



February 9, 2016

39 Summit LLC  
c/o Ms. Andrea Milner  
cc: Mrs. Cassandra Beresini  
314 Lytton Avenue, Suite 100  
Palo Alto, California 94301

IGES Project No. 02132-002

Subject: Response to Additional Review Comments - Engineering  
Geotechnical Investigation Report and Soil Nail Wall  
Lot 39R of Powder Mountain Resort  
8365 East Summit Pass  
Weber County, Utah

Ms. Milner:

As requested, IGES has prepared the following response to additional review comments regarding the referenced geotechnical report and soil nail wall design package for Lot 39, part of the larger Powder Mountain Resort expansion project in Weber County, Utah. The review comments to be addressed were prepared by Taylor Geotechnical (TG); the comments were posted on Miradi (Weber County Website), and were provided to IGES on February 9, 2016 via email. The latest review comments are in regards to the review response by IGES (2016), which was prepared in response to TG's first engineering review comments posted on December 1, 2015. For convenience, the review comments will be presented first, followed by our response.

**Comment No. 1**

*“Based on the slope analysis completed for the November 4, 2015 IGES document, the anticipated surcharge from the proposed home did not appear to be incorporated into the analysis. TG recommends IGES show the location of the home loads in the slope analysis.*

*Based on the slope analysis completed for the January 12, 2016 IGES document, the anticipated surcharge from the proposed home did not appear to be incorporated into the analysis. TG recommends IGES show the location of the home loads in the slope analysis.”*

**Response to Comment No. 1**

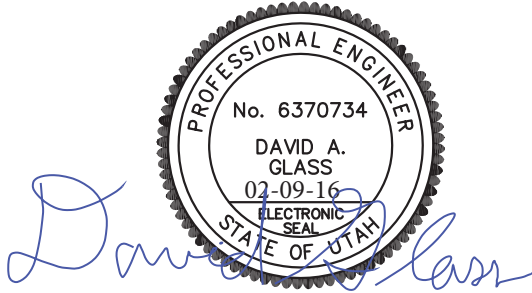
The slope stability analysis for both submittals (IGES, 2016 and 2015a) are the same. IGES has revised the slope stability analysis by adding a surcharge to simulate the load from a home. The surcharge assumes exterior continuous footings, 24” wide, with a 1,500 psf bearing stress. A floor surcharge of 200 psf has also been assumed. The reduction in driving force due to the removal of soil for the basement excavation has been conservatively neglected.

A summary of the revised slope stability analysis is attached.

**Closure**

We appreciate the opportunity to provide you with our services. If you have any questions please contact the undersigned at your convenience (801) 748-4044.

Respectfully Submitted,  
IGES, Inc.



David A. Glass, P.E.  
Senior Geotechnical Engineer

Attachments:

