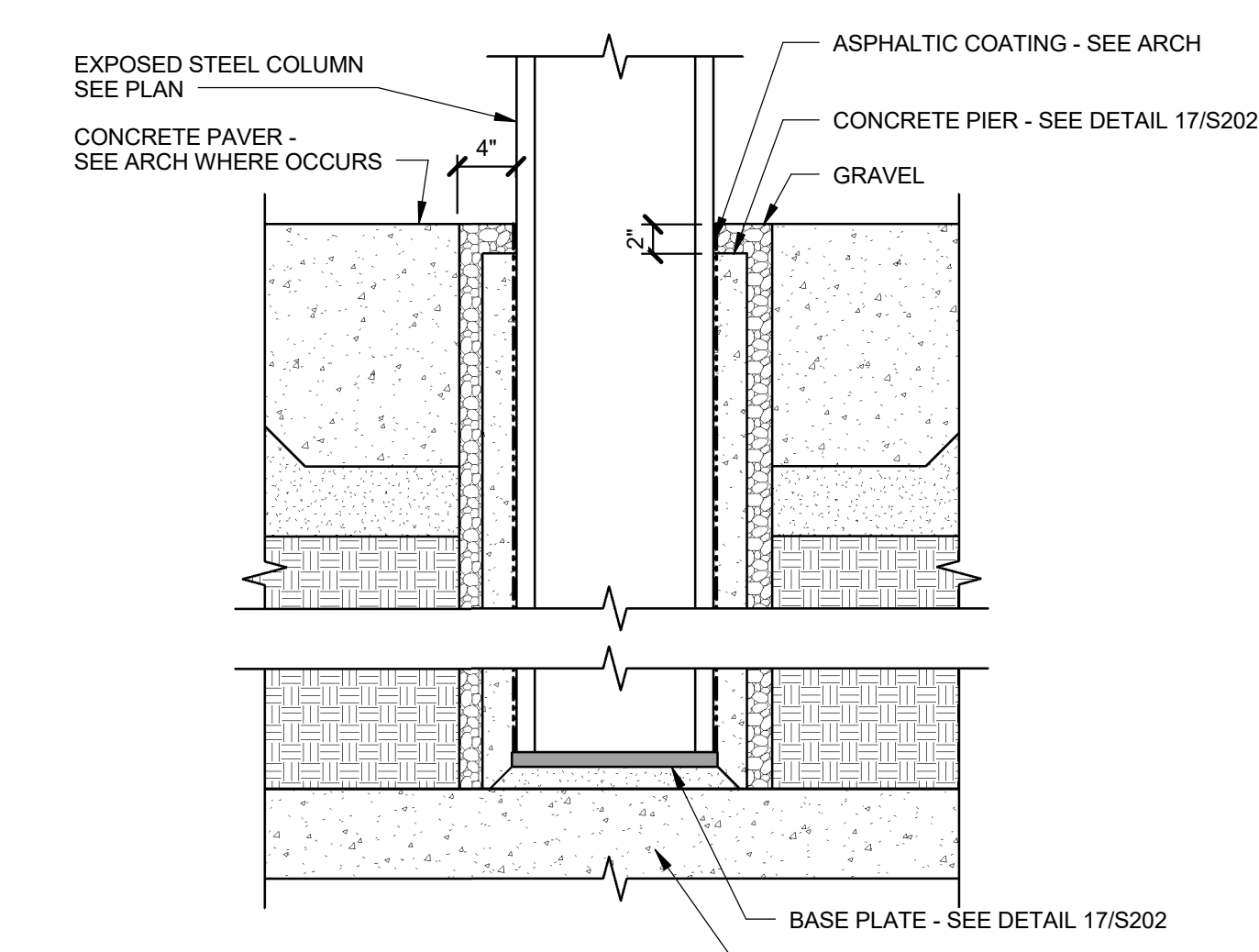
15
DETAIL
SCALE: NONE
S202



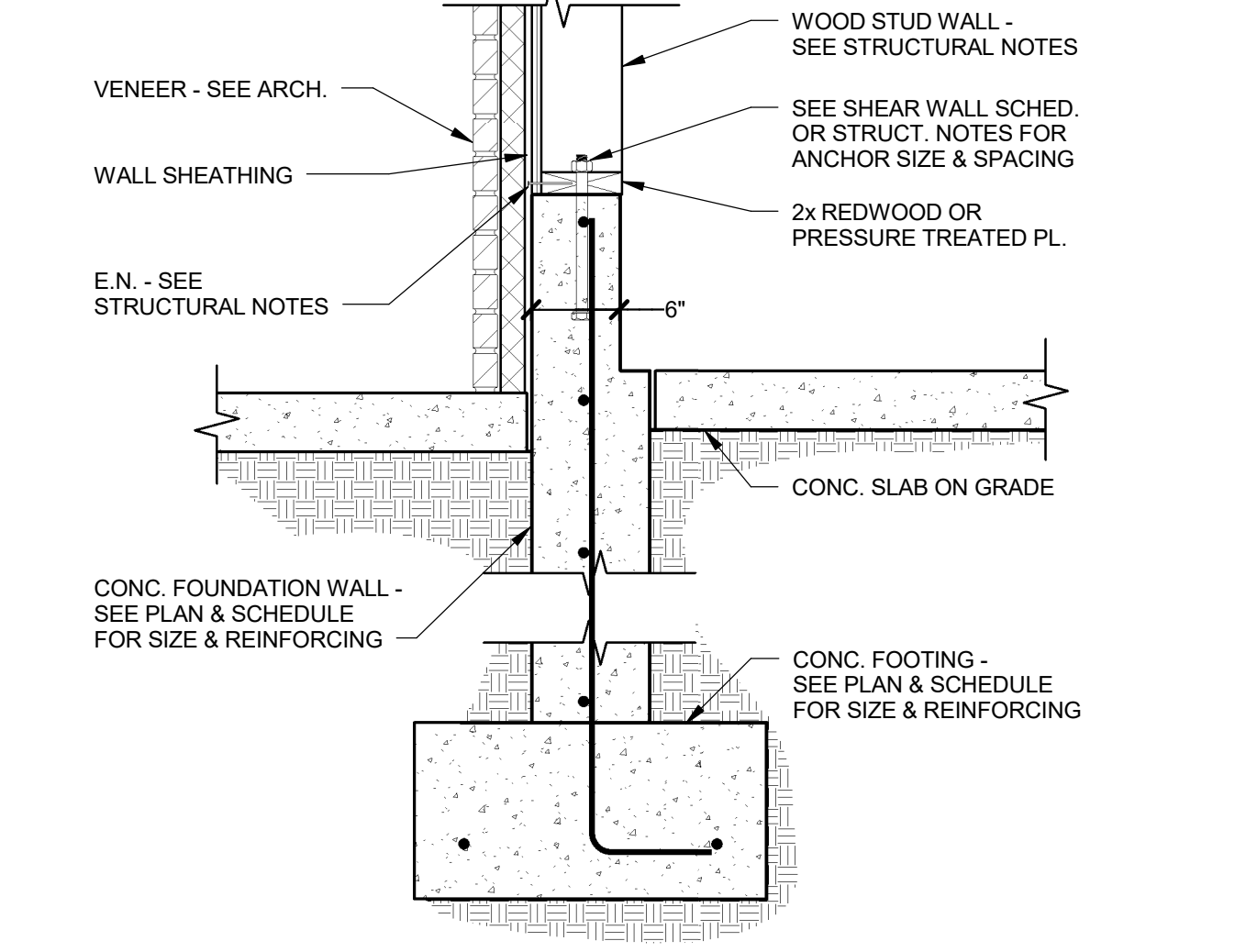
16
DETAIL
SCALE: NONE
S202



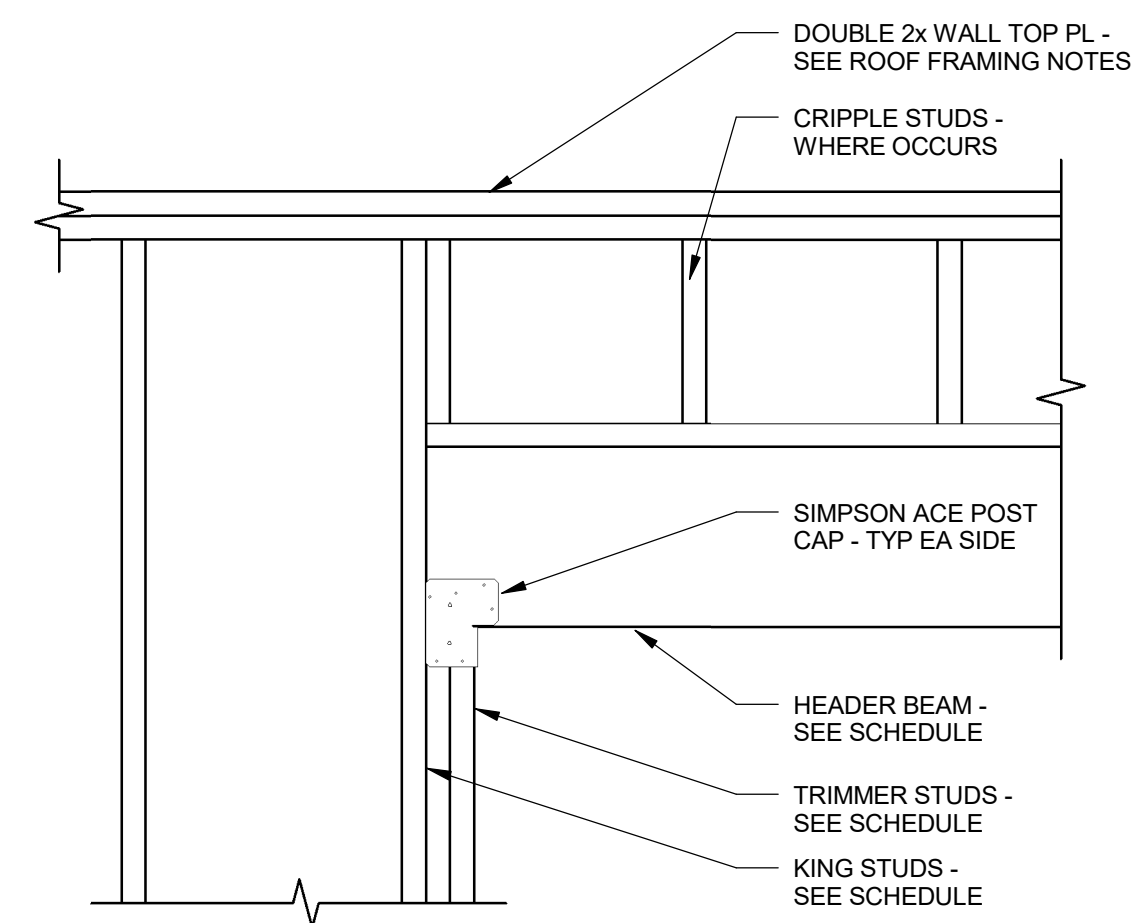
17
DETAIL
SCALE: NONE
S202



18
DETAIL
SCALE: NONE
S202

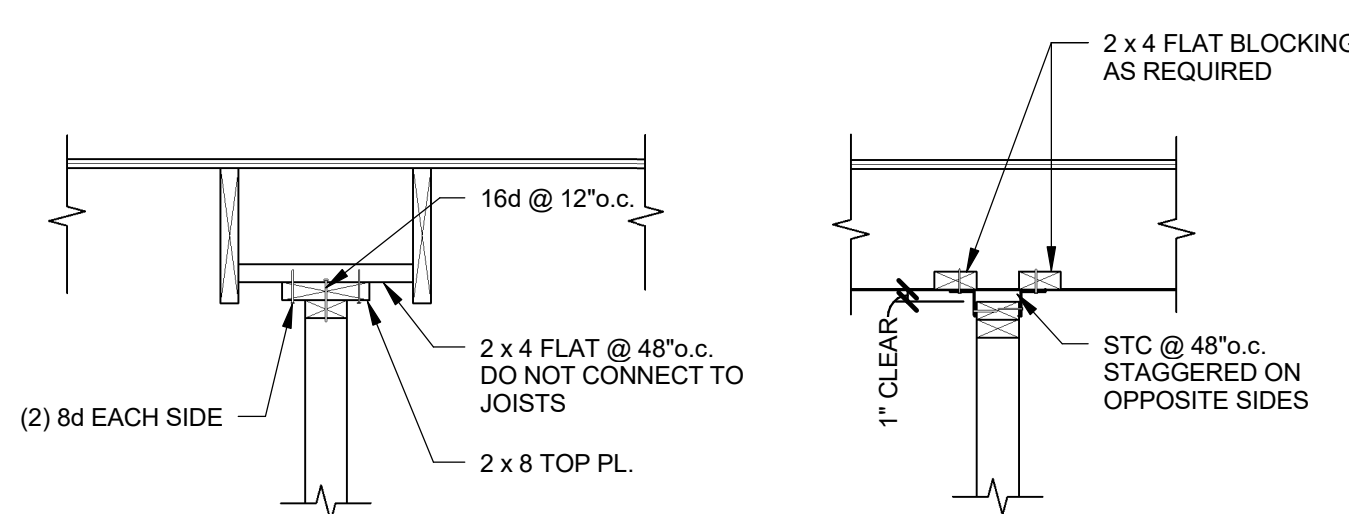


19
DETAIL
SCALE: NONE
S202



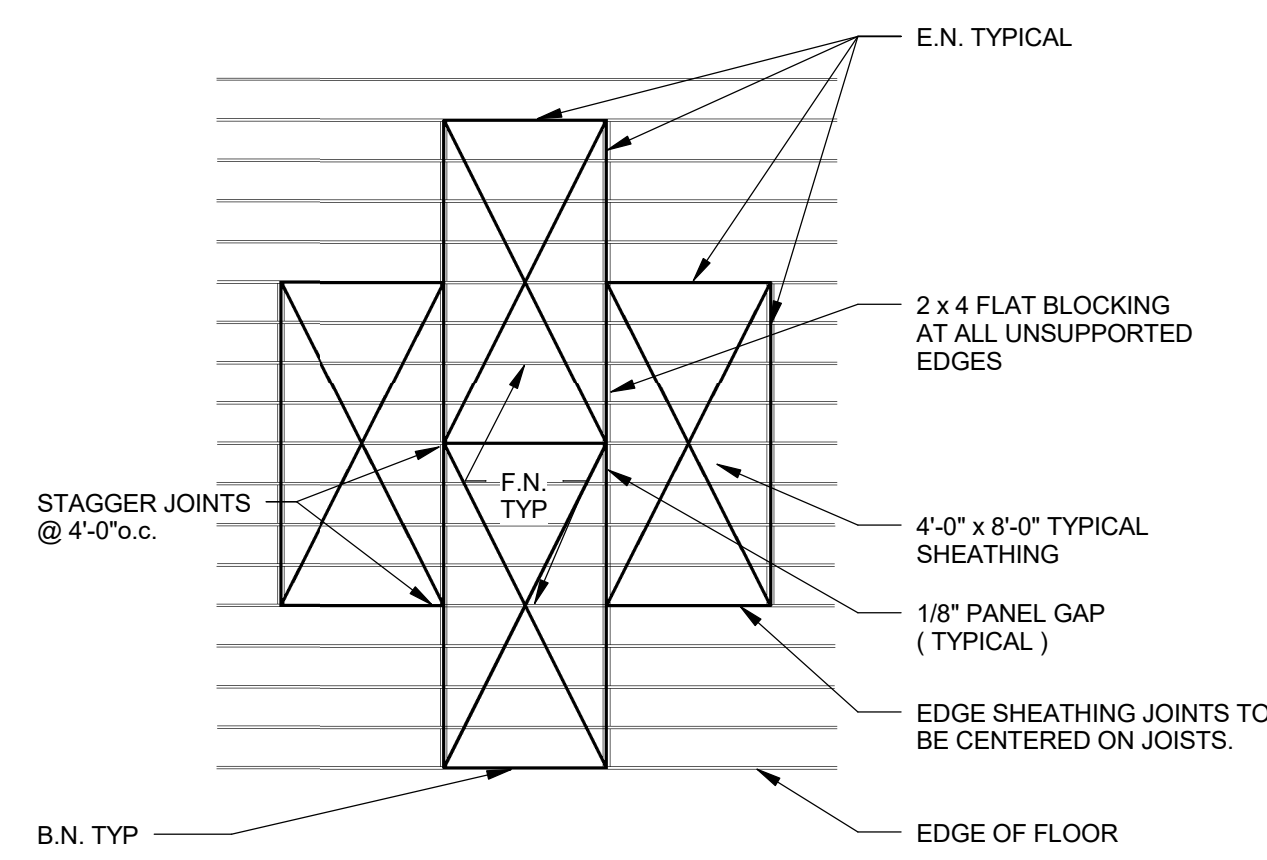
OPENING SIZE	KING STUDS	TRIMMER STUDS	HEADER
UP TO 6'-0"	(1) 2x6	(1) 2x6	(3) 2x10
6'-1" TO 10'-0"	(1) 2x6	(2) 2x6	(3) 1 3/4" x 11 7/8" LVL

DETAIL SCALE: NONE **1** S203

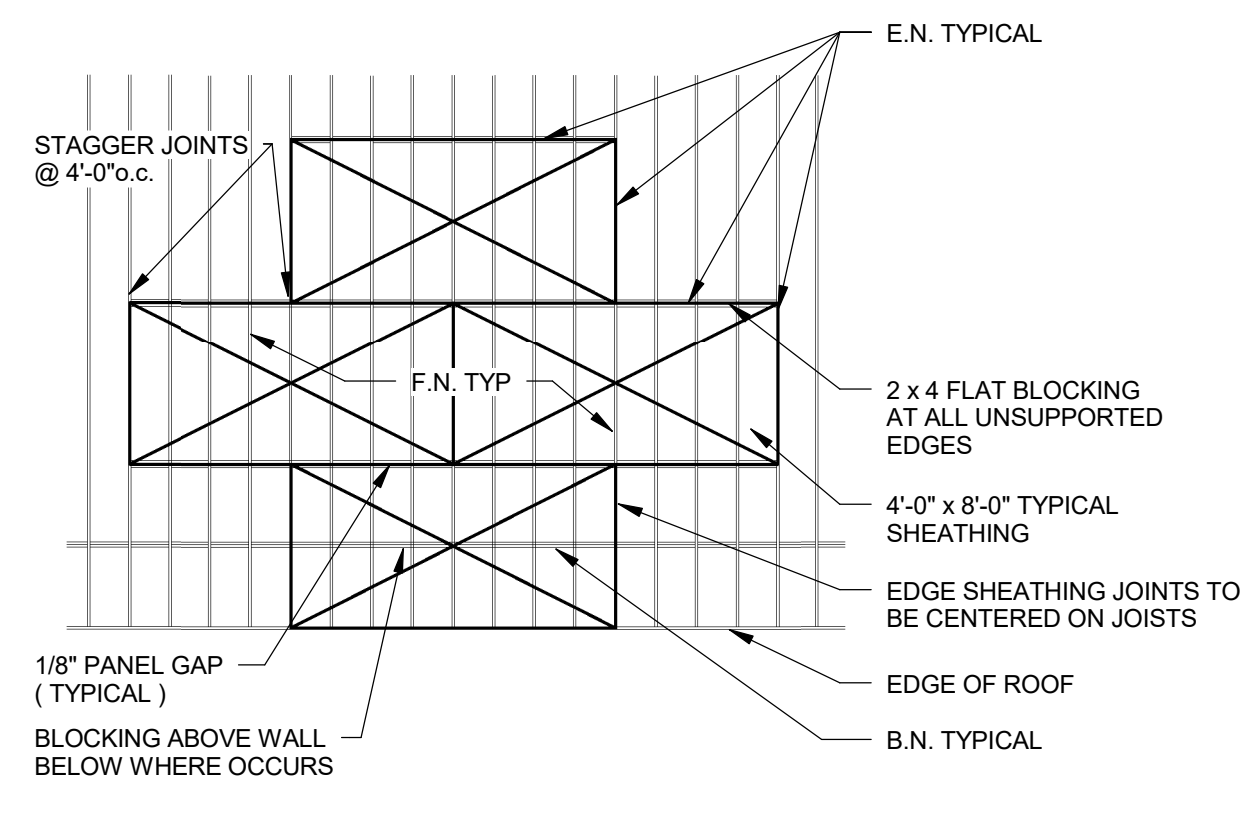


WALL PARALLEL TO JOISTS WALL PERPENDICULAR TO JOISTS

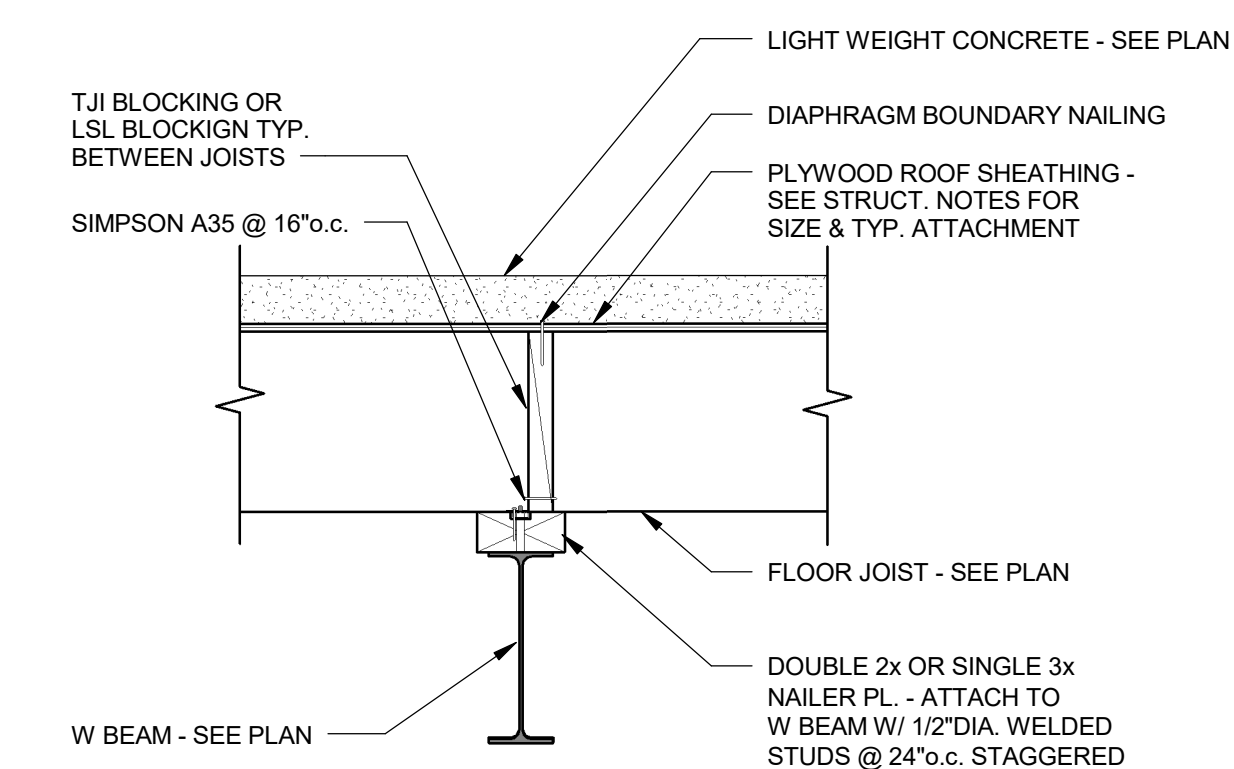
DETAIL SCALE: NONE **2** S203



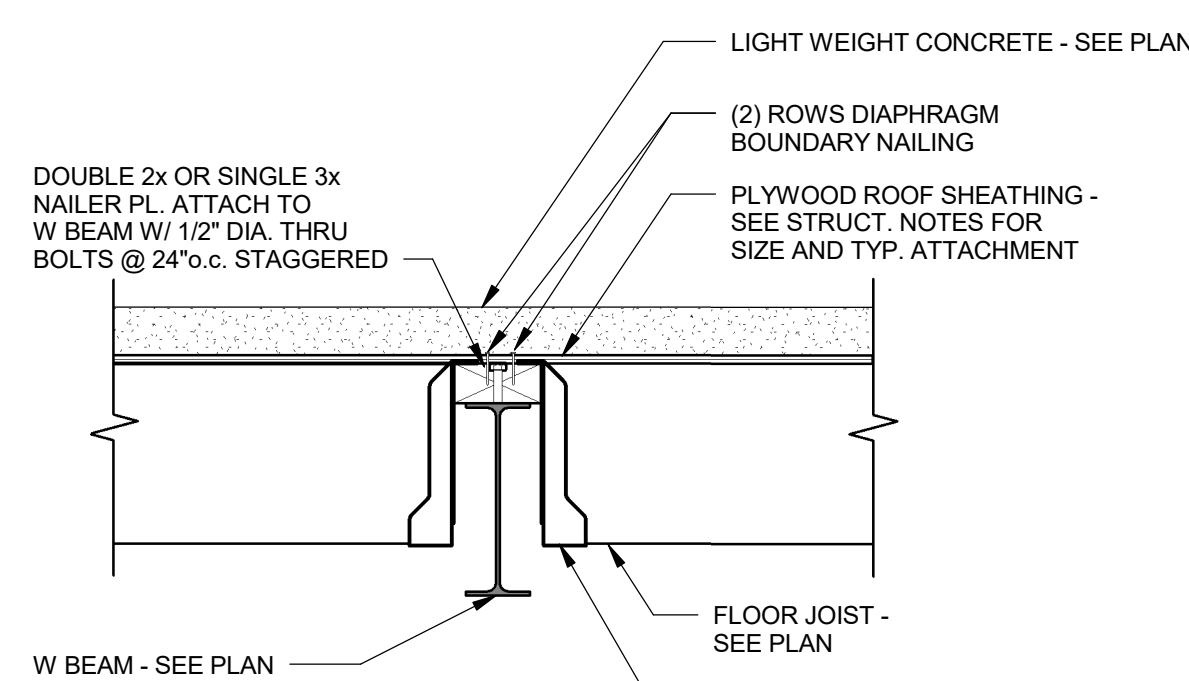
TYPICAL SHEATHING LAYOUT @ FLOOR SCALE: NONE



