- DRAWINGS ARE PRELIMINARY AND NOT FOR CONSTRUCTION UNLESS STRUCTURAL ENGINEERS WET STAMP IS AFFIXED TO DRAWINGS. ANY DISCREPANCIES IN THE DRAWINGS, NOTES AND SPECIFICATIONS, SHALL BE REPORTED TO ENGINEER/ARCHITECT FOR CLARIFICATION. THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS ELEVATIONS, AND TOP OF CONC. PRIOR TO PROCEEDING WITH ANY
- WORK OR FABRICATION. THE CONTRACTOR IS RESPONSIBLE FOR ALL BRACING AND SHORING DURING CONSTRUCTION.
- CONTRACTOR TO SUBMIT A REQUEST TO ENGINEER & ARCHITECT FOR ANY SUBSTITUTION OF MATERIALS OR PRODUCTS SPECIFIED ON THE
- STRUCTURAL DESIGN PER 2012 INTERNATIONAL BUILDING CODE (IBC).
- ALL CONSTRUCTION TO CONFORM TO 2012 IBC. THE FOLLOWING NOTES APPLY UNLESS SHOWN OTHERWISE. THESE DRAWINGS HAVE BEEN PREPARED SOLELY FOR THE USE IN THE CONSTRUCTION OF A PROPOSED BUILDING TO WHICH THESE NOTES ARE ATTACHED. THE DRAWINGS SHALL NOT BE USED IN
- OTHER LOCATION WITHOUT THE WRITTEN CONSENT OF THE ENGINEER. THE OWNER SHALL NOTIFY ENGINEER IF ANY UNIQUE SOILS CONDITIONS EXIST ON SITE WHICH MAY BE DETECTED DURING
 - CONSTRUCTION. THESE INCLUDE BUT SHALL NOT BE LIMITED TO: 1. SATURATED SOIL AT FOOTING SUBGRADE

WHOLE OR IN PART, FOR FABRICATION OR CONSTRUCTION AT ANY

- 2. GROUNDWATER
- 3. UNDOCUMENTED FILL 4. CLAY SOIL WITH SWELL OR COLLAPSE POTENTIAL
- 5. FILL BEING PLACED BELOW FOOTINGS
- EPIC ENGINEERING CANNOT BE HELD RESPONSIBLE FOR SOIL CONDITIONS THAT ARE NOT BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO WORK PROCEEDING. IT IS THE RESPONSIBILITY OF THE OWNER TO HIRE A GEO-TECHNICAL ENGINEER IF NEEDED. THE DESIGN OF THE FOUNDATION SYSTEM SHALL BE BASED ON THE ALLOWABLE SOIL BEARING PRESSURES ALLOWED IN TABLE 1804.2 OF THE 2012 IBC
- THE CONTRACTOR SHALL VISUALLY INSPECT THE SITE PRIOR TO WORK PROCEEDING AND SHALL NOTIFY ENGINEER IF ANY UNIQUE SOIL CONDITIONS EXIST THAT COULD AFFECT THE PERFORMANCE OF THE FOUNDATION SYSTEM PRIOR TO ANY WORK PROCEEDING.

SECTION 2 - CONCRETE

STRUCTURAL CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3,000 PSI. CONCRETE FOR SLABS ON GRADE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AND A MAXIMUM WATER/CEMENT RATIO OF 0.5 MINIMUM CEMENT CONTENT SHALL BE 5 SACK/CU. YD. MAXIMUM AGGREGATE SHALL BE 3/4", INCLUDE 4% TO 6% AIR ENTRAINMENT WITH SLUMP NOT TO EXCEED 4".

CAST IN PLACE CONCRETE

CONCRETE FORM WORK TO BE OF ADEQUATE SIZE AND STRENGTH PROPERLY BRACED TO PREVENT SAGGING OR BULGING. PROTECT ALL CONCRETE FROM FREEZING TEMPERATURES. REFER TO DRAWING FOR DIMENSIONS OF CONCRETE MEMBERS AND SIZE AND LOCATION OF ALL REINFORCEMENT.

- NO FOOTING SHALL BE PLACED ON DISTURBED (OR FROZEN) SOIL (IF DISTURBED, COMPACT SOIL IN 6" LIFTS TO 95% OR MAXIMUM DRY DENSITY PER ASTM D1557). FOOTINGS SHALL BE STEPPED DOWN ONE (1) VERTICALLY TO ONE AND ONE HALF (1 1/2) HORIZONTALLY,
- UNLESS BULK HEADED & STOPPED VERTICALLY. FOUNDATION WALLS: REINFORCE PER DRAWINGS. DO NOT BACKFILL WALLS UNTIL MAIN FLOOR IS FRAMED, THE SUBFLOOR INSTALLED, SHEATHED AND CONCRETE HAS CURED A MINIMUM OF 7 DAYS. SEE SPECIAL PROVISIONS FOR COLD WEATHER CONCRETE BELOW. USE HAND OPERATED COMPACTION EQUIPMENT ADJACENT TO NEWLY
- PLACED CONCRETE BASEMENT WALLS. CONCRETE PADS AND THICKENED SLABS: REFER TO DRAWINGS AS TO SIZE AND REINFORCEMENT.
- REFER TO DRAWINGS AS TO SIZE AND REINFORCEMENT
- FIREPLACE FOOTINGS AND CMU WALLS: REFER TO DRAWINGS FOR SIZE AND REINFORCEMENT.
- REINFORCEMENT BARS: REINFORCEMENT SHALL BE PER ASTM A615, GRADE 60 FOR #5 BARS AND LARGER, GRADE 40 FOR #3 AND #4 BARS. ALL REBAR LAPPED 30 TIMES DIAMETER, REBAR AT FOOTINGS TO HAVE 3" CLEAR COVER OF CONCRETE (U.N.O. ON DRAWINGS).PROVIDE CORNER BARS WITH 18" LEGS AT THE CORNERS OF ALL WALLS AND FOOTINGS, SIZE AND PLACEMENT TO MATCH HORIZONTAL REINFORCEMENT.
- COLD-WEATHER CONCRETING: CONTRACTOR SHALL SUBMIT TO ENGINEER FOR REVIEW THE PROPOSED MEASURES TO SATISFY PLACEMENT & CURING OF CONCRETE DURING COLD WEATHER. FOR OPTIMUM STRENGTH GAIN. IT IS RECOMMENDED TO CONSIDER A BLEND OF TYPE I AND TYPE II CEMENT WITH A 6 BAG MIX, LOW SAND TO AGGREGATE RATIO, BATCHED TO A 1" SLUMP WITH SUPER PLASTICIZER ADDED FOR 4"-5" SLUMP WORKABILITY, 1%-2% NON-CHLORINE ACCELERATOR & CONCRETE MAINTAINED AT 50° MINIMUM FOR 7 DAYS. AVOID MORE THAN 25° TEMPERATURE CHANGE PER DAY WHEN HEATING IS
- TERMINATED. ANCHOR BOLTS AND HOLDOWN: ANCHOR BOLTS TO BE ASTM A307, 5/8"Øx10" EMBEDDED IN FOUNDATION WALLS @ 32" O.C. (MAX) U.N.O. (SEE FOUNDATION PLAN FOR REQUIREMENTS AT SHEAR WALLS). BOLTS TO BE WITHIN 1'-0" OF SILL PLATES ENDS (COORDINATE WITH GENERAL CONTRACTOR). MINIMUM OF TWO ANCHOR BOLTS PER SILL PLATE.
- ALL POSTS SUPPORTED BY ISOLATED FOOTINGS TO HAVE POST ANCHORS UNLESS SPACED IN STUD WALLS. REFER TO DRAWINGS FOR HOLDOWN REQUIREMENTS. INSTALL REQUIRED EMBEDDED ITEMS PER MANUFACTURER'S
- CATALOG TO ENGAGE HOLDOWN CONSTRUCTION AND CRACK CONTROL JOINTS ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED TO REMOVE DUST, CHIPS AND OTHER FOREIGN MATERIAL PRIOR TO PLACING ADJACENT CONCRETE. CRACK CONTROL JOINTS IN SLABS SHALL HAVE A MAXIMUM SPACING OF 15'-0" IN BOTH DIRECTIONS. THE CONTRACTOR SHALL SUBMIT THE DETAILS AND PROPOSED LOCATIONS OF CONSTRUCTION JOINTS AND CRACK CONTROL JOINTS FOR REVIEW
- VAPOR BARRIER: VAPOR BARRIER TO BE 6 MIL POLYETHYLENE SHEET PLACED ON UNDISTURBED SOIL. VAPOR BARRIER UNDER SLAB ON GRADE, PLACED ON COMPACTED GRAVEL WITH 3/4" TO 1-1/2" OF DAMP SAND BETWEEN POLYETHYLENE VAPOR BARRIER AND CONCRETE.

BEFORE STARTING CONSTRUCTION.

EMBEDDED HOLDOWNS EMBEDDED ITEMS FOR HD TYPE HOLDOWN TO BE ASTM A307 HEX HEADED BOLT IN THE DIAMETER AS SPECIFIED BY THE MANUFACTURER FOR THE HD. ALL BOLTS TO HAVE 3" MIN. CONCRETE SIDE COVER EMBEDMENT DEPTHS ARE 15" FOR BOLTS UP TO AND INCLUDING 3/4" DIA., 24" DEPTH FOR BOLTS OVER 3/4" U.N.O. TYPICAL REINFORCEMENT TO

PASS UNINTERRUPTED ALONGSIDE HOLD DOWN AS APPLICABLE. COUPLER NUTS MAY BE USED TO EXTEND THE HOLD DOWN ANCHOR THROUGH THE FLOOR PLATE TO THE SHEAR WALL CHORD. **EPOXY ANCHORS:** ANCHORING ADHESIVE SHALL BE A TWO-COMPONENT HIGH-SOLIDS, EPOXY

SYSTEM SUPPLIED IN MANUFACTURER'S STANDARD CARTRIDGE AND DISPENSED THROUGH A STATIC-MIXING NOZZLE SUPPLIED BY THE MANUFACTURER. THE ADHESIVE ANCHOR SHALL HAVE BEEN TESTED AND QUALIFIED FOR PERFORMANCE IN UN-CRACKED CONCRETE PER ICC-ES AC308. ADHESIVE SHALL BE SET-XP EPOXY-TIE ADHESIVE FROM SIMPSON STRONG-TIE, PLEASANTON, CA. ANCHORS SHALL BE INSTALLED PER SIMPSON STRONG-TIE INSTRUCTIONS FOR SET-XP **EPOXY-TIE ADHESIVE**

NOTE: THE USE OF EPOXY ANCHORS REQUIRES SPECIAL INSPECTION OF INSTALLATION PER CURRENT ICO REPORT, CONTRACTOR TO PROVIDE SPECIAL INSPECTION REPORTS TO ENGINEER, BUILDING OFFICIAL & ARCHITECT.