References

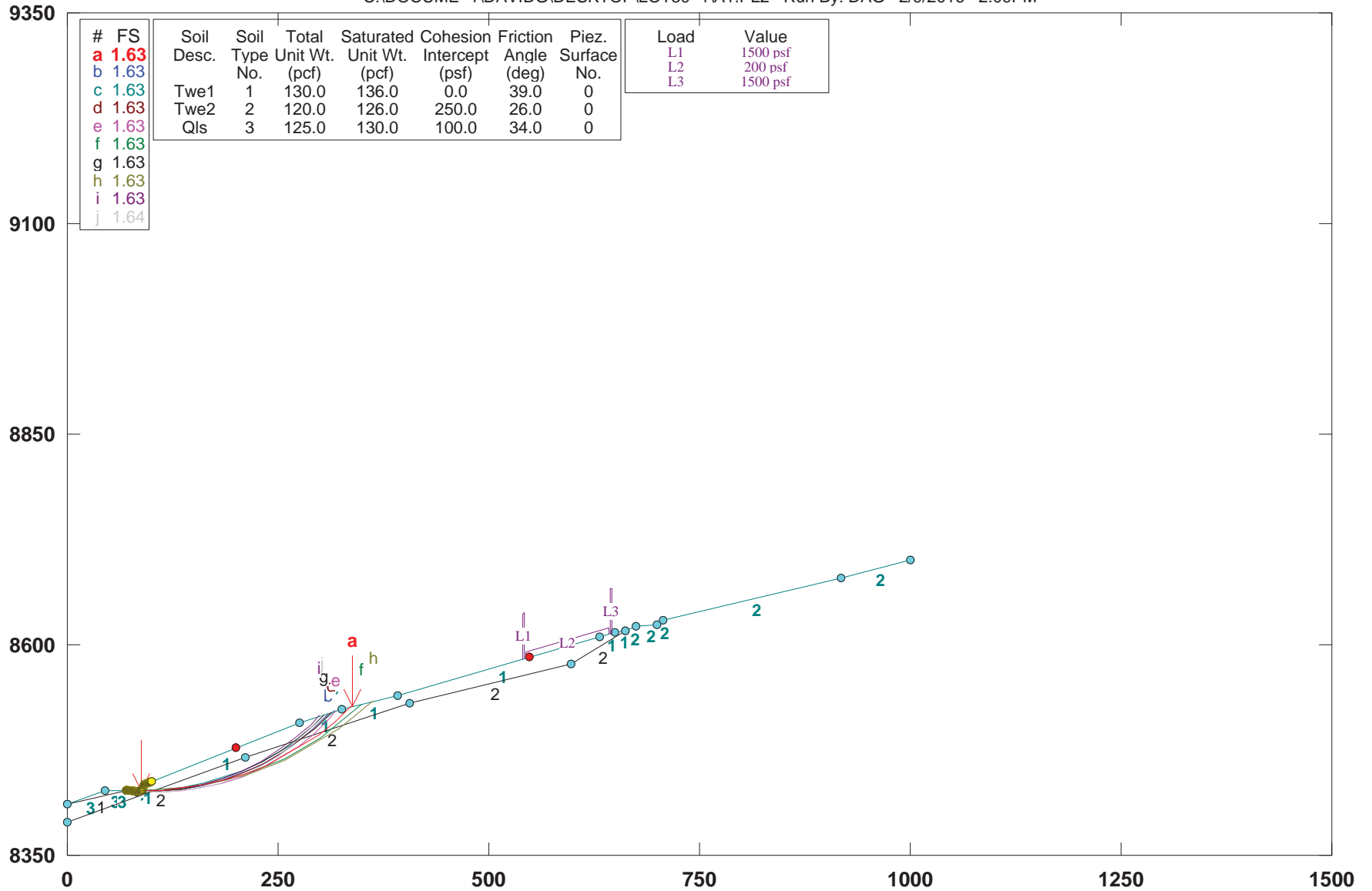
Revised Slope Stability Analysis

References

- IGES, Inc., 2016, Response to Review Comments – Engineering, Geotechnical Investigation Report and Soil Nail Wall, Lot 39R of Powder Mountain Resort, 8365 East Summit Pass, Weber County, Utah, Project No. 02132-002, dated January 12, 2016.
- IGES, Inc., 2015a, Response to Additional Review Comments – Geology, Geotechnical Investigation Report, Lot 39R of Powder Mountain Resort, 8365 East Summit Pass, Weber County, Utah, Project No. 02132-002, dated November 4, 2015.
- IGES, Inc., 2015b, Response to Review Comments – Geology, Geotechnical Investigation Report, Lot 39R of Powder Mountain Resort, 8365 East Summit Pass, Weber County, Utah, Project No. 02132-002, dated September 23, 2015.
- IGES, Inc., 2015c, Design Package, Permanent Shoring System, Howery Residence, 8365 East Summit Pass (Lot 39R), Summit Eden Development, Weber County, Utah, Project No. 02132-001, dated July 6, 2015, latest revision August 27, 2015.
- IGES, Inc., 2015d, Geotechnical Investigation Report, Lot 39R of Powder Mountain Resort, 8365 East Summit Pass, Weber County, Utah Project No. 02052-001, dated June 3, 2015.
- IGES, Inc., 2012a, Design Geotechnical Investigation, Powder Mountain Resort, Weber County, Utah, Project No. 01628-003, dated November 9, 2012.
- IGES, Inc., 2012b, Preliminary Geotechnical Investigation, Powder Mountain Resort, Weber County, Utah, Project No. 01628-001, dated July 26, 2012.

# Lot 39; A-A'; 02132-002; Post-LS Failure; Setback; Static

C:\DOCUME~1\DAVIDG\DESKTOP\LOT39-1\A1.PL2 Run By: DAG 2/9/2016 2:09PM



#	FS
a	1.63
b	1.63
c	1.63
d	1.63
e	1.63
f	1.63
g	1.63
h	1.63
i	1.63
j	1.64

Soil Desc.	Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Piez. Surface No.
Twe1	1	130.0	136.0	0.0	39.0	0
Twe2	2	120.0	126.0	250.0	26.0	0
Qls	3	125.0	130.0	100.0	34.0	0

Load	Value
L1	1500 psf
L2	200 psf
L3	1500 psf

GSTABL7 v.2 FSmin=1.63

Safety Factors Are Calculated By The Modified Bishop Method



\*\*\* GSTABL7 \*\*\*

\*\* GSTABL7 by Garry H. Gregory, P.E. \*\*

\*\* Original Version 1.0, January 1996; Current Version 2.002,  
December 2001 \*\*

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\*\*\*\*\*

SLOPE STABILITY ANALYSIS SYSTEM

Modified Bishop, Simplified Janbu, or GLE Method of Slices.  
(Includes Spencer & Morgenstern-Price Type Analysis)  
Including Pier/Pile, Reinforcement, Soil Nail, Tieback,  
Nonlinear Undrained Shear Strength, Curved Phi Envelope,  
Anisotropic Soil, Fiber-Reinforced Soil, Boundary Loads, Water  
Surfaces, Pseudo-Static Earthquake, and Applied Force Options.