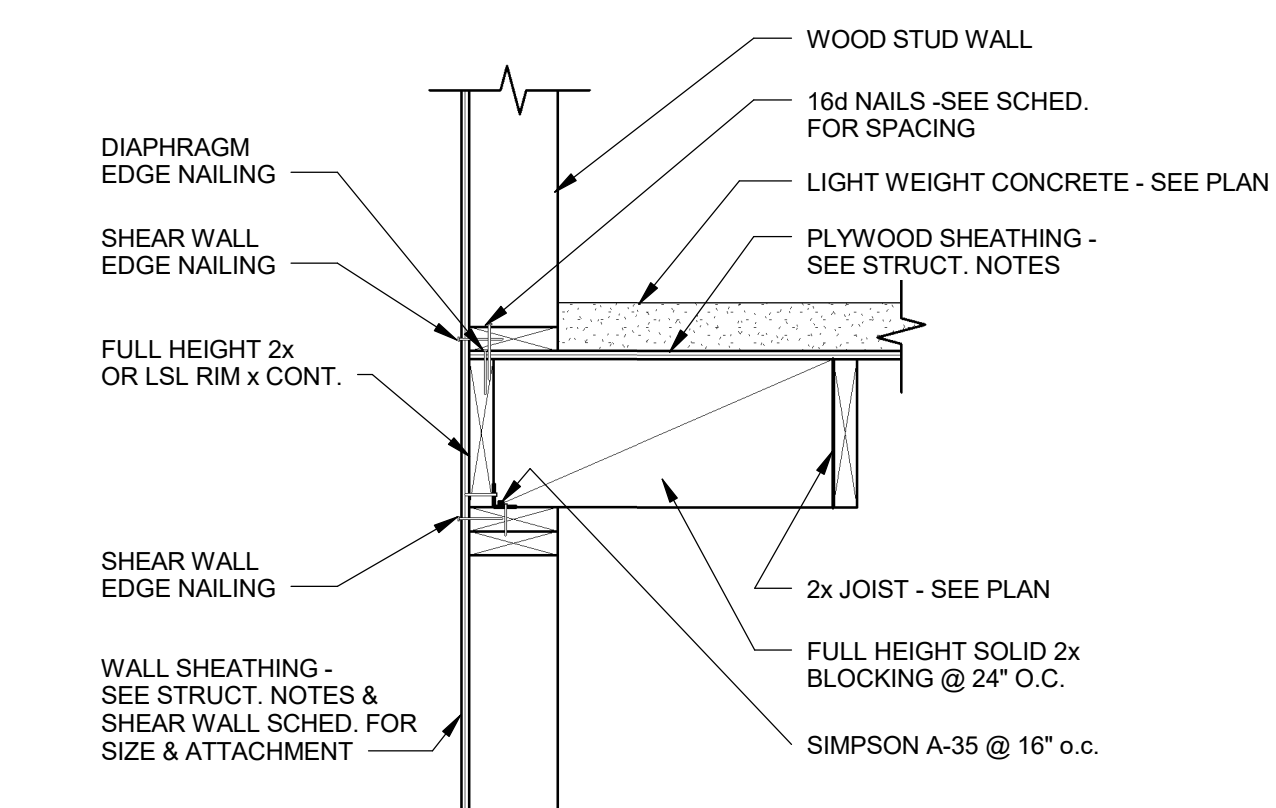
3 S203



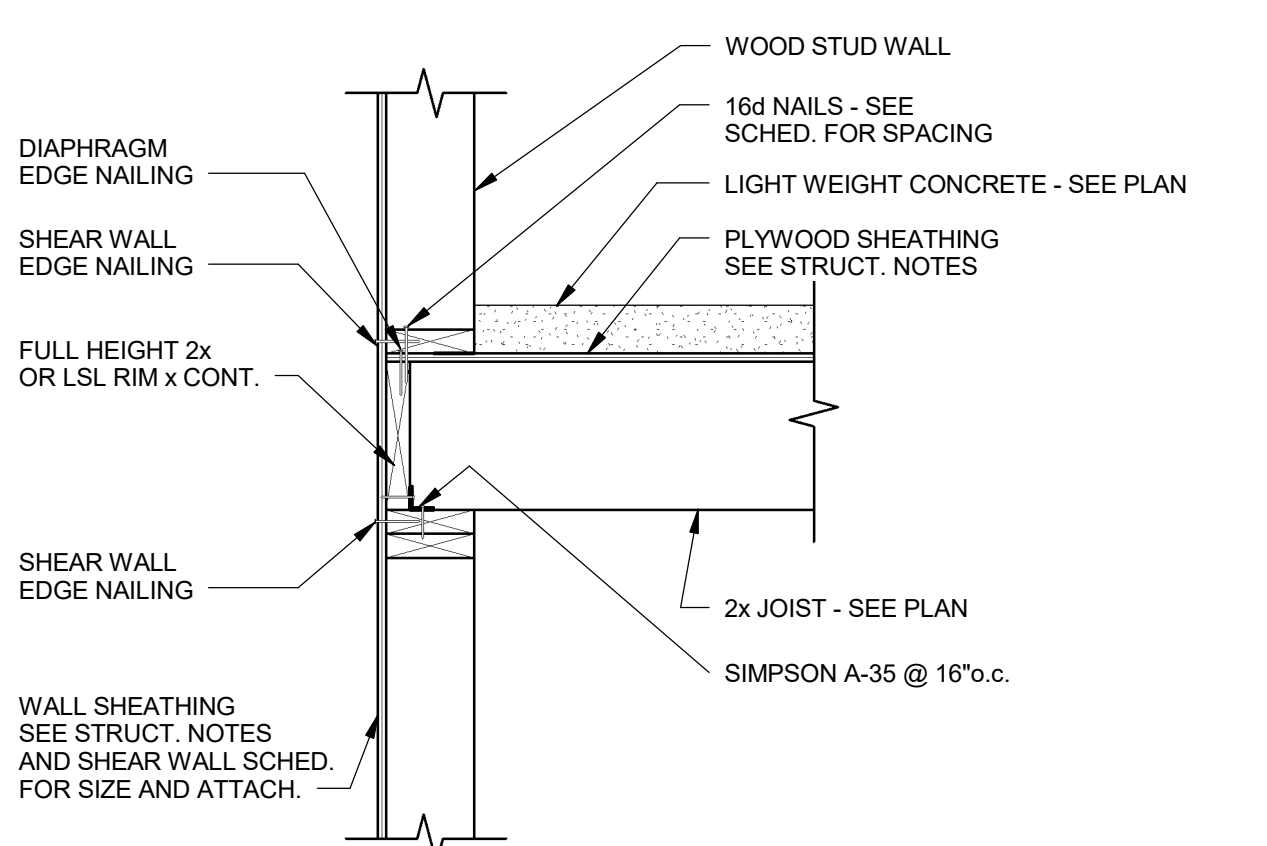
DETAIL SCALE: NONE **4** S203



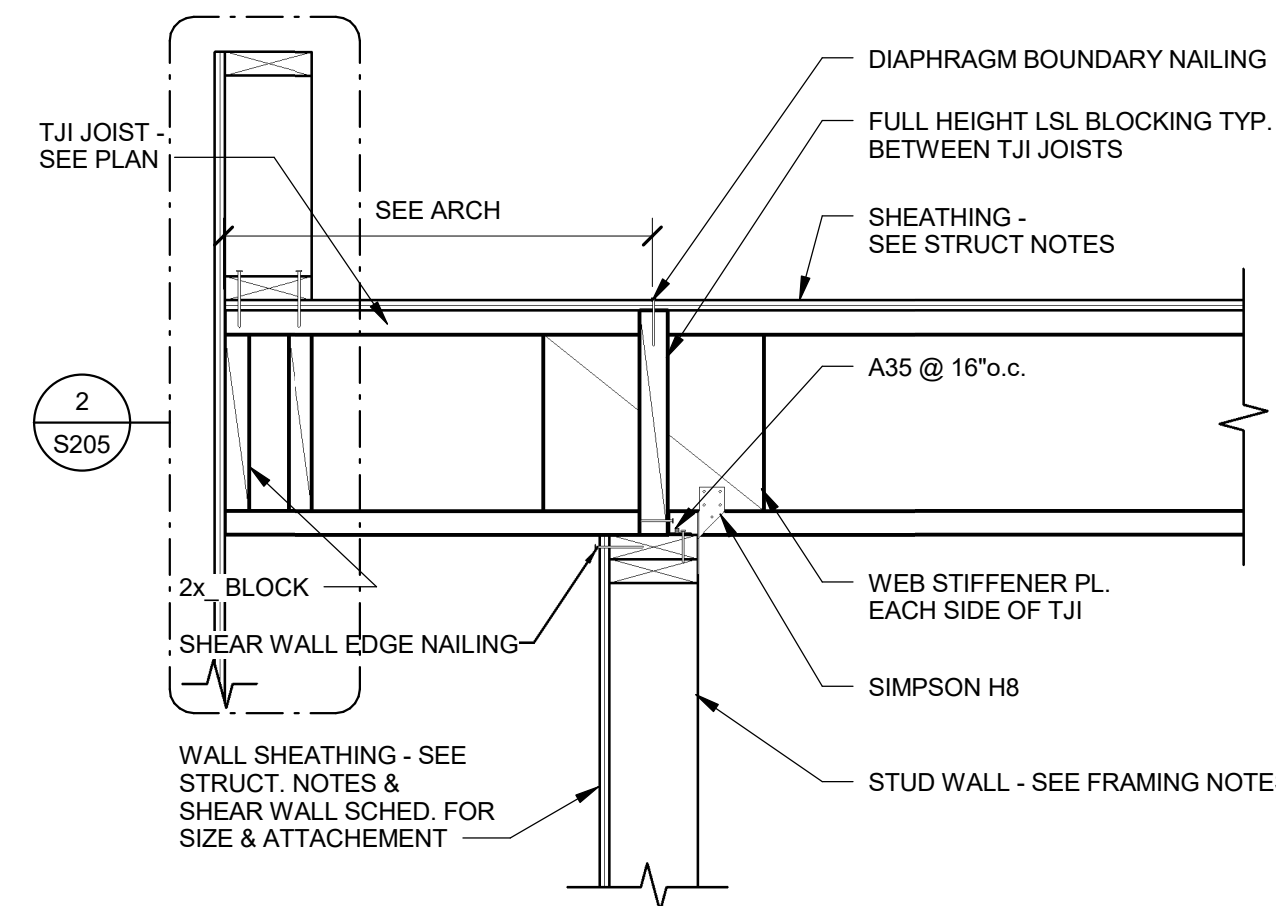
DETAIL SCALE: NONE **5** S203



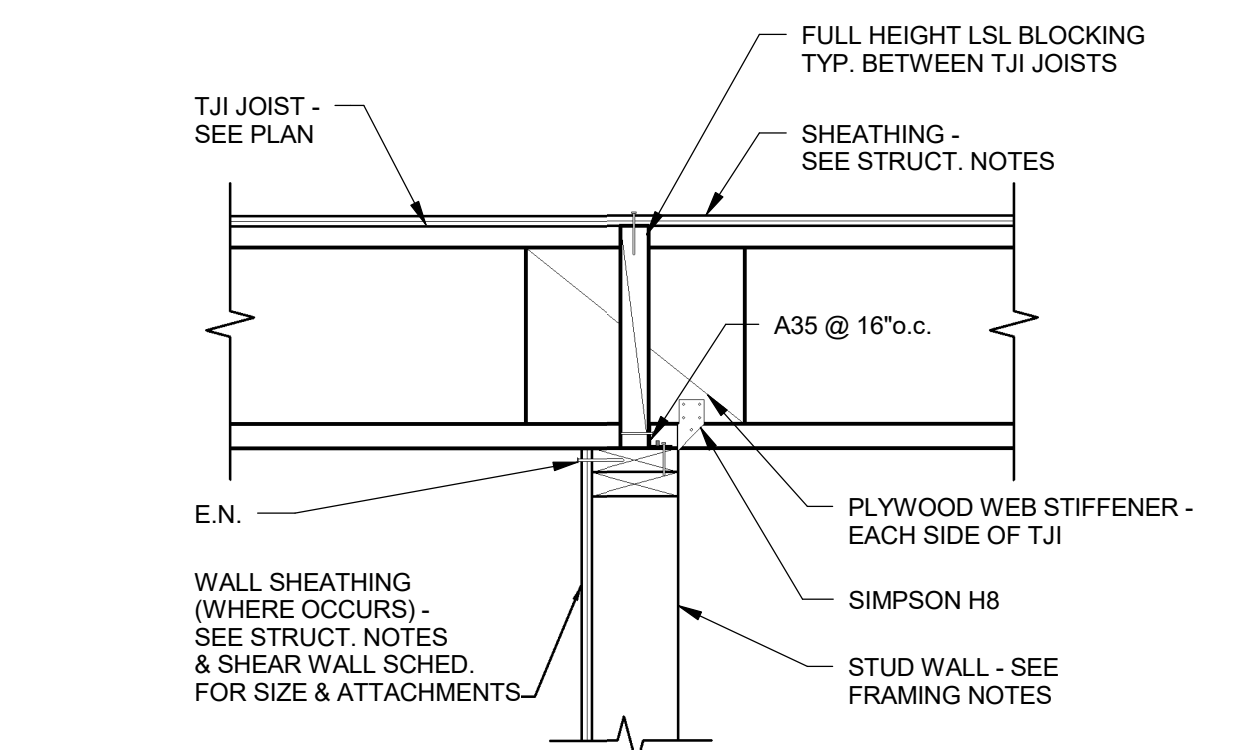
DETAIL SCALE: NONE **6** S203



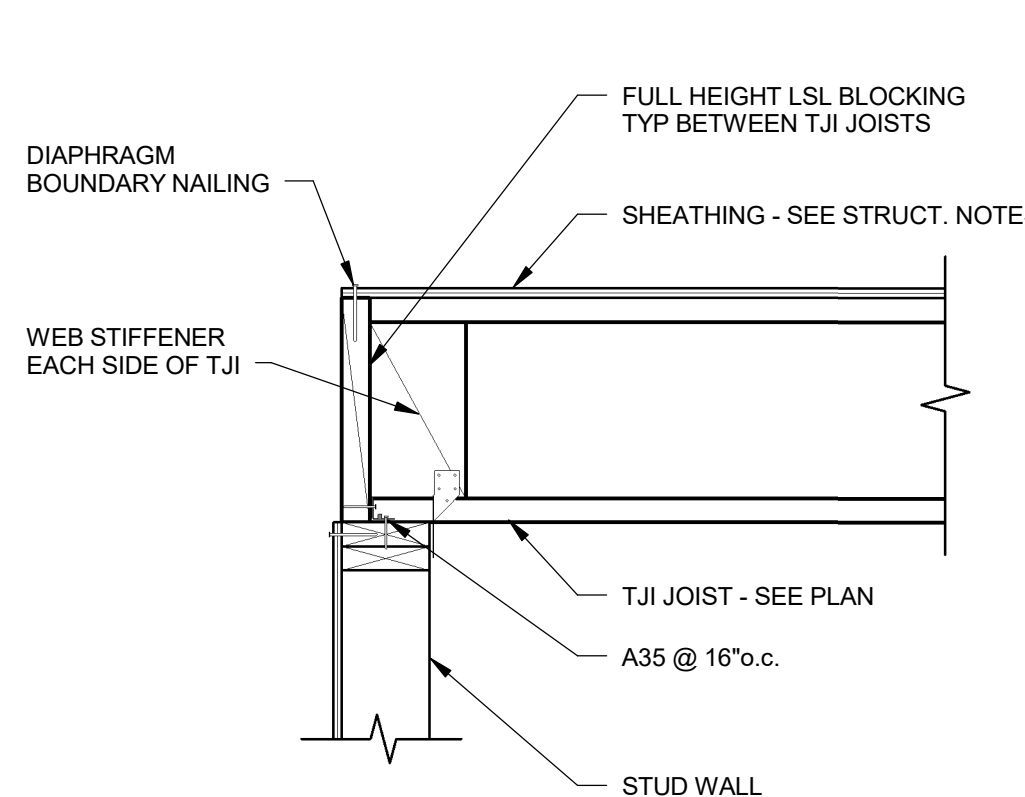
DETAIL SCALE: NONE **7** S203



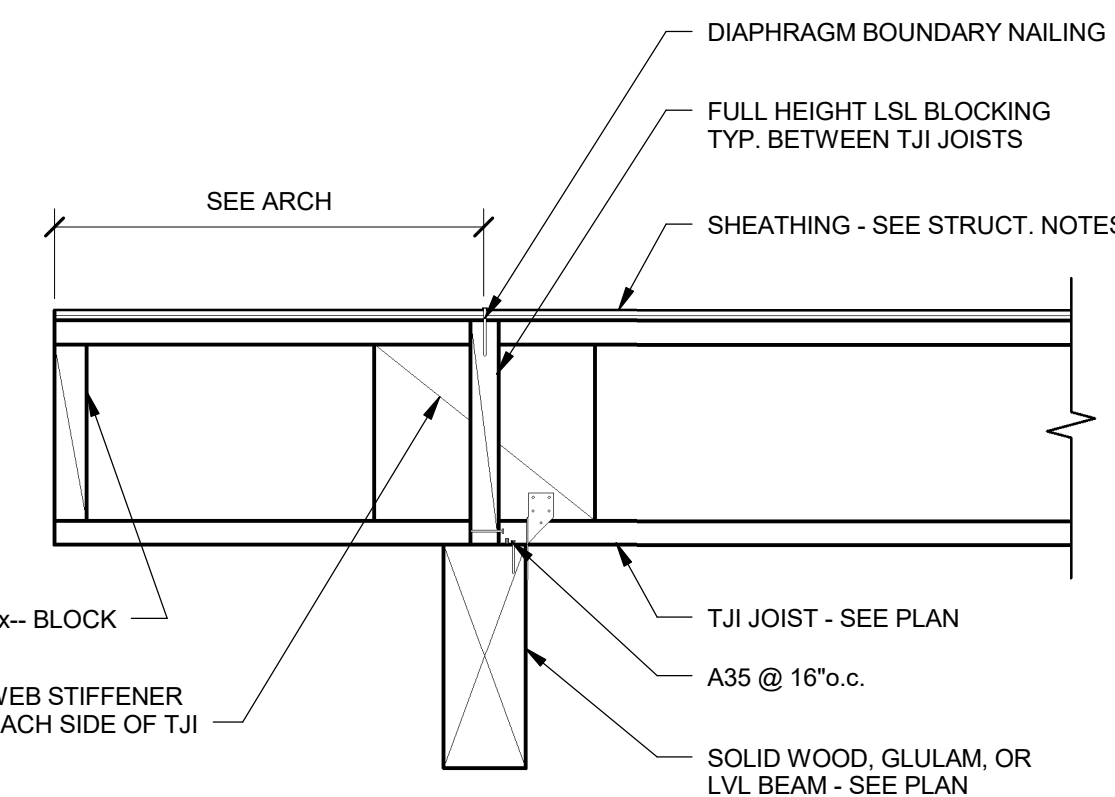
DETAIL SCALE: NONE **8** S203



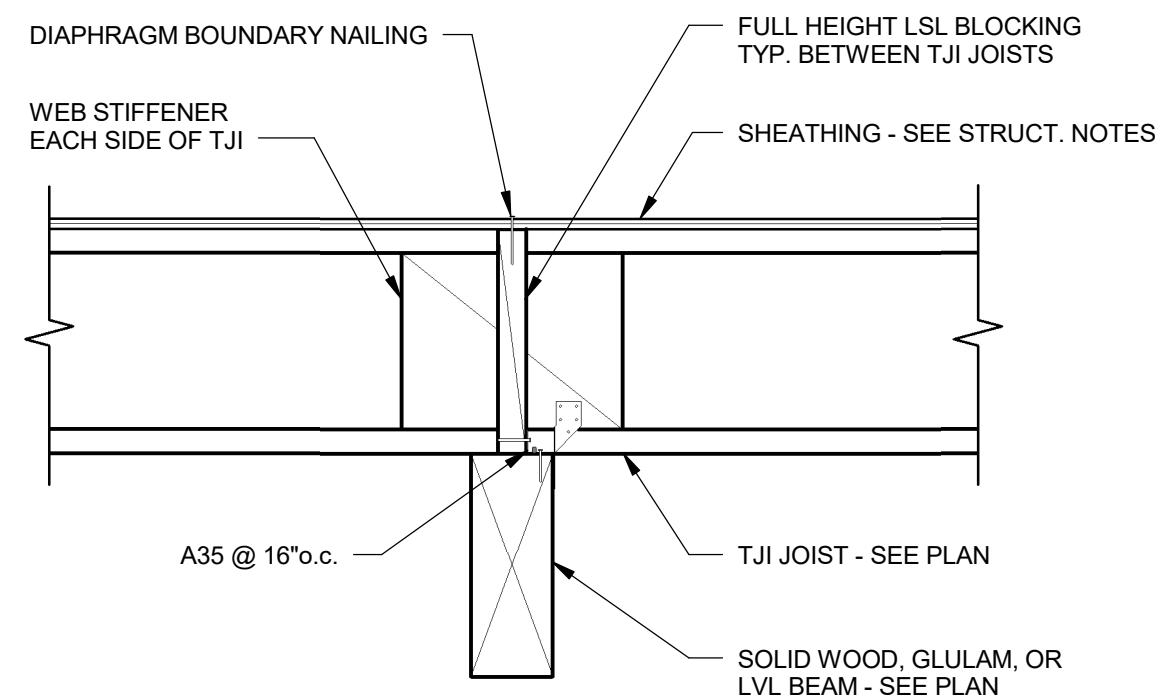
DETAIL SCALE: NONE **9** S203



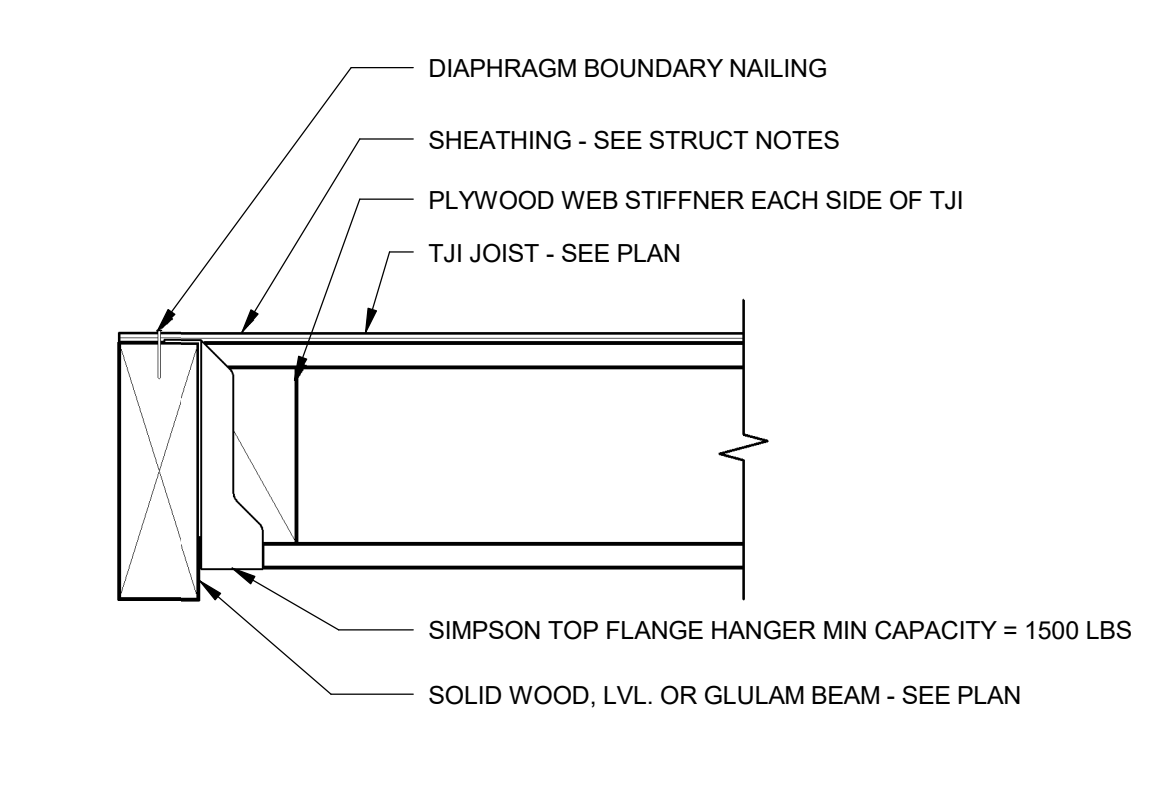
DETAIL NOT USED SCALE: NONE **10** S203



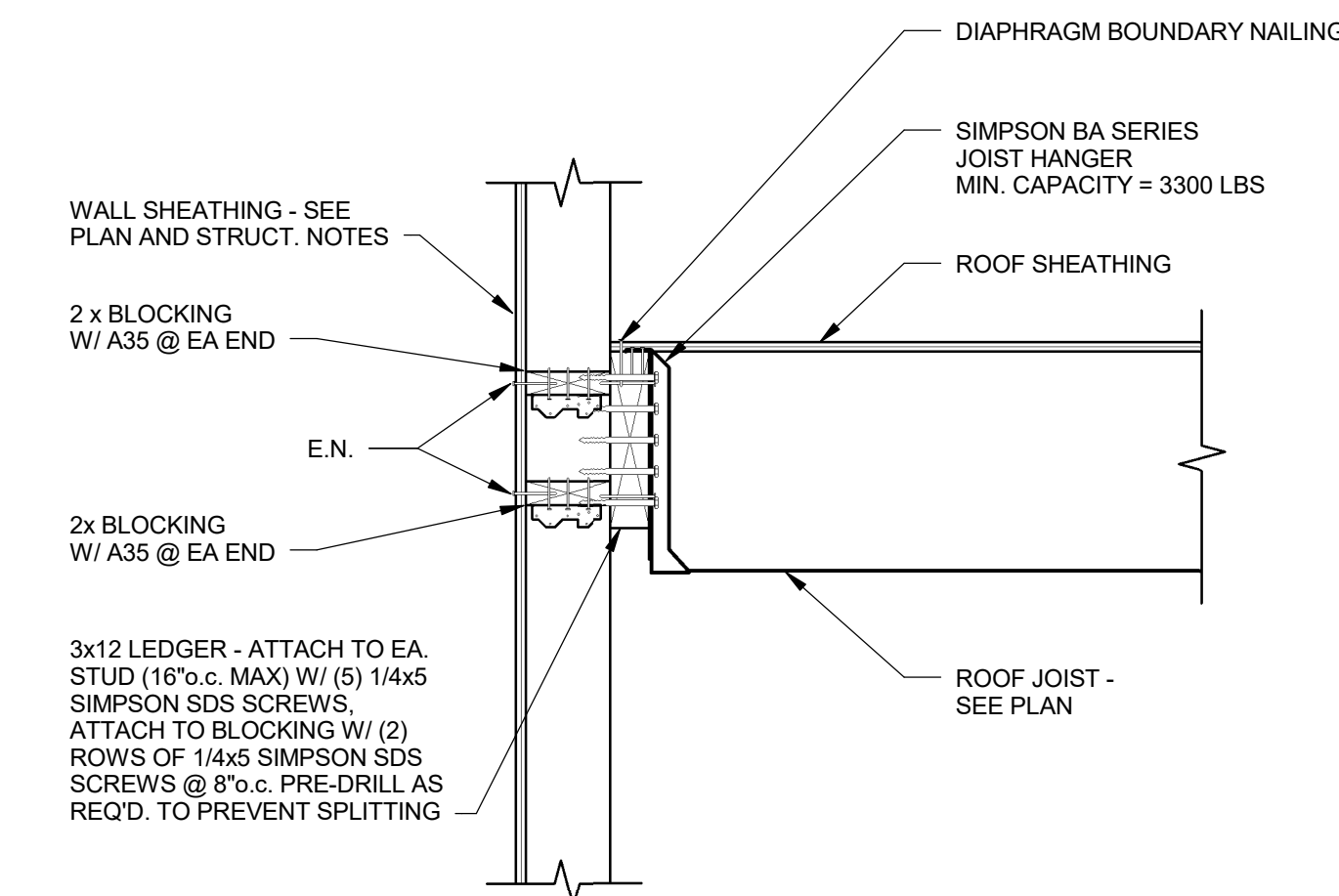
DETAIL NOT USED SCALE: NONE **11** S203



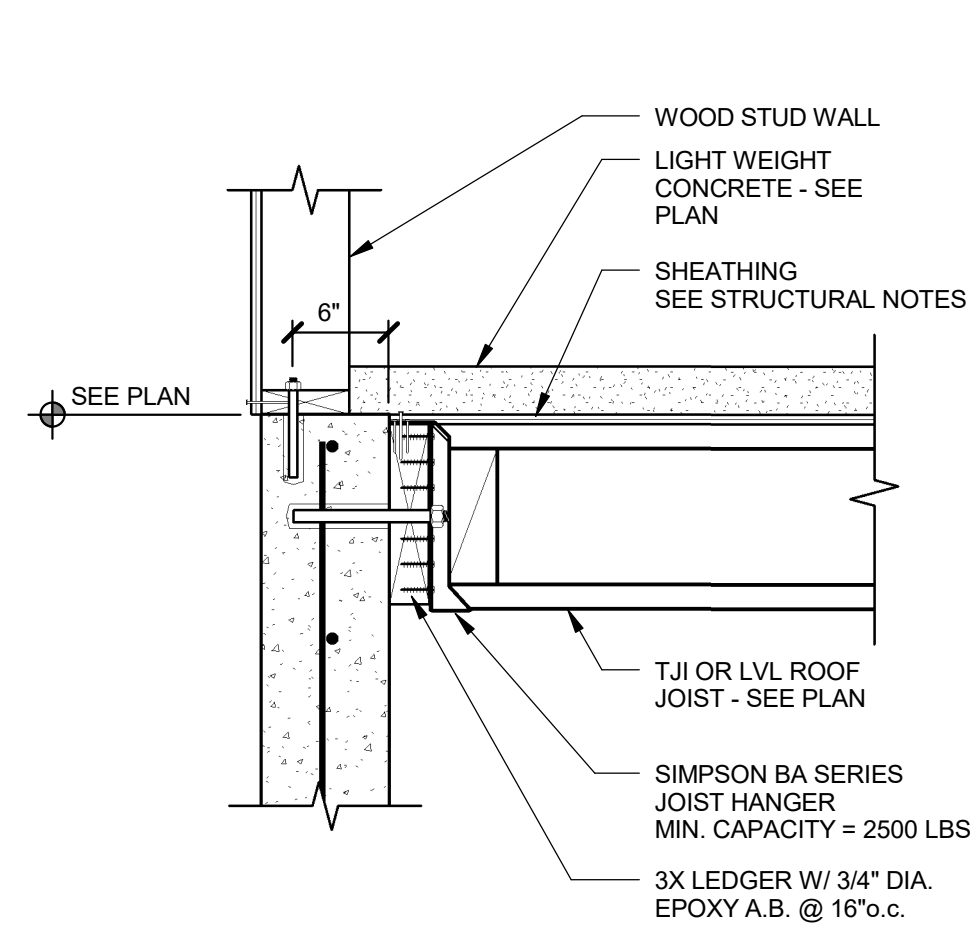
DETAIL NOT USED SCALE: NONE **12** S203