SECTION 3 - FRAMING LUMBER

I. SAWN STRUCTURAL LUMBER

- SAWN LUMBER SHALL BE DOUGLAS FIR-LARCH (DF-L) NO.2 OR BETTER FOR ALL 2 INCH AND 4 INCH NOMINAL LUMBER AND DF-L NO.2 OR BETTER FOR 6 INCH NOMINAL AND LARGER STRUCTURAL MEMBERS (U.N.O.)
- WOOD BEARING ON OR INSTALLED WITHIN 1" OF MASONRY OR CONCRETE SHALL BE PRESSURE TREATED WITH AN APPROVED PRESERVATIVE. PROVIDE MILD STEEL PLATE WASHERS AT ALL BOLT HEADS AND NUTS BEARING ON WOOD.
- ALL FRAMING DETAILS SHALL BE IN ACCORDANCE WITH CHAPTER 23 OF THE 2012 IBC, UNLESS OTHERWISE NOTED ON THE DRAWINGS. ALL FRAMING NAILING SHALL CONFORM TO TABLE 2304.9.1 OF THE IBC UNLESS OTHERWISE SHOWN. PROVIDE STEEL STRAPS AT PIPES IN STUD WALLS AS REQUIRED BY IBC CHAPTER 23. PLUMBING AND ELECTRICAL RUNS IN STUD WALLS SHALL CONFORM TO CHAPTER 23. BOLTS SHALL BE STANDARD MACHINE BOLTS (A307) ALL NAILS SHALL BE COMMON WIRE OR GALVANIZED BOX NAILS. IF PNEUMATIC NAILERS ARE TO BE USED, CONTRACTOR MUST SUBMIT A SCHEDULE OF NAILS DESIRED AS SUBSTITUTION TO THE ARCHITECT OR ENGINEER FOR REVIEW. A CHANGE IN THE NUMBER OF NAILS OR A CLOSER NAIL SPACING MAY
- BE REQUIRED. METAL HANGERS AND CONNECTORS SHALL BE FULLY NAILED OR BOLTED UNLESS OTHERWISE NOTED ON THE DRAWINGS. METAL HANGERS OR CONNECTORS SHOWN ON THE DRAWINGS SHALL BE MANUFACTURED BY SIMPSON COMPANY. METAL HANGERS OR CONNECTORS BY OTHER MANUFACTURES MAY BE CONSIDERED WHERE LOAD CAPACITY AND DIMENSIONS ARE EQUAL OR BETTER ALL SUBSTITUTIONS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW
- PROVIDE SOLID BLOCKING BELOW ALL BEARING WALLS. PROVIDE SOLID VERTICAL BLOCKING IN FLOOR SPACE TO MATCH STUD BUNDLE OR SOLID COLUMN ABOVE AND BELOW VERTICAL BLOCKING AT WOOD I-JOISTS SHALL BE 1/16" LONGER THAN JOIST IS DEEP. MINIMUM POST TO BE TWO 2x STUDS BEARING AT EACH END OF HEADER U.N.O. FOR BEAMS FRAMING PERPENDICULAR TO BEARING WALLS PROVIDE FULL WIDTH BEAM POCKET WITH FILLER AS REQUIRED AND KING STUD BOTH SIDES. STITCH STUD BUNDLES TOGETHER WITH 16d COMMON @ 18" O.C. MAXIMUM (U.N.O.) WHERE FLOOR BEAMS ARE FRAMED FLUSH WITHIN FLOOR AND TOP FLANGE HANGERS ARE SPECIFIED, BEAMS ARE TO BE BLOCKED UP TO JOIST HEIGHT WITH FULL WIDTH DF-L SPACER AS REQUIRED.
- FIRE BLOCK STUD SPACED AT SOFFITS, FLOOR AND CEILING JOIST LINES, AT 10' VERTICALLY AND HORIZONTALLY, AND AT OPENINGS BETWEEN ATTIC SPACES FOR FACTORY BUILT CHIMNEYS, AND AT OTHER LOCATIONS NOT SPECIFICALLY MENTIONED WHICH COULD AFFORD PASSAGE FOR FLAMES.
- STRUCTURAL GLUED-LAMINATED TIMBER ALL GLUED-LAMINATED TIMBER SHALL BE COMBINATION 24F-V4 FOR SIMPLY SUPPORTED BEAMS, COMBINATION 24F-V8 FOR BEAMS CONTINUOUS OVER SUPPORTS, AND COMBINATION L2 FOR COLUMNS (U.N.O.) FABRICATION TO BE IN ACCORDANCE WITH AITC 117. PROVIDE WET-USE ADHESIVES. MAXIMUM MOISTURE CONTENT SHALL BE 15% PROVIDE MILD STEEL PLATE WASHERS AT ALL BOLT HEADS AND NUTS BEARING ON WOOD. WOOD BEARING ON OR WITHIN 1" OF MASONRY OR CONCRETE SHALL BE TREATED WITH AN APPROVED PRESERVATIVE. SEAL END GRAIN OF ALL EXTERIOR EXPOSED BEAMS
- MANUFACTURED JOISTS SIZE AND SPACING HAVE BEEN DETERMINED PER THE MANUFACTURES STANDARDS. SUBSTITUTION OF PRODUCTS BY OTHER MANUFACTURERS REQUIRES APPROVAL OF ENGINEER OF RECORD. JOIST SHALL BE ERECTED, INSTALLED,
- AND BRACED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. LAMINATED VENEER LUMBER (LVL) PRODUCTS SPECIFIED HEREIN AS LVL AND PSL SHALL CONFORM TO THE PERFORMANCE CRITERIA OF LVL AND PSL PRODUCTS AS MANUFACTURED BY TRUSS JOIST AS MICRO-LAM AND PARALLAM. SUBSTITUTES ARE ACCEPTABLE PROVIDED THEY HAVE THE SAME STRUCTURAL VALUES. ANY SUBSTITUTIONS MUST BE SUBMITTED TO
- THE ENGINEER FOR REVIEW. WOOD SHEATHING ALL WOOD SHEATHING SHALL BE APA RATED EXPOSURE 1 PLYWOOD OR OSB WITH THICKNESS, VENEER GRADES AND SPAN RATING AS NOTED HEREIN OR ON DRAWINGS

INCLUDING NON-LOAD BEARING ARCHITECTURAL BEAMS.

ROOF SHEATHING 5/8" WITH MINIMUM (40/20) SPAN RATING.

FLOOR SHEATHING

- 3/4" OSB GLUED AND NAILED EXTERIOR WALL AND SHEAR WALL SHEATHING
- 7/16" WITH MINIMUM (24") SPAN RATING. ROOF AND FLOOR SHEATHING TO BE LAID UP WITH FACE GRAIN PERPENDICULAR TO SUPPORTS AND END JOINTS STAGGERED 4'-0" O.C. INSTALL ROOF SHEATHING WITH 1/8" SPACE AT ALL PANEL EDGES. NAIL ROOF SHEATHING WITH 8d @ 6" O.C. AT SUPPORTED PANEL AND 12" O.C. AT INTERMEDIATE FRAMING, FLOOR SHEATHING WITH 10d @ 6" O.C. AT SUPPORTED PANEL EDGES AND 10" O.C. FIELD, U.N.O. HOLES ARE NOT PERMITTED IN DIAPHRAGMS UNLESS REVIEWED BY ENGINEER. NAIL EXTERIOR WALL SHEATHING WITH 8d @ 6" O.C. EDGES AND 12" O.C. FIELD U.N.O. IN SHEAR WALL SCHEDULE. OFFSET VERTICAL JOINTS 4'-0" O.C. INSTALL WITH 1/8" GAP AT BUTT ENDS.
- WOOD SHEAR WALLS NO.14 GAGE STAPLES WITH MINIMUM 7/16 OD CROWN AND 1-3/8" LENGTH MAY BE USED ONE FOR ONE IN LIEU OF 8d NAILS. WHERE SUBSTITUTING FOR 10d NAILS USE 3 STAPLES FOR EACH 2 NAILS.
 - WHERE PLYWOOD PANELS ARE APPLIED TO BOTH SIDES OF SHEAR WALL, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, OR FRAMING MEMBERS SHALL BE 3" (NOMINAL) WIDE AND NAILS ON EACH SIDE SHALL BE
 - ALLOWABLE SHEAR VALUES IN SHEAR WALL TABLE ARE FOR DOUGLAS FIR FRAMING MEMBERS (GROUP II). NO SUBSTITUTION OF LESSER GROUPS WILL BE ALLOWED. FASTENERS EXPOSED TO WEATHER SHALL BE ZINC COATED BY HOT DIP GALVANIZING, MECHANICALLY DEPOSITED, OR ELECTRO-DEPOSITED.

SECTION 4 - STRUCTURAL STEEL AND MISCELLANEOUS METALS

- ALL STRUCTURAL STEEL SHALL COMPLY WITH THE PREFERRED ASTM MATERIAL SPECIFICATION FOR VARIOUS SHAPES PER TABLES 2-3 AND 2-4 OF AISC'S STEEL CONSTRUCTION
- MANUAL (THIRTEENTH EDITION) ALL WORK SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", & AISC 341 FOR FABRICATION OF LATERAL ELEMENTS. SHOP DRAWINGS SHALL BE SUBMITTED FOR THE OWNER'S REPRESENTATIVES REVIEW BEFORE COMMENCING FABRICATION. SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. ALL WELDING SHALL BE DONE BY "STRUCTURAL WELDING CODE", AWS D1.1 ALL FIELD WELDING TO BE ACCOMPLISHED BY AWS CERTIFIED WELDERS.
- ALL STEEL ANCHORS, TIES AND OTHER MEMBERS TO BE EMBEDDED IN CONCRETE OR MASONRY SHALL BE LEFT UNPAINTED. ALL MACHINE BOLTS SHALL BE ASTM A307 U.N.O. (SEE CONNECTION SCHEDULE FOR A325 BOLTS) AND SHALL BE PROVIDED WITH LOCK WASHERS UNDER NUTS OR SELF LOCKING NUTS. ALL NUTS, BOLTS, WASHERS AND MISC. STEEL EXPOSED TO WEATHER SHALL
- WELDED HEADED STUDS (WHS)+ TYPICAL WELD OF WHS TO STEEL SHALL BE FILLET WELD ALL AROUND SIZE EQUAL TO ONE-HALF THE DIAMETER OF THE STUD.

SECTION 5 - LIGHT GAGE METAL FRAMING.