\*\*\*\*\*

Analysis Run Date: 2/9/2016  
Time of Run: 2:09PM  
Run By: DAG  
Input Data Filename: C:\al.  
Output Filename: C:\al.OUT  
Unit System: English  
  
Plotted Output Filename: C:\al.PLT

PROBLEM DESCRIPTION: Lot 39; A-A'; 02132-002; Post-LS Failure  
; Setback; Static

BOUNDARY COORDINATES

16 Top Boundaries  
21 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	0.00	8410.00	45.00	8427.00	3
2	45.00	8427.00	70.00	8427.50	3
3	70.00	8427.50	83.00	8426.00	3
4	83.00	8426.00	89.00	8427.70	1
5	89.00	8427.70	91.10	8434.30	1
6	91.10	8434.30	275.00	8508.00	1
7	275.00	8508.00	326.00	8524.30	1
8	326.00	8524.30	392.00	8540.50	1
9	392.00	8540.50	632.00	8610.00	1

10	632.00	8610.00	650.00	8614.40	1
11	650.00	8614.40	662.00	8616.10	1
12	662.00	8616.10	675.00	8622.00	2
13	675.00	8622.00	700.00	8623.60	2
14	700.00	8623.60	706.00	8629.20	2
15	706.00	8629.20	917.00	8680.00	2
16	917.00	8680.00	1000.00	8700.00	2
17	0.00	8410.00	70.00	8427.50	1
18	0.00	8390.00	212.00	8466.00	2
19	212.00	8466.00	406.00	8530.00	2
20	406.00	8530.00	597.00	8578.00	2
21	597.00	8578.00	662.00	8616.10	2

User Specified Y-Origin = 8350.00(ft)

ISOTROPIC SOIL PARAMETERS

3 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	136.0	0.0	39.0	0.00	0.0	0
2	120.0	126.0	250.0	26.0	0.00	0.0	0
3	125.0	130.0	100.0	34.0	0.00	0.0	0

BOUNDARY LOAD(S)

3 Load(s) Specified

Load No.	X-Left (ft)	X-Right (ft)	Intensity (psf)	Deflection (deg)
1	540.00	542.00	1500.0	0.0
2	543.00	643.00	200.0	0.0
3	644.00	646.00	1500.0	0.0

NOTE - Intensity Is Specified As A Uniformly Distributed Force Acting On A Horizontally Projected Surface.

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Circular Surfaces, Has Been Specified.

2500 Trial Surfaces Have Been Generated.

50 Surface(s) Initiate(s) From Each Of 50 Points Equally Spaced  
 Along The Ground Surface Between X = 70.00(ft)  
 and X = 100.00(ft)

Circle Center At X = 95.31 ; Y = 8770.59 ; and Radius = 343.33

Factor of Safety  
 \*\*\* 1.628 \*\*\*

Each Surface Terminates Between X = 200.00(ft)  
 and X = 548.00(ft)

Unless Further Limitations Were Imposed, The Minimum Elevation  
 At Which A Surface Extends Is Y = 0.00(ft)

Individual data on the 19 slices

25.00(ft) Line Segments Define Each Trial Failure Surface.

Restrictions Have Been Imposed Upon The Angle Of Initiation.  
 The Angle Has Been Restricted Between The Angles Of -30.0  
 And 10.0 deg.

Following Is Displayed The Most Critical Of The Trial  
 Failure Surfaces Evaluated.

\* \* Safety Factors Are Calculated By The Modified Bishop Method \* \*

Total Number of Trial Surfaces Evaluated = 2500

Statistical Data On All Valid FS Values:  
 FS Max = 2.581 FS Min = 1.628 FS Ave = 1.990  
 Standard Deviation = 0.188 Coefficient of Variation = 9.42 %

Slice No.	Width (ft)	Weight (lbs)	Water Force		Tie Force		Earthquake Force		
			Top (lbs)	Bot (lbs)	Norm (lbs)	Tan (lbs)	Hor (lbs)	Ver (lbs)	Surcharge Load (lbs)
1	1.2	27.0	0.0	0.0	0.	0.	0.0	0.0	0.0
2	2.1	988.0	0.0	0.0	0.	0.	0.0	0.0	0.0
3	13.8	17117.7	0.0	0.0	0.	0.	0.0	0.0	0.0
4	7.9	13982.6	0.0	0.0	0.	0.	0.0	0.0	0.0
5	24.9	60552.2	0.0	0.0	0.	0.	0.0	0.0	0.0
6	24.7	80562.8	0.0	0.0	0.	0.	0.0	0.0	0.0
7	24.3	94078.3	0.0	0.0	0.	0.	0.0	0.0	0.0
8	23.8	101041.2	0.0	0.0	0.	0.	0.0	0.0	0.0
9	1.5	6532.4	0.0	0.0	0.	0.	0.0	0.0	0.0
10	21.7	95144.5	0.0	0.0	0.	0.	0.0	0.0	0.0
11	22.5	96369.4	0.0	0.0	0.	0.	0.0	0.0	0.0
12	18.8	74804.9	0.0	0.0	0.	0.	0.0	0.0	0.0
13	2.9	10760.9	0.0	0.0	0.	0.	0.0	0.0	0.0
14	20.7	67287.9	0.0	0.0	0.	0.	0.0	0.0	0.0
15	1.4	3993.7	0.0	0.0	0.	0.	0.0	0.0	0.0
16	18.2	39338.8	0.0	0.0	0.	0.	0.0	0.0	0.0
17	7.9	10244.1	0.0	0.0	0.	0.	0.0	0.0	0.0
18	10.6	5664.5	0.0	0.0	0.	0.	0.0	0.0	0.0
19	0.7	26.4	0.0	0.0	0.	0.	0.0	0.0	0.0