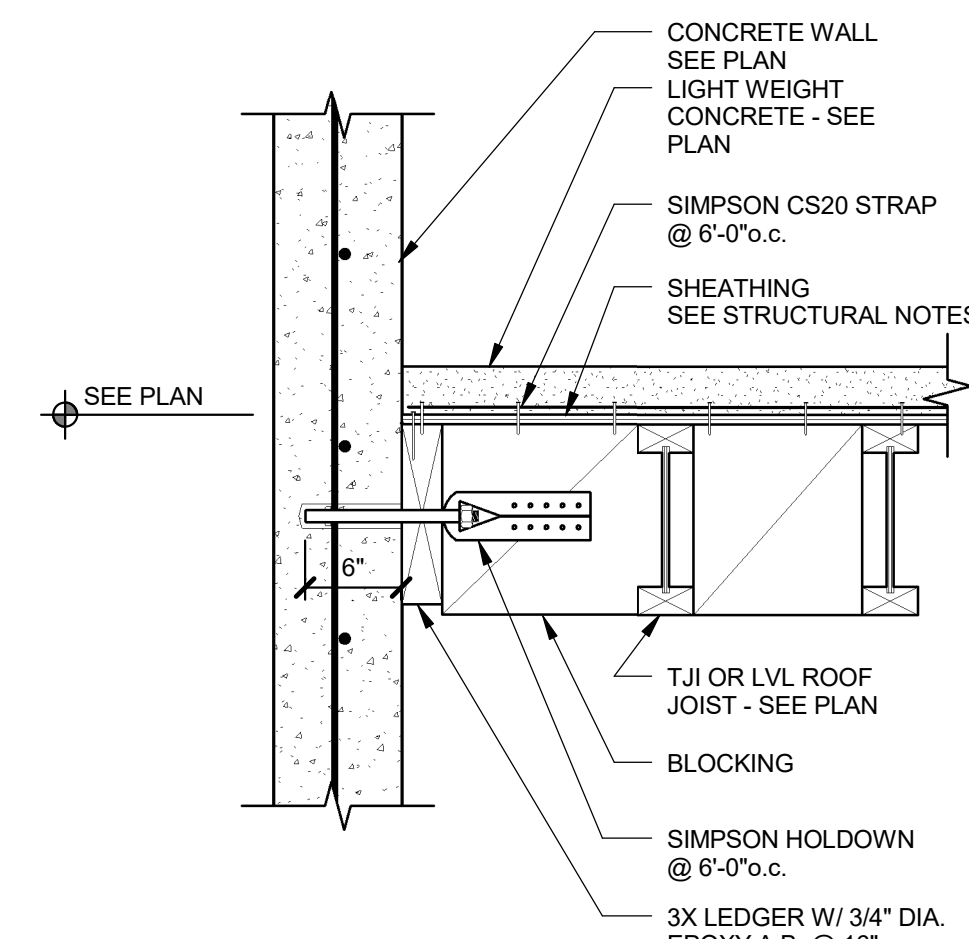
DETAIL NOT USED SCALE: NONE **13** S203



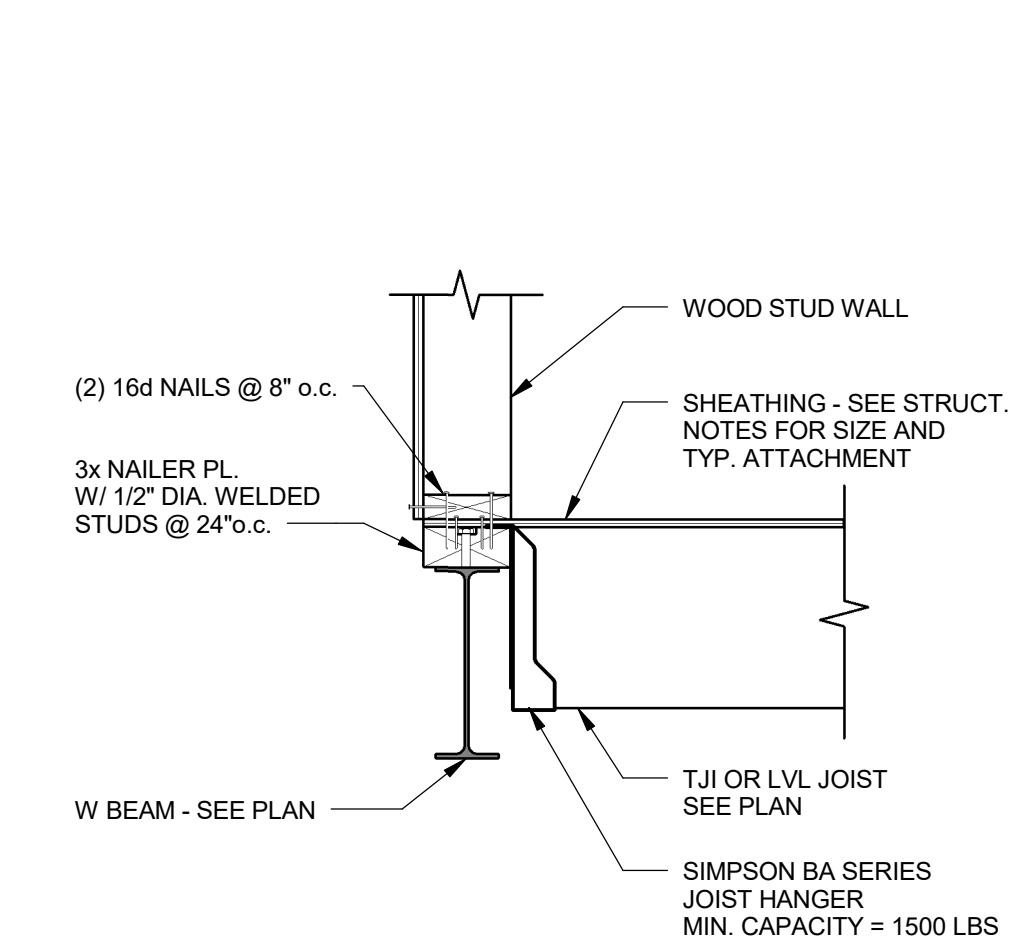
DETAIL SCALE: NONE **14** S203



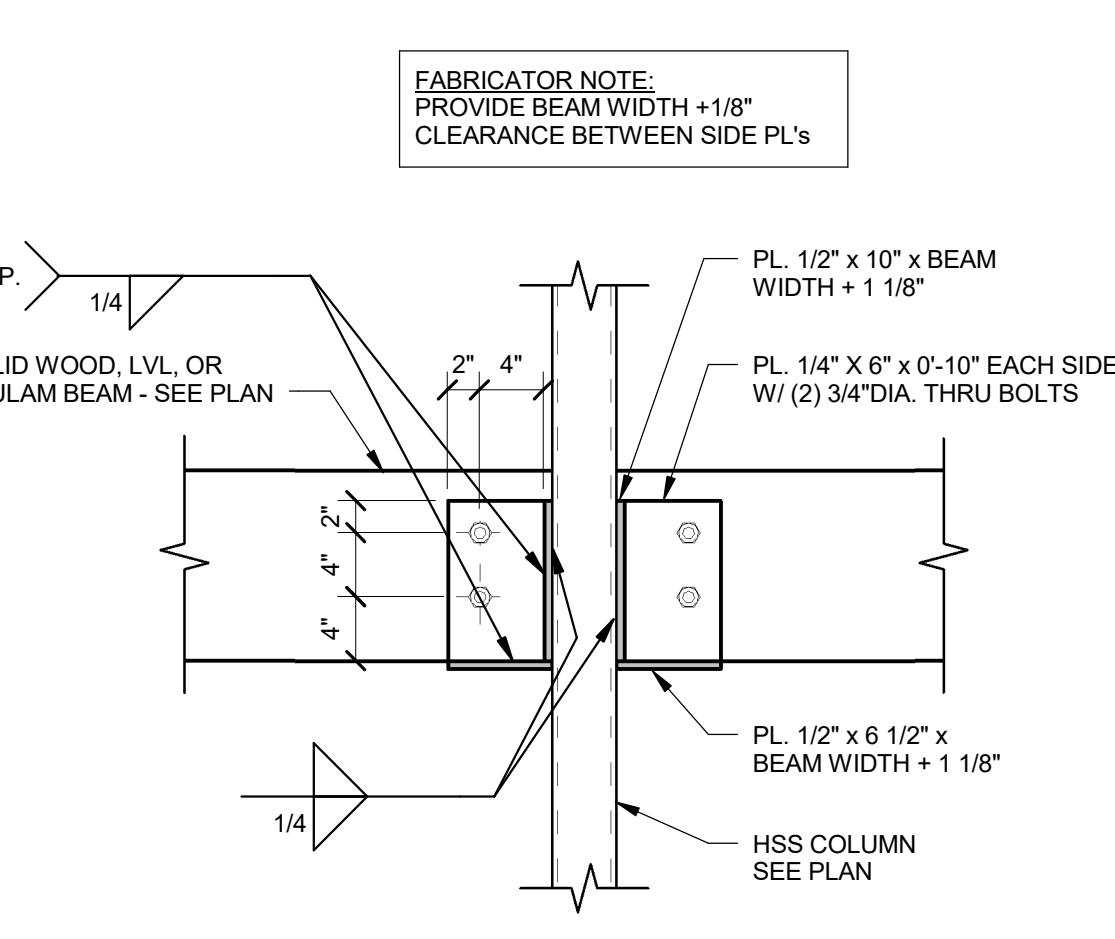
DETAIL SCALE: NONE **15** S203



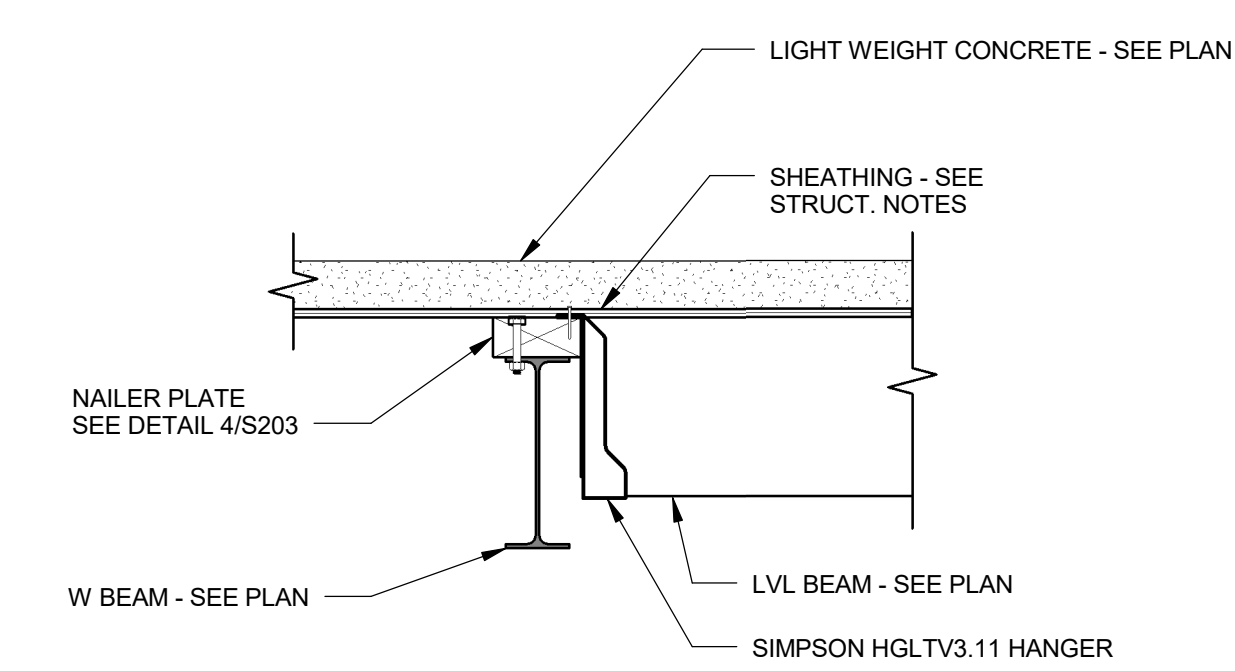
DETAIL SCALE: NONE **16** S203



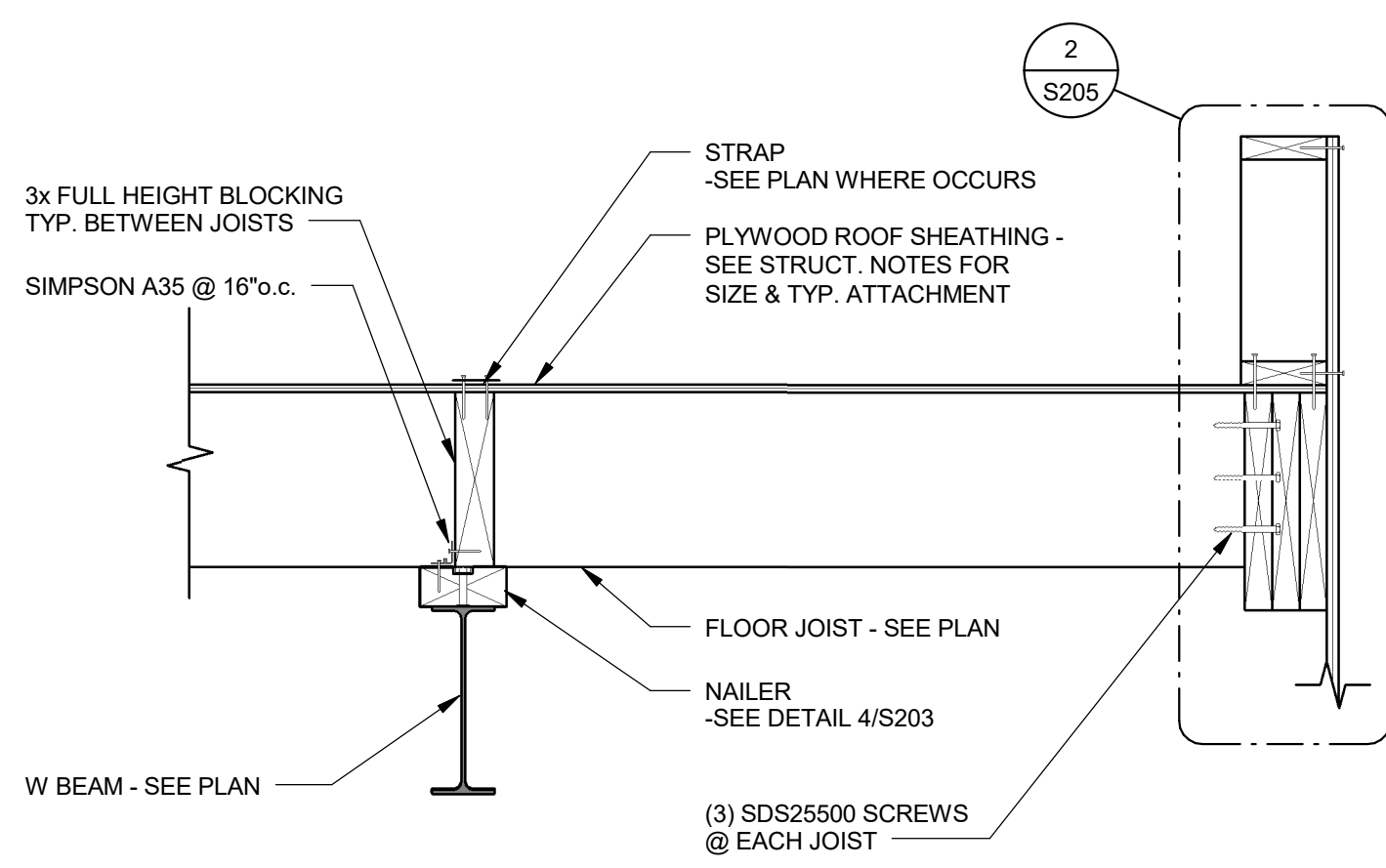
DETAIL SCALE: NONE **17** S203



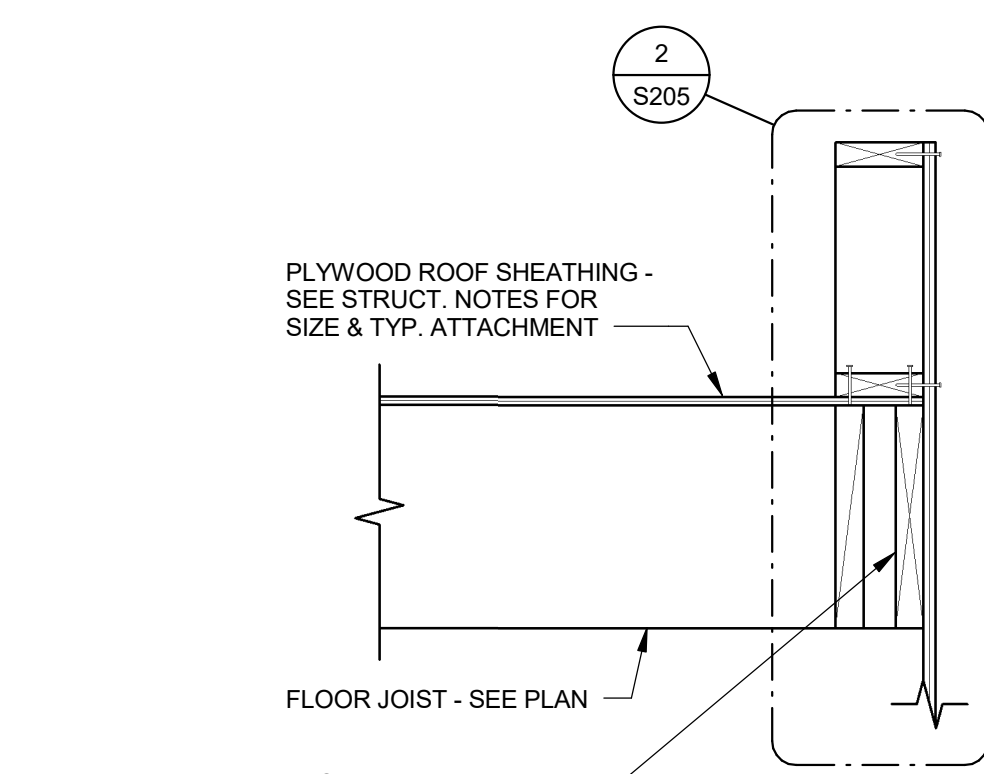
DETAIL SCALE: NONE **18** S203



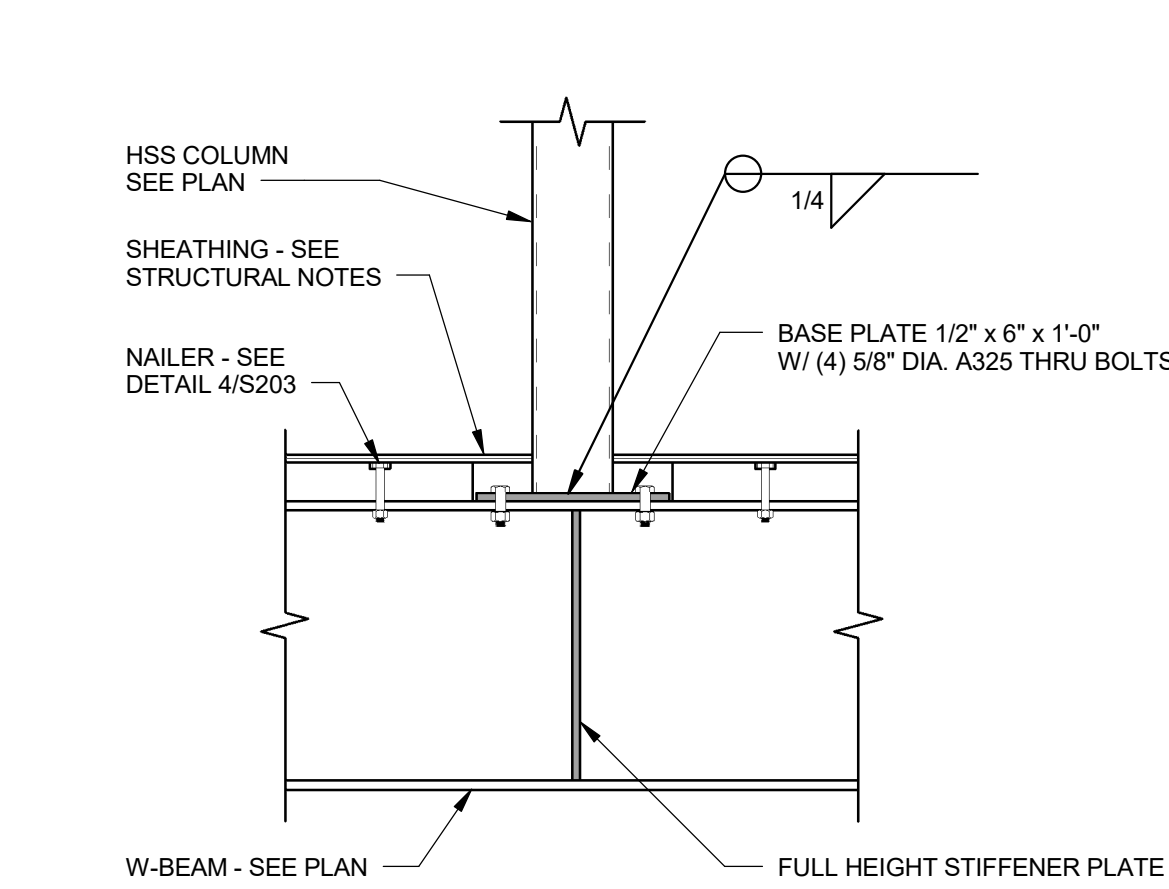
DETAIL SCALE: NONE **19** S203



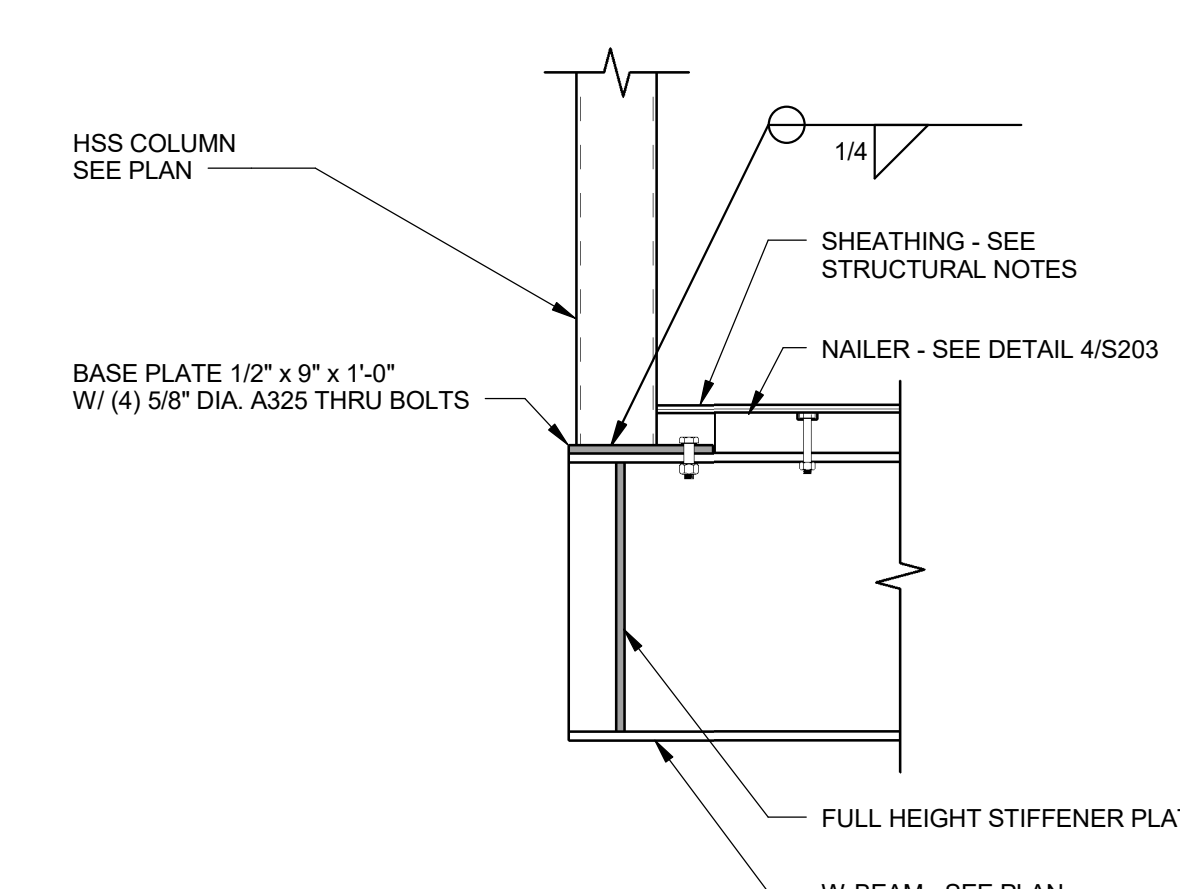
DETAIL
SCALE: NONE
1
S204



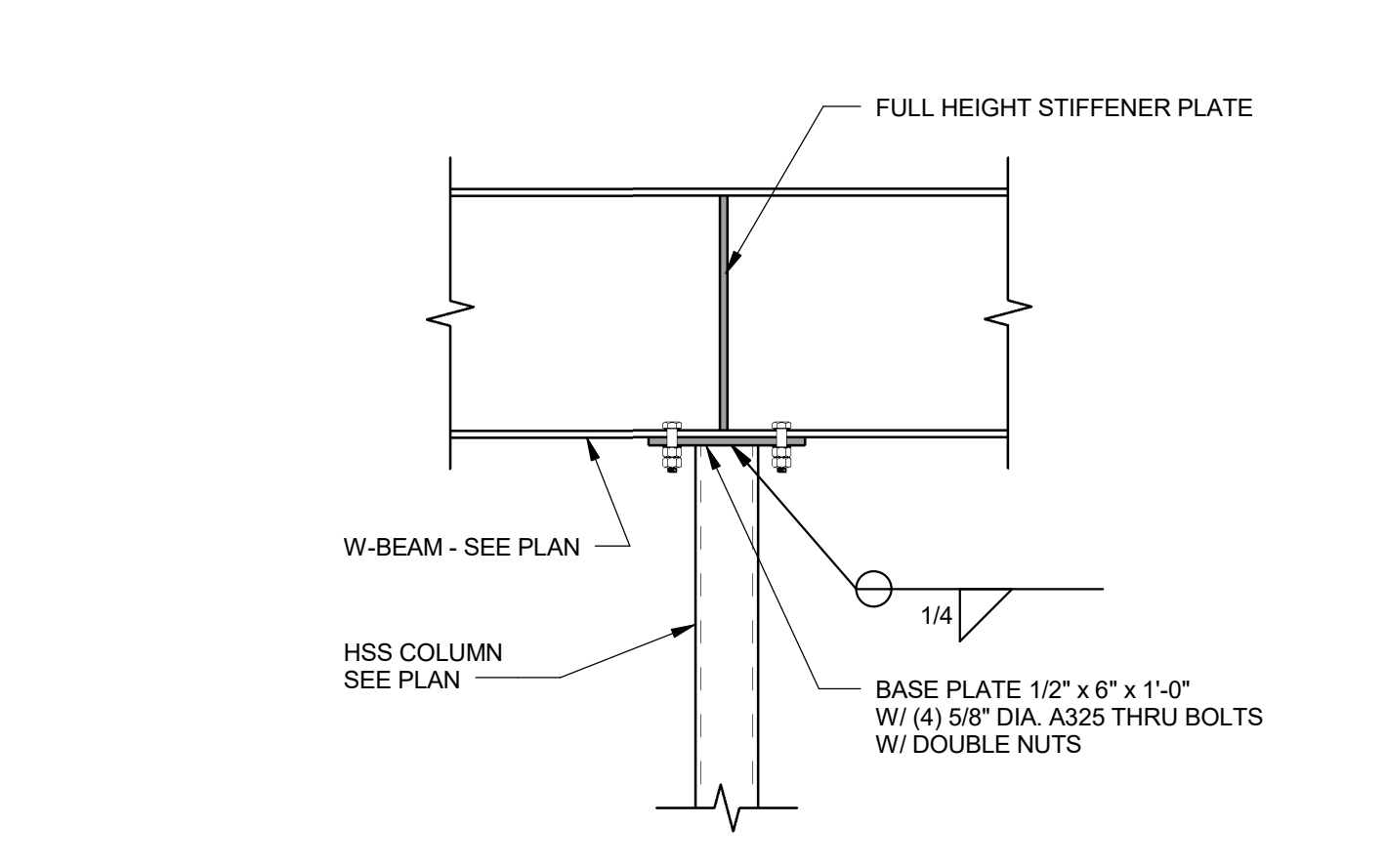
DETAIL
SCALE: NONE
2
S204



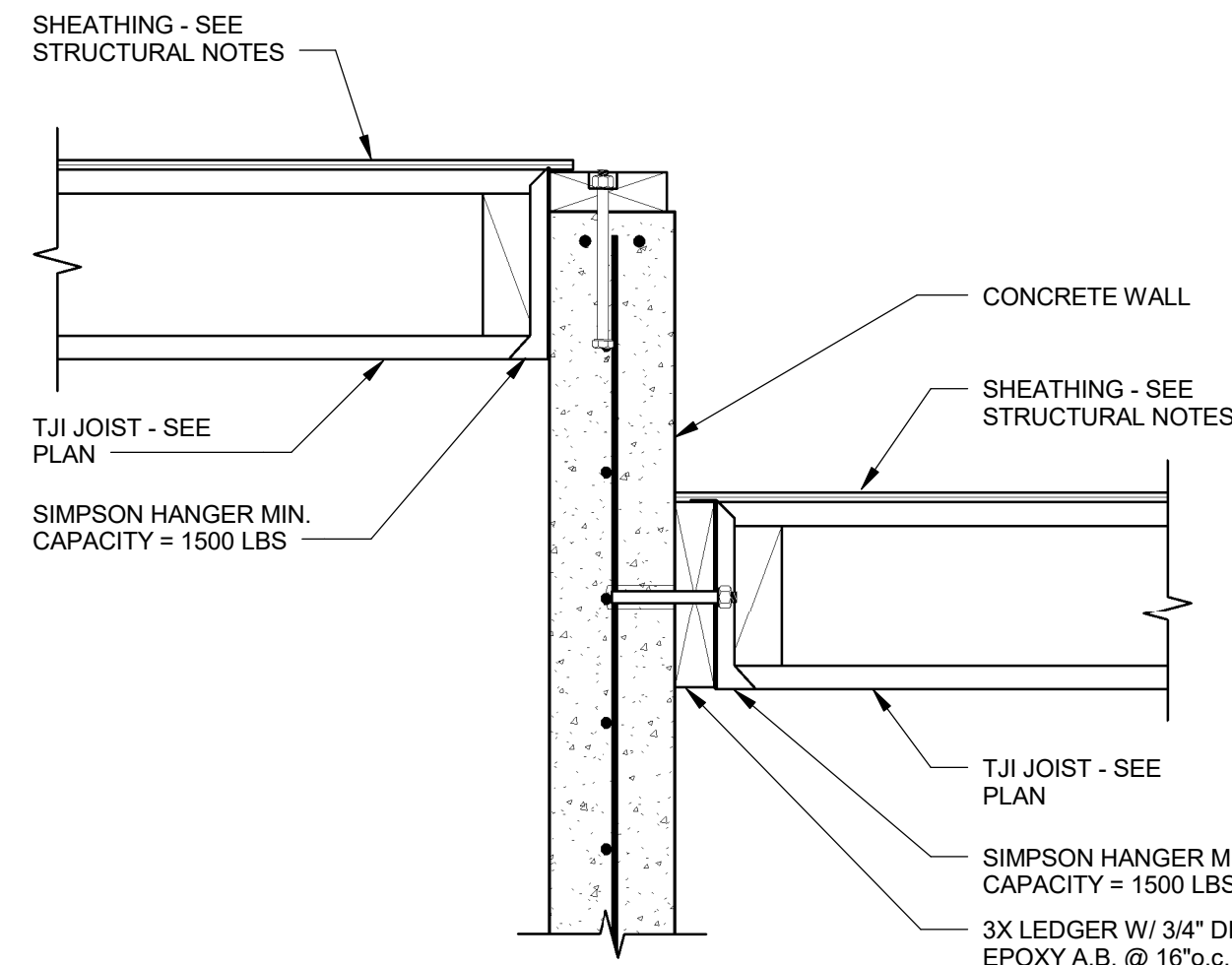
DETAIL
SCALE: NONE
3
S204