- DESIGN, FABRICATION AND ERECTION OF LIGHT-GAGE METAL FRAMING SHALL COMPLY WITH REQUIREMENTS OF: ASIC "MANUAL OF STEEL CONSTRUCTION", AWS "STRUCTURAL WELDING CODE", AISI "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", AND ICBO REPORT ER-4943P
- FRAMING SHOWN ON PLANS ARE MINIMUM SIZES AND CONDITIONS. SUBSTITUTION OF FRAMING MEMBERS SHALL BE APPROVED BY ARCHITECT AND ENGINEERS. THEY SHALL HAVE CAPACITY FOR GRAVITY LOADS AND LATERAL LOADS EQUAL TO OR BETTER THAN SPECIFIED FRAMING MEMBERS AND SHALL BE ICBO APPROVED.
- ALL COMPONENTS SHALL BE GALVANIZED ACCORDING TO REQUIREMENTS OF ASTM A-525 FOR MINIMUM G-60 COATING.
- ALL 14, 16, 18, AND 20 GAGE STUDS, AND ALL TRACK, BRIDGING, END CLOSURES AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE MINIMUM REQUIREMENTS OF ASTM A-446, WITH A MINIMUM YIELD OF 33 KSI FOR 18 AND 20 GAGE, AND 50 KSI FOR 16 GAGE, FOR STUDS AND 33 KSI FOR RUNNERS, BRIDGING, END CLOSURES AND
- ALL WELDS SHALL BE ACCOMPLISHED USING 1/8" AWS TYPE 6013 OR 7014 ROD WITH A WELDING HEAT OF 60 TO 110 AMPERES DEPENDING ON THE GAGE OF MATERIAL AND THE FIT OF THE PARTS. WIRE TYING OF FRAMING COMPONENTS IS NOT PERMITTED.
- ADEQUATE LATERAL BRACING MUST BE PROVIDED DURING CONSTRUCTION. UNLESS NOTED OTHERWISE, METAL FRAMING MEMBERS AT LOAD BEARING, SHEAR WALLS AND EXTERIOR WALLS SHALL BE 20 GAGE FOR STUDS, 16 GAGE FOR TOP AND BOTTOM RUNNER.
- 16 GAGE FOR STUDS AT HOLD DOWNS. RUNNER TRACKS: INSTALL CONTINUOUS TRACKS SIZED TO MATCH STUDS. ALIGN TRACKS ACCURATELY TO LAYOUT AT BASE AND TOPE OF STUDS. UNLESS INDICATED OTHERWISE, SECURE TRACKS AS RECOMMENDED BY STUD MANUFACTURE FOR TYPE OF CONSTRUCTION INVOLVED, DO NOT EXCEED 24" O.C. SPACING FOR NAIL OR POWER-DRIVEN FASTENERS, OR 16"O.C. FOR OTHER TYPES OF ATTACHMENT. PROVIDE FASTENERS AT CORNERS
- AND ENDS OF TRACK. FASTENINGS: FASTENING OF COMPONENTS SHALL BE WITH SELF-DRILLING SCREWS OR BY WELDING. SCREWS AND WELDS SHALL BE OF SUFFICIENT SIZE TO ENSURE THE STRENGTH OF THE CONNECTION. WIRE TYING OF COMPONENTS SHALL NOT BE PERMITTED. ALL WELDS SHALL BE TOUCHED UP WITH A ZINC-RICH PAINT. FASTENING OF PLYWOOD DIAPHRAGMS AND SILL PLATES SHALL BE AS INDICATED IN STRUCTURAL NOTES, AND DETAILS.
- STUD SECTIONS USED AS RAFTERS OR JOISTS SHALL BE UNPUNCHED. ALL NON-LOAD BEARING WALLS SHALL BE ERECTED SO AS TO ALLOW FOR PROPER DEFLECTION OF THE STRUCTURE ABOVE. THE TOPS OF ALL SUCH WALLS SHALL BE HORIZONTALLY BRACED TO THE STRUCTURE ABOVE AT A MAXIMUM OF 8'-0" O.C.

SECTION 6 - METAL DECKING

- ALL DECK SHALL BE 3-SPAN CONTINUOUS MINIMUM. IN AREAS WHERE 3-SPAN CONDITIONS ARE NOT POSSIBLE, THE CONTRACTOR SHALL PROVIDE HEAVIER GAGE DECK AS REQUIRED TO PROVIDE THE EQUIVALENT LOADING OF THE DECK UNDER A THREE SPAN CONDITION. STEEL ROOF DECK SHALL NOT BE USED TO SUPPORT LOADS FROM PLUMBING, HVAC DUCTS, LIGHT FIXTURES, ARCHITECTURAL ELEMENTS OR EQUIPMENT OF ANY KIND, UNLESS SPECIFICALLY NOTED. LIGHT WEIGHT SUSPENDED ACOUSTICAL CEILINGS WITH A TOTAL
- WEIGHT OF 50 LBS PER ATTACHMENT MAY BE HUNG FROM ROOF DECK. THE HANGERS SHALL BE STAGGERED TO DISTRIBUTE THE LOADS OVER MULTIPLE DECK FLUTES.
- CLINCH SEAMS BEFORE WELDING INTERLOCKING SEAMS.

ADHERE TO THE PAINTED DECK.

- STEEL ROOF DECK SHALL BE 1. 1/2" DEEP X 22 GAGE MINIMUM PAINTED, TYPE "B" WIDE RIB DECK WITH INTERLOCKING SIDE SEAMS WITH THE FOLLOWING PROPERTIES:
- MINIMUM I (IN 4/FT) = 0.175MINIMUM ALLOWABLE DECK DIAPHRAGM SHEAR VALUES SHALL BE 500 LB/FT. FOR A 7'-0"
- SHEAR REQUIREMENTS.):
 - (7 WELDS PER 36" SHEET).
- 6" O.C. TO ALL SUPPORTS PARALLEL TO DECK CORRUGATIONS. HILTI POWER DRIVEN FASTENERS ARE ACCEPTABLE AS AN ALTERNATIVE TO WELDS PROVIDED THE CONNECTION MEETS THE DIAPHRAGM SHEER CAPACITY GIVEN ABOVE. FASTENERS ARE USED, A HILTI REPRESENTATIVE SHALL BE PRESENT BEFORE THE DECKING IS INSTALLED TO MAKE SURE THE INSTALLER IS PROPERLY TRAINED IN USING THE EQUIPMENT. THE HILTI REPRESENTATIVE SHALL ALSO MAKE A SITE VISIT THE DAY
- FASTENERS ARE BEING INSTALLED CORRECTLY. ATTACH INTERLOCKING SEAMS WITH 1 1/2" LONG TOP SEAM WELDS AT 24" O.C. MAXIMUM OR WITH VERCO UNCLICK SYSTEM AT 24" O.C. MAXIMUM OR WITH ASC DELTA GRIP SYSTEM AT 24" O.C. MAXIMUM. CLOSER SPACING MAY BE USED TO DEVELOP MINIMUM SHEAR REQUIREMENTS. A STANDARD BUTTON PUNCH CAN NOT BE USED IN PLACE OF VERCO
- PROVIDE A 2-INCH MINIMUM BEARING AND A 4-INCH LAP AT THE SPLICE POINTS.

SUBSTITUTION FOR ANY SPECIFIED STRUCTURAL COMPONENT MUST BE REQUESTED IN WRITING BY THE CONTRACTOR. THE ENGINEER WILL REVIEW THE REQUESTED ALTERNATIVE & RESPOND IN WRITING. ADDITIONAL SUPERVISION OR SPECIAL INSPECTION MAY BR REQUIRED FOR THE

SECTION 9 - JOB SAFETY

THE ENGINEER HAS NOT BEEN RETAINED NOR COMPENSATED TO PROVIDE DESIGN AND/OR CONSTRUCTION REVIEW SERVICES RELATED TO THE CONTRACTOR'S SAFETY PRECAUTIONS OR TO MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES FOR THE CONTRACTOR TO PERFORM HIS WORK. THE UNDERTAKING OF PERIODIC SITE VISITS BY THE ENGINEER SHALL NOT BE CONSTRUED AS SUPERVISION OF ACTUAL CONSTRUCTION NOR MAKE HIM RESPONSIBLE FOR PROVIDING A SAFE PLACE FOR THE PERFORMANCE OF WORK BY THE CONTRACTOR, SUBCONTRACTORS, SUPPLIERS OR THEIR EMPLOYEES, OR FOR ACCESS, VISIT, USE WORK, OR OCCUPANCY BY ANY PERSON.

- STEEL DECK SHALL COMPLY WITH THE LATEST REQUIREMENTS OF THE STEEL DECK
- ALL DECK SUPPORTING MEMBERS SHALL BE DRY BEFORE WELDING.
- YIELD STRESS OF THE 22 GAGE STEEL DECK SHALL BE LIMITED TO A MAXIMUM OF 50 KSI. WHERE DECK IS TO RECEIVE SPRAYED-ON FIRE PROOFING, PAINTED DECK SHALL BE COATED WITH SPECIAL PAINT THAT WILL ALLOW THE SPRAYED-ON FIRE PROOFING TO

SECTION 7 - STEEL ROOF DECK

- MINIMUM S (IN3/FT) = 0.187
- WELD STEEL ROOF DECK TO SUPPORTING FRAMING MEMBERS WITH 3/4" DIAMETER PUDDLE WELDS AT THE FOLLOWING SPACING (CLOSER SPACING MAY BE USED TO DEVELOP MINIMUM
 - 6" O.C. TO ALL SUPPORTS PERPENDICULAR TO DECK CORRUGATIONS
- CALL HILTI AT 800-879-8000 EXTENSION 6337 FOR CONNECTION INFORMATION COMPARISON. IF HILTI POWER DRIVEN FASTENERS ARE USED, THE CONTRACTOR SHALL SUBMIT HILTI'S CALCULATIONS TO THE ARCHITECT/ENGINEER FOR REVIEW. ALSO IF HILTI POWER DRIVEN AFTER DECK HAS BEEN STARTED TO BE INSTALLED TO VERIFY THE POWER DRIVEN
- PUNCHLOCK OR DELTA GRIP.

REQUESTED SUBSTITUTION.