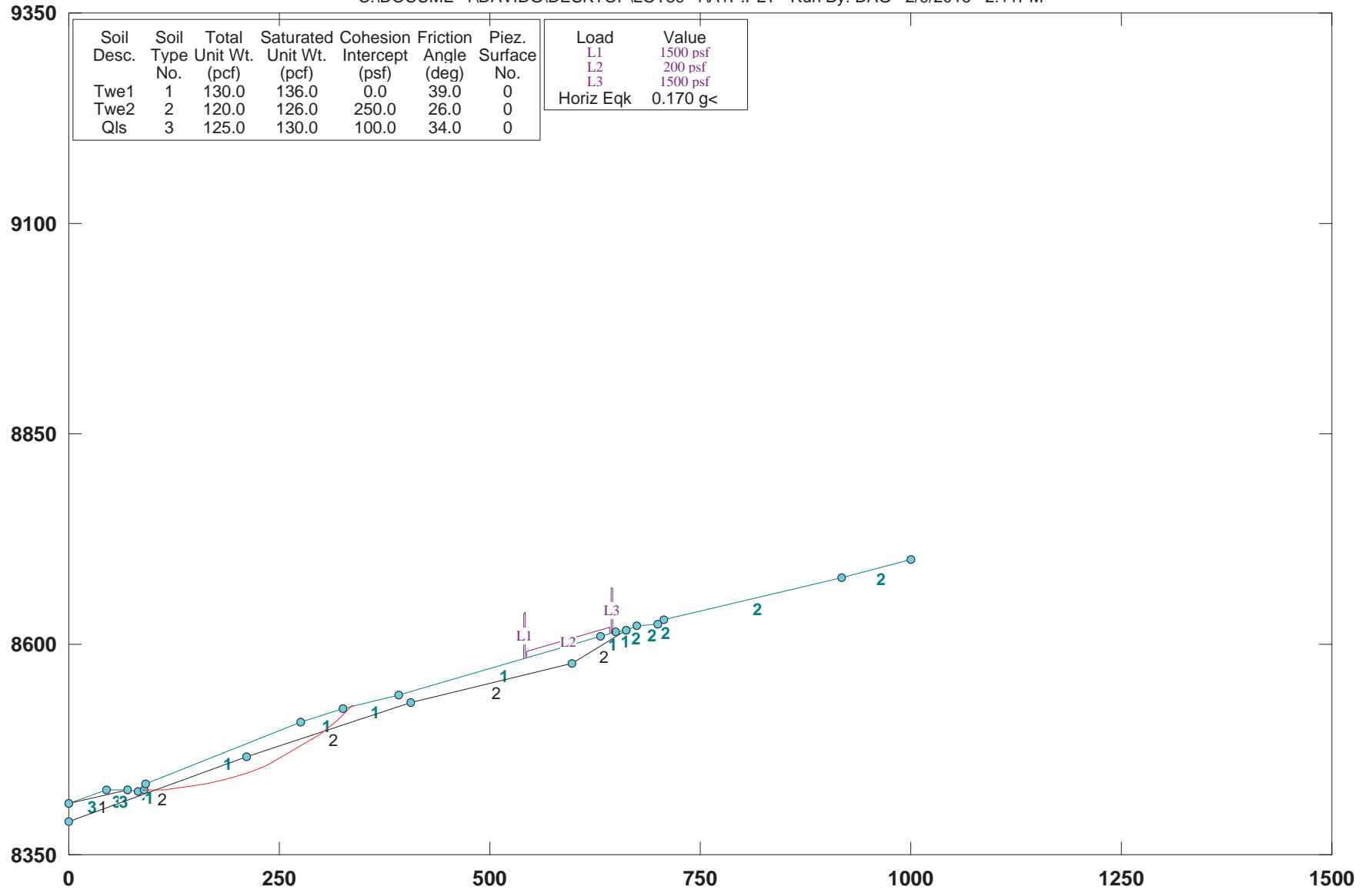
Failure Surface Specified By 13 Coordinate Points

\*\*\*\* END OF GSTABL7 OUTPUT \*\*\*\*

Point No.	X-Surf (ft)	Y-Surf (ft)
1	87.76	8427.35
2	112.75	8427.71
3	137.66	8429.89
4	162.34	8433.87
5	186.66	8439.64
6	210.50	8447.17
7	233.73	8456.41
8	256.23	8467.31
9	277.87	8479.83
10	298.55	8493.88
11	318.14	8509.41
12	336.56	8526.32
13	337.26	8527.06