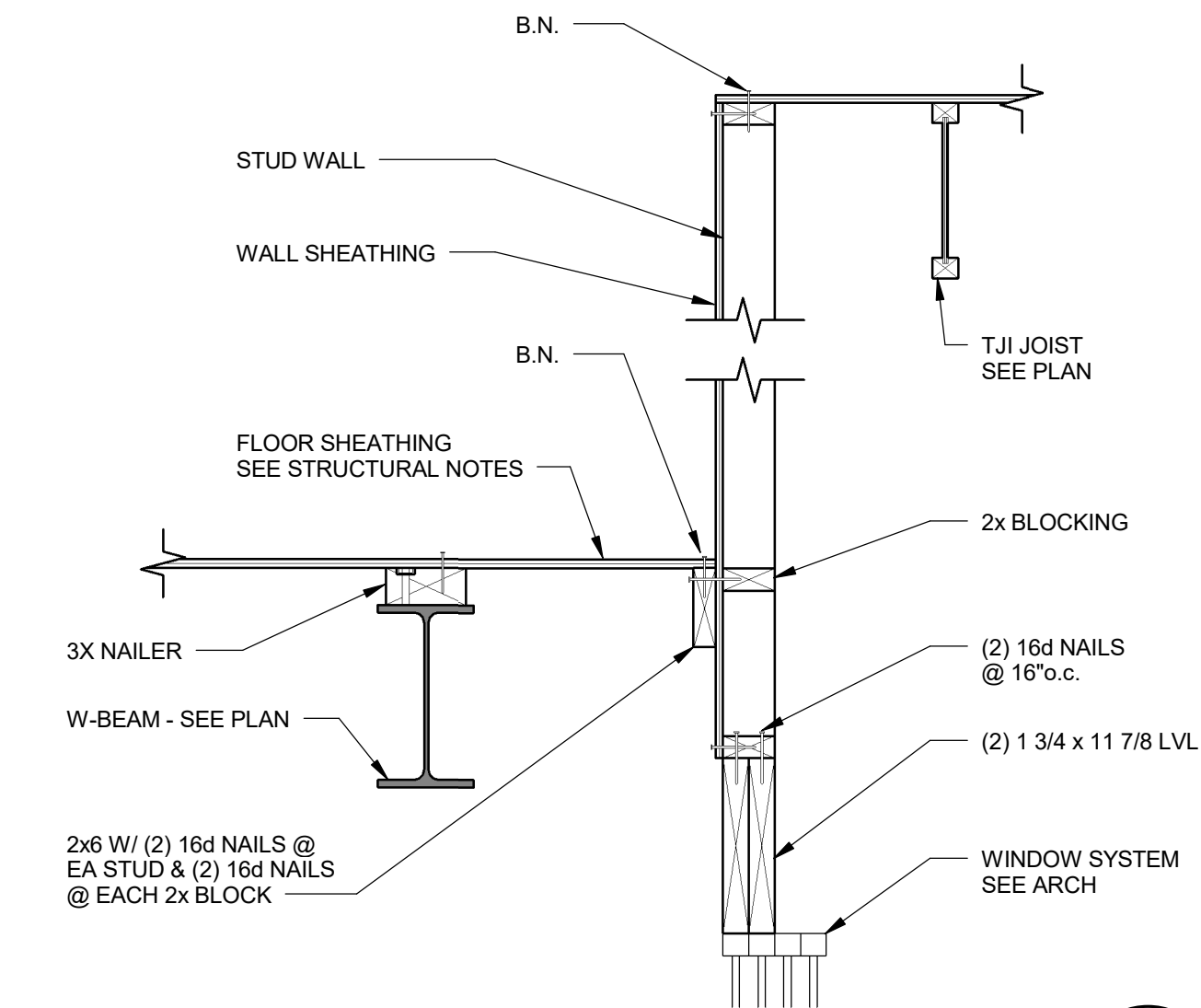
DETAIL
SCALE: NONE
4
S204



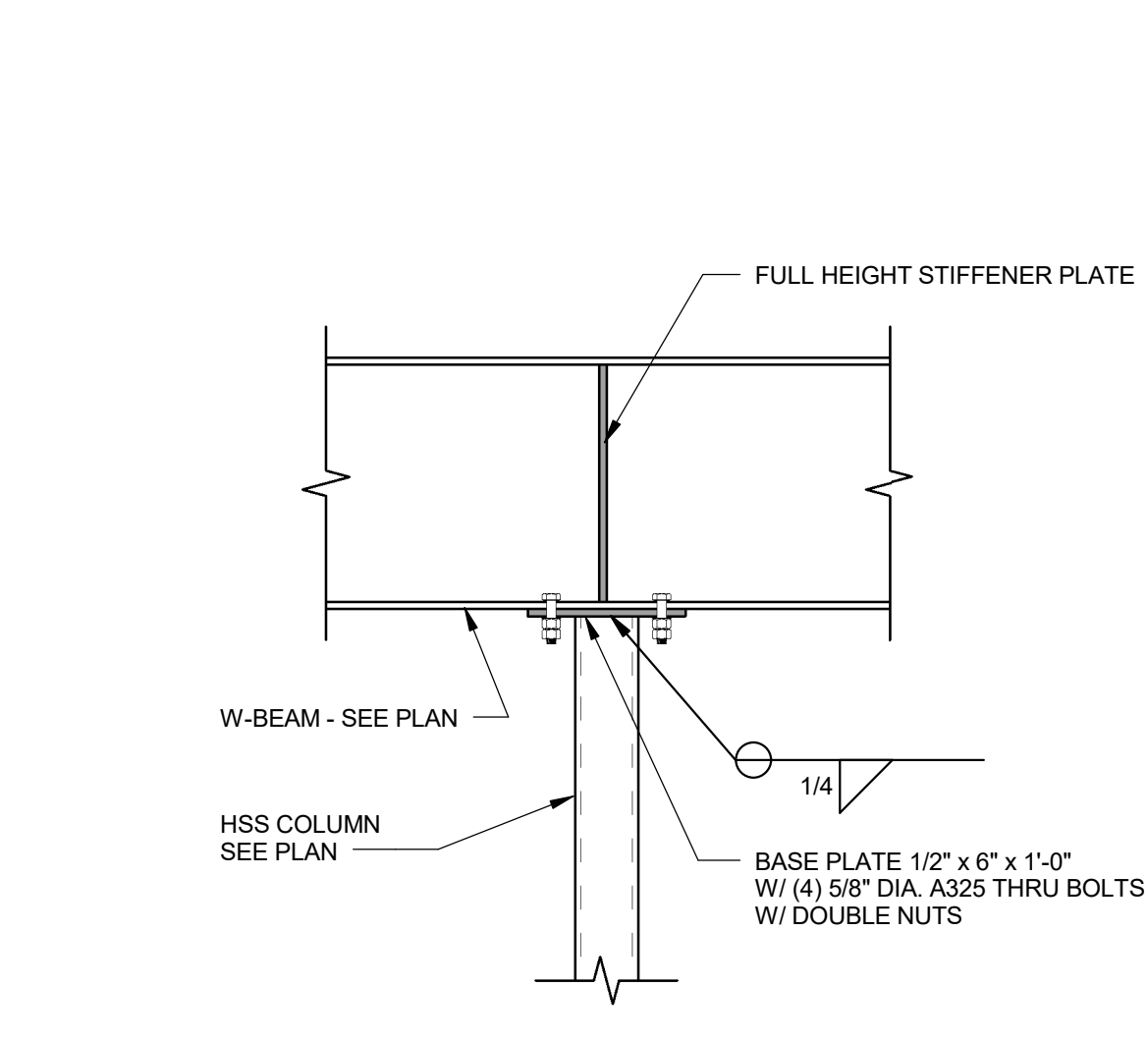
DETAIL
SCALE: NONE
5
S204



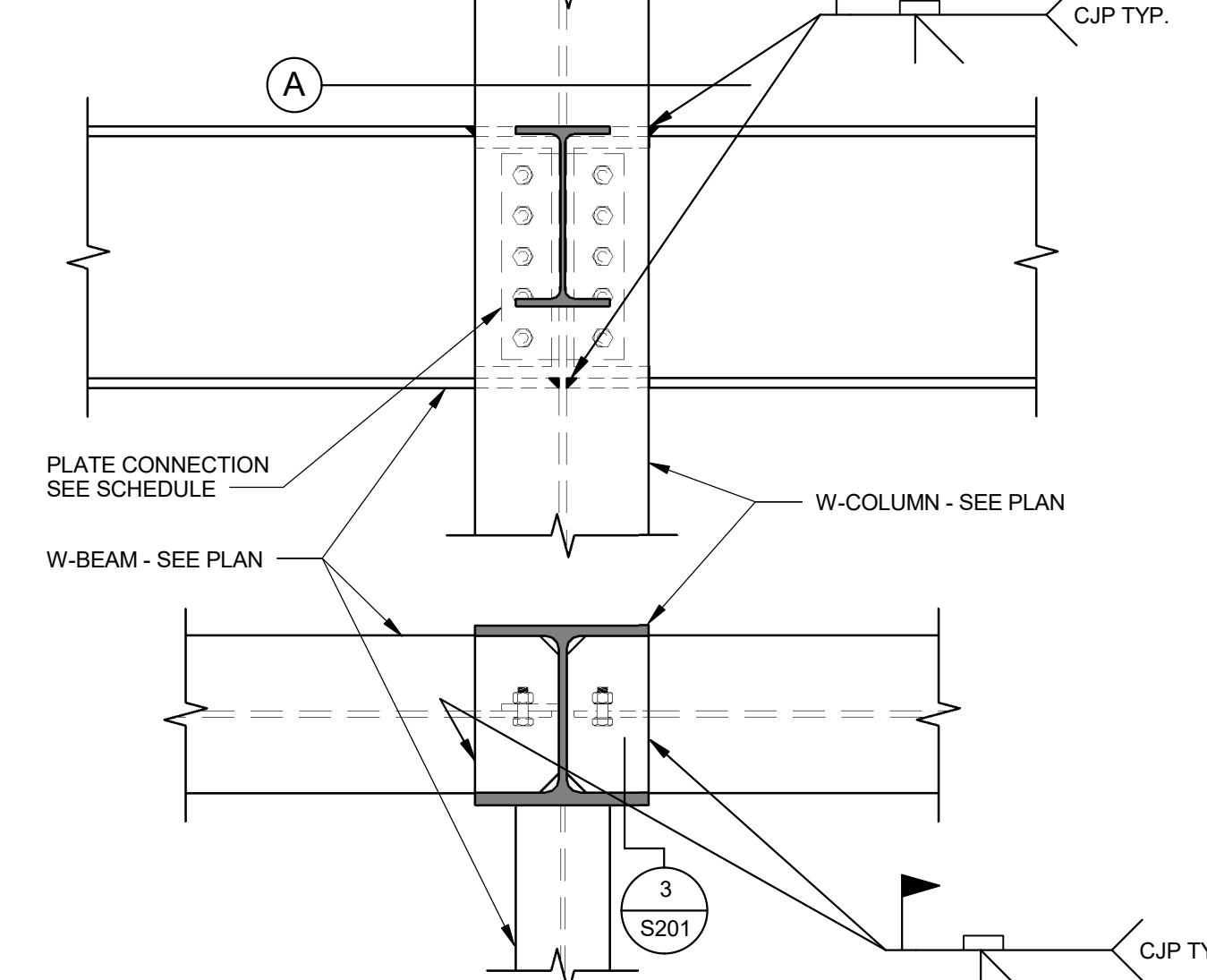
DETAIL
SCALE: NONE
6
S204



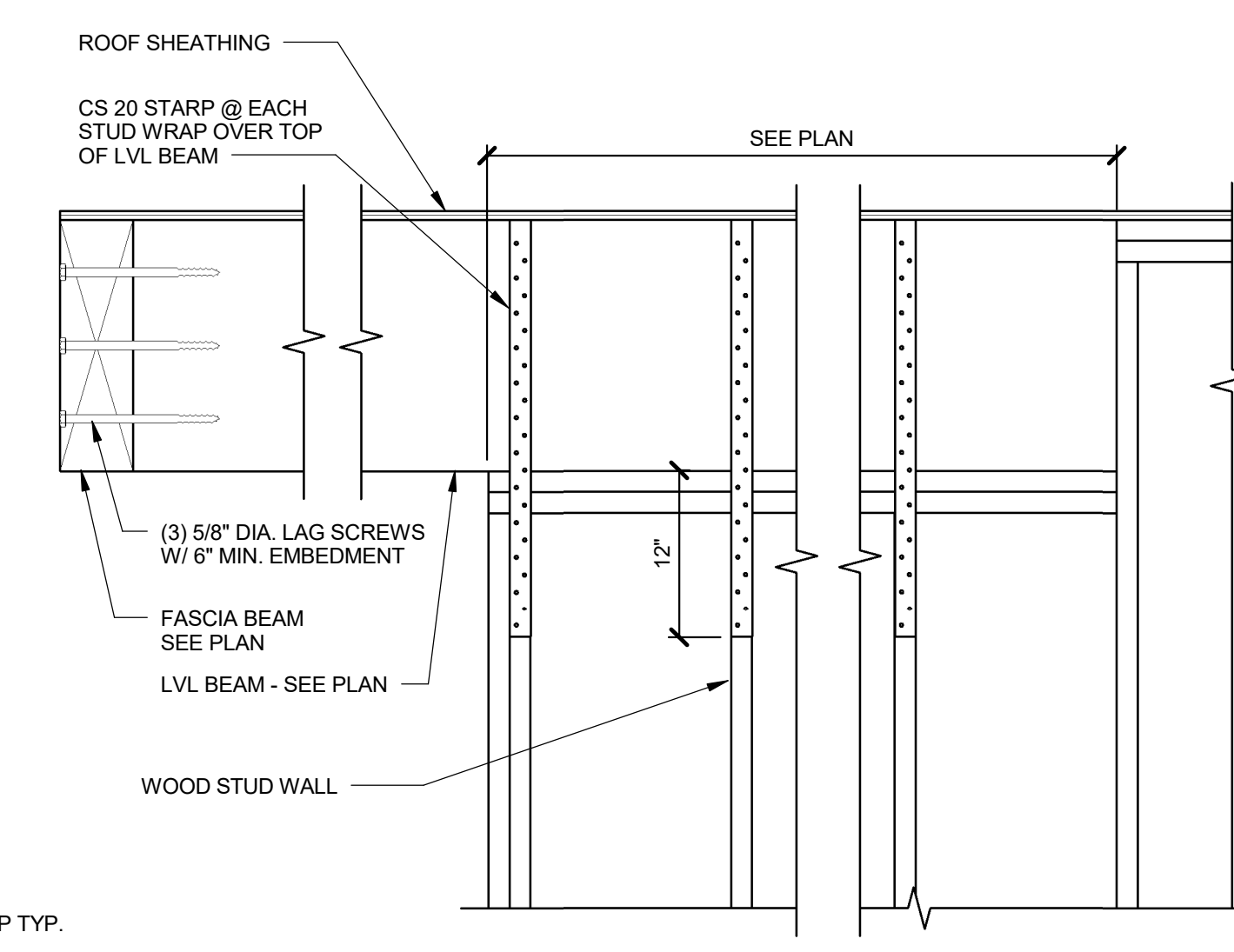
DETAIL
SCALE: NONE
7
S204



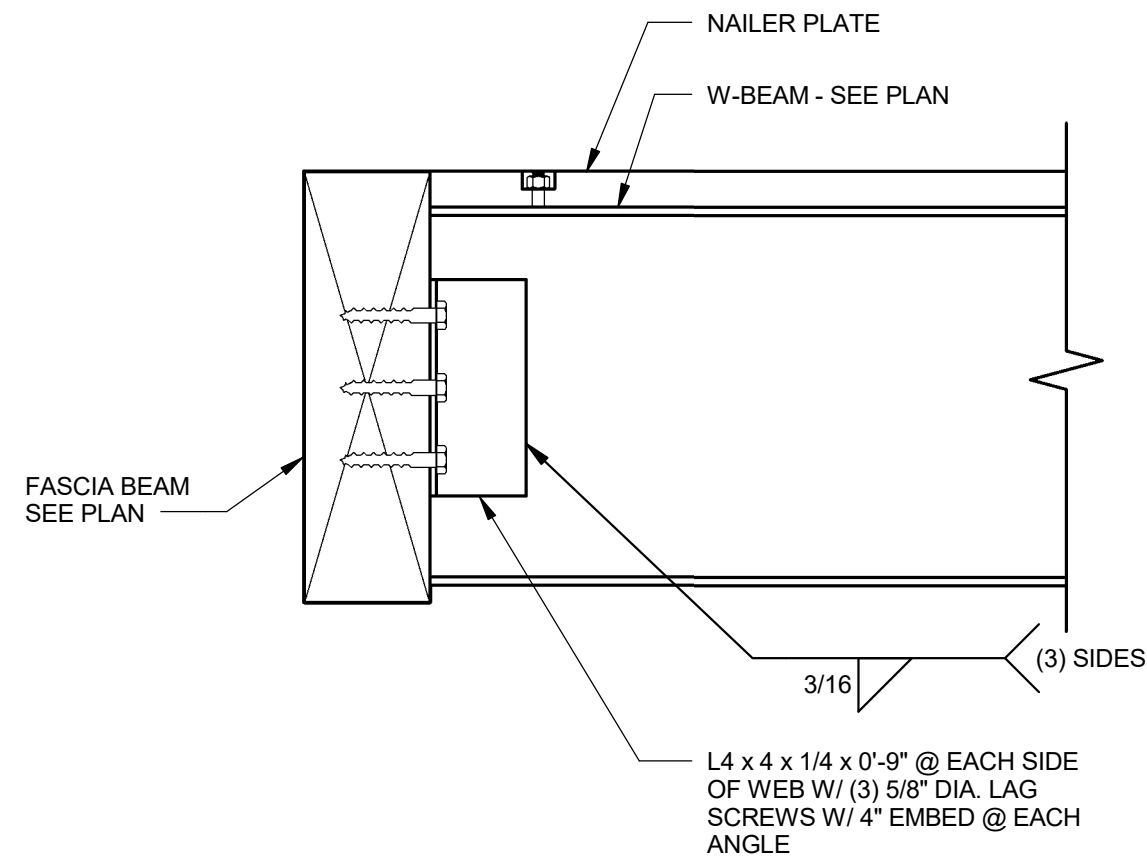
DETAIL
SCALE: NONE
8
S204



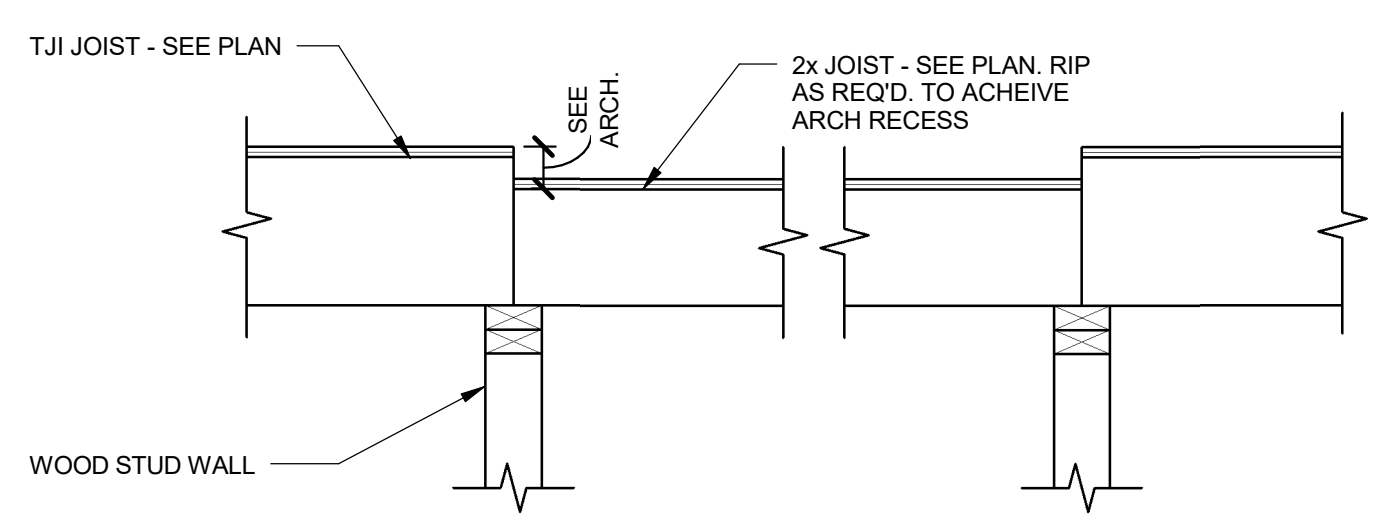
DETAIL
SCALE: NONE
9
S204



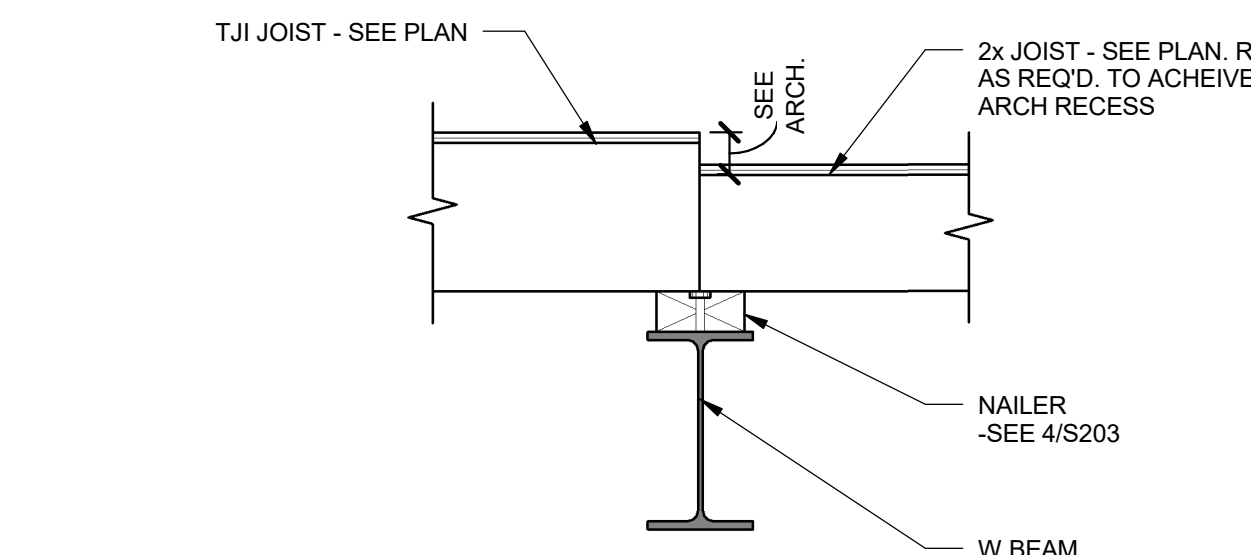
DETAIL
SCALE: NONE
10
S204



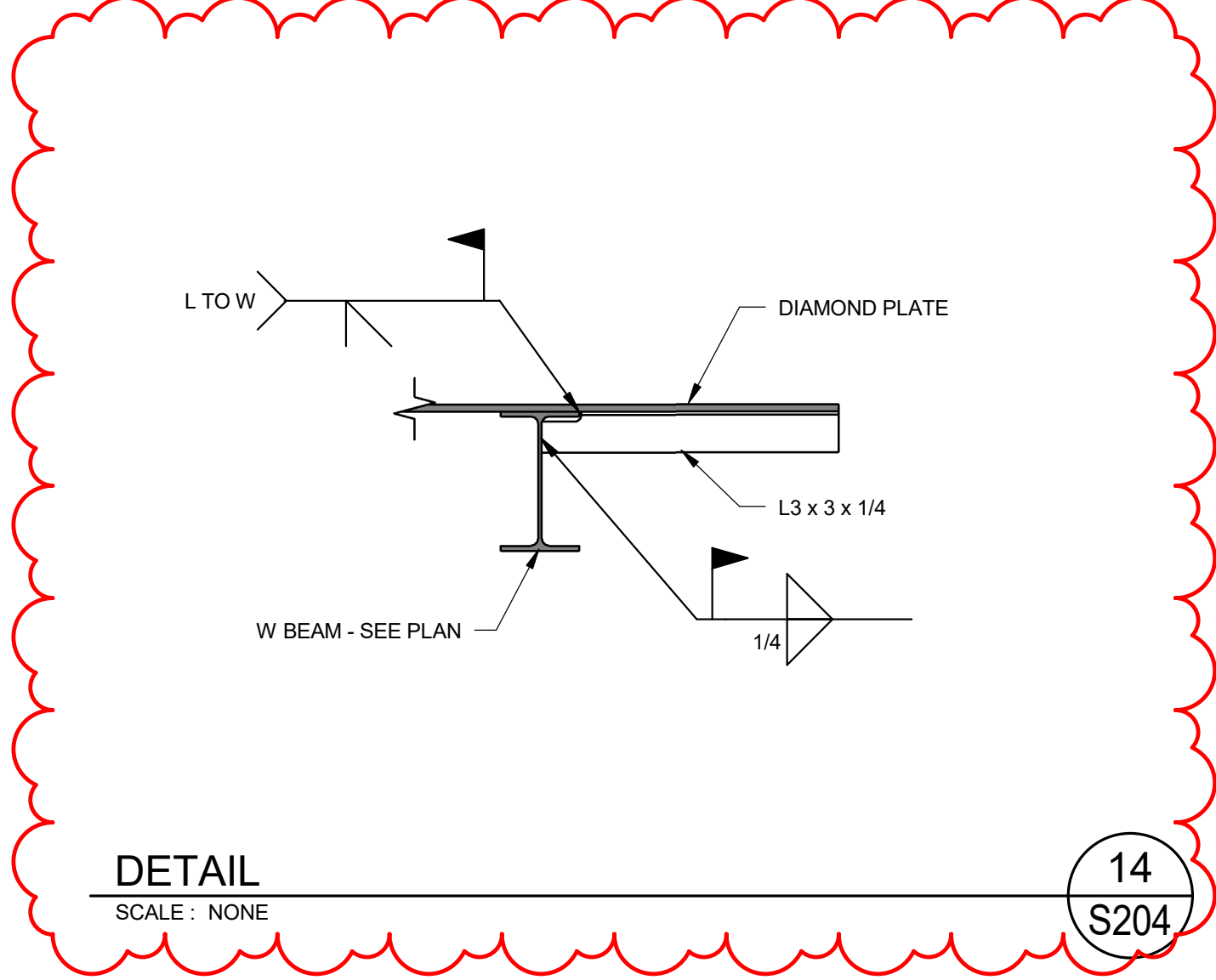
DETAIL
SCALE: NONE
11
S204



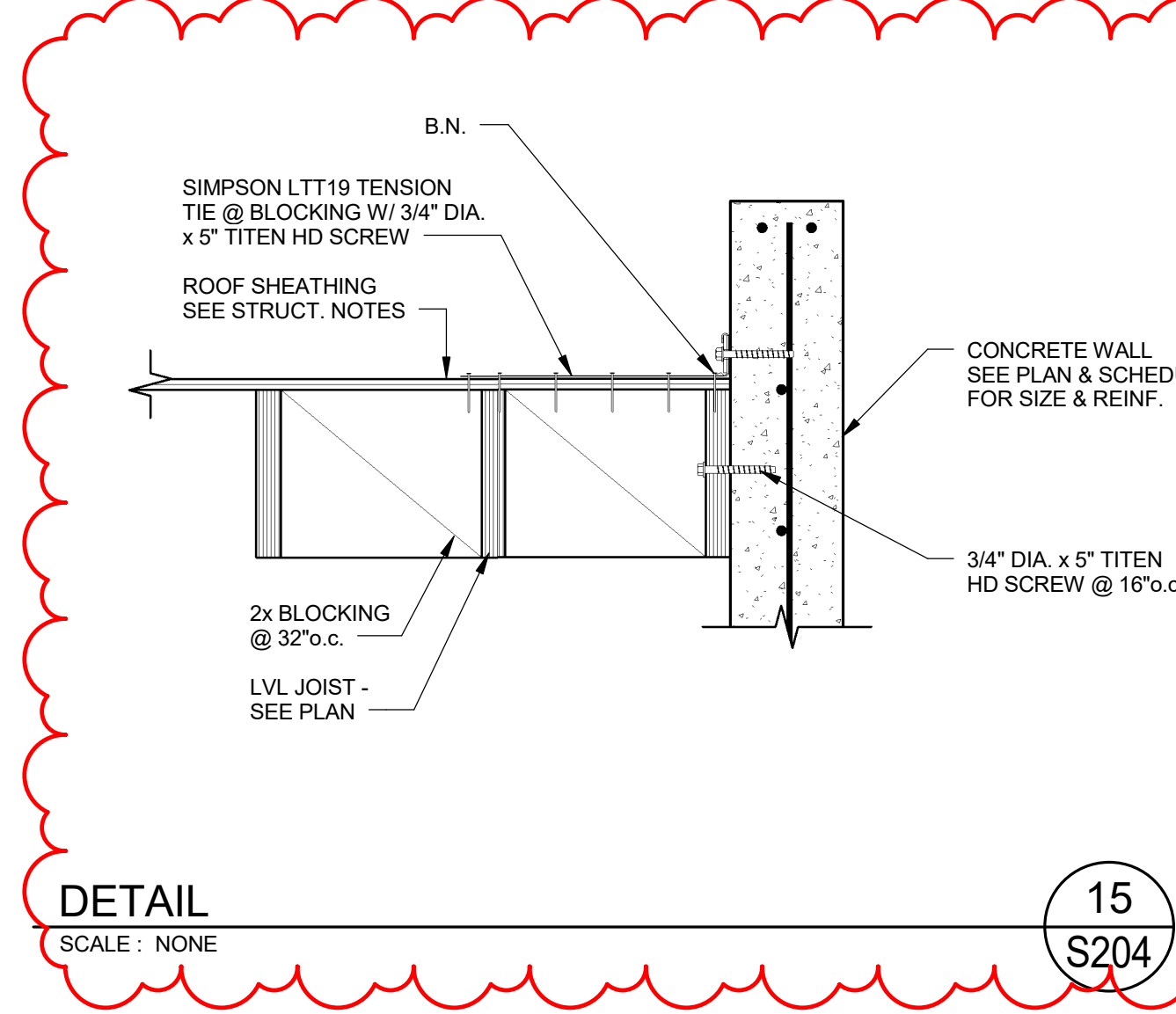
DETAIL
SCALE: NONE
12
S204



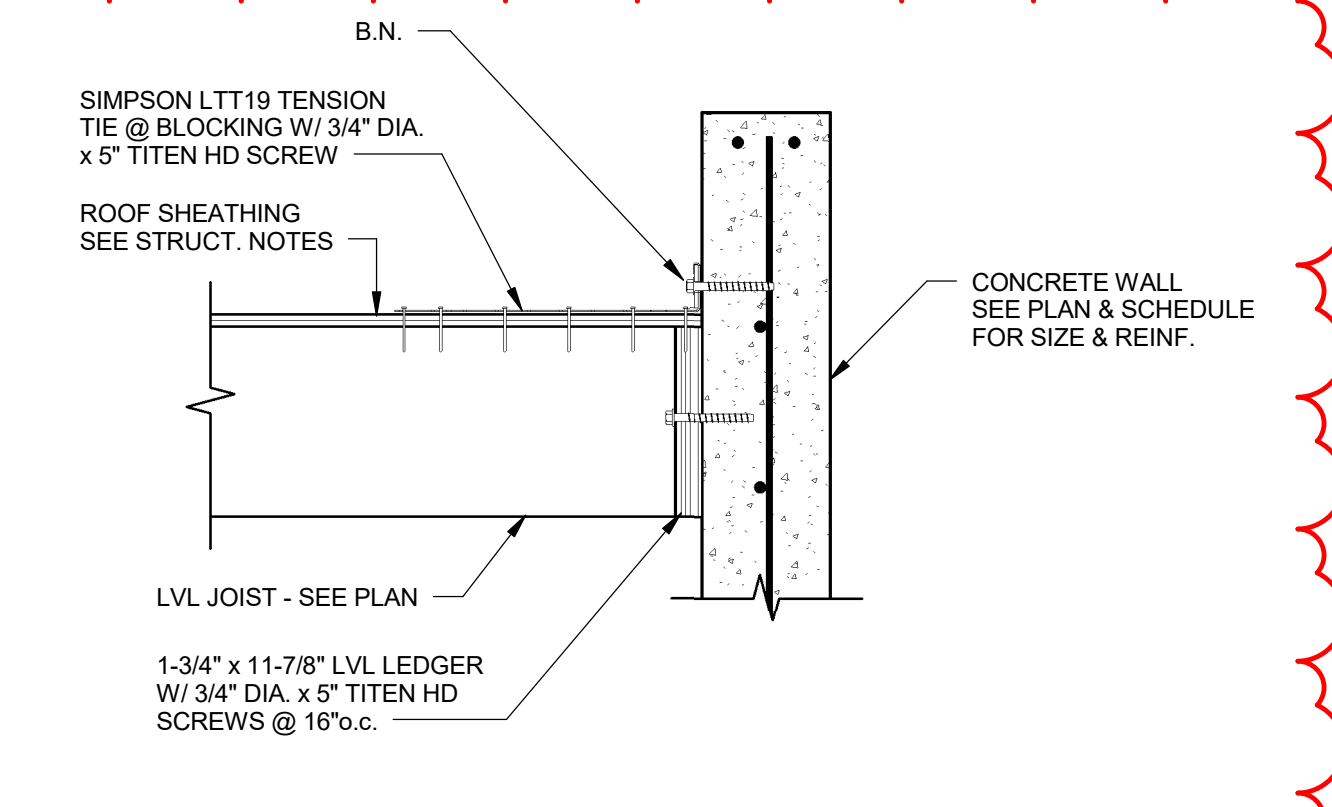
DETAIL
SCALE: NONE
13
S204



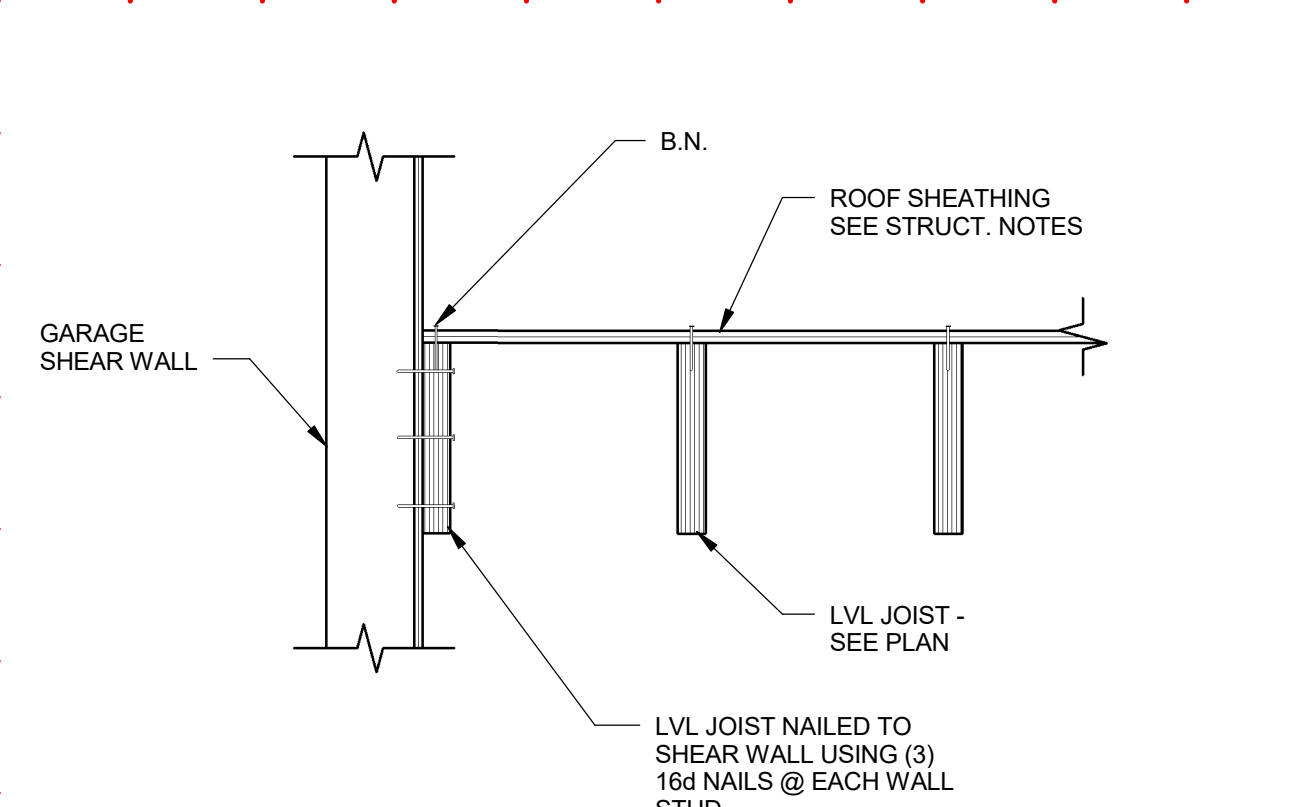
DETAIL