2012 IBC/IRC Governing Code Occupancy Category

CONSTRUCTION NOTES

Importance Factor: Soil Properties: Site Class Soil Bearing Pressure 5000 psf Seismic Design: 0.825 0.183 Seismic Design Category D 4.5/6.5

Basic Wind Speed 115 mph Exposure **Ground Snow Load** Roof Snow Load

65 psf

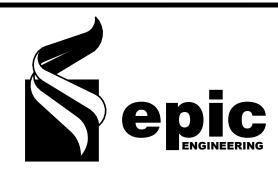
12 psf

Reduced Roof Snow Load

Roof Dead Load

Floor Live Load

Floor Dead Load



MAY 2015

MARK DATE DESCRIPTION

DESIGNER: PW REVIEWED: AJH

PROJECT#

1" = 1'-0"

14SM2068

5/14/2015 SCALES

PROJECT NAME:

FALCONE RESIDENCE

PROJECT LOCATION: 7947 EAST HEARTWOOD DRIVE

WEBER COUNTY, UT

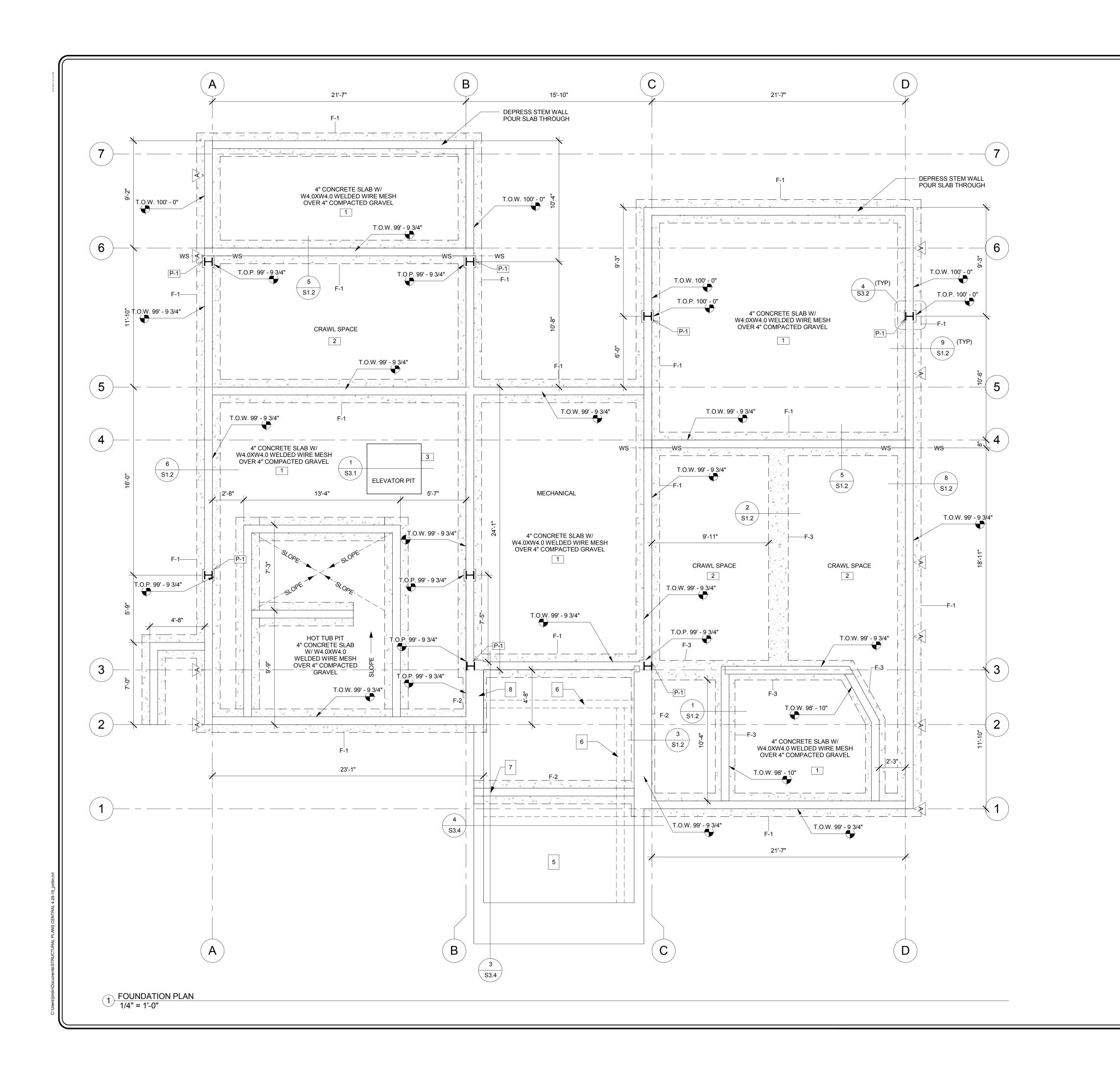
SHEET TITLE:

GENERAL NOTES

PLAN SET: **PERMI7**

S0.1

SHEET



STRUCTURAL FOUNDATION SCHEDULE

F-1 24" CONT. 12" (3) #4 BAR CONT.
F-2 36" CONT. 14" (4) #4 BAR CONT.

MARK | WIDTH | LENGTH | DEPTH | REINFORCEMENT

FOUNDATION WALL SCHEDULE

HEIGHT THICKNESS VERT. REINFORCEMENT HORIZ. REINFORCEMENT

| <u><</u> 4' | 8" | #4 @ 32" O.C. | 4 - #4 BARS |
|-----------------|-----|---------------|-------------|
| <u><</u> 6' | 8" | #4 @ 24" O.C. | 5 - #4 BARS |
| <u><</u> 8' | 8" | #4 @ 24" O.C. | 6 - #4 BARS |
| <u><</u> 9' | 8" | #4 @ 16" O.C. | 7 - #4 BARS |
| <u><</u> 12' | 10" | #5 @ 16" O.C. | 9 - #5 BARS |

*12' FOUNDATION WALL REQUIRES F-2 FOOTING

MARK | WIDTH | LENGTH | REINFORCEMENT

PIER SCHEDULE

P-1 12" 12" W/ (8) #4 VERT. BARS & #3 RINGS @ 10" O.C. W/ (3) IN TOP 5"

MARK HOLD DOWN TYPE Allow. LOAD

A Simpson STHD10RJ Embedded Hold down 2940 lbs

LEGEND

MARK COMMENTS

WS WALL STEP

NOTE: FOOTINGS ARE REQUIRED TO BEAR DIRECTLY ON BEDROCK. CONTRACTOR SHALL STEP FOOTINGS AS FIELD CONDITIONS AND DEPTH OF BEDROCK REQUIRES.

REFER TO GEO TECH REPORT

5/8" ANCHOR BOLTS @ 24" O.C. W/ 7" MINIMUM EMBEDMENT DEPTH AND 3" X 3" X 0.229" PLATE WASHERS ON ALL ANCHOR BOLTS.
(2) ANCHOR BOLTS MIN. PER SHEAR WALL.

REFER TO ARCHITECTURAL PLANS FOR ALL DIMENSIONS

MARK

COMMENTS

1 PLACE 6 MIL VAPOR BARRIER BELOW SLAB
2 PLACE 6 MIL VAPOR BARRIER OVER SOIL, LAP ALL EDGES 6"
3 REFER TO ARCHITECTURALS FOR LOCATION
4 6" SLAB W/ #4 @ 12" O.C. E.W.
5 10" THICK POOL FLOOR - SEE SECTION.
6 POOL WALL, SEE SECTION.

18" THICK WALL - SEE SECTION. WALL WIDTH REDUCES TO 10" AT

WALL TO BE PLACED BELOW POOL SLAB - SEE SECTION

CANTILEVERED PORTION

CONSTRUCTION NOTES

FOUNDATION NOTES:

1. ALLOWABLE SOIL PRESSURE USED IN DESIGN = 5000 PSF.

2. DO NOT PLACE BACKFILL AGAINST FOUNDATION WALLS UNTIL BRACING FLOOR IS IN PLACE OR ADEQUATE SHORING IS INSTALLED.

3. ALL FOUNDATION WALLS ARE 8" THICK UNLESS NOTED OTHERWISE ON PLAN. REFER TO CONCRETE NOTES AND PLANS FOR WALL REINFORCEMENT, TYPE, AND SIZE OF ATTACH. ANCHORS REQUIRED.

CONCRETE NOTES:

1. PERFORM ALL CONCRETE WORK IN ACCORDANCE WITH ACI 301-05.

2. ALL CONCRETE SHALL BE STONE AGGREGATE AND HAVE A MIN COMPRESSIVE STRENGTH OF 3000 PSI TYPICAL, 4000 PSI AT SLABS ON GRADE, WITHIN 28 DAYS AFTER 2,500 PSI COMPRESSIVE STRENGTH (F'C) WAS ASSUMED IN THE CALCULATIONS. PLACING.

3. ALL METAL REINFORCEMENT SHALL CONFORM TO A.S.T.M. A615 AND SUPPLEMENT (S1), GRADE 60, WITH A MIN YIELD STRENGTH OF 60 KSI.

4. ALL REINFORCING BARS SHALL BE DETAILED, BOLSTERED AND SUPPORTED IN ACCORDANCE WITH ACI 315, 318, AND PUBLICATION SP-66.

66.

5. ALL REINFORCEMENT BARS SHALL BE SECURELY ANCHORED TO THE FORMS AND SPACED FROM THEM AS FOLLOWS: (A) FOR CONCRETE NOT EXPOSED DIRECTLY TO THE GROUND OR WEATHER, 3/4" IN SLABS, JOISTS AND WALLS; 1-1/2" IN PIERS, COLUMNS, BEAMS, AND GIRDERS. (B) FOR CONCRETE EXPOSED TO THE GROUND OR WEATHER, 2" IN WALLS, PIERS AND COLUMNS; 3" ABOVE BOTTOM OF FOOTINGS.

6. ALL SPLICES IN CONT REINFORCING BARS SHALL LAP 30 BAR DIA'S. ALL SUCH SPLICES SHALL BE MADE IN A REGION OF COMPRESSION UNLESS SHOWN OTHERWISE.

7. PROVIDE 1/4" MIN AMPLITUDE ROUGHENED JOINT IN TOP OF ALL.

7. PROVIDE 1/4" MIN AMPLITUDE ROUGHENED JOINT IN TOP OF ALL FOOTINGS.

8. LARGE AREAS OF SLAB ON GRADE SHALL BE PLACED IN CHECKERBOARD FASHION IN LENGTHS NOT TO EXCEED 24'-0" IN ANY DIRECTION.

9. PLACE CONTROL JOINTS IN SLABS AT 12'-0" O.C. IN EACH DIRECTION BY SAW CUTTING OR PREMOLDED STRIP, 1/4TH THE SLAB THICKNESS.

10. REINFORCE ALL CONCRETE WALLS AS SHOWN ON THE PLANS.

11. USE (2) #4 AT TOPS, BOTTOM, AND SIDES OF ALL OPENINGS

12. ALL DOWELS SHALL HAVE AT LEAST 30 BAR DIA EMBEDMENT AND/OR STANDARD HOOK AT ENDS.

13. PROVIDE STD. CORNER BARS AT ALL INTERSECTING CORNERS OF WALLS AND FOOTINGS. USE SAME SIZE AND SPACING AS HORIZONTAL REINFORCEMENT.