# Lot 39; A-A'; 02132-002; Post-LS Failure; Setback; P-Static

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GSTABL7 v.2 FSmin=1.08

Factor Of Safety Is Calculated By The Modified Bishop Method



\*\*\* GSTABL7 \*\*\*

\*\* GSTABL7 by Garry H. Gregory, P.E. \*\*

\*\* Original Version 1.0, January 1996; Current Version 2.002,  
December 2001 \*\*

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SLOPE STABILITY ANALYSIS SYSTEM

Modified Bishop, Simplified Janbu, or GLE Method of Slices.  
(Includes Spencer & Morgenstern-Price Type Analysis)  
Including Pier/Pile, Reinforcement, Soil Nail, Tieback,  
Nonlinear Undrained Shear Strength, Curved Phi Envelope,  
Anisotropic Soil, Fiber-Reinforced Soil, Boundary Loads, Water  
Surfaces, Pseudo-Static Earthquake, and Applied Force Options.

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Analysis Run Date: 2/9/2016  
Time of Run: 2:11PM  
Run By: DAG  
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Output Filename: C:alp.OUT  
Unit System: English  
  
Plotted Output Filename: C:alp.PLT

PROBLEM DESCRIPTION: Lot 39; A-A'; 02132-002; Post-LS Failure  
; Setback; P-Static

BOUNDARY COORDINATES

16 Top Boundaries  
21 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	0.00	8410.00	45.00	8427.00	3
2	45.00	8427.00	70.00	8427.50	3
3	70.00	8427.50	83.00	8426.00	3
4	83.00	8426.00	89.00	8427.70	1
5	89.00	8427.70	91.10	8434.30	1
6	91.10	8434.30	275.00	8508.00	1
7	275.00	8508.00	326.00	8524.30	1
8	326.00	8524.30	392.00	8540.50	1
9	392.00	8540.50	632.00	8610.00	1

10	632.00	8610.00	650.00	8614.40	1
11	650.00	8614.40	662.00	8616.10	1
12	662.00	8616.10	675.00	8622.00	2
13	675.00	8622.00	700.00	8623.60	2
14	700.00	8623.60	706.00	8629.20	2
15	706.00	8629.20	917.00	8680.00	2
16	917.00	8680.00	1000.00	8700.00	2
17	0.00	8410.00	70.00	8427.50	1
18	0.00	8390.00	212.00	8466.00	2
19	212.00	8466.00	406.00	8530.00	2
20	406.00	8530.00	597.00	8578.00	2
21	597.00	8578.00	662.00	8616.10	2

User Specified Y-Origin = 8350.00(ft)

ISOTROPIC SOIL PARAMETERS

3 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	136.0	0.0	39.0	0.00	0.0	0
2	120.0	126.0	250.0	26.0	0.00	0.0	0
3	125.0	130.0	100.0	34.0	0.00	0.0	0

BOUNDARY LOAD(S)

3 Load(s) Specified

Load No.	X-Left (ft)	X-Right (ft)	Intensity (psf)	Deflection (deg)
1	540.00	542.00	1500.0	0.0
2	543.00	643.00	200.0	0.0
3	644.00	646.00	1500.0	0.0

NOTE - Intensity Is Specified As A Uniformly Distributed Force Acting On A Horizontally Projected Surface.

A Horizontal Earthquake Loading Coefficient Of0.170 Has Been Assigned

A Vertical Earthquake Loading Coefficient Of0.000 Has Been Assigned



1

Cavitation Pressure = 0.0(psf)

Trial Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	87.76	8427.35
2	112.75	8427.71
3	137.66	8429.89
4	162.34	8433.87
5	186.66	8439.64
6	210.50	8447.17
7	233.73	8456.41
8	256.23	8467.31
9	277.87	8479.83
10	298.55	8493.88
11	318.14	8509.41
12	336.56	8526.32
13	337.26	8527.06