SCALE: NONE
14
S204



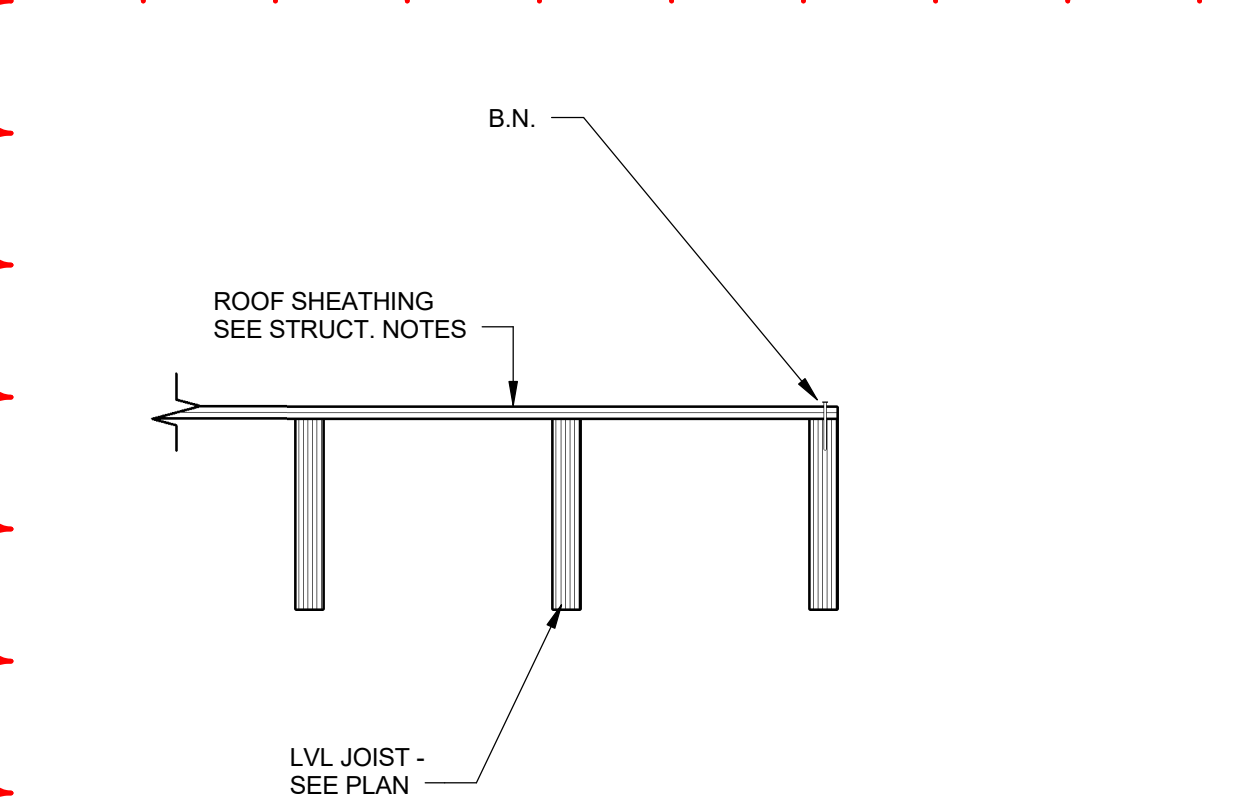
DETAIL
SCALE: NONE
15
S204



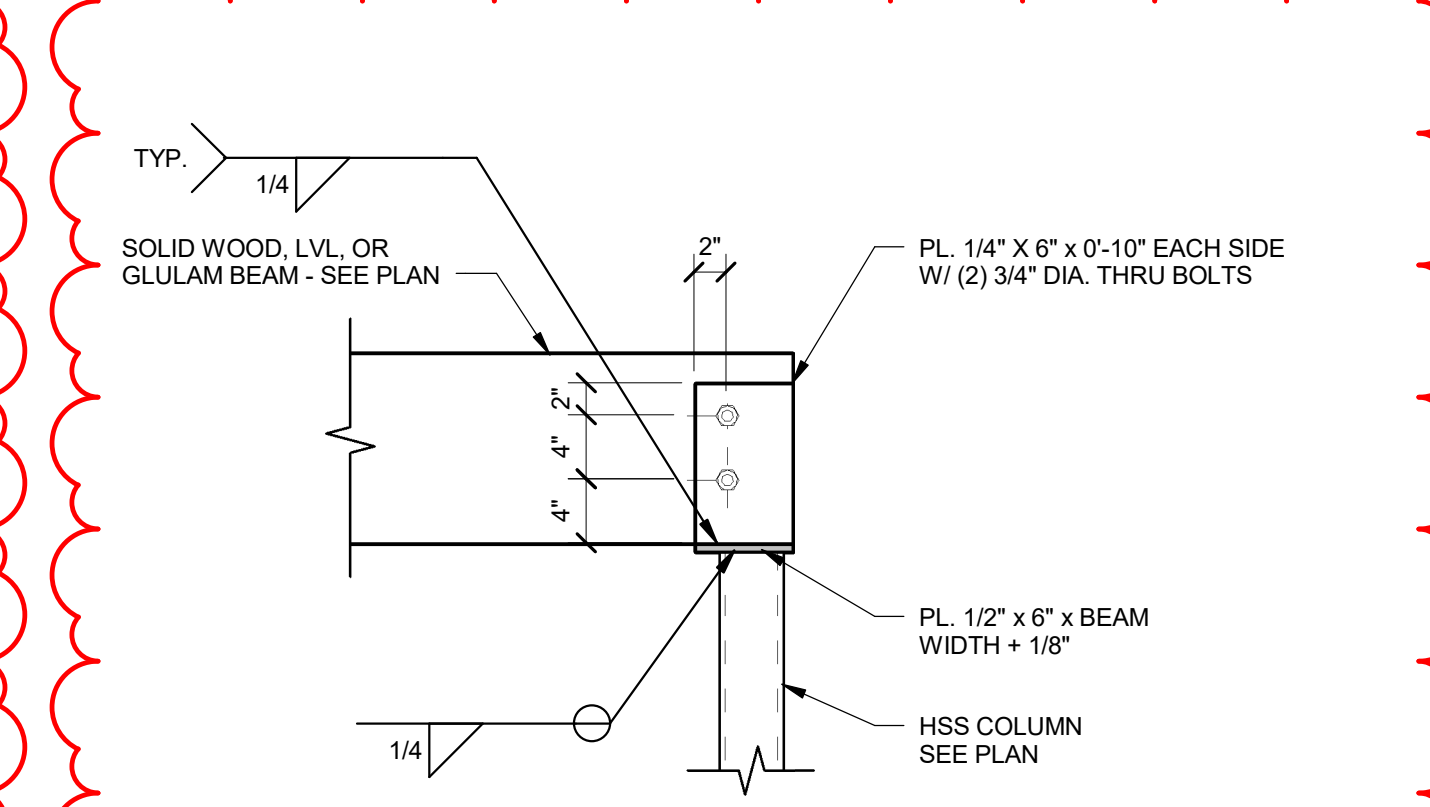
DETAIL
SCALE: NONE
16
S204



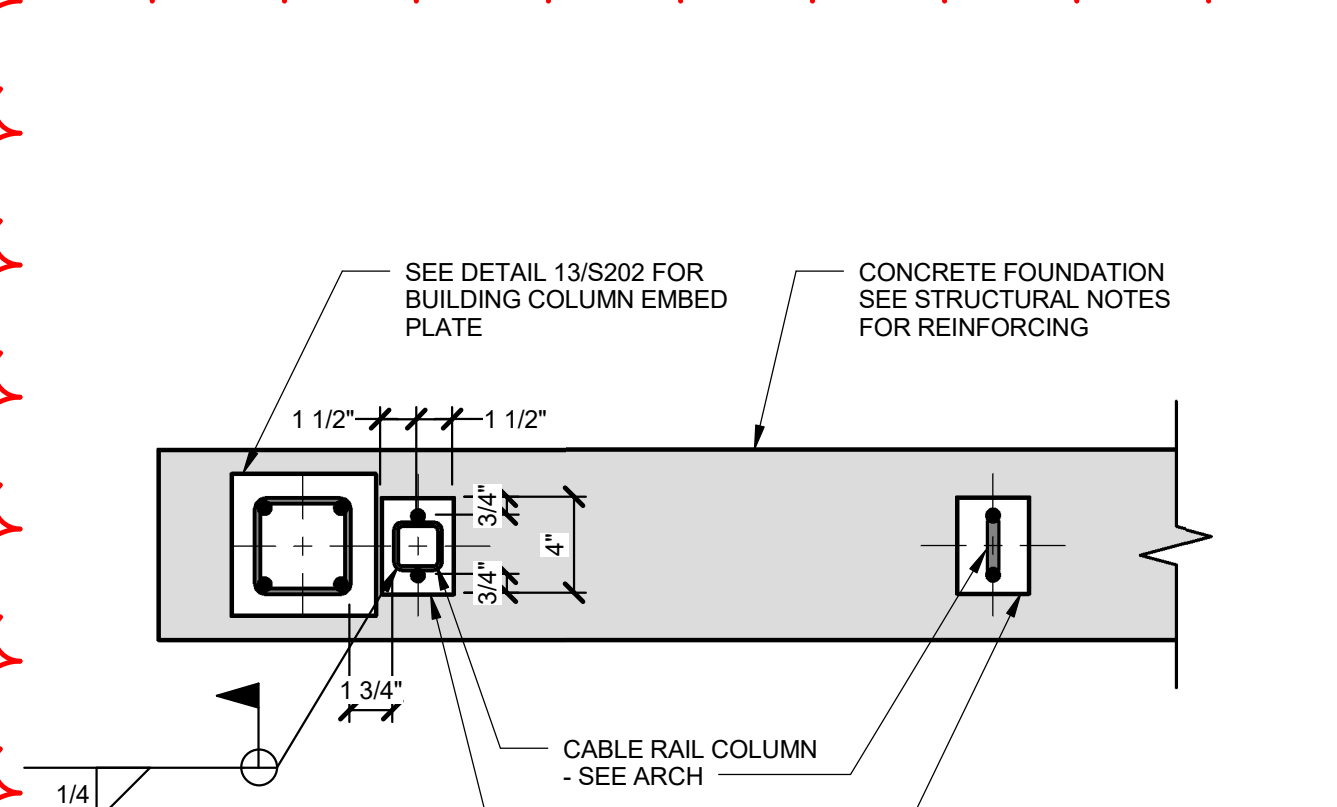
DETAIL
SCALE: NONE
17
S204



DETAIL
SCALE: NONE
18
S204

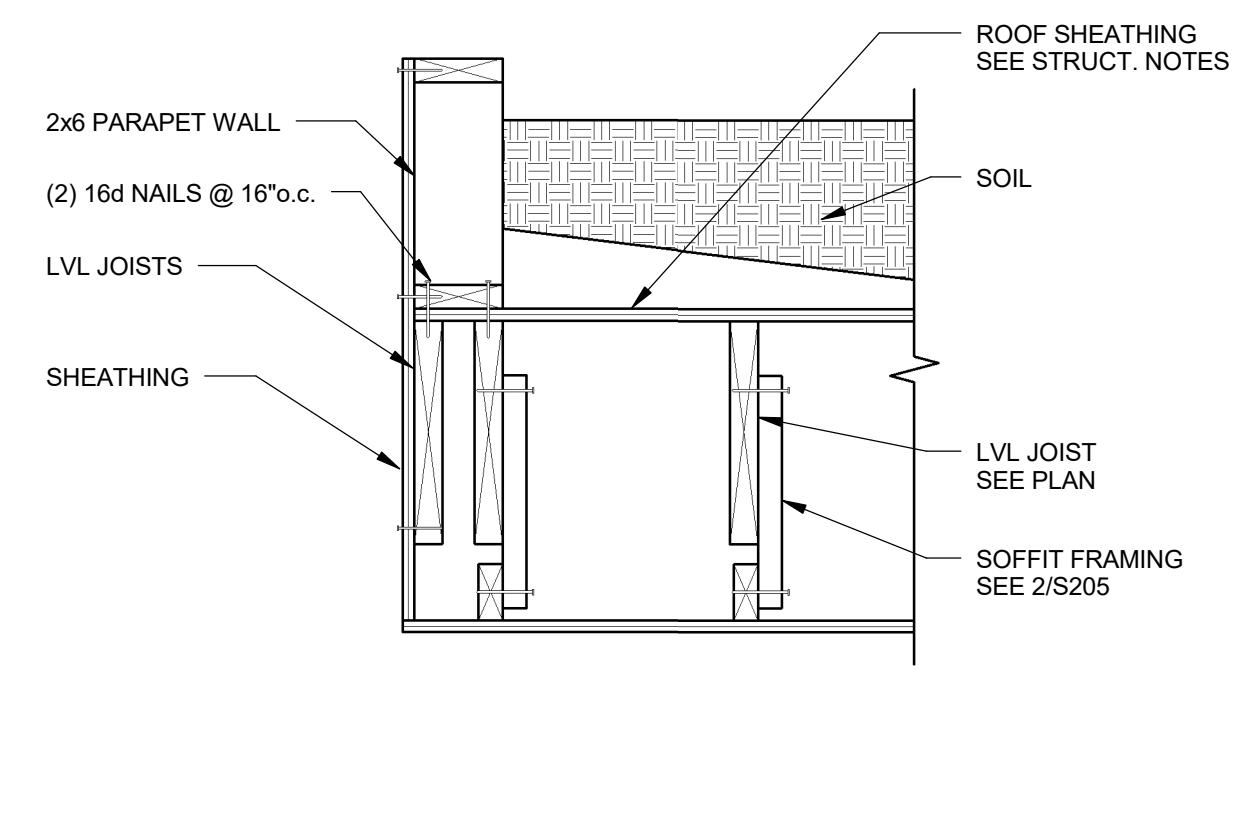


DETAIL
SCALE: NONE
19
S204

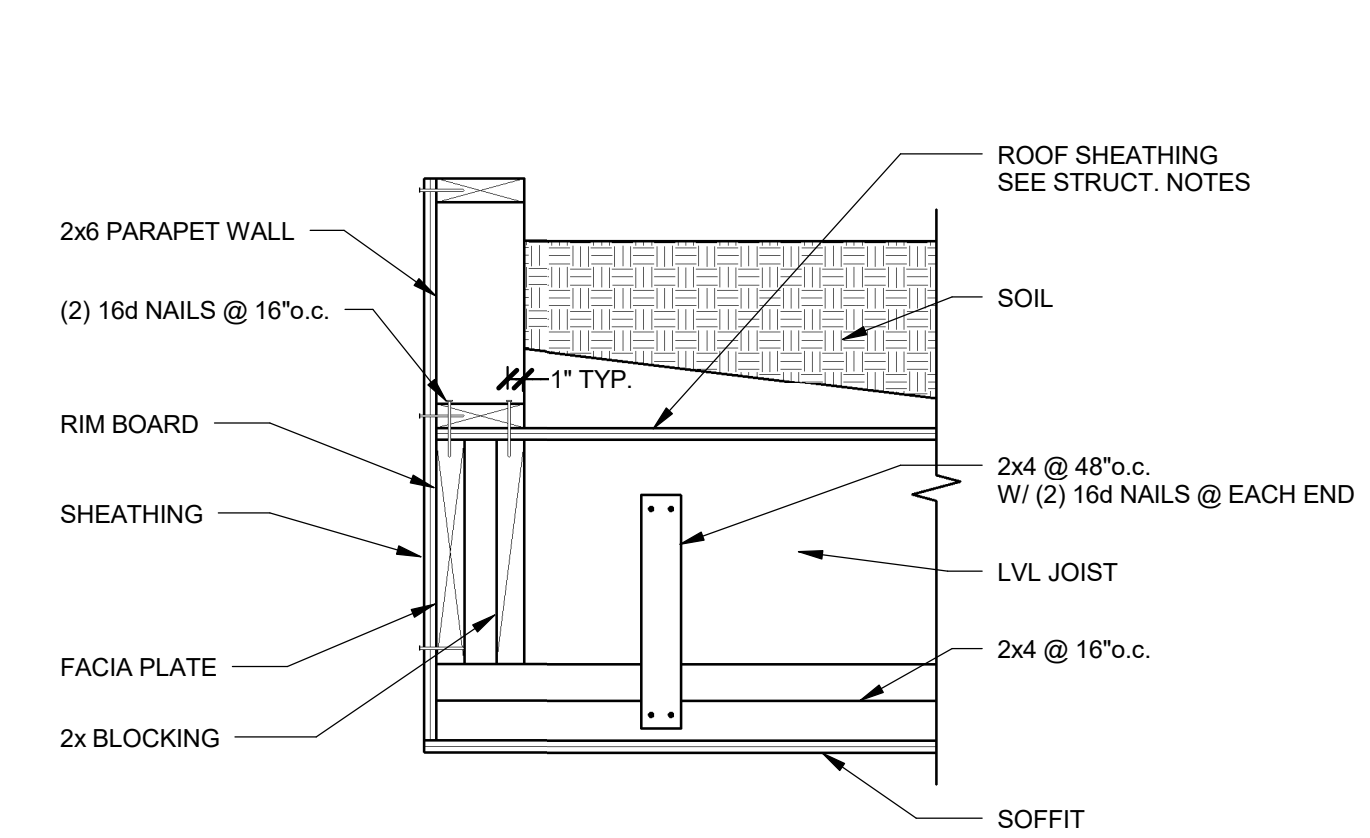


DETAIL
SCALE: NONE
20
S204

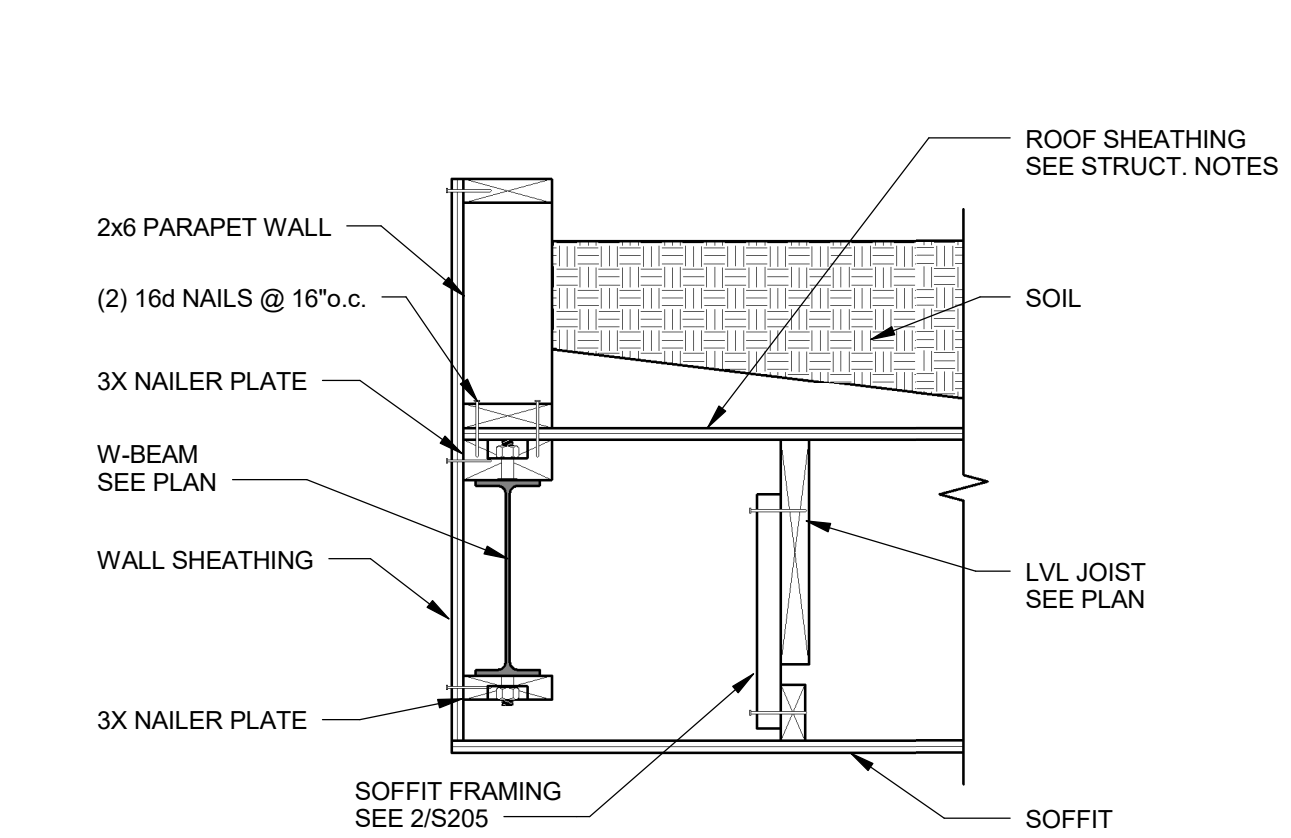
FOR CONSTRUCTION



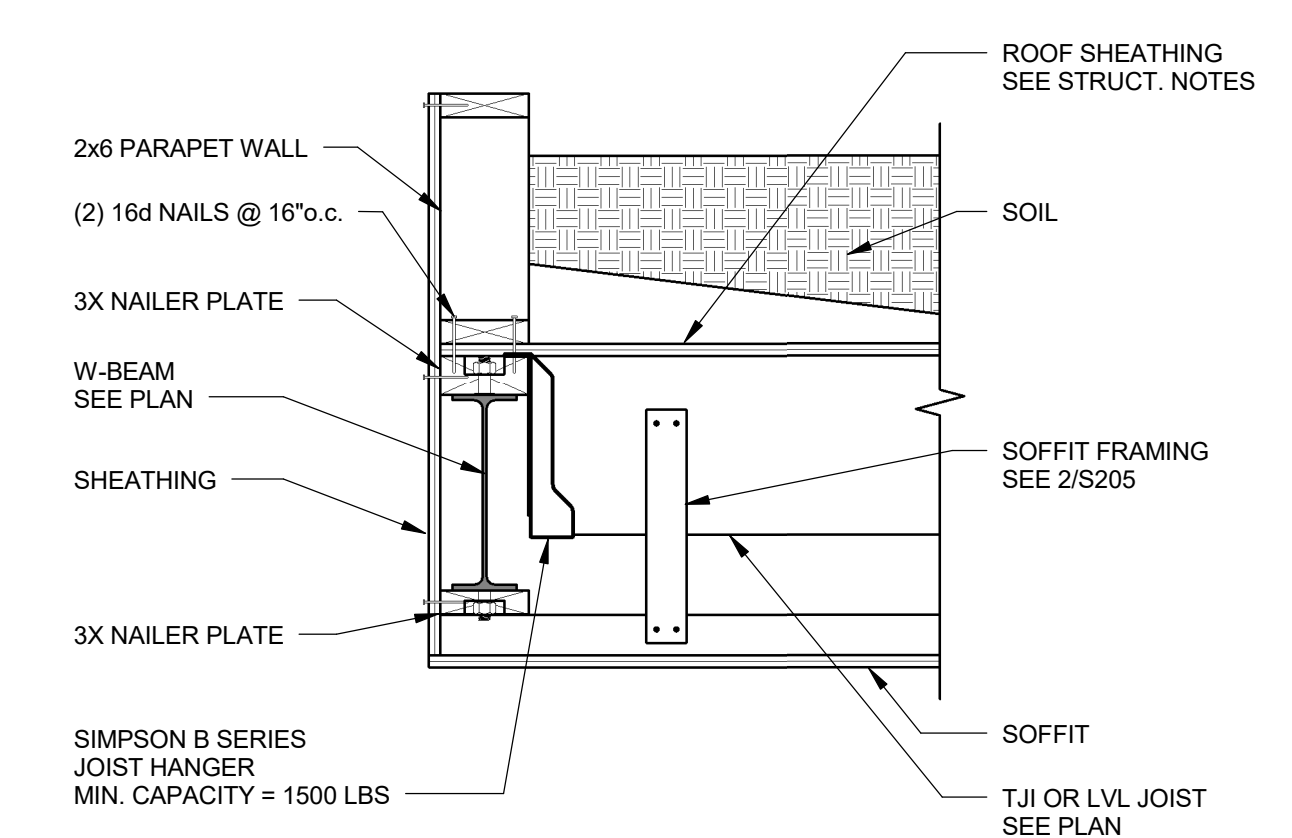
1
HIGH ROOF PARALLEL TO JOIST
SCALE: NONE
S205



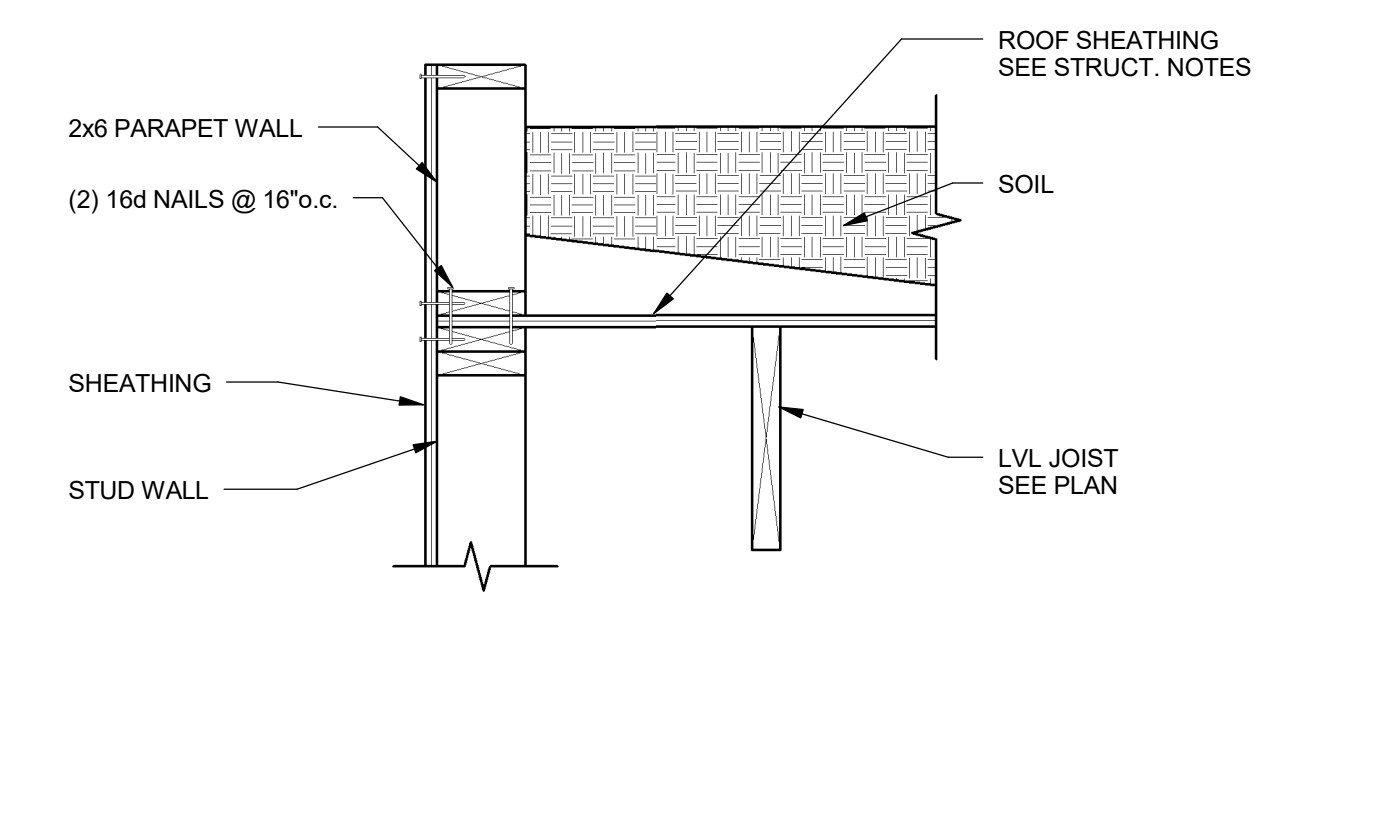
2
HIGH ROOF PERPENDICULAR TO JOIST
SCALE: NONE
S205



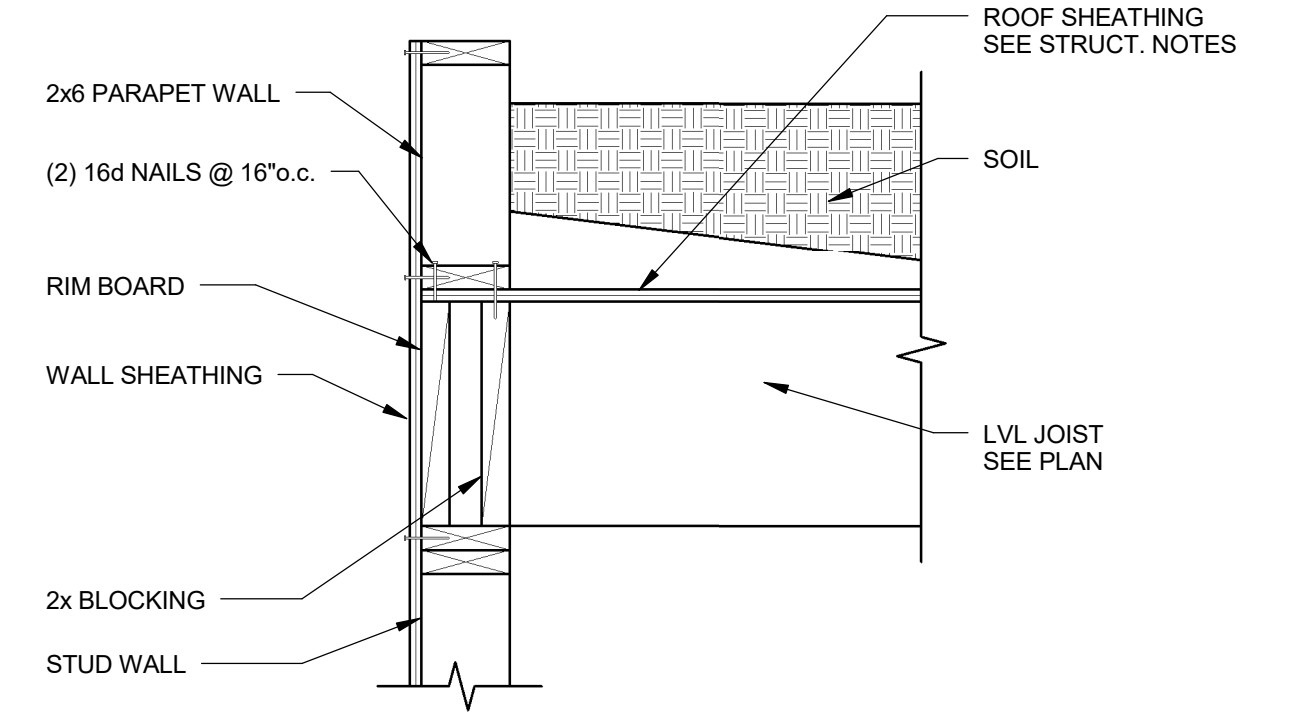
3
HIGH ROOF PERPENDICULAR TO JOIST
SCALE: NONE
S205