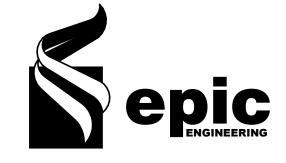
14. CONTRACTOR IS RESPONSIBLE FOR ALL FORMING AND BRACING REQUIREMENTS TO ENSURE THAT THE FORMS ARE STABLE AND PLUMB DURING CONCRETE PLACEMENT

15. ALL FOOTINGS TO BE A MIN OF 30" BELOW FINISHED GRADE

16. PROVIDE CONCRETE MIX WITH A MIN COMPRESSIVE STRENGTH OF 3,000 PSI

16. PROVIDE CONCRETE MIX WITH A MIN COMPRESSIVE STRENGTH O 3,000 PSI 17. TOP OF FOUNDATION WALL TO BE A MIN OF 6" ABOVE ADJACENT FINISH GRADE

DATE
MAY 2015



REVISIONS

MARK DATE DESCRIPTION

DRAWN: JKC
DESIGNER: PW
REVIEWED: AJH

PROJECT # **14SM2068**

SCALES 5/14/2015

indicated

BAR
FL

PROJECT NAME:

FALCONE RESIDENCE

PROJECT LOCATION:

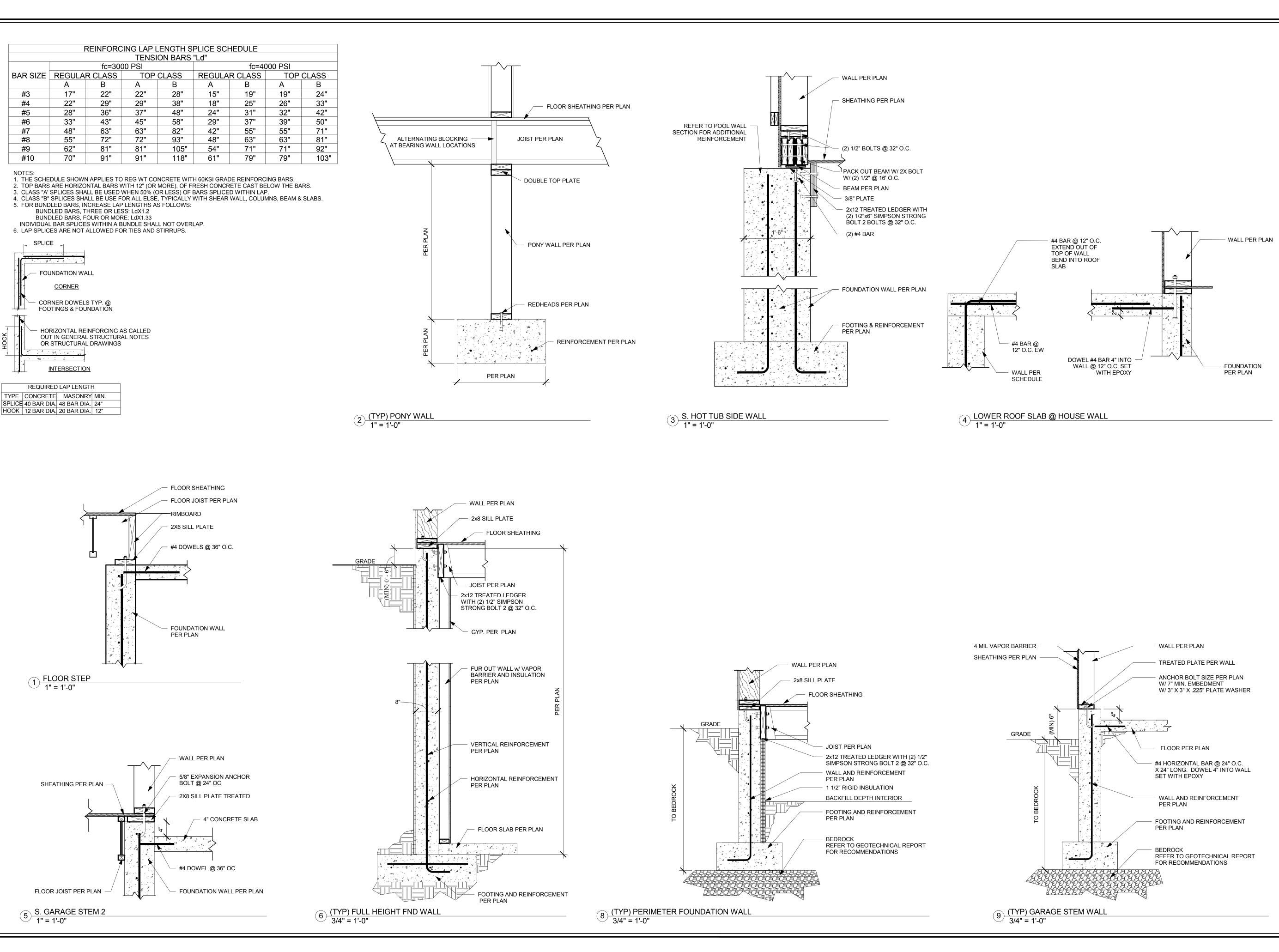
7947 EAST HEARTWOOD DRIVE WEBER COUNTY, UT

SHEET TITLE:

FOUNDATION PLAN

PERMIT SHEET

S1.1



CONSTRUCTION NOTES ALLOWABLE SOIL PRESSURE USED IN DESIGN = 5000 PSF. DO NOT PLACE BACKFILL AGAINST FOUNDATION WALLS UNTIL BRACING FLOOR IS IN PLACE OR ADEQUATE SHORING IS INSTALLED.
 ALL FOUNDATION WALLS ARE 8" THICK UNLESS NOTED OTHERWISE ON PLAN. REFER TO CONCRETE NOTES AND PLANS FOR WALL REINFORCEMEN TYPE, AND SIZE OF ATTACH. ANCHORS REQUIRED. CONCRETE NOTES:

1. PERFORM ALL CONCRETE WORK IN ACCORDANCE WITH ACI 301-84.
2. ALL CONCRETE SHALL BE STONE AGGREGATE AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI TYPICAL, 4000 PSI AT SLABS ON GRADE, WITHIN 28 DAYS AFTER 2,500 PSI COMPRESSIVE STRENGTH (F'C) WAS ASSUMED IN THE CALCULATIONS. PLACING.
3. ALL METAL REINFORCEMENT SHALL CONFORM TO A.S.T.M. A615 AND SUPPLEMENT (S1), GRADE 60, WITH A MINIMUM YIELD STRENGTH OF 60 KSI.
4. ALL REINFORCING BARS SHALL BE DETAILED, BOLSTERED AND SUPPORTED IN ACCORDANCE WITH ACI 315, 318, AND PUBLICATION SP-66.
5. ALL REINFORCEMENT BARS SHALL BE SECURELY ANCHORED TO THE FORMS AND SPACED FROM THEM AS FOLLOWS: (A) FOR CONCRETE NOT EXPOSED DIRECTLY TO THE GROUND OR WEATHER, 3/4" IN SLABS, JOISTS AND WALLS; 1-1/2" IN PIERS, COLUMNS, BEAMS, AND GIRDERS. (B) FOR CONCRETE EXPOSED TO THE GROUND OR WEATHER, 2" IN WALLS, PIERS CONCRETE NOTES: CONCRETE EXPOSED TO THE GROUND OR WEATHER, 2" IN WALLS, PIERS AND COLUMNS; 3" ABOVE BOTTOM OF FOOTINGS. 6. ALL SPLICES IN CONTINUOUS REINFORCING BARS SHALL LAP 30 BAR DIAMETERS. ALL SUCH SPLICES SHALL BE MADE IN A REGION OF COMPRESSION UNLESS SHOWN OTHERWISE.
7. PROVIDE 1/4" MINIMUM AMPLITUDE ROUGHENED JOINT IN TOP OF ALL FOUTINGS. 8. LARGE AREAS OF SLAB ON GRADE SHALL BE PLACED IN CHECKERBOARD FASHION IN LENGTHS NOT TO EXCEED 24'-0" IN ANY DIRECTION. SAW CUTTING OR PREMOLDED STRIP, 1/4TH THE SLAB THICKNESS.

10. REINFORCE ALL CONCRETE WALLS AS SHOWN ON THE PLANS. 1. USE (2) #4 AT TOPS, BOTTOM, AND SIDES OF ALL OPENINGS 11. OSE (2) #A TOPS, DOTTOM, AND SIDES OF ALL OF ENINGS
12. ALL DOWELS SHALL HAVE AT LEAST 30 BAR DIAMETER EMBEDMENT
AND/OR STANDARD HOOK AT ENDS. 13. PROVIDE STD. CORNER BARS AT ALL INTERSECTING CORNERS OF WALLS AND FOOTINGS. USE SAME SIZE AND SPACING AS HORIZONTAL REINFORCEMENT.

14. CONTRACTOR IS RESPONSIBLE FOR ALL FORMING AND BRACING REQUIREMENTS TO ENSURE THAT THE FORMS ARE STABLE AND PLUMB DURING CONCRETE PLACEMENT

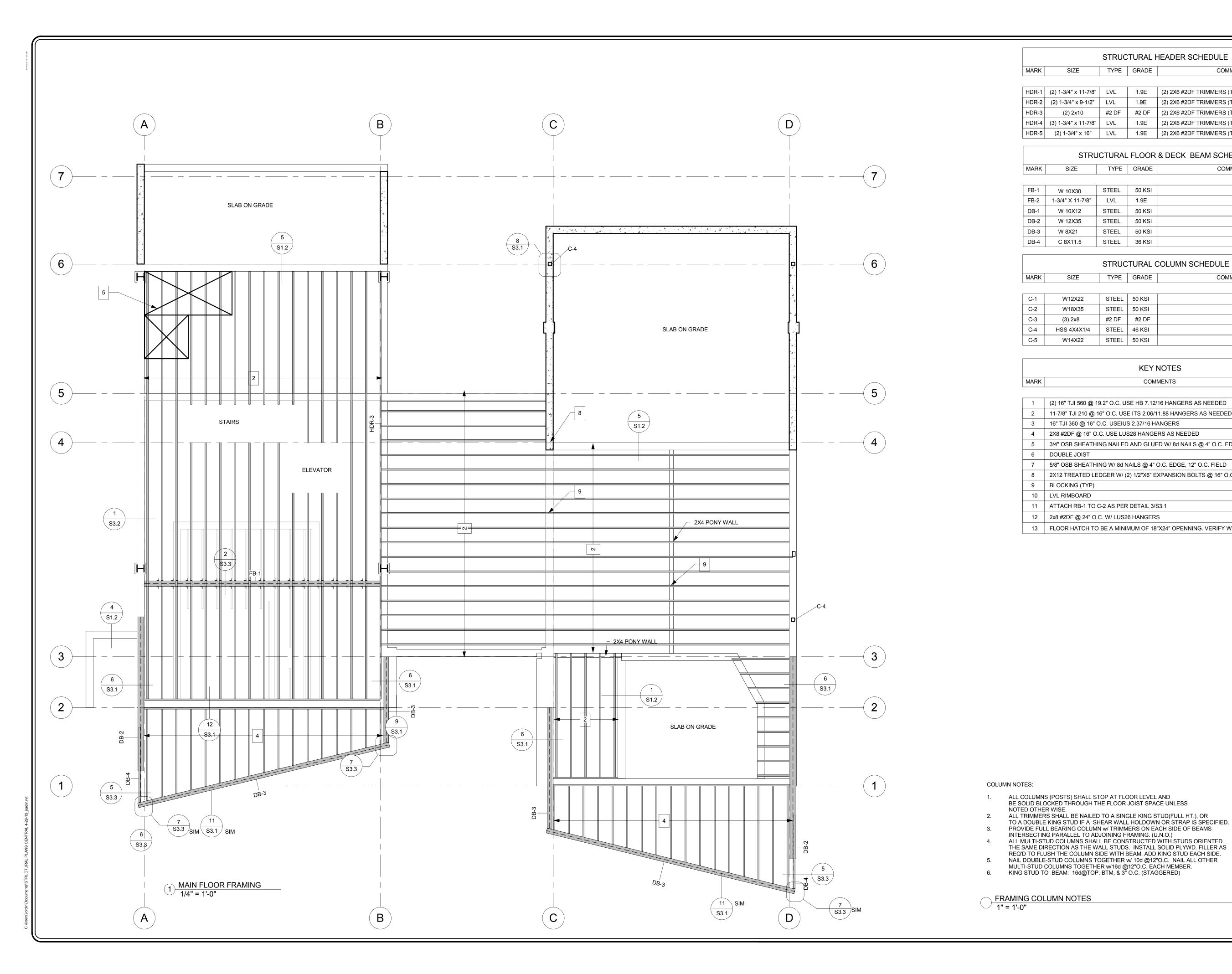
15. ALL FOOTINGS TO BE A MINIMUM OF 36" BELOW FINISHED GRADE -IRC 16. PROVIDE CONCRETE MIX WITH A MINIMUM COMPRESSIVE STRENGTH OF 16. PROVIDE CONCRETE MIX WITH A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI -IRC R402
17. TOP OF FOUNDATION WALL TO BE A MINIMUM OF 6 INCHES ABOVE ADJACENT FINISH GRADE.-IRC R404.1.6
18. CONCRETE FLOOR SLABS, EXCEPT THOSE IN UNHEATED ACCESSORY STRUCTURES, SHALL HAVE A VAPOR RETARDER CONSISTING OF A 6 MIL (.006 INCH)POLYETHYLENE BARRIER OR APPROVED VAPOR RETARDER WITH JOINTS LAPPED NOT LESS THAN 6 INCHES PLACED BETWEEN THE CONCRETE FLOOR SLAB AND THE BASE COURSE MATERIAL
19. SLAB ON GRADE FLOORS AT EXTERIOR PERIMETER FOUNDATION WALLS THAT ARE ABOVE GRADE ARE REQUIRED TO BE INSULATED. ALSO BETWEEN THE WALL AND THE END OF THE SLAB A THERMO BREAK IS REQUIRED. IECC 102.4.1, 802.2.7 DATE **MAY 2015** MARK DATE DESCRIPTION DESIGNER: PW REVIEWED: AJH ADAM J. HUFF 5/14/2015 PROJECT# 14SM2068 SCALES As indicated PROJECT NAME: FALCONE RESIDENCE PROJECT LOCATION: **7947 EAST HEARTWOOD DRIVE WEBER COUNTY, UT** SHEET TITLE: **FOUNDATION SCHEDULES**