Circle Center At X = 95.30 ; Y = 8770.61; and Radius = 343.35

\* \* Factor Of Safety Is Calculated By The Modified Bishop Method \* \*

Factor Of Safety For The Preceding Specified Surface = 1.077

\*\*\*Table 1 - Individual Data on the 19 Slices\*\*\*

Slice No.	Width (ft)	Weight (lbs)	Water	Water	Tie	Tie	Earthquake		
			Force Top (lbs)	Force Bot (lbs)	Force Norm (lbs)	Force Tan (lbs)	Force Hor (lbs)	Force Ver (lbs)	Force Surcharge Load (lbs)
1	1.2	26.9	0.0	0.0	0.0	0.0	4.6	0.0	0.0
2	2.1	987.8	0.0	0.0	0.0	0.0	167.9	0.0	0.0
3	13.8	17123.6	0.0	0.0	0.0	0.0	2911.0	0.0	0.0
4	7.9	13968.0	0.0	0.0	0.0	0.0	2374.6	0.0	0.0
5	24.9	60555.9	0.0	0.0	0.0	0.0	10294.5	0.0	0.0
6	24.7	80561.4	0.0	0.0	0.0	0.0	13695.4	0.0	0.0
7	24.3	94067.8	0.0	0.0	0.0	0.0	15991.5	0.0	0.0
8	23.8	101039.0	0.0	0.0	0.0	0.0	17176.6	0.0	0.0
9	1.5	6547.8	0.0	0.0	0.0	0.0	1113.1	0.0	0.0
10	21.7	95122.0	0.0	0.0	0.0	0.0	16170.7	0.0	0.0
11	22.5	96387.7	0.0	0.0	0.0	0.0	16385.9	0.0	0.0
12	18.8	74808.1	0.0	0.0	0.0	0.0	12717.4	0.0	0.0
13	2.9	10754.2	0.0	0.0	0.0	0.0	1828.2	0.0	0.0
14	20.7	67308.1	0.0	0.0	0.0	0.0	11442.4	0.0	0.0

15	1.5	4019.8	0.0	0.0	0.0	0.0	683.4	0.0	0.0
16	18.1	39301.6	0.0	0.0	0.0	0.0	6681.3	0.0	0.0
17	7.9	10244.9	0.0	0.0	0.0	0.0	1741.6	0.0	0.0
18	10.6	5660.1	0.0	0.0	0.0	0.0	962.2	0.0	0.0
19	0.7	26.0	0.0	0.0	0.0	0.0	4.4	0.0	0.0

\*\*\*Table 2 - Base Stress Data on the 19 Slices\*\*\*

Slice No. *	Alpha (deg)	X-Coord. Slice Cntr (ft)	Base Leng. (ft)	Available Shear Strength (psf)	Mobilized Shear Stress (psf)
1	0.83	88.38	1.24	17.39	2.76
2	0.83	90.05	2.10	376.79	8.24
3	0.83	97.99	13.77	995.93	18.19
4	0.83	108.81	7.88	1107.65	26.02
5	5.00	125.21	25.01	1380.93	211.22
6	9.16	150.00	25.00	1716.64	513.24
7	13.35	174.50	25.00	1929.17	868.83
8	17.53	198.58	25.00	2027.04	1217.38
9	21.69	211.25	1.61	2015.79	1501.03
10	21.69	222.86	23.39	2020.85	1503.49
11	25.85	244.98	25.00	1918.38	1680.90
12	30.05	265.61	21.69	1738.24	1727.76
13	30.05	276.43	3.32	1646.10	1625.18
14	34.19	288.21	25.00	1405.00	1513.04
15	38.41	299.28	1.85	1176.51	1348.16
16	38.41	309.07	23.14	1099.27	1055.04
17	42.55	322.07	10.67	624.37	649.62
18	42.55	331.28	14.33	256.75	267.23
19	46.75	336.91	1.02	16.74	21.53

Sum of the Resisting Forces (including Pier/Pile, Tieback, Reinforcing Soil Nail, and Applied Forces if applicable) = 414392.16 (lbs)

Average Available Shear Strength (including Tieback, Pier/Pile, Reinforcing, Soil Nail, and Applied Forces if applicable) = 1501.30(psf)

Sum of the Driving Forces = 384919.50 (lbs)

Average Mobilized Shear Stress = 1394.53(psf)