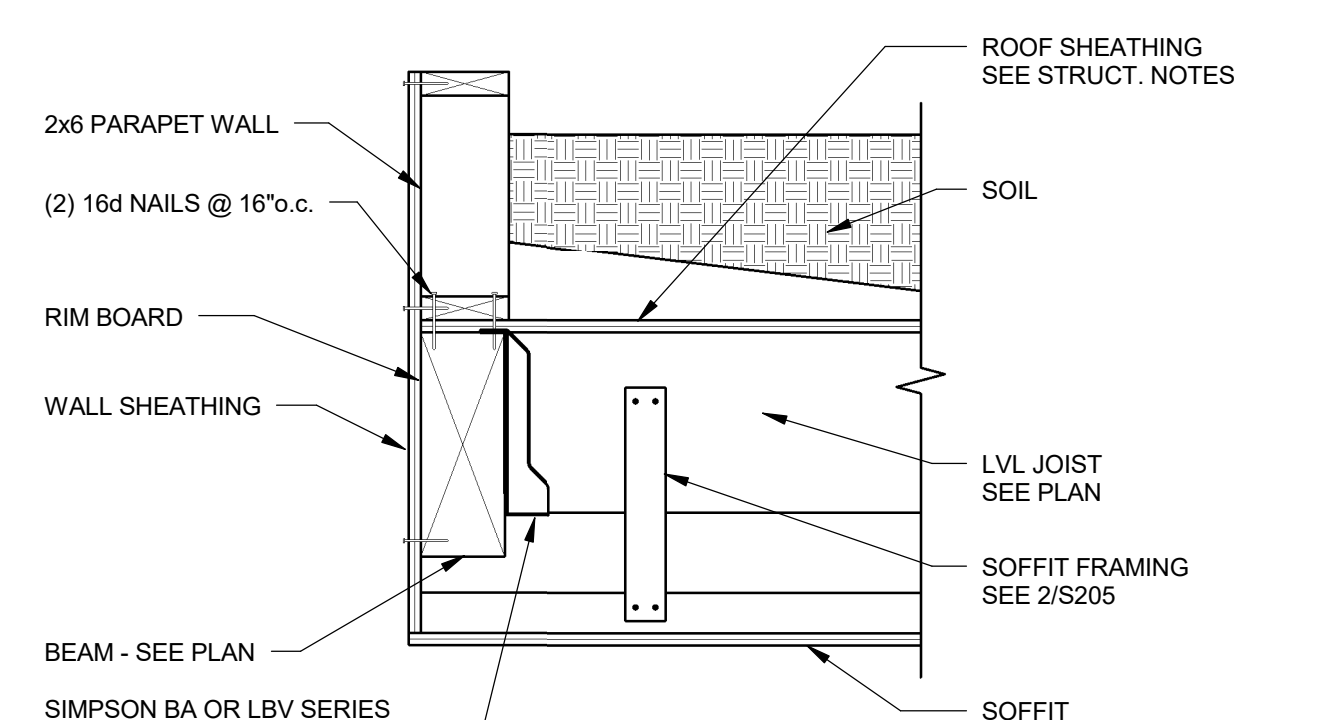
4
DETAIL
SCALE: NONE
S205



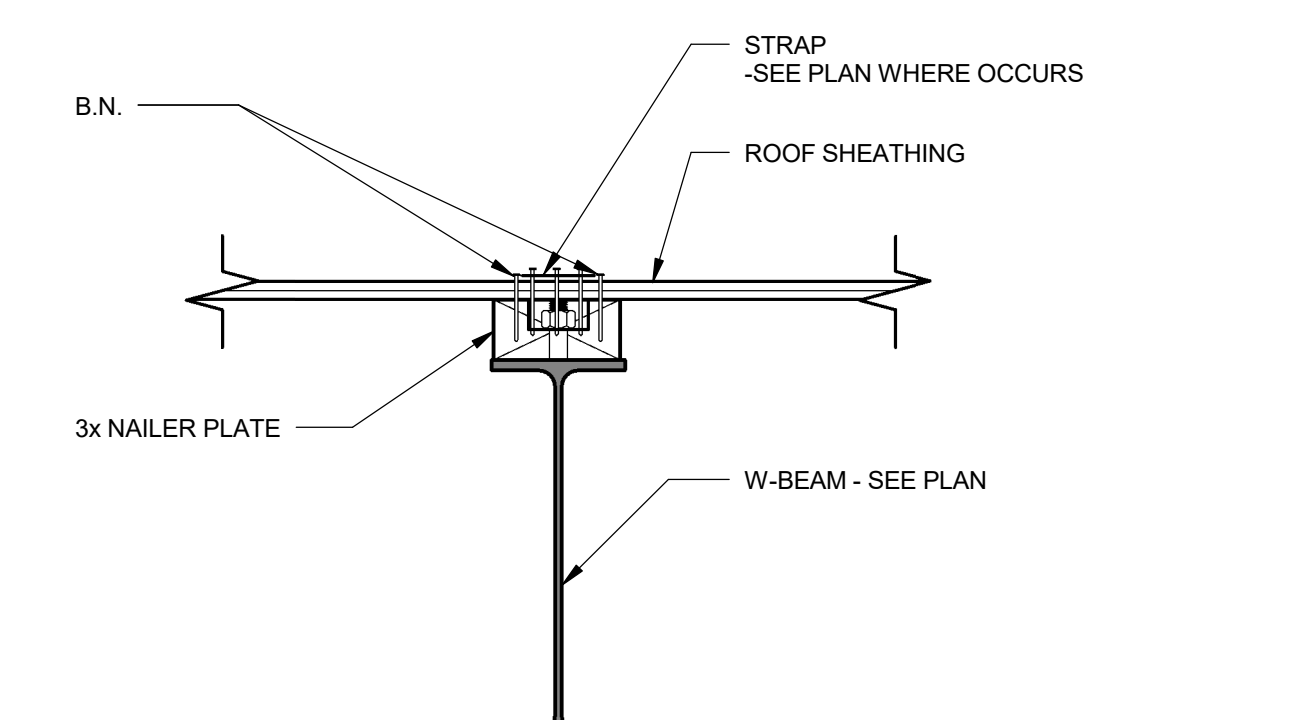
5
DETAIL
SCALE: NONE
S205



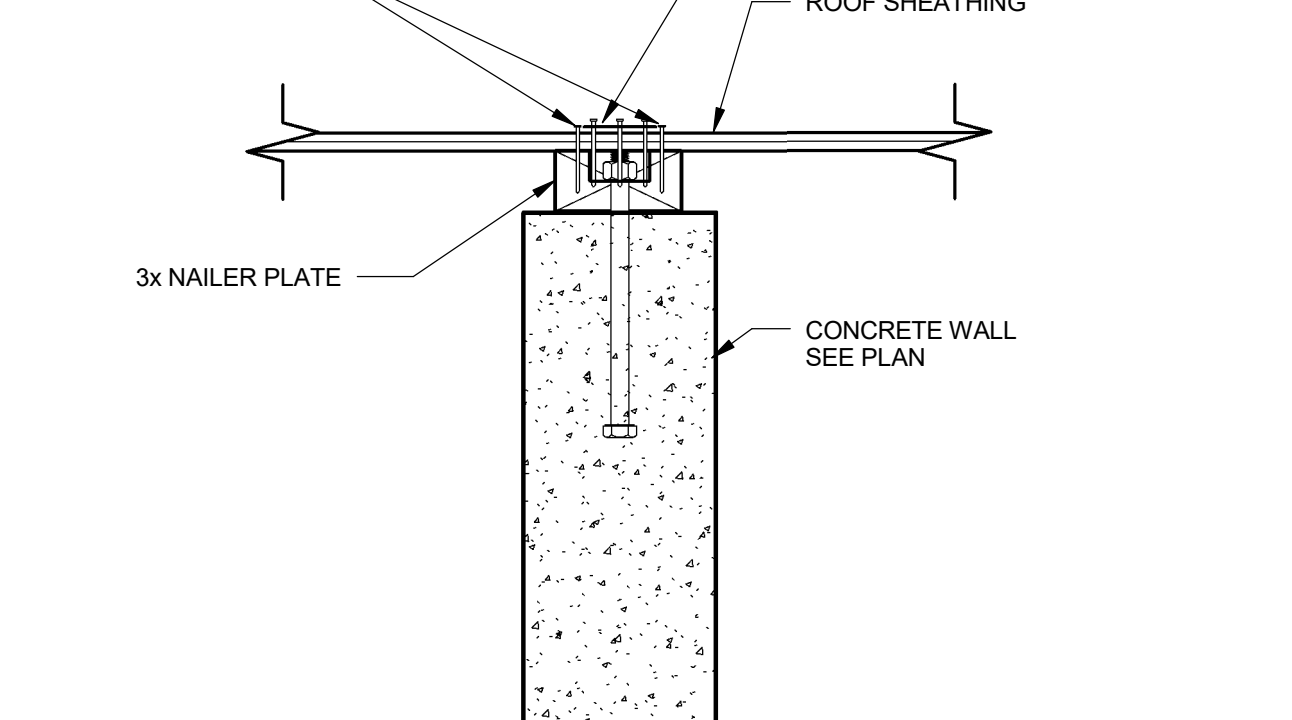
6
DETAIL
SCALE: NONE
S205



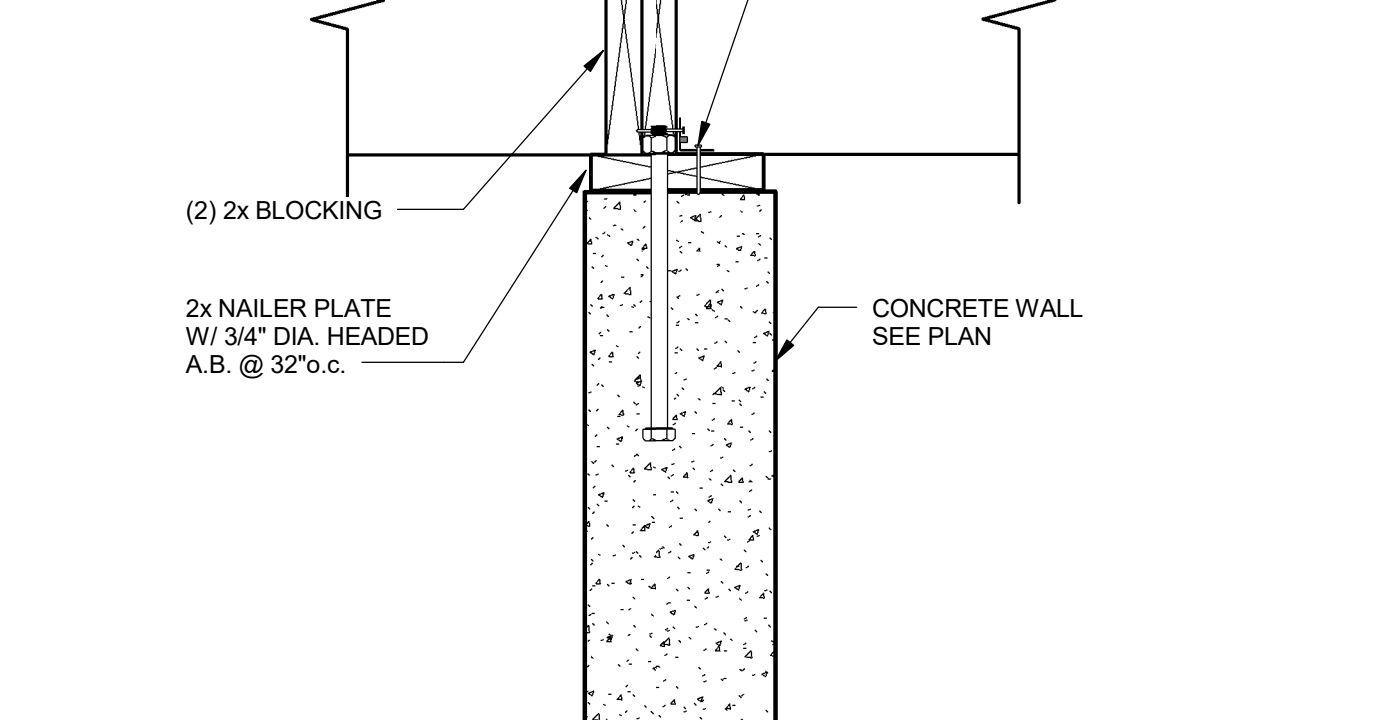
7
DETAIL
SCALE: NONE
S205



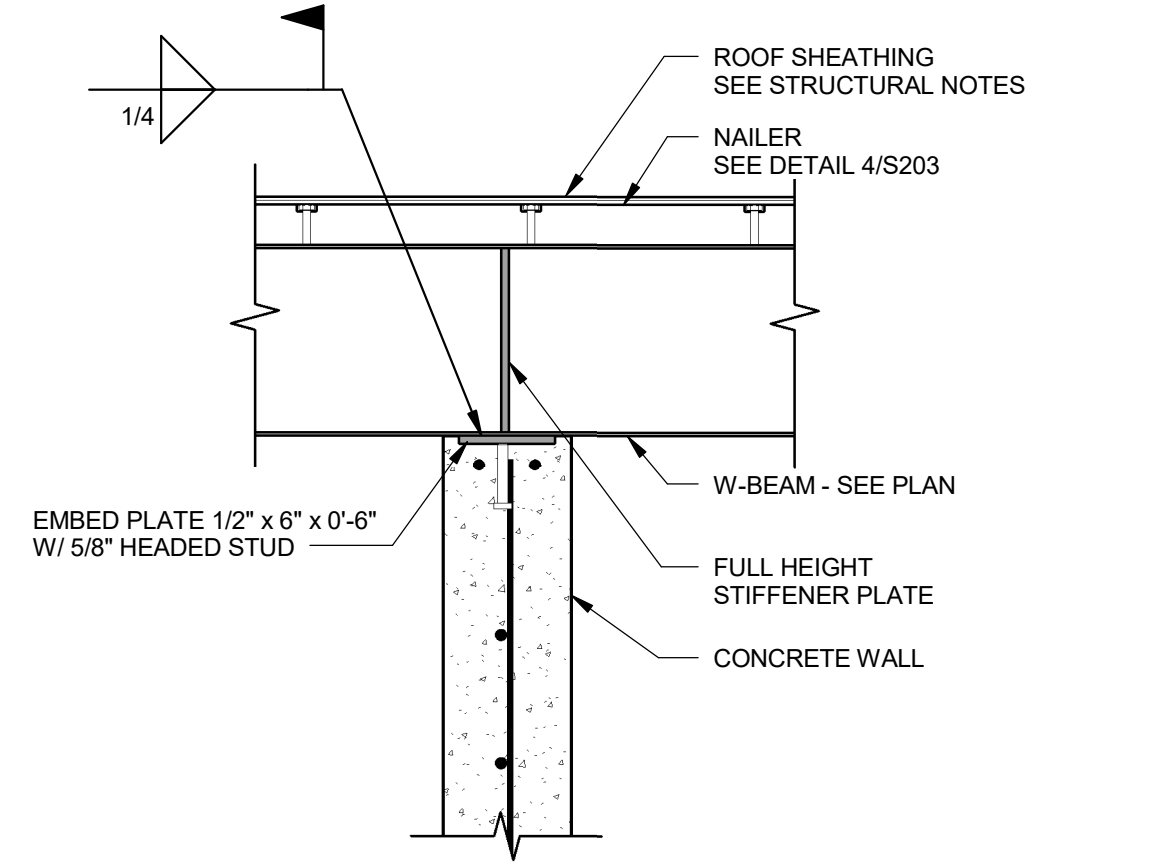
8
DETAIL
SCALE: NONE
S205



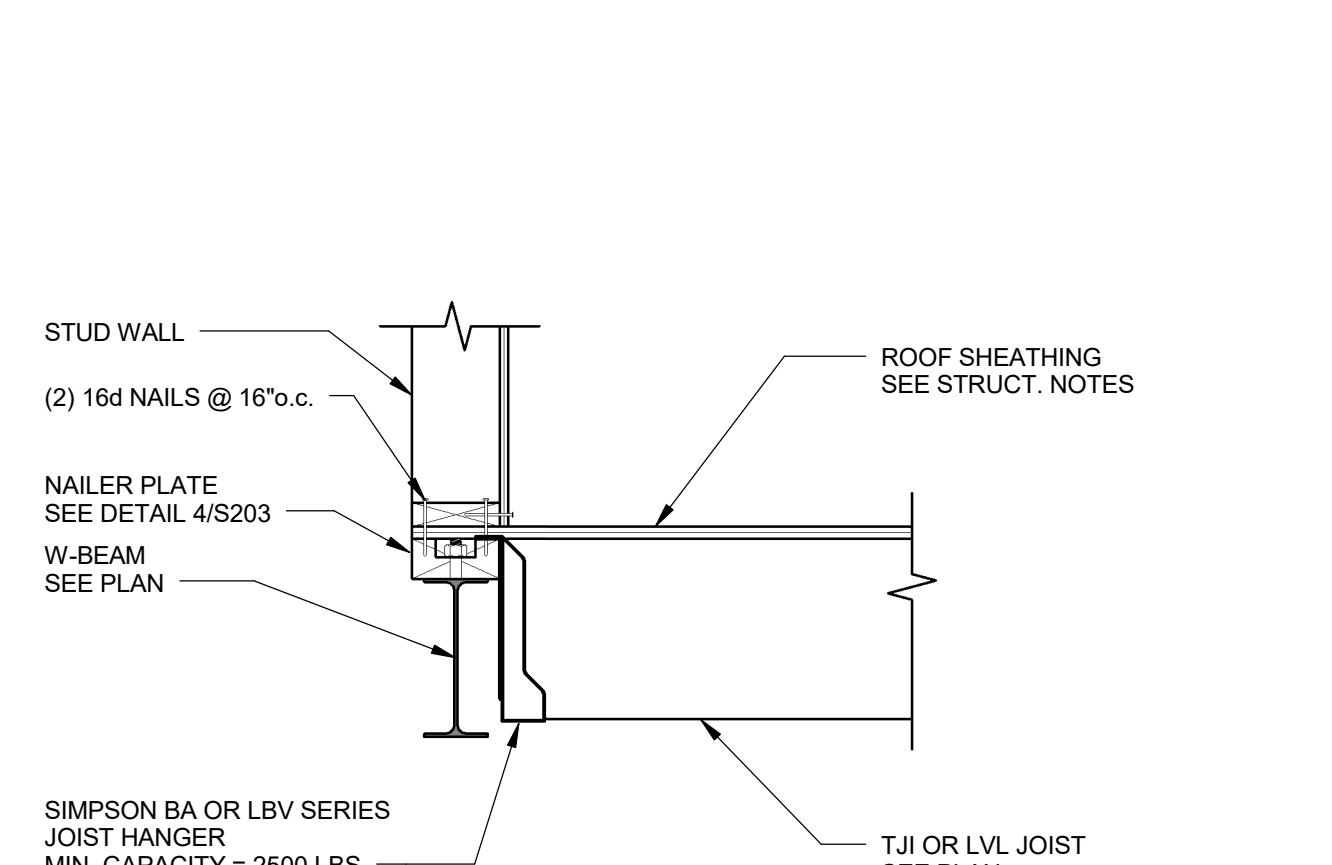
9
DETAIL
SCALE: NONE
S205



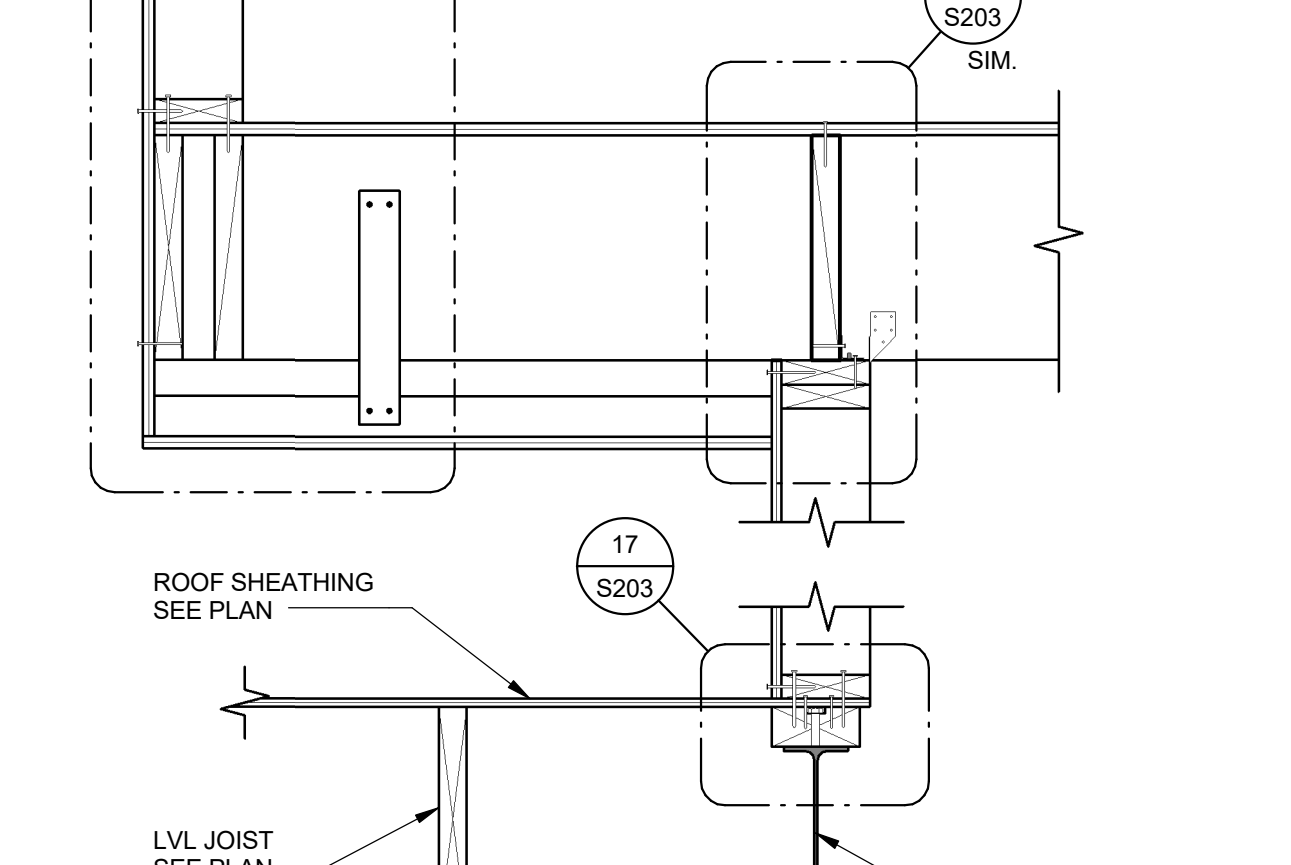
10
DETAIL
SCALE: NONE
S205



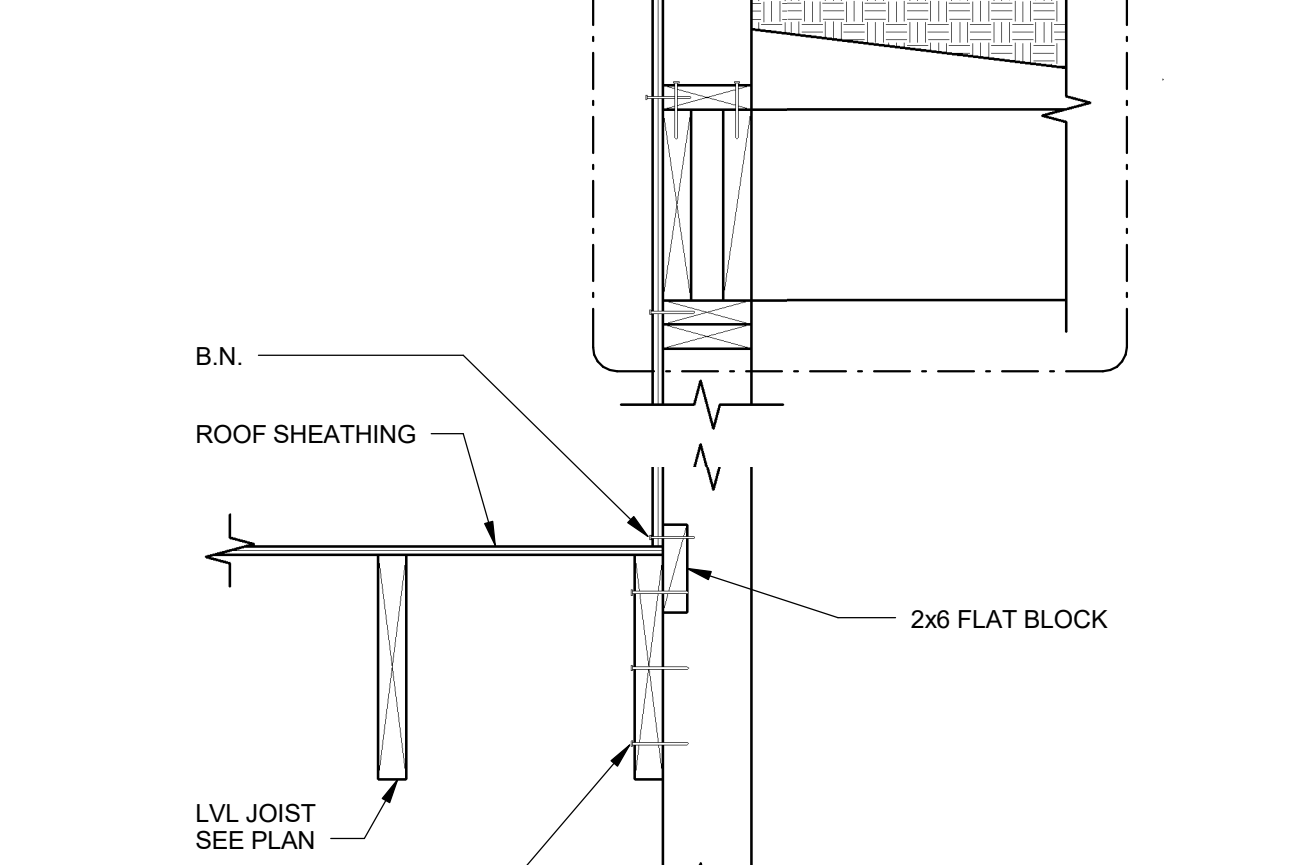
11
DETAIL
SCALE: NONE
S205



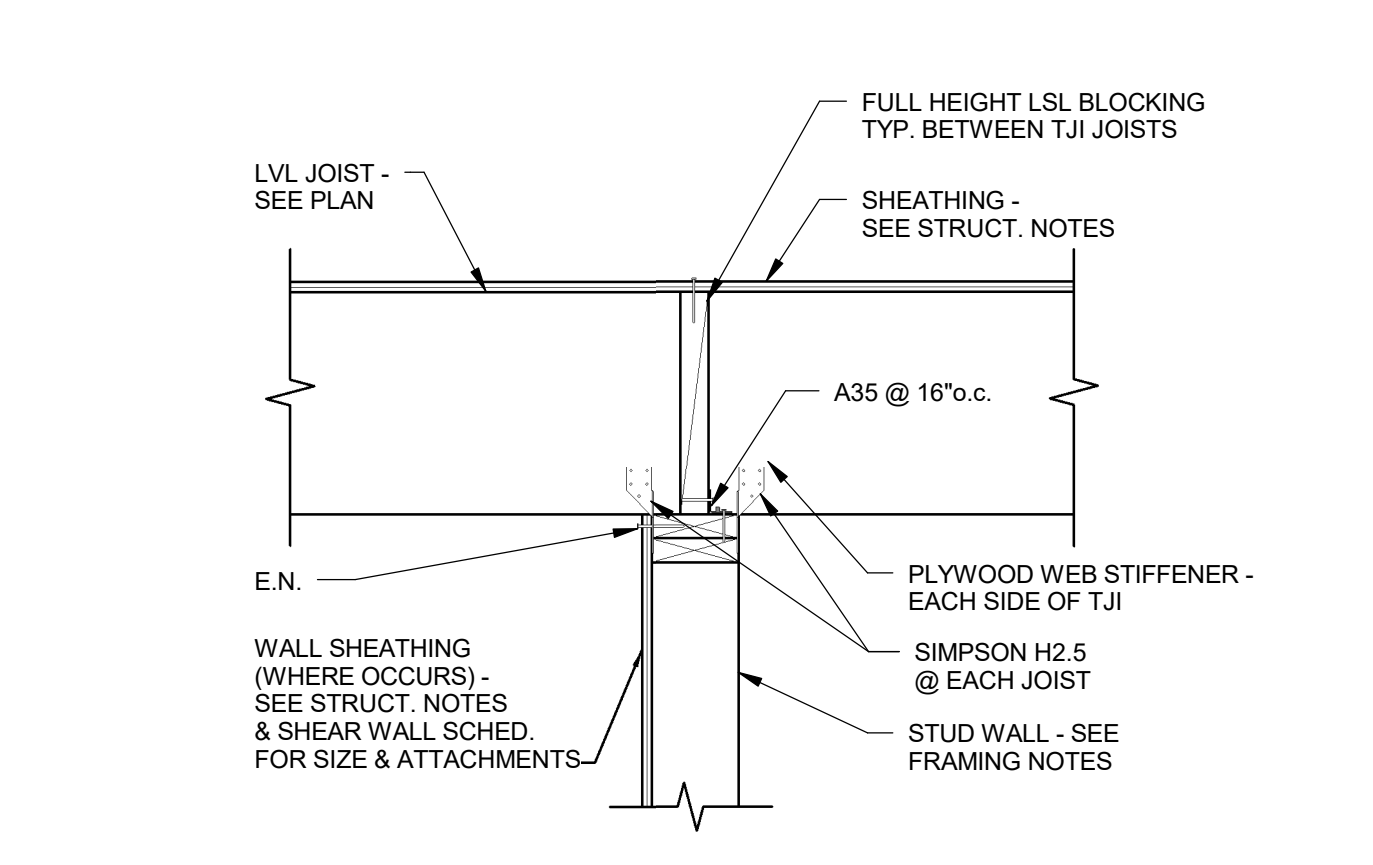
12
DETAIL
SCALE: NONE
S205



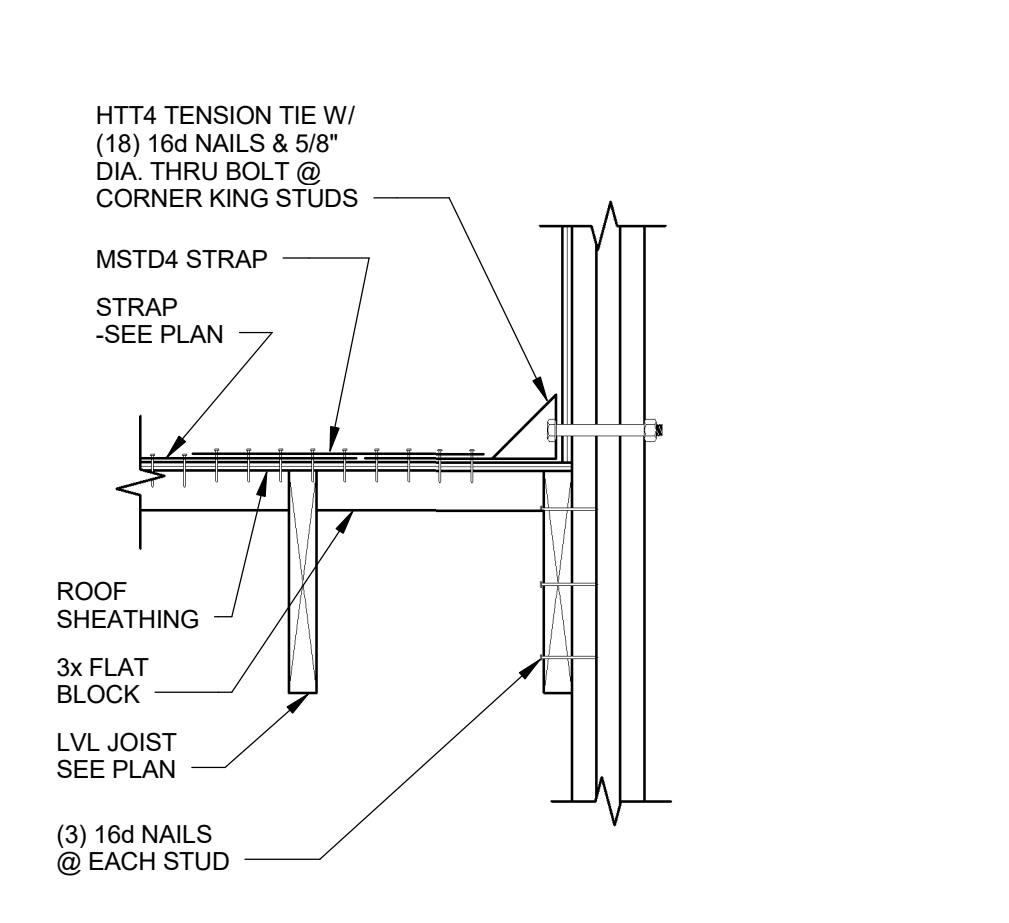
13
DETAIL
SCALE: NONE
S205



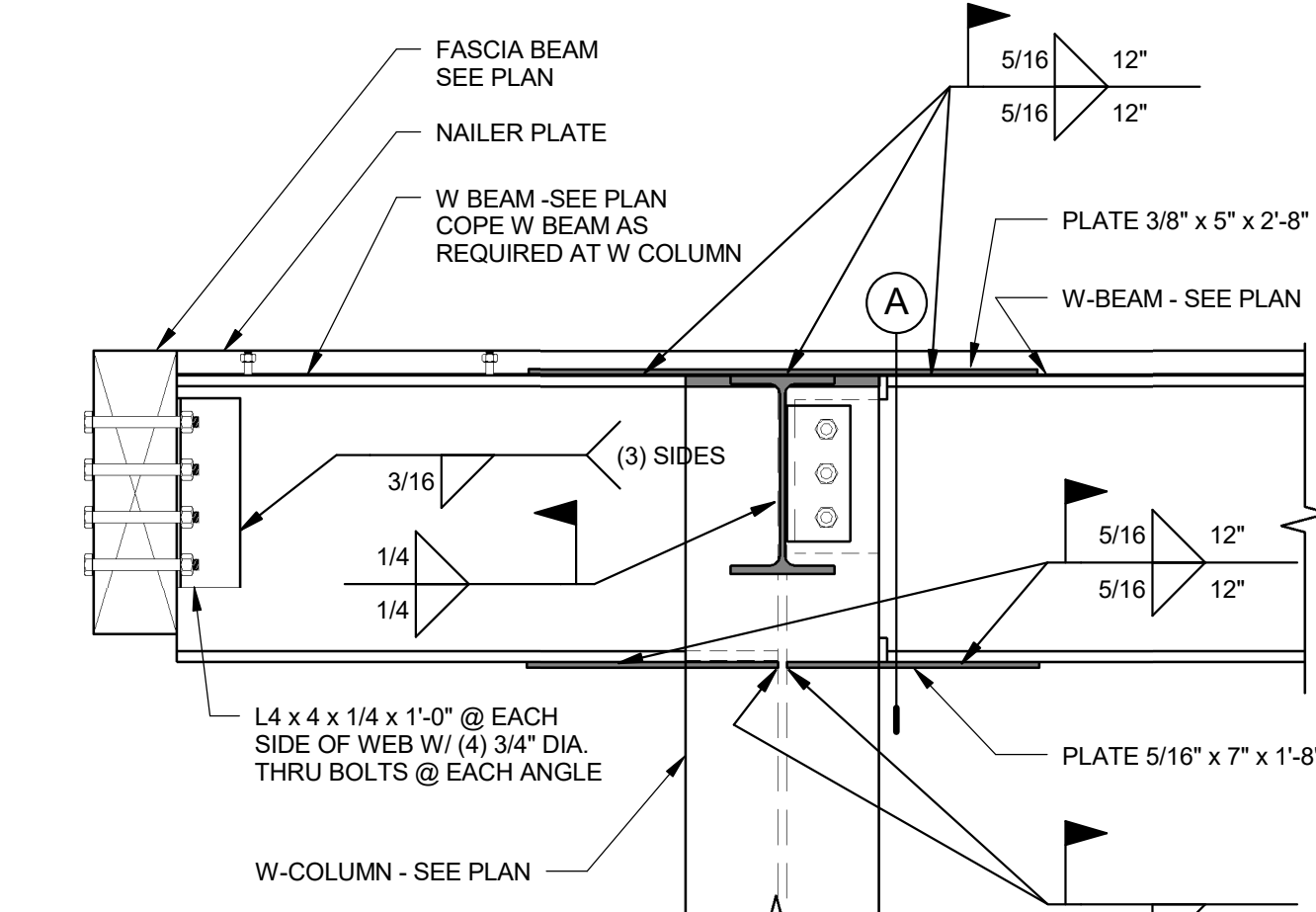
14
DETAIL
SCALE: NONE
S205



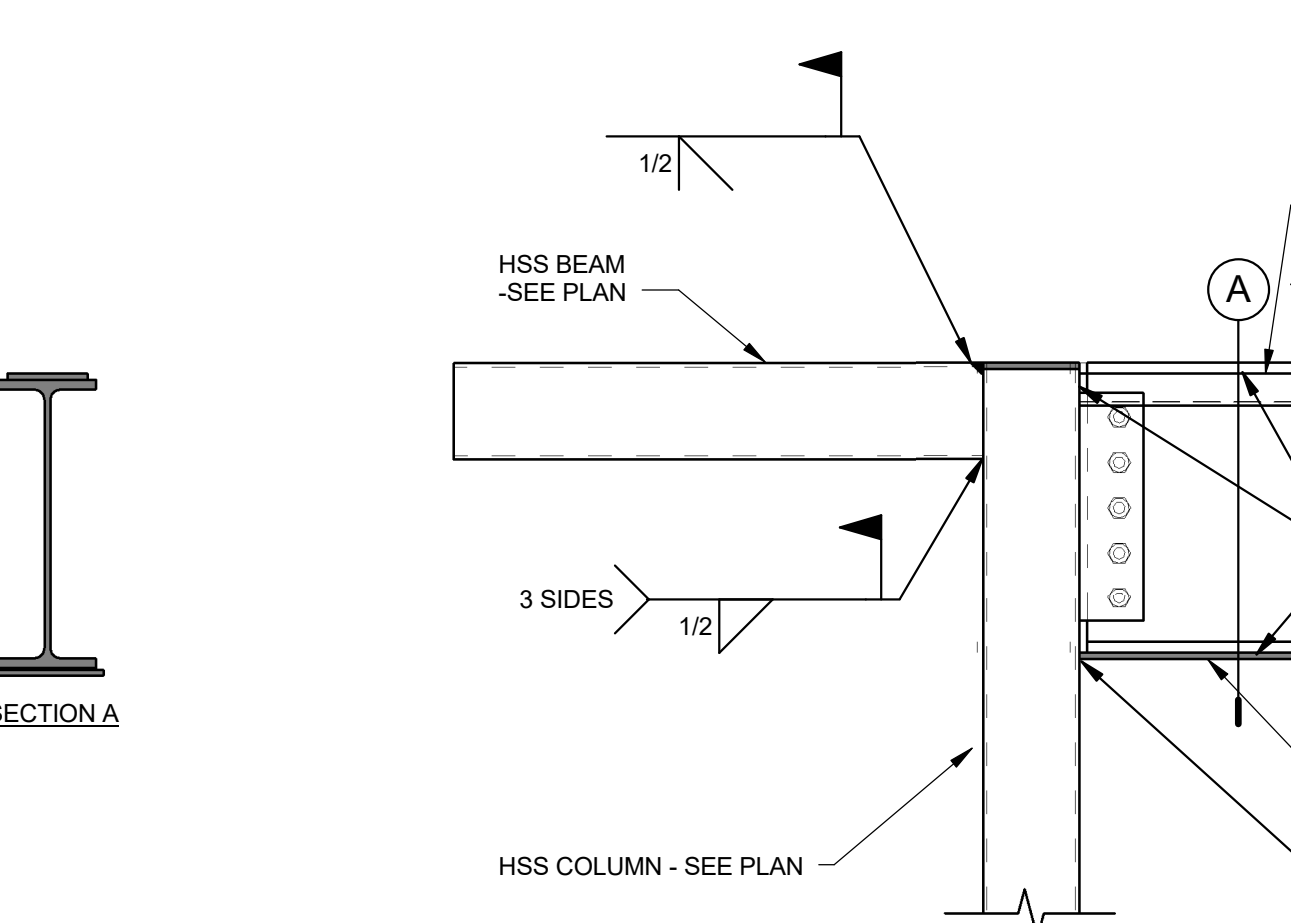
15
DETAIL
SCALE: NONE
S205



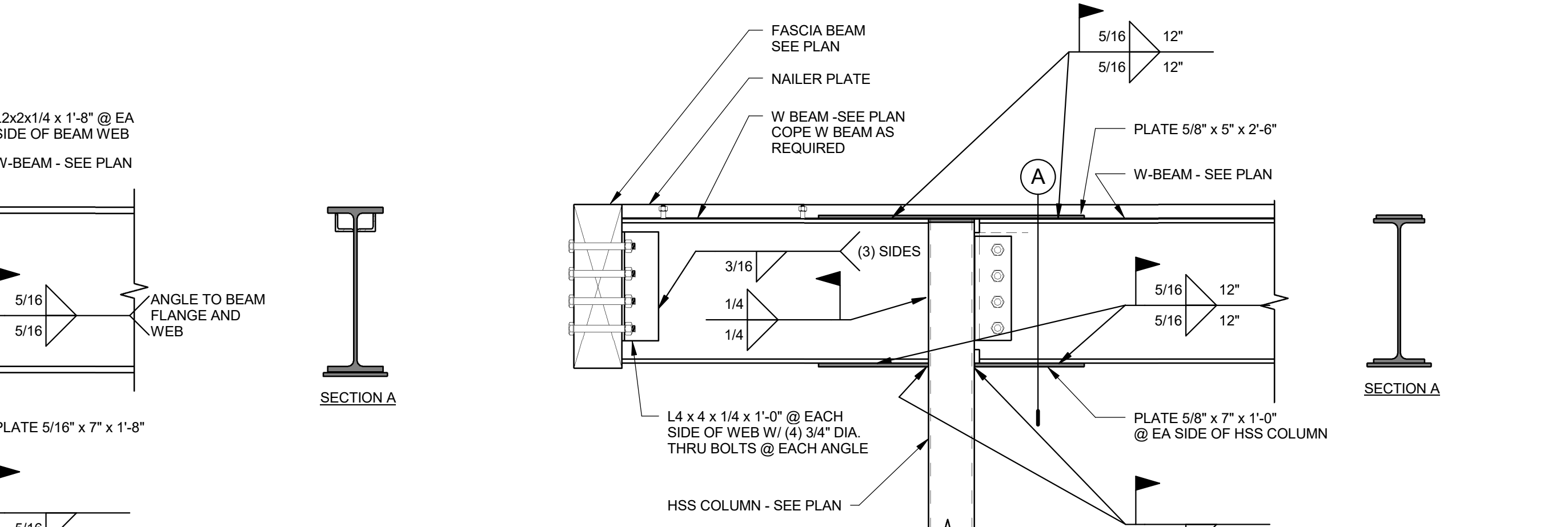
16
DETAIL
SCALE: NONE
S205



17
DETAIL
SCALE: NONE
S205

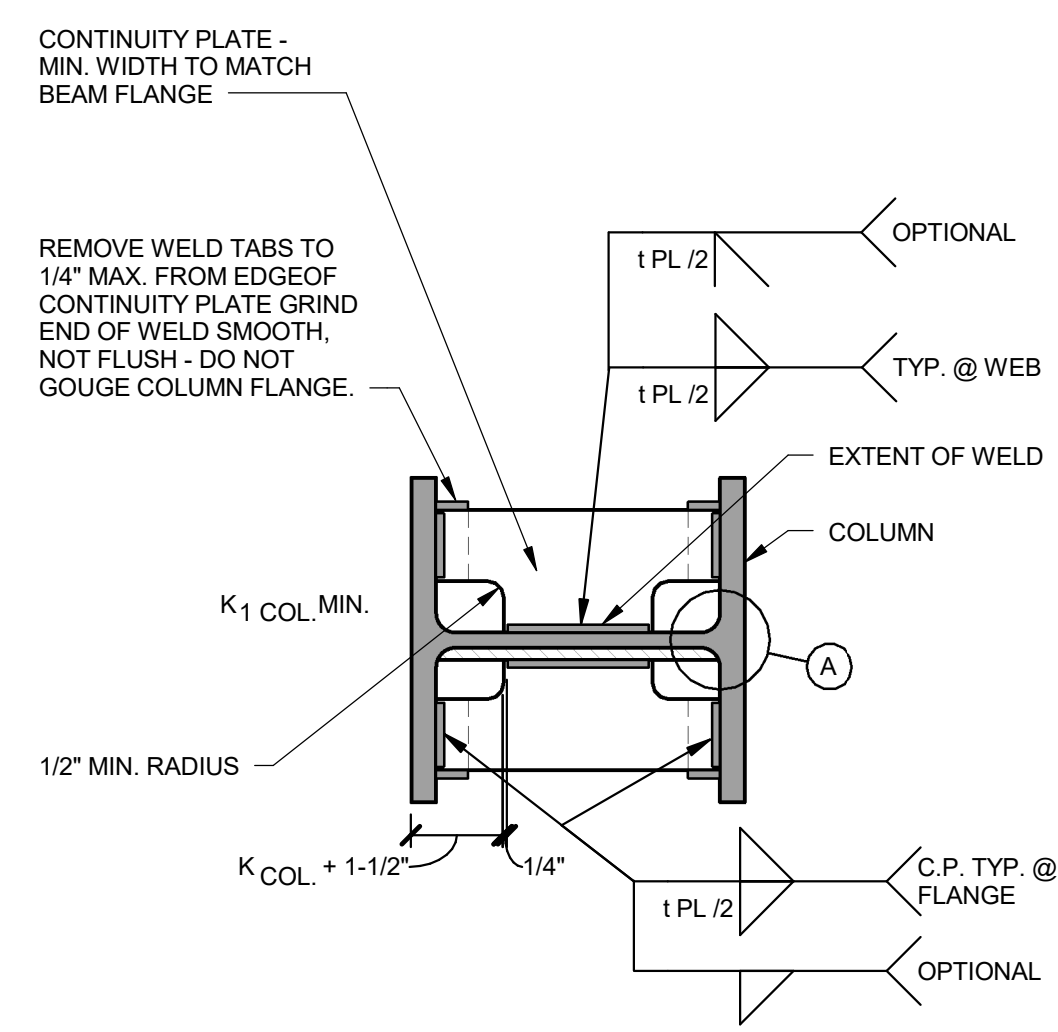


18
DETAIL
SCALE: NONE
S205



19
DETAIL
SCALE: NONE
S205

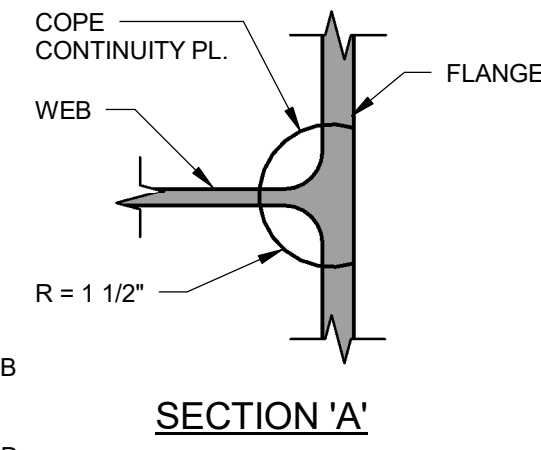
FOR CONSTRUCTION



TYPICAL MOMENT FRAME CONNECTION DETAIL

SCALE: NONE

1
S301



SECTION 'A'