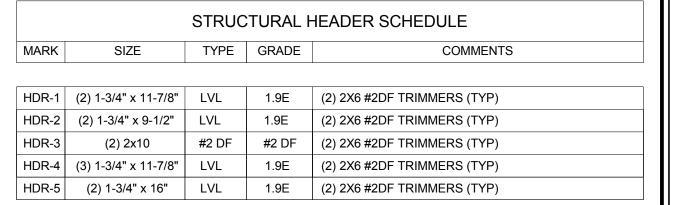
PLAN SET:

PERMIT

SHEET

S1.2





| STRUCTURAL FLOOR & DECK BEAM SCHEDULE | | | | | | | |
|---------------------------------------|------------------|-------|--------|----------|--|--|--|
| MARK | SIZE | TYPE | GRADE | COMMENTS | | | |
| | | | | | | | |
| FB-1 | W 10X30 | STEEL | 50 KSI | | | | |
| FB-2 | 1-3/4" X 11-7/8" | LVL | 1.9E | | | | |
| DB-1 | W 10X12 | STEEL | 50 KSI | | | | |
| DB-2 | W 12X35 | STEEL | 50 KSI | | | | |
| | | | | | | | |

| STRUCTURAL COLUMN SCHEDULE | | | | | | | |
|-------------------------------|-------------|-------|--------|--|--|--|--|
| MARK SIZE TYPE GRADE COMMENTS | | | | | | | |
| | | | | | | | |
| C-1 | W12X22 | STEEL | 50 KSI | | | | |
| C-2 | W18X35 | STEEL | 50 KSI | | | | |
| C-3 | (3) 2x8 | #2 DF | #2 DF | | | | |
| C-4 | HSS 4X4X1/4 | STEEL | 46 KSI | | | | |
| C-5 | W14X22 | STEEL | 50 KSI | | | | |

| | KEY NOTES | | | | | |
|------|-----------|--|--|--|--|--|
| MARK | COMMENTS | | | | | |
| | | | | | | |

| | 1 | (2) 16" TJI 560 @ 19.2" O.C. USE HB 7.12/16 HANGERS AS NEEDED |
|--|---|-----------------------------------------------------------------|
| | 2 | 11-7/8" TJI 210 @ 16" O.C. USE ITS 2.06/11.88 HANGERS AS NEEDED |
| | | |

STEEL 36 KSI

- 3 16" TJI 360 @ 16" O.C. USEIUS 2.37/16 HANGERS 4 2X8 #2DF @ 16" O.C. USE LUS28 HANGERS AS NEEDED
- 5 3/4" OSB SHEATHING NAILED AND GLUED W/ 8d NAILS @ 4" O.C. EDGE, 12" O.C. FIELD

C 8X11.5

- 7 5/8" OSB SHEATHING W/ 8d NAILS @ 4" O.C. EDGE, 12" O.C. FIELD
- 8 2X12 TREATED LEDGER W/ (2) 1/2"X6" EXPANSION BOLTS @ 16" O.C. 9 BLOCKING (TYP)
- 10 LVL RIMBOARD
- 11 ATTACH RB-1 TO C-2 AS PER DETAIL 3/S3.1
- 12 2x8 #2DF @ 24" O.C. W/ LUS26 HANGERS
- 13 FLOOR HATCH TO BE A MINIMUM OF 18"X24" OPENNING. VERIFY W/ ARCHITECT ON LOCATION

CONSTRUCTION NOTES

FRAMING NOTES:

1. PROVIDE A MINIMUM 18 INCH CLEARANCE FOR WOOD JOISTS AND 12 INCHES CLEARANCE FOR WOOD GIRDERS IN THE CRAWL.SPACE UNLESS PROPER REDWOOD OR PRESSURE TREATED WOOD IS SPECIFIED - IRC R319.1

2. ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY INCLUDING LEDGERS AND FURRING WALLS MUST BE PRESERVATIVE TREATED OR FOUNDATION GRADE REDWOOD - IRC 319

3. PROVIDE 1/2" AIRSPACE AT TOPS, SIDES AND ENDS OF GIRDERS ENTERING EXTERIOR CONCRETE OR MASONRY WALLS UNLESS WOODS RESISTANT TO DECAY ARE USED IRCR319

4. NO WOOD SHALL BE NEARER THAN 8 INCHES TO EARTH UNLESS SEPARATED BY CONCRETE AT LEAST 3 INCHES IN THICKNESS WITH AN IMPERVIOUS MEMBRANE INSTALLED BETWEEN THE EARTH AND THE CONCRETE. THIS INCLUDES DECKS AND SIDING - IRC R319

5. ACCESSIBLE BELOW-FLOOR AREAS SHALL BE PROVIDED WITH A MINIMUM 18" X 24" ACCESS OPENING. IRC R408.3. FOR ACCESS TO MECHANICAL EQUIPMENT IN THESE AREAS. SEE IRC M1305.1.4

6. PROVIDE A MINIMUM 22" 30" ATTIC ACCESS IN A HALLWAY OR OTHER READILY ACCESSIBLE LOCATION -IRC R807. SEE M1305.1.3 FOR ACCESS TO FURNACES AND OTHER MECHANICAL EQUIPMENT IN ATTICS.

7. PROVIDE A MINIMUM 02" 30" ATTIC ACCESS IN A HALLWAY OR OTHER READILY ACCESSIBLE LOCATION -IRC
R807. SEE M1305.1.3 FOR ACCESS TO FURNACES AND OTHER MECHANICAL EQUIPMENT IN ATTICS.

7. PROVIDE A MINIMUM 02" 30" ATTIC ACCESS IN A HALLWAY OR OTHER MECHANICAL EQUIPMENT IN ATTICS.

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7. PROVIDE A MINIMUM 02" 30" ATTIC ACCESS OF AND OTHER MECHANICAL EQUIPMENT IN ATTICS.

9. PROVIDE A MINIMUM OF 2 INCH THICK PLOWOOD PLANKS FOR DECK IF DECK JOISTS ARE SPACED GREATER.

10 ON CENTER. -IRC R501.2

8. FIRE BLOCK STUD SPACES AT SOFFITS, FLOOR AND CEILING JOIST LINES, AT 10 FT. VERTICALLY AND HORIZONTALLY, AND AT OPENINGS BETWEEN ATTIC SPACES AND CHIMNEY SPACES FOR FACTORY BUILT CHIMNEYS, AND AT ANY OTHER LOCATIONS NOT SPECIFICALLY MENTIONED WHICH COULD AFFORD PASSAGE FOR FLAMES - IRC R602.8

9. ALL HEADERS TYPICAL FOR ALL 2X6 WALLS ARE (2) 2 X 10

#2DF U.N.O.

10. ALL HEADERS TYPICAL FOR ALL 2X6 WALLS ARE (3)2 X 10 #2DF U.N.O.

MAY 2015



MARK DATE DESCRIPTION

REVIEWED: AJH

PROJECT#

14SM2068

5/14/2015

SCALES

PROJECT NAME:

FALCONE RESIDENCE

PROJECT LOCATION:

7947 EAST HEARTWOOD DRIVE WEBER COUNTY, UT

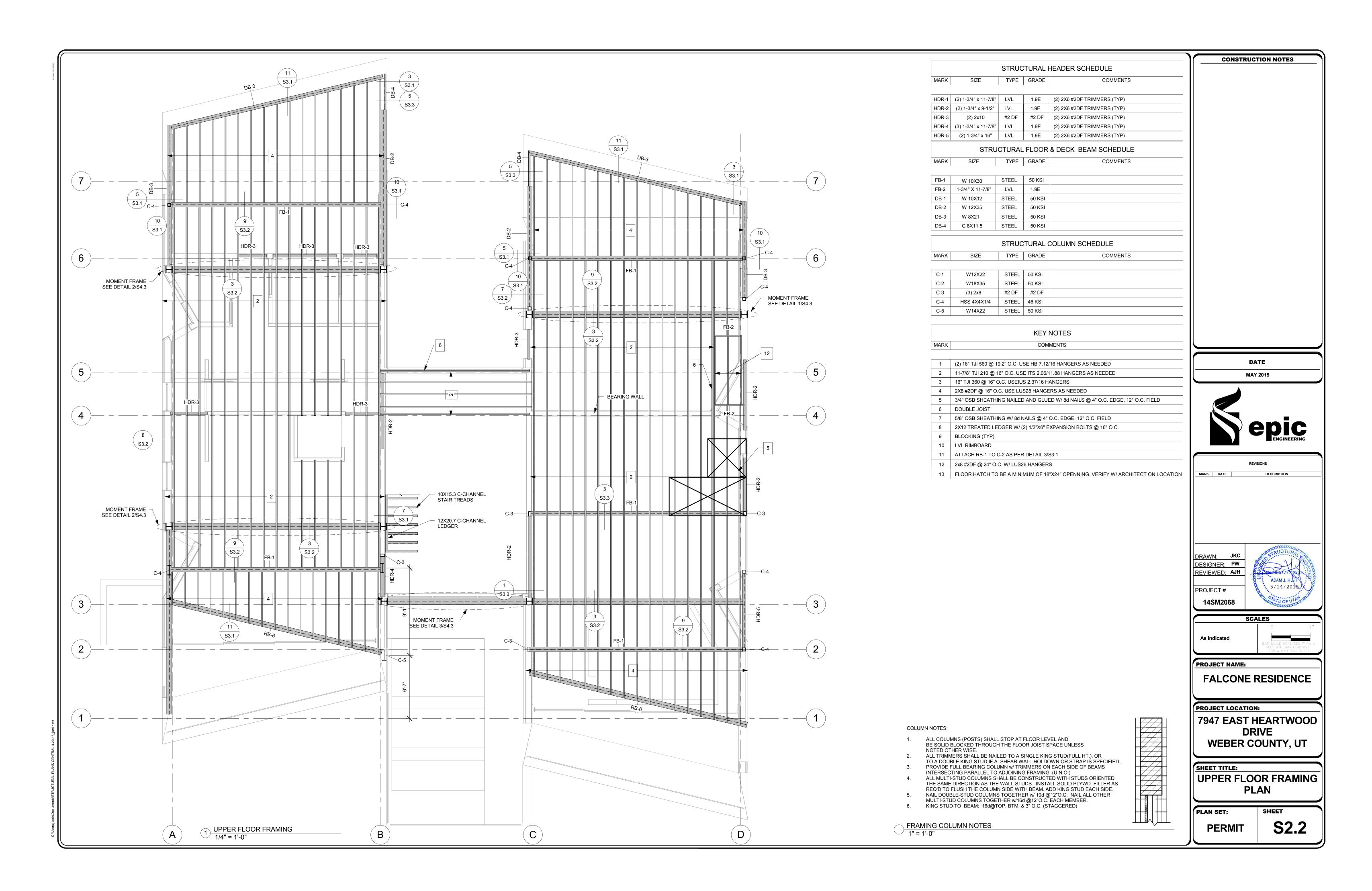
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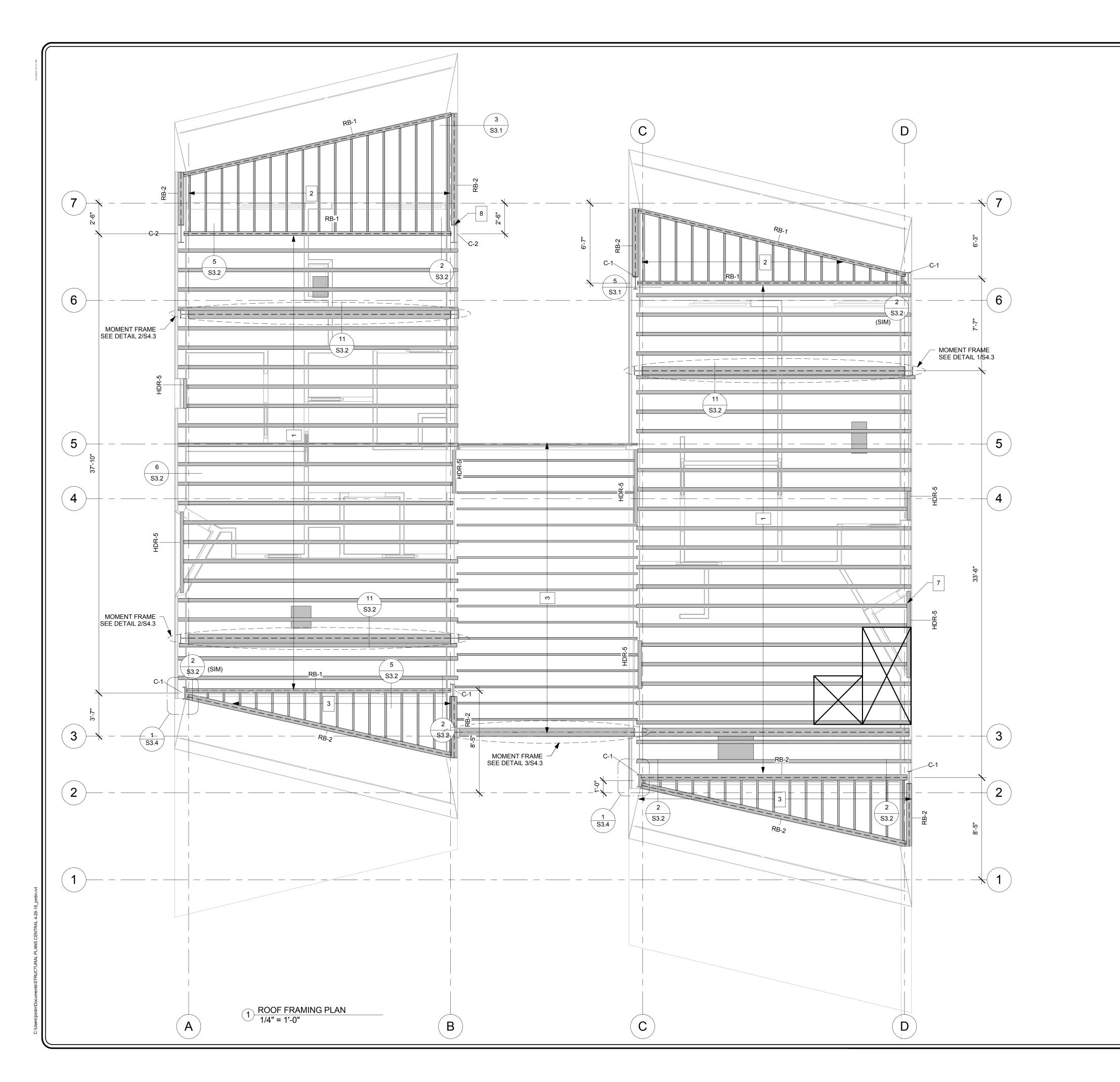
MAIN FLOOR FRAMING **PLAN**

S2.1

PLAN SET: SHEET **PERMIT**

PROVIDE FULL BEARING COLUMN w/ TRIMMERS ON EACH SIDE OF BEAMS INTERSECTING PARALLEL TO ADJOINING FRAMING. (U.N.O.)
ALL MULTI-STUD COLUMNS SHALL BE CONSTRUCTED WITH STUDS ORIENTED THE SAME DIRECTION AS THE WALL STUDS. INSTALL SOLID PLYWD. FILLER AS REQ'D TO FLUSH THE COLUMN SIDE WITH BEAM. ADD KING STUD EACH SIDE. NAIL DOUBLE-STUD COLUMNS TOGETHER W/ 10d @12"O.C. NAIL ALL OTHER MULTI-STUD COLUMNS TOGETHER W/16d @12"O.C. EACH MEMBER. KING STUD TO BEAM: 16d@TOP, BTM, & 3" O.C. (STAGGERED)





| | STRUCTURAL HEADER SCHEDULE | | | | | |
|-------------------------------|----------------------------|-------|-------|-----------------------------|--|--|
| MARK SIZE TYPE GRADE COMMENTS | | | | COMMENTS | | |
| | | | | | | |
| HDR-1 | (2) 1-3/4" x 11-7/8" | LVL | 1.9E | (2) 2X6 #2DF TRIMMERS (TYP) | | |
| HDR-2 | (2) 1-3/4" x 9-1/2" | LVL | 1.9E | (2) 2X6 #2DF TRIMMERS (TYP) | | |
| HDR-3 | (2) 2x10 | #2 DF | #2 DF | (2) 2X6 #2DF TRIMMERS (TYP) | | |
| HDR-4 | (3) 1-3/4" x 11-7/8" | LVL | 1.9E | (2) 2X6 #2DF TRIMMERS (TYP) | | |
| HDR-5 | (2) 1-3/4" x 16" | LVL | 1.9E | (2) 2X6 #2DF TRIMMERS (TYP) | | |

| STRUCTURAL ROOF BEAM SCHEDULE | | | | | | |
|-------------------------------|---------|-------|--------|----------|--|--|
| MARK | SIZE | TYPE | GRADE | COMMENTS | | |
| | | | | | | |
| RB-1 | W 12X22 | STEEL | 50 KSI | | | |
| RB-2 | W 16X26 | STEEL | 50 KSI | | | |
| RB-3 | W 14X38 | STEEL | 50 KSI | | | |

| STRUCTURAL COLUMN SCHEDULE | | | | | |
|----------------------------|-------------|-------|--------|----------|--|
| MARK SIZE TYPE GRADE | | | | COMMENTS | |
| | | | | | |
| C-1 | W12X22 | STEEL | 50 KSI | | |
| C-2 | W18X35 | STEEL | 50 KSI | | |
| C-3 | (3) 2x8 | #2 DF | #2 DF | | |
| C-4 | HSS 4X4X1/4 | STEEL | 46 KSI | | |
| C-5 | W14X22 | STEEL | 50 KSI | | |

| | KEY NOTES | |
|------|-----------|--|
| MARK | COMMENTS | |

| RB-4 | W 12X26 | STEEL | 50 KSI

12 2x8 #2DF @ 24" O.C. W/ LUS26 HANGERS

RB-5 (2) 1-3/4" x 11-7/8" LVL 1.9E

| 1 | (2) 16" TJI 560 @ 19.2" O.C. USE HB 7.12/16 HANGERS AS NEEDED |
|----|--------------------------------------------------------------------------------|
| 2 | 11-7/8" TJI 210 @ 16" O.C. USE ITS 2.06/11.88 HANGERS AS NEEDED |
| 3 | 16" TJI 360 @ 16" O.C. USEIUS 2.37/16 HANGERS |
| 4 | 2X8 #2DF @ 16" O.C. USE LUS28 HANGERS AS NEEDED |
| 5 | 3/4" OSB SHEATHING NAILED AND GLUED W/ 8d NAILS @ 4" O.C. EDGE, 12" O.C. FIELD |
| 6 | DOUBLE JOIST |
| 7 | 5/8" OSB SHEATHING W/ 8d NAILS @ 4" O.C. EDGE, 12" O.C. FIELD |
| 8 | 2X12 TREATED LEDGER W/ (2) 1/2"X6" EXPANSION BOLTS @ 16" O.C. |
| 9 | BLOCKING (TYP) |
| 10 | LVL RIMBOARD |
| 11 | ATTACH RB-1 TO C-2 AS PER DETAIL 3/S3.1 |

13 FLOOR HATCH TO BE A MINIMUM OF 18"X24" OPENNING. VERIFY W/ ARCHITECT ON LOCATION

CONSTRUCTION NOTES

1. PROVIDE A MINIMUM 18 INCH CLEARANCE FOR WOOD JOISTS AND 12 INCHES CLEARANCE FOR WOOD GIRDERS IN THE CRAWLSPACE UNLESS PROPER REDWOOD OR PRESSURE TREATED WOOD IS SPECIFIED - IRC R319.1

2. ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY INCLUDING LEDGERS AND FURRING WALLS MUST BE PRESERVATIVE TREATED OR FOUNDATION GRADE REDWOOD - IRC 319

3. PROVIDE 1/2" AIRSPACE AT TOPS, SIDES AND ENDS OF GIRDERS ENTERING EXTERIOR CONCRETE OR MASONRY WALLS UNLESS WOODS RESISTANT TO DECAY ARE USED IRCR319

4. NO WOOD SHALL BE NEARER THAN 8 INCHES TO

IMPERVIOUS MEMBRANE INSTALLED BETWEEN
THE EARTH AND THE CONCRETE. THIS INCLUDES
DECKS AND SIDING - IRC R319
5. ACCESSIBLE BELOW-FLOOR AREAS SHALL BE
PROVIDED WITH A MINIMUM 18" X 24" ACCESS
OPENING. IRC R408.3. FOR ACCESS TO
MECHANICAL EQUIPMENT IN THESE AREAS. SEE
IRC M1305.1.4

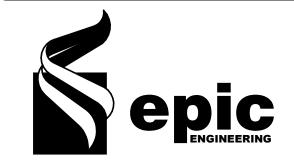
EARTH UNLESS SEPARATED BY CONCRETE AT LEAST 3 INCHES IN THICKNESS WITH AN

6. PROVIDE A MINIMUM 22" 30" ATTIC ACCESS IN A HALLWAY OR OTHER READILY ACCESSIBLE LOCATION -IRC R807. SEE M1305.1.3 FOR ACCESS TO FURNACES

AND OTHER MECHANICAL EQUIPMENT IN ATTICS.
7. PROVIDE A MINIMUM OF 2 INCH THICK
REDWOOD PLANKS FOR DECK IF DECK JOIST
SPACING IS 16" ON CENTER OR GREATER.
NOMINAL 1" THICK PLANKING SHALL NOT BE USED
WHERE DECK JOISTS ARE SPACED GREATER
THAN 12" ON CENTER. -IRC R501.2
8. FIRE BLOCK STUD SPACES AT SOFFITS, FLOOR
AND CEILING JOIST LINES, AT 10 FT. VERTICALLY
AND HORIZONTALLY, AND AT OPENINGS BETWEEN
ATTIC SPACES AND CHIMNEY SPACES FOR
FACTORY BUILT CHIMNEYS, AND AT ANY OTHER
LOCATIONS NOT SPECIFICALLY MENTIONED
WHICH COULD AFFORD PASSAGE FOR FLAMES -

IRC R602.8
9. ALL HEADERS TYPICAL FOR ALL 2X4 WALLS
ARE (2) 2 X 10 #2DF U.N.O.
10. ALL HEADERS TYPICAL FOR ALL 2X6 WALLS
ARE (3)2 X 10 #2DF U.N.O.

MAY 2015



REVISIONS

MARK DATE DESCRIPTION

DRAWN: JKC
DESIGNER: PW
REVIEWED: AJH

PROJECT # **14SM2068**

ADAM J. HUFF 5/14/2015

1/4" = 1'-0"

PROJECT NAME:

FALCONE RESIDENCE

SCALES

PROJECT LOCATION:

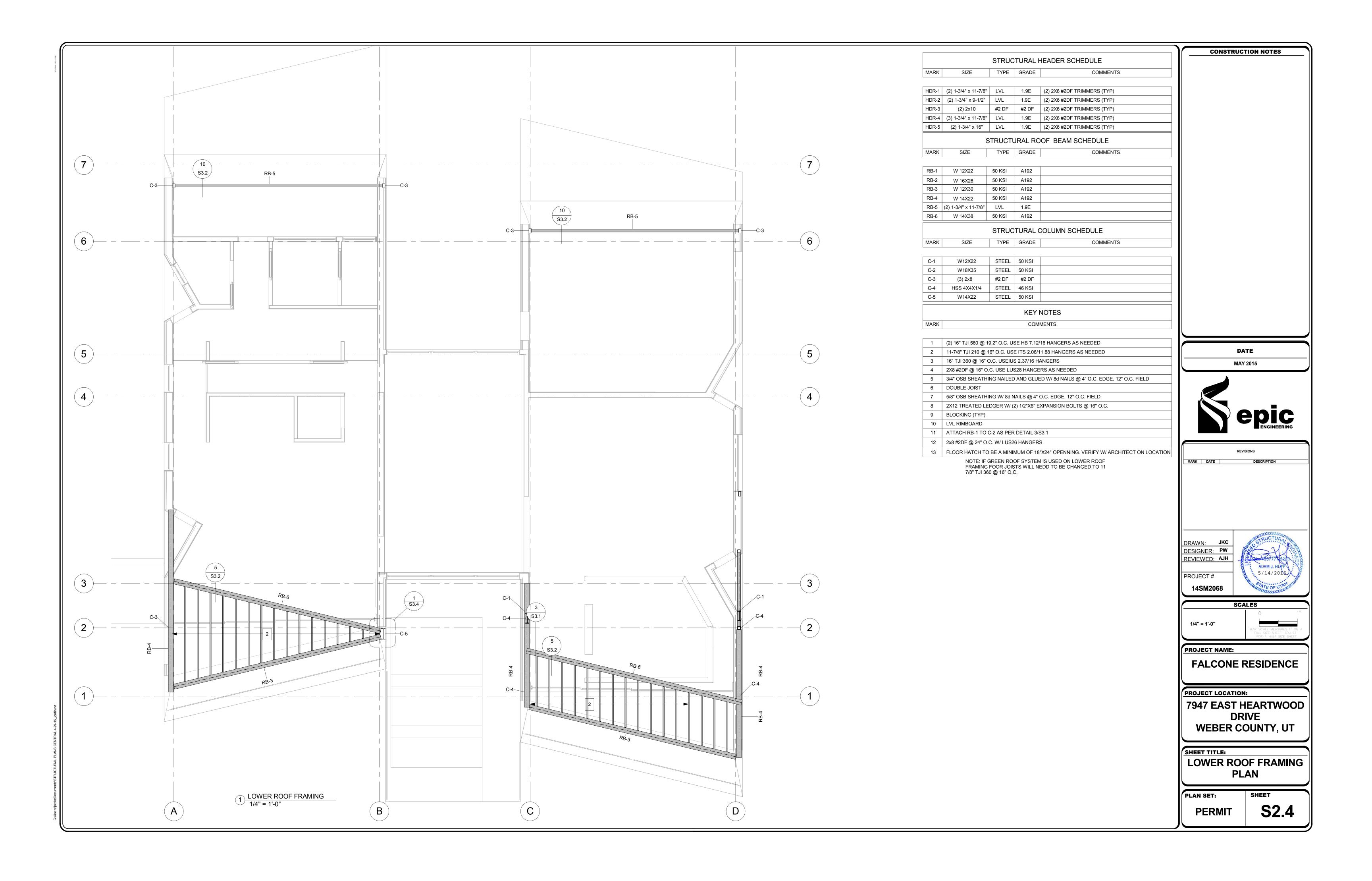
7947 EAST HEARTWOOD DRIVE WEBER COUNTY, UT

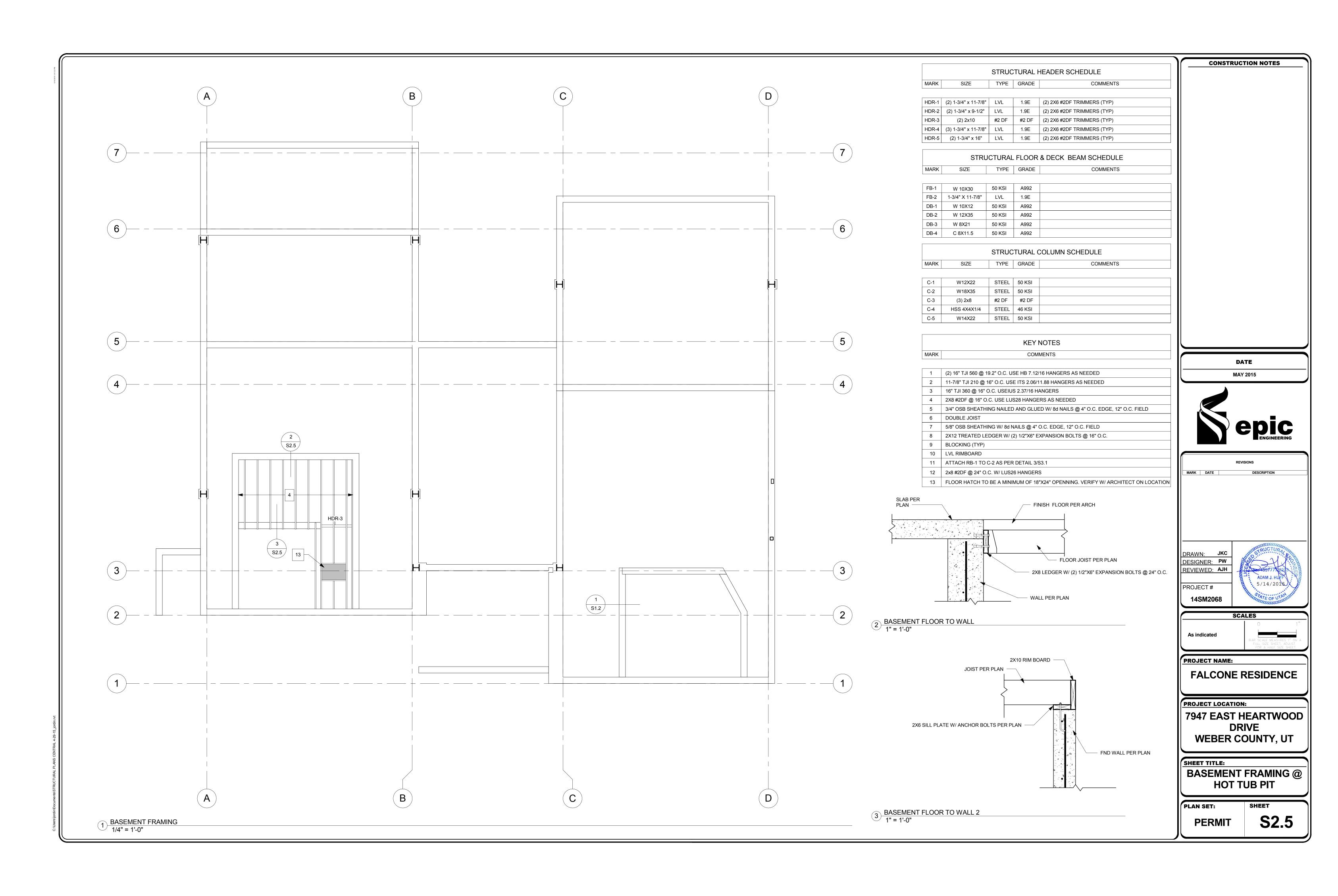
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