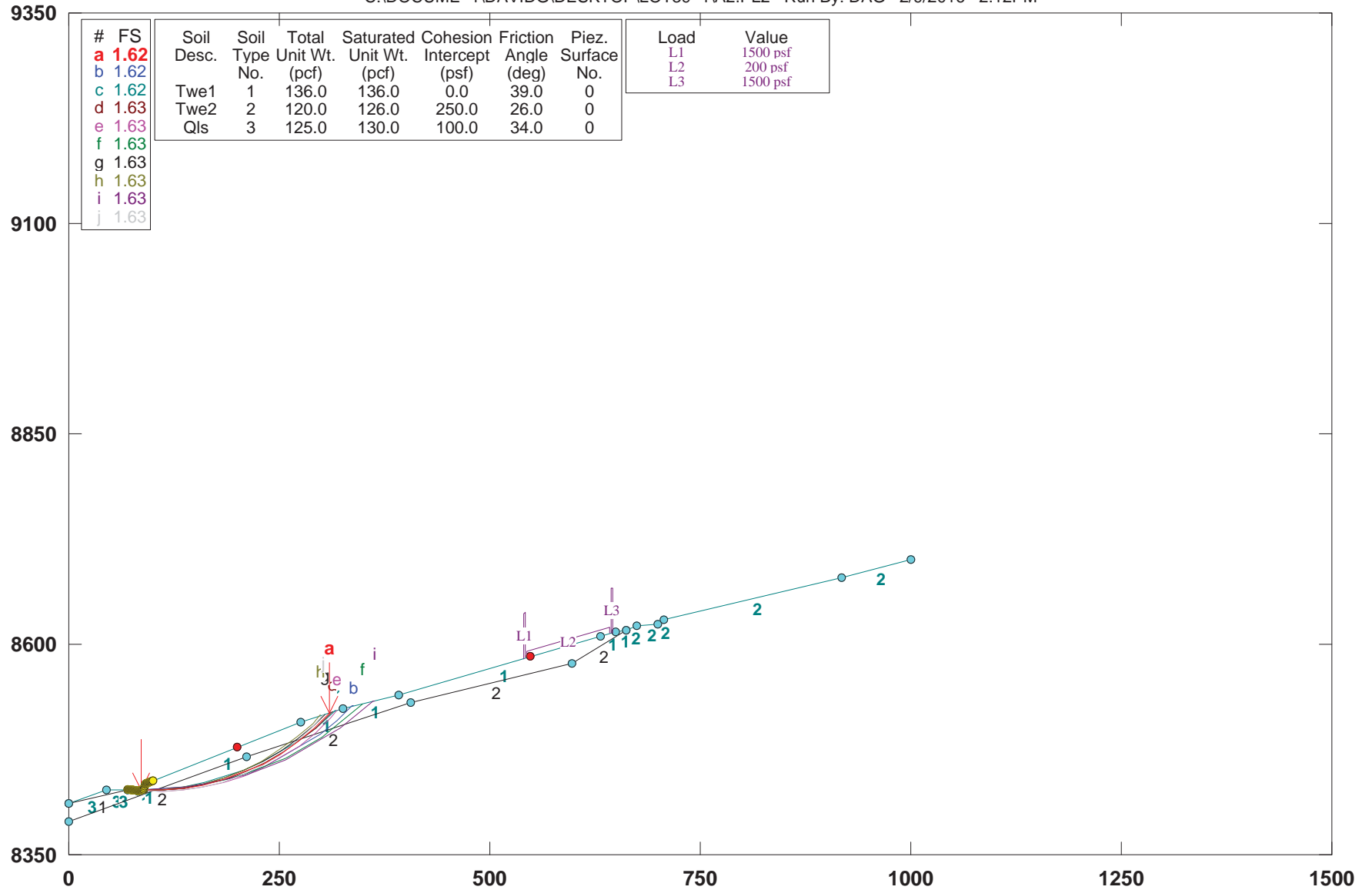
Total length of the failure surface = 276.02(ft)

CAUTION - Factor Of Safety Is Calculated By The Modified Bishop Method. This Method Is Valid Only If The Failure Surface Approximates A Circular Arc.

\*\*\*\* END OF GSTABL7 OUTPUT \*\*\*\*

# Lot 39; A-A'; 02132-002; Post-LS Failure; Setback; Sat. Unit Weight; Static

C:\DOCUME~1\DAVIDG\DESKTOP\LOT39-1\A2.PL2 Run By: DAG 2/9/2016 2:12PM



#	FS	Soil Desc.	Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Piez. Surface No.
a	1.62	Twe1	1	136.0	136.0	0.0	39.0	0
b	1.62	Twe2	2	120.0	126.0	250.0	26.0	0
c	1.62	Qls	3	125.0	130.0	100.0	34.0	0
d	1.63							
e	1.63							
f	1.63							
g	1.63							
h	1.63							
i	1.63							
j	1.63							

Load	Value
L1	1500 psf
L2	200 psf
L3	1500 psf

GSTABL7 v.2 FSmin=1.62  
Safety Factors Are Calculated By The Modified Bishop Method



\*\*\* GSTABL7 \*\*\*

\*\* GSTABL7 by Garry H. Gregory, P.E. \*\*

\*\* Original Version 1.0, January 1996; Current Version 2.002,  
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Nonlinear Undrained Shear Strength, Curved Phi Envelope,  
Anisotropic Soil, Fiber-Reinforced Soil, Boundary Loads, Water  
Surfaces, Pseudo-Static Earthquake, and Applied Force Options.

\*\*\*\*\*

Analysis Run Date: 2/9/2016  
Time of Run: 2:12PM  
Run By: DAG  
Input Data Filename: C:a2.  
Output Filename: C:a2.OUT  
Unit System: English  
  
Plotted Output Filename: C:a2.PLT

PROBLEM DESCRIPTION: Lot 39; A-A'; 02132-002; Post-LS Failure  
; Setback; Sat. Unit Weight; Static

BOUNDARY COORDINATES

16 Top Boundaries  
21 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	0.00	8410.00	45.00	8427.00	3
2	45.00	8427.00	70.00	8427.50	3
3	70.00	8427.50	83.00	8426.00	3
4	83.00	8426.00	89.00	8427.70	1
5	89.00	8427.70	91.10	8434.30	1
6	91.10	8434.30	275.00	8508.00	1
7	275.00	8508.00	326.00	8524.30	1
8	326.00	8524.30	392.00	8540.50	1
9	392.00	8540.50	632.00	8610.00	1