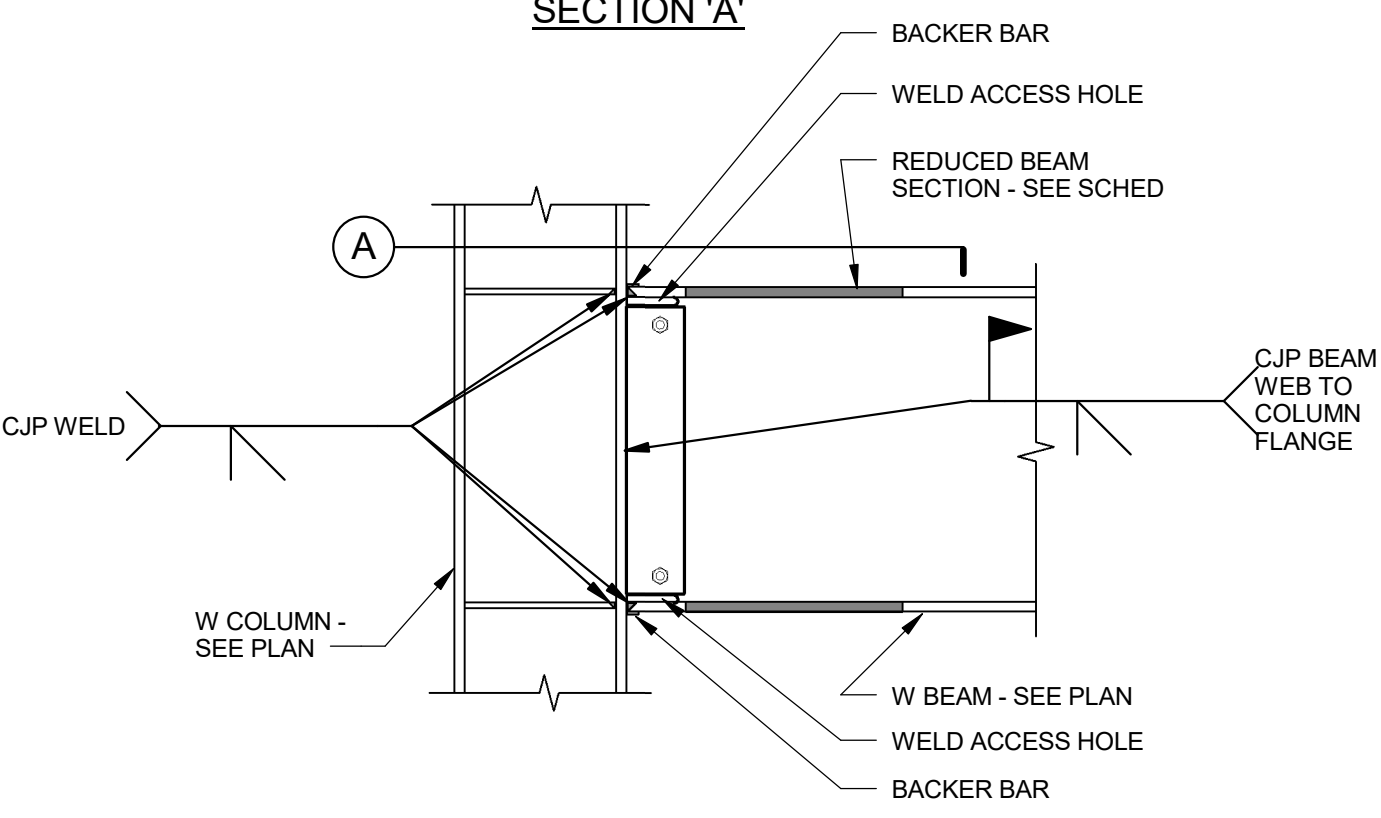
TYPICAL MOMENT FRAME CONNECTION DETAIL

SCALE: NONE

2
S301

BEAM	'a'	'b'	'c'
W12 x 35	4"	9"	3/4"
W12 x 79	7"	9"	1-1/4"
W12 x 87	7"	9"	1-1/4"

SECTION 'A'



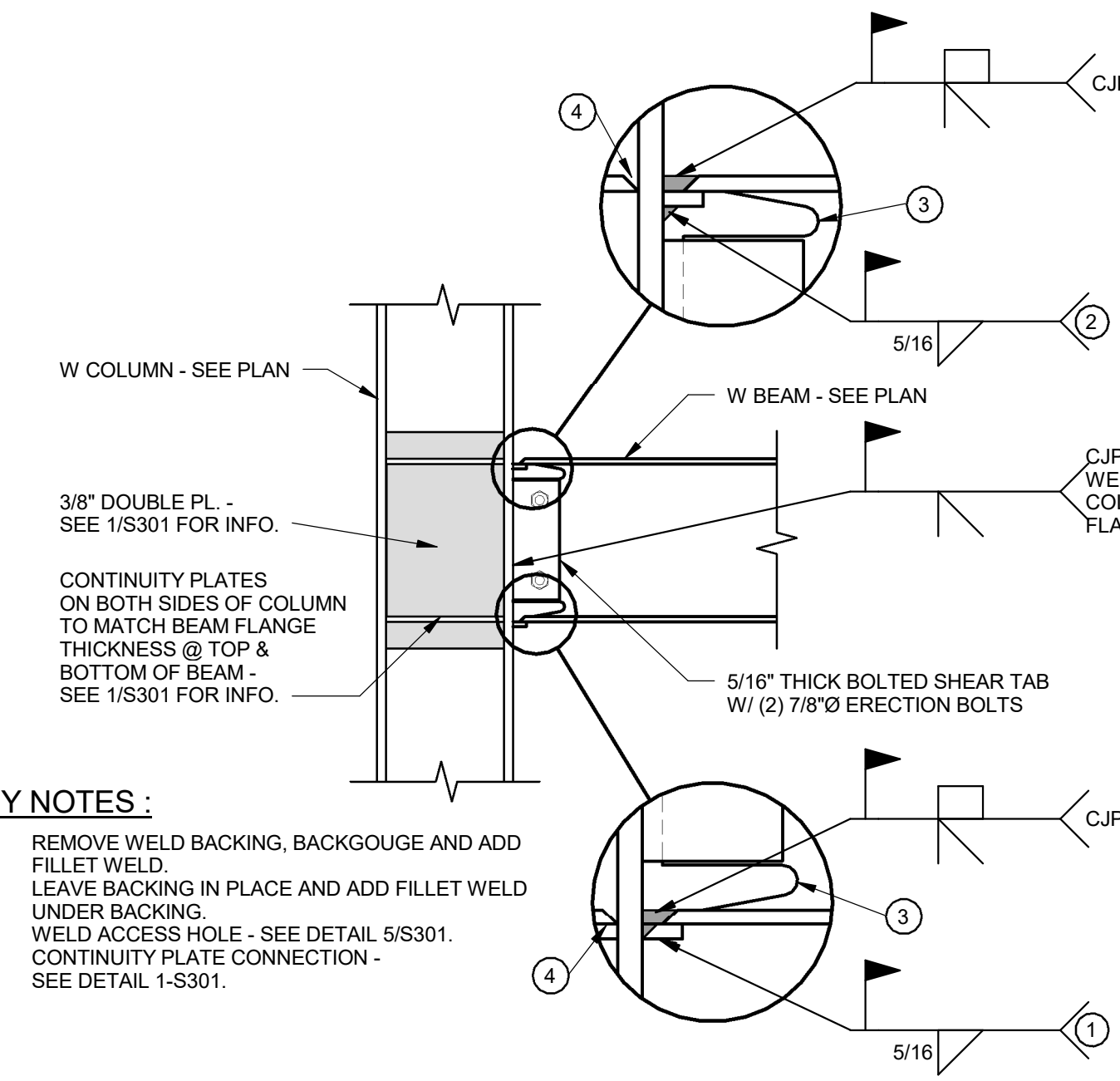
KEY NOTES:

1. REMOVE WELD BACKING, BACKGOUGE AND ADD FILLET WELD.
2. LEAVE BACKING IN PLACE AND ADD FILLET WELD UNDER BACKING.
3. WELD ACCESS HOLE - SEE DETAIL 5/S301.
4. CONTINUITY PLATE CONNECTION - SEE DETAIL 1-S301.

DETAIL

SCALE: NONE

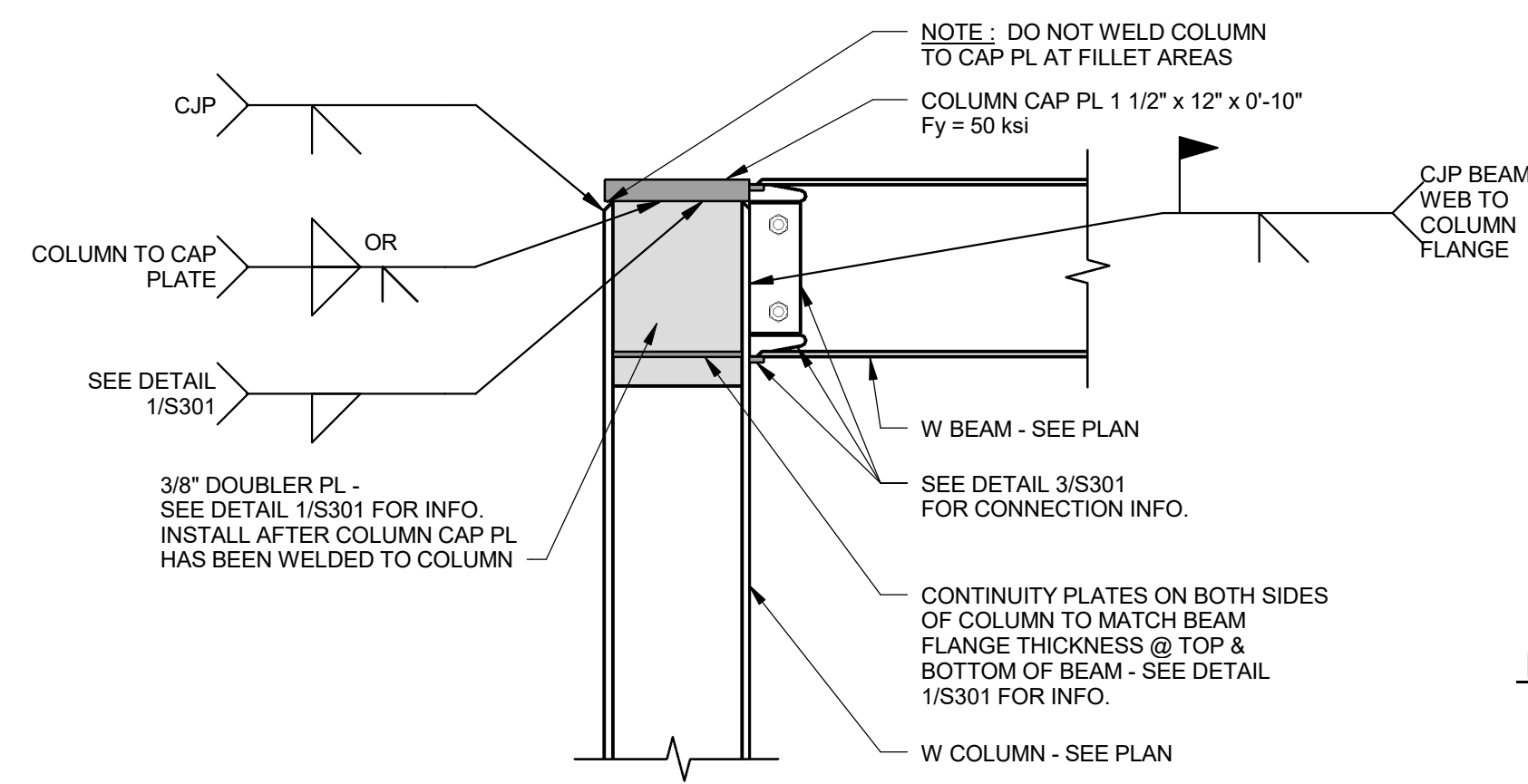
3
S301



MOMENT FRAME CONNECTION DETAIL

SCALE: NONE

4
S301



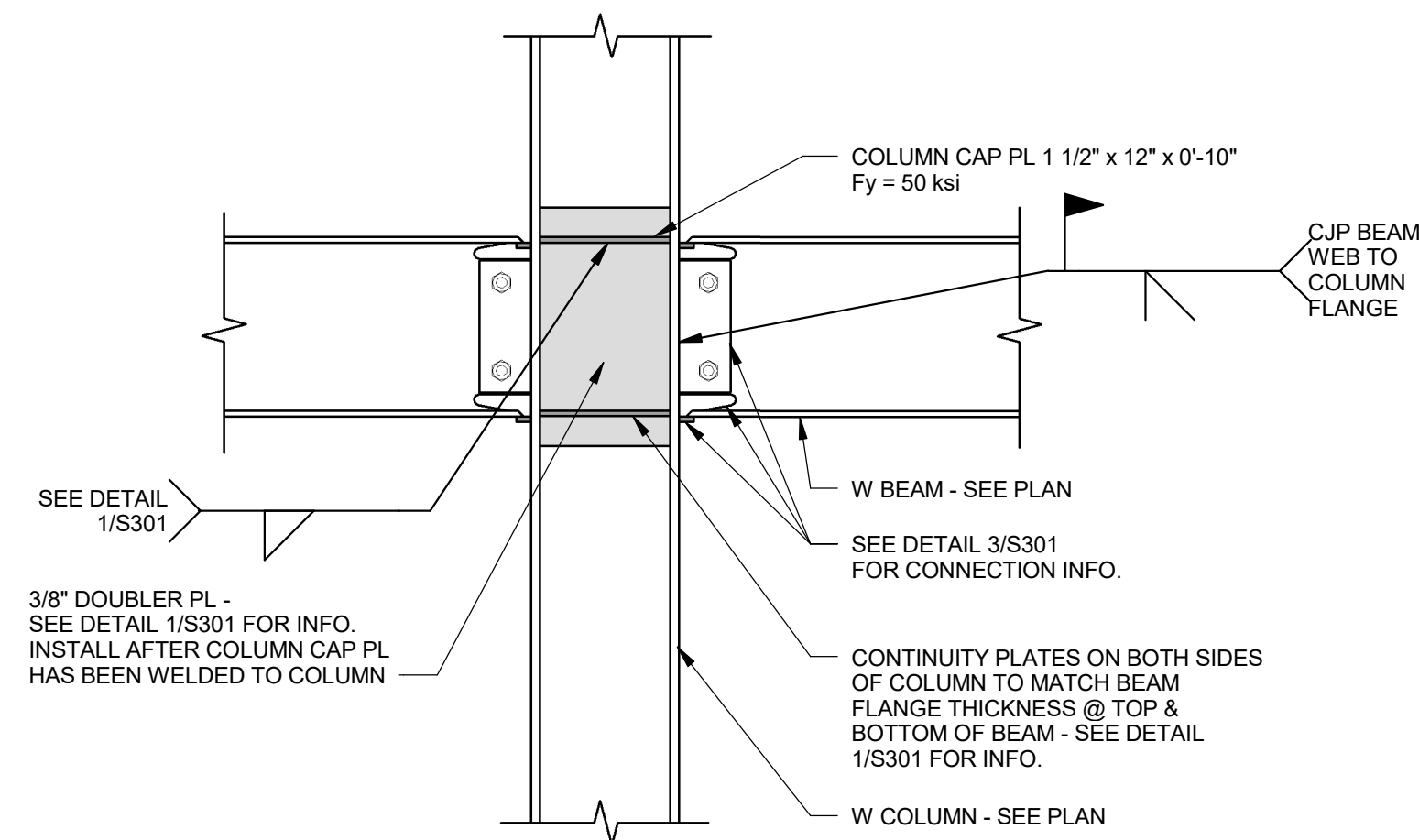
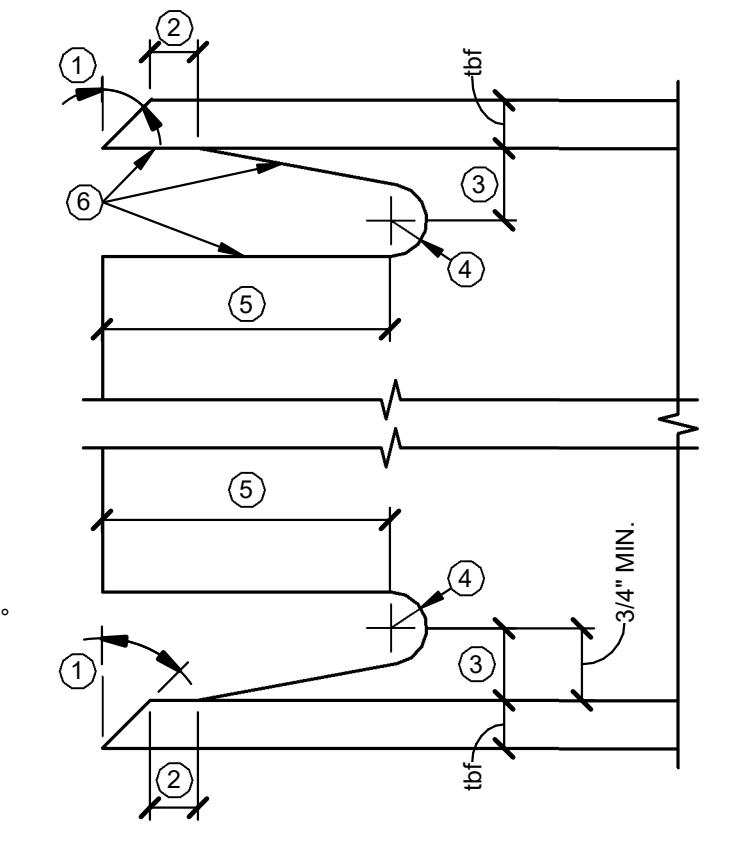
DETAIL

SCALE: NONE

5
S301

KEY NOTES:

1. BEVEL AS REQUIRED BY AWS D1.1 FOR SELECTED GROOVE WELD PROCEDURE.
2. LARGER OF 1/4" OR 1/2 INCH (PLUS 1/2 1/4", OR MINUS 1/4 1/4").
3. 3/4 1/4" TO 1 1/4", 3/4" MINIMUM (+ 1/4 INCH).
4. 3/8" MINIMUM RADIUS (PLUS NOT LIMITED, OR MINUS 0).
5. 3 1/4" (+ 1/2 INCH).
6. SEE FEMA-355, RECOMMENDED SPECIFICATIONS AND QUALITY ASSURANCE GUIDELINES FOR STEEL MOMENT-FRAME CONSTRUCTION FOR SEISMIC APPLICATIONS, FOR FABRICATION DETAILS INCLUDING CUTTING METHODS AND SMOOTHNESS REQUIREMENTS.



MOMENT FRAME CONNECTION DETAIL

SCALE: NONE

6
S301

FOR CONSTRUCTION

ENGINEERS
structural consultants
1584 W. Park, Ok. Okemah, Okla. 74404
PH: 801-782-6008 FAX: 801-782-4856

A R W

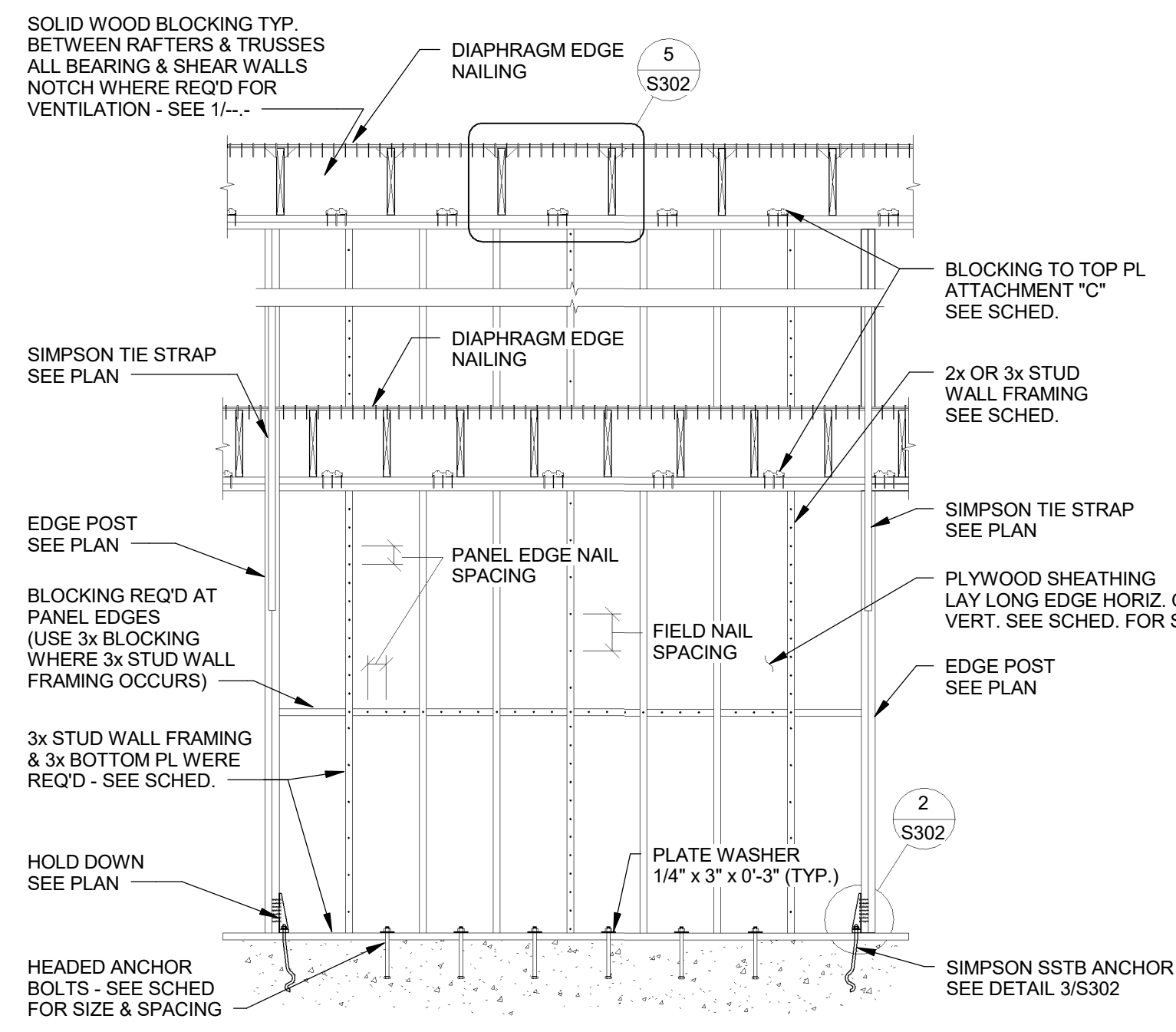
39 SUMMIT, LLC
Summit at Powder Mountain Lot 39
8365 E. SUMMIT PASS

DETAILS

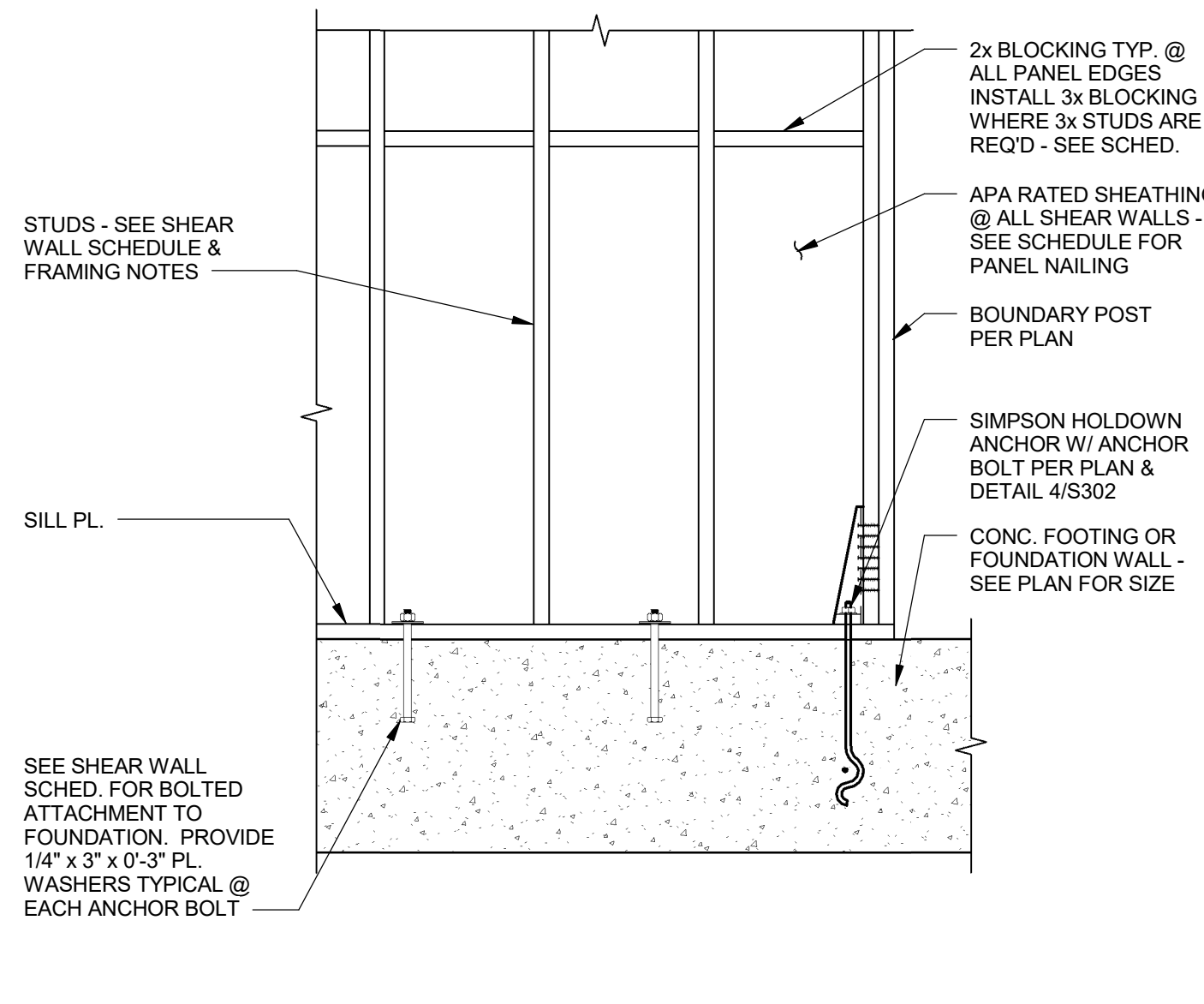
June 26, 2015	No. 37496
November 2, 2015	Top Me-Std.
June 27, 2017	07/25/17
September 22, 2017	
October 25, 2017	

2014168

S301



DETAIL
SCALE: NONE
1
S302



TYP. HOLD DOWN DETAIL
SCALE: NONE
2
S302

HOLD DOWN EMBED SCHEDULE

HOLD DOWN SIZE	ANCHOR SIZE	MIN. EMBEDMENT DEPTH "L _e "
HDU2	SSTB16	12-5/8"
HDU4	SSTB20	16-5/8"
HDU5	SSTB24	24-7/8"
HDU8	SSTB28	24-7/8"

SECTION A

LOCATE APPROX. 45 DEG.

PLACE SSTB ANCHOR DIAGONAL IN CORNER @ END WALL APPLICATION

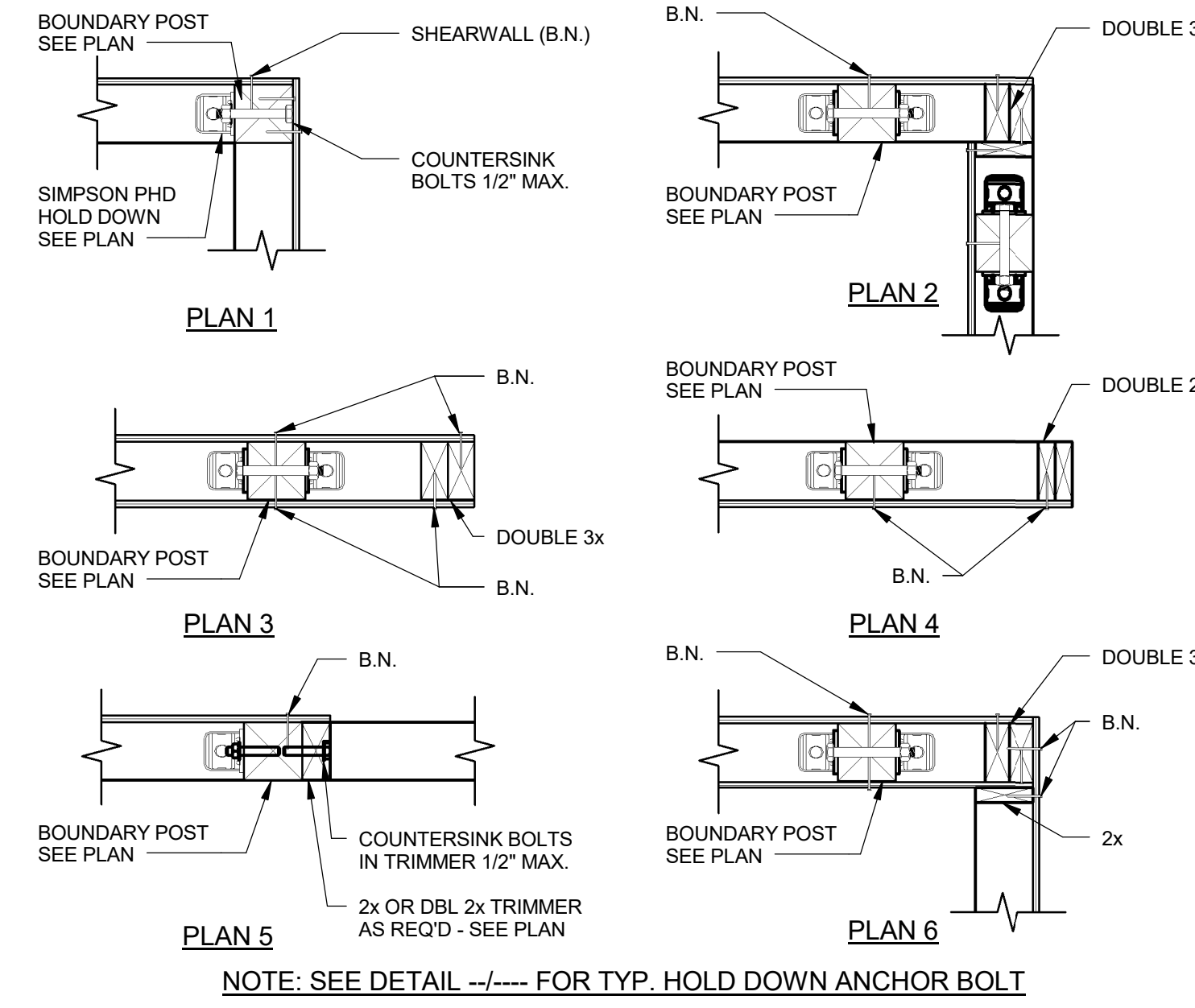
CONTINUOUS WALL INSTALLATION

NOTE: AT INTERIOR HOLD DOWNS ON CONC. SLAB ON METAL DECK, WELD THREADED ROD EQUIVALENT SIZE AND STRENGTH TO TOP OF W-BEAM AT INTERIOR HOLD DOWNS ON THICKENED SLAB FOOTINGS. PROVIDE HEADED ANCHOR BOLT OF EQUIVALENT SIZE W/ 8" MIN. EMBEDMENT.

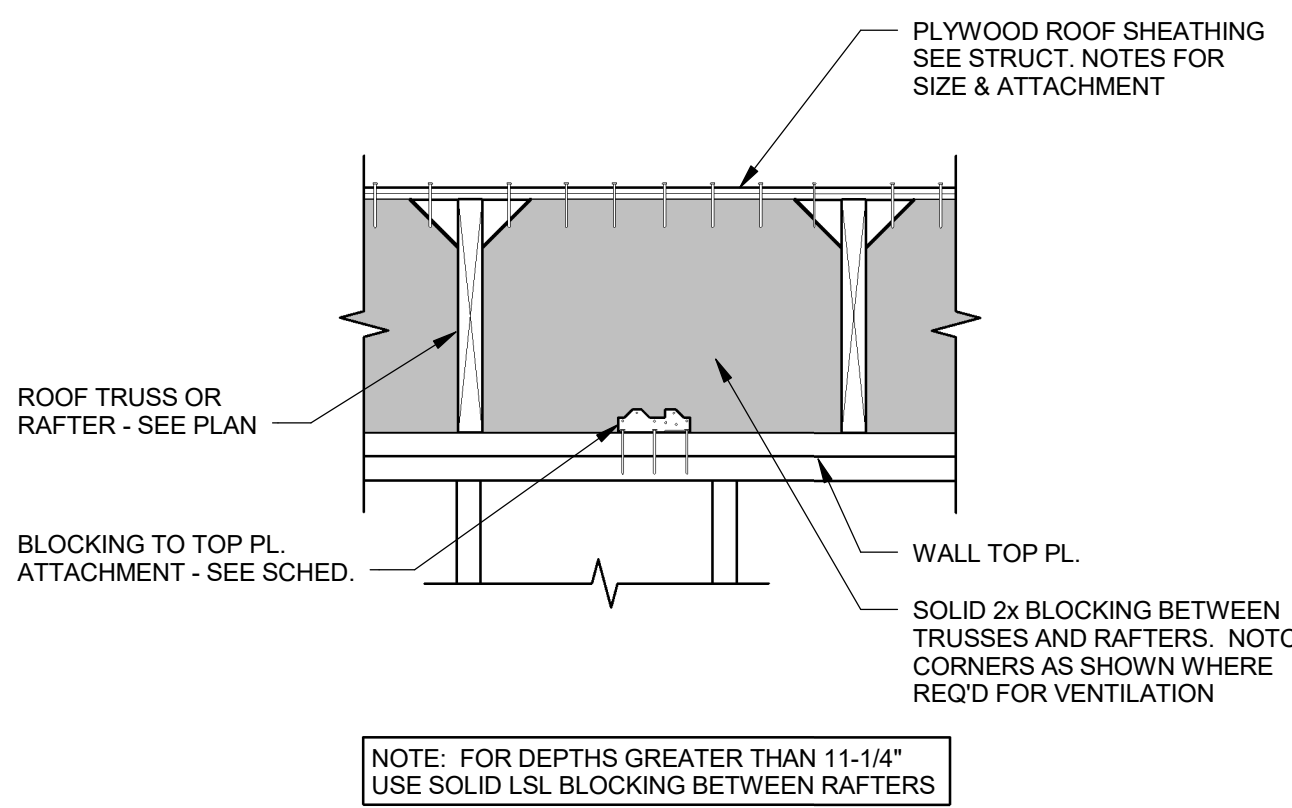
CORNER / END WALL INSTALLATION

1-3/4" MIN.

HOLD DOWN EMBED SCHEDULE
SCALE: NONE
3
S302



DETAIL
SCALE: NONE
4
S302

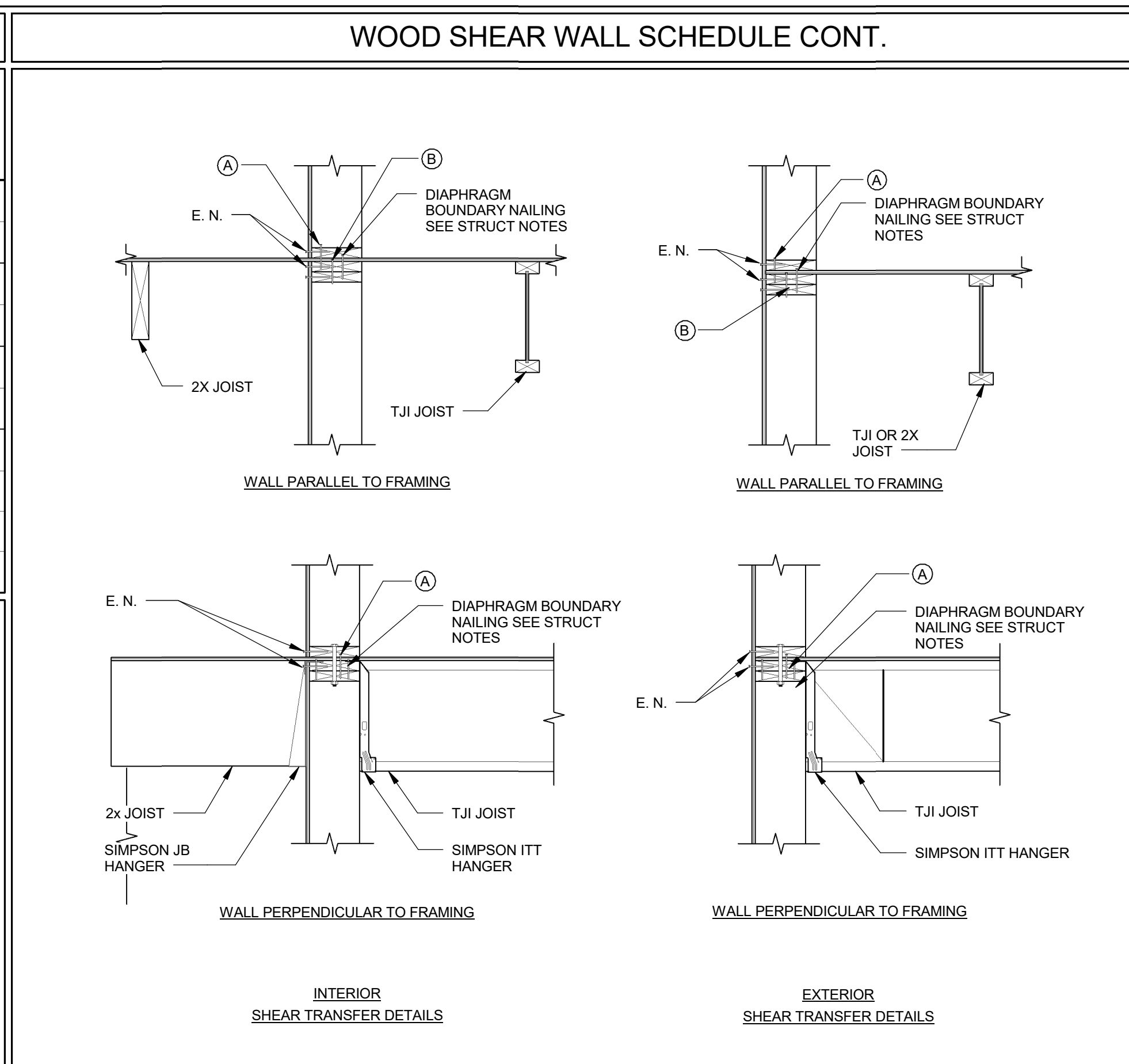


DETAIL
SCALE: NONE
5
S302

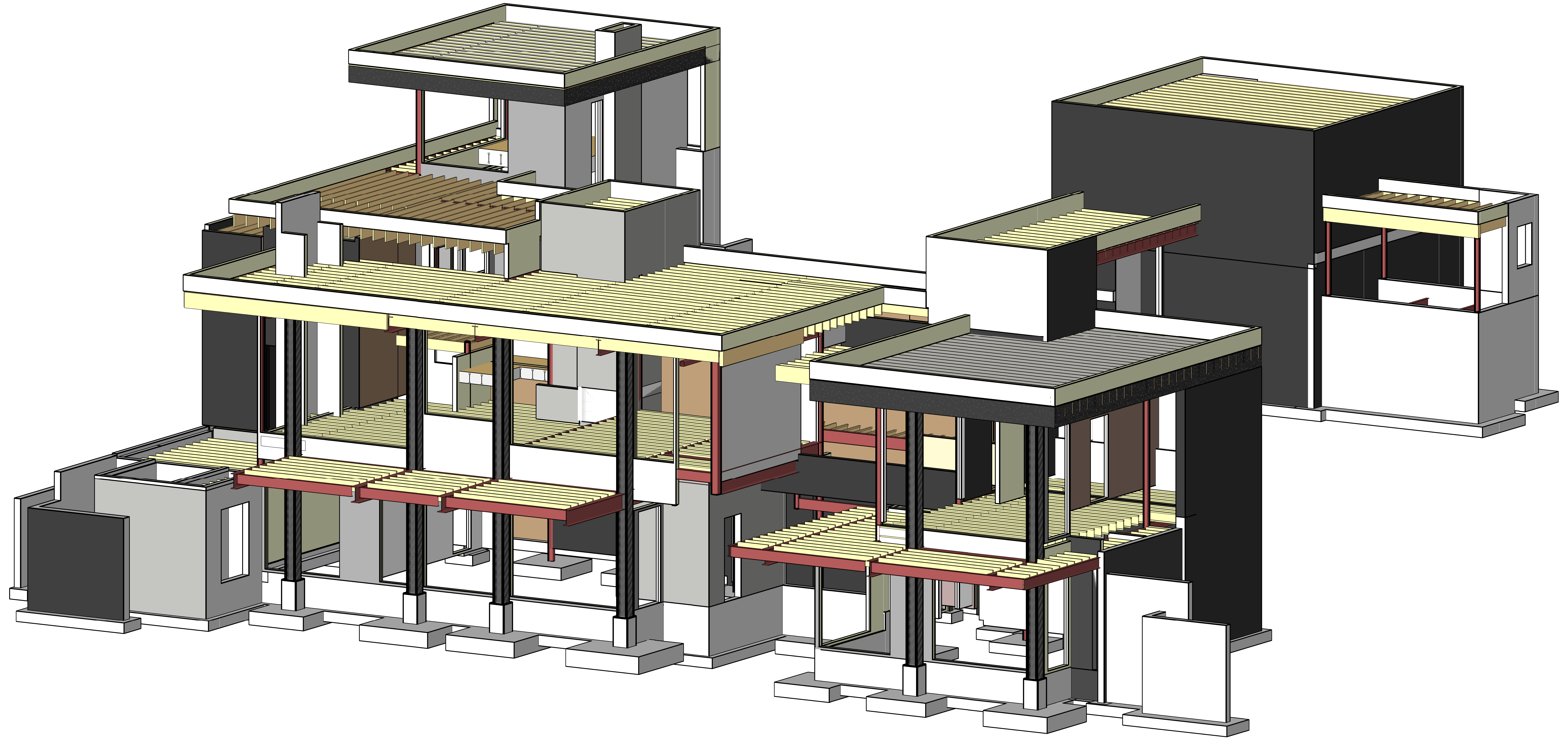
WOOD SHEAR WALL SCHEDULE											
WALL MARK	LEVEL	(NOTE 6) PLYWOOD SHEATHING (CDX U.N.O.)	EDGE NAILING (E.N.) (SEE NOTES 2 & 3)	NOMINAL BOTTOM PLATE SIZE	(NOTE 7) NOM. STUD SIZE (MIN.)	CONNECTION NAILING			A.B. @ FOUND.		COMMENTS
						BOTTOM PL. (A) (SEE NOTE 4) (L-LAG (ST)-STAGGER)	NAILING TOP PL. TOGETHER (B)	BLKG. TO TOP PL. (C)	DIA.	SPA.	
SW-1	1ST TO 2ND	15/32"	6"	2x	2x	--	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
	2ND TO ROOF	15/32"	6"	2x	2x	(2) 16d @ 16" o.c.	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
SW-2	1ST TO 2ND	15/32"	4"	2x	2x	--	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
	2ND TO ROOF	15/32"	4"	2x	2x	(2) 16d @ 16" o.c.	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
SW-3	1ST TO 2ND	15/32"	3"	3x	3x	--	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
	2ND TO ROOF	15/32"	3"	3x	3x	(2) 16d @ 8" o.c.	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	32" o.c.	
SW-4	1ST TO 2ND	15/32"	2"	3x	3x	--	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	16" o.c.	
	2ND TO ROOF	15/32"	2"	3x	3x	(2) 16d @ 8" o.c.	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	16" o.c.	
SW-5	1ST TO 2ND	15/32" BOTH FACES	2"	3x	3x	--	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	8" o.c.	
	2ND TO ROOF	15/32" BOTH FACES	2"	3x	3x	(2) 16d @ 8" o.c.	(2) 16d @ 6" o.c.	A35 @ 16" o.c.	5/8" DIA.	8" o.c.	

NOTES:

- ALL SHEATHING PANEL EDGES TO BE BLOCKED. USE 3x BLOCKING WHERE 3x STUDS ARE REQ'D.
- ALL NAILS TO BE COMMON OR GALV. BOX.
- FIELD NAILING TO BE SAME NAILS @ 12" o.c.
- (C) CONNECTION IS FOR 2ND FLOOR AND ABOVE.
- AT SHEAR WALLS W/ SHEATHING ON BOTH SIDES, BOTH VERTICAL AND HORIZONTAL JOINTS ON OPPOSITE SIDES OF THE WALL SHALL BE STAGGERED.
- STAGGER E.N. AT DOUBLE TOP PLATES.
- 3x NOMINAL FRAMING MEMBERS TO OCCUR AT ABUTTING PANEL EDGES. 2x NOMINAL FRAMING MEMBERS MAY BE USED AT INTERIOR OF PANEL, UNLESS NOTED OTHERWISE IN FLOOR FRAMING NOTES. (2) 2x NAILED TOGETHER W/ (2) 16d NAILS @ 6" o.c. OR 4x NOMINAL FRAMING MEMBERS OF THE SAME DEPTH AND LUMBER GRADE MAY BE USED IN LIEU OF 3x MEMBERS AT CONTRACTOR OPTION.
- SHEATHING SHALL BE STAMPED W/ APA STAMP. O.S.B. OF EQUIVALENT THICKNESS, GRADE, AND RATING MAY BE USED IN LIEU OF PLYWOOD.
- SEE THIS SHEET FOR TYPICAL SHEAR TRANSFER DETAILS.



FOR CONSTRUCTION



NOTE: THIS VIEW REPRESENTS A SCHEMATIC RENDERING ONLY AND IS NOT INTENDED TO CONVEY CONSTRUCTION INFORMATION. ALL CONSTRUCTION SHALL COMPLY WITH SPECIFIC NOTES AND DETAILS WITHIN THE STRUCTURAL DRAWINGS.

FOR CONSTRUCTION