10	632.00	8610.00	650.00	8614.40	1
11	650.00	8614.40	662.00	8616.10	1
12	662.00	8616.10	675.00	8622.00	2
13	675.00	8622.00	700.00	8623.60	2
14	700.00	8623.60	706.00	8629.20	2
15	706.00	8629.20	917.00	8680.00	2
16	917.00	8680.00	1000.00	8700.00	2
17	0.00	8410.00	70.00	8427.50	1
18	0.00	8390.00	212.00	8466.00	2
19	212.00	8466.00	406.00	8530.00	2
20	406.00	8530.00	597.00	8578.00	2
21	597.00	8578.00	662.00	8616.10	2

User Specified Y-Origin = 8350.00(ft)

ISOTROPIC SOIL PARAMETERS

3 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	136.0	136.0	0.0	39.0	0.00	0.0	0
2	120.0	126.0	250.0	26.0	0.00	0.0	0
3	125.0	130.0	100.0	34.0	0.00	0.0	0

BOUNDARY LOAD(S)

3 Load(s) Specified

Load No.	X-Left (ft)	X-Right (ft)	Intensity (psf)	Deflection (deg)
1	540.00	542.00	1500.0	0.0
2	543.00	643.00	200.0	0.0
3	644.00	646.00	1500.0	0.0

NOTE - Intensity Is Specified As A Uniformly Distributed Force Acting On A Horizontally Projected Surface.

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Circular Surfaces, Has Been Specified.

2500 Trial Surfaces Have Been Generated.

50 Surface(s) Initiate(s) From Each Of 50 Points Equally Spaced  
 Along The Ground Surface Between X = 70.00(ft)  
 and X = 100.00(ft)

Circle Center At X = 99.36 ; Y = 8710.44 ; and Radius = 284.13

Factor of Safety  
 \*\*\* 1.621 \*\*\*

Each Surface Terminates Between X = 200.00(ft)  
 and X = 548.00(ft)

Unless Further Limitations Were Imposed, The Minimum Elevation  
 At Which A Surface Extends Is Y = 0.00(ft)

Individual data on the 17 slices

25.00(ft) Line Segments Define Each Trial Failure Surface.

Restrictions Have Been Imposed Upon The Angle Of Initiation.  
 The Angle Has Been Restricted Between The Angles Of -30.0  
 And 10.0 deg.

Following Is Displayed The Most Critical Of The Trial  
 Failure Surfaces Evaluated.

\* \* Safety Factors Are Calculated By The Modified Bishop Method \* \*

Total Number of Trial Surfaces Evaluated = 2500

Statistical Data On All Valid FS Values:  
 FS Max = 2.582 FS Min = 1.621 FS Ave = 1.987  
 Standard Deviation = 0.189 Coefficient of Variation = 9.52 %

Slice No.	Width (ft)	Weight (lbs)	Water Force		Tie Force		Earthquake Force Surcharge		
			Top (lbs)	Bot (lbs)	Norm (lbs)	Tan (lbs)	Hor (lbs)	Ver (lbs)	Load (lbs)
1	3.7	267.8	0.0	0.0	0.	0.	0.0	0.0	0.0
2	2.1	1248.7	0.0	0.0	0.	0.	0.0	0.0	0.0
3	10.9	14648.0	0.0	0.0	0.	0.	0.0	0.0	0.0
4	8.3	15394.9	0.0	0.0	0.	0.	0.0	0.0	0.0
5	24.9	63299.7	0.0	0.0	0.	0.	0.0	0.0	0.0
6	24.6	83044.4	0.0	0.0	0.	0.	0.0	0.0	0.0
7	24.2	94802.6	0.0	0.0	0.	0.	0.0	0.0	0.0
8	23.5	98571.2	0.0	0.0	0.	0.	0.0	0.0	0.0
9	4.5	18909.1	0.0	0.0	0.	0.	0.0	0.0	0.0
10	18.2	75893.0	0.0	0.0	0.	0.	0.0	0.0	0.0
11	21.7	84304.9	0.0	0.0	0.	0.	0.0	0.0	0.0
12	20.5	68008.9	0.0	0.0	0.	0.	0.0	0.0	0.0
13	1.7	4980.5	0.0	0.0	0.	0.	0.0	0.0	0.0
14	0.9	2653.5	0.0	0.0	0.	0.	0.0	0.0	0.0
15	16.5	36923.2	0.0	0.0	0.	0.	0.0	0.0	0.0
16	17.7	14710.2	0.0	0.0	0.	0.	0.0	0.0	0.0
17	0.1	0.7	0.0	0.0	0.	0.	0.0	0.0	0.0

\*\*\*\* END OF GSTABL7 OUTPUT \*\*\*\*

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	85.31	8426.65
2	110.31	8426.52
3	135.22	8428.58
4	159.86	8432.82
5	184.03	8439.21
6	207.54	8447.71
7	230.22	8458.23
8	251.88	8470.71
9	272.36	8485.04
10	291.51	8501.12
11	309.16	8518.82
12	309.27	8518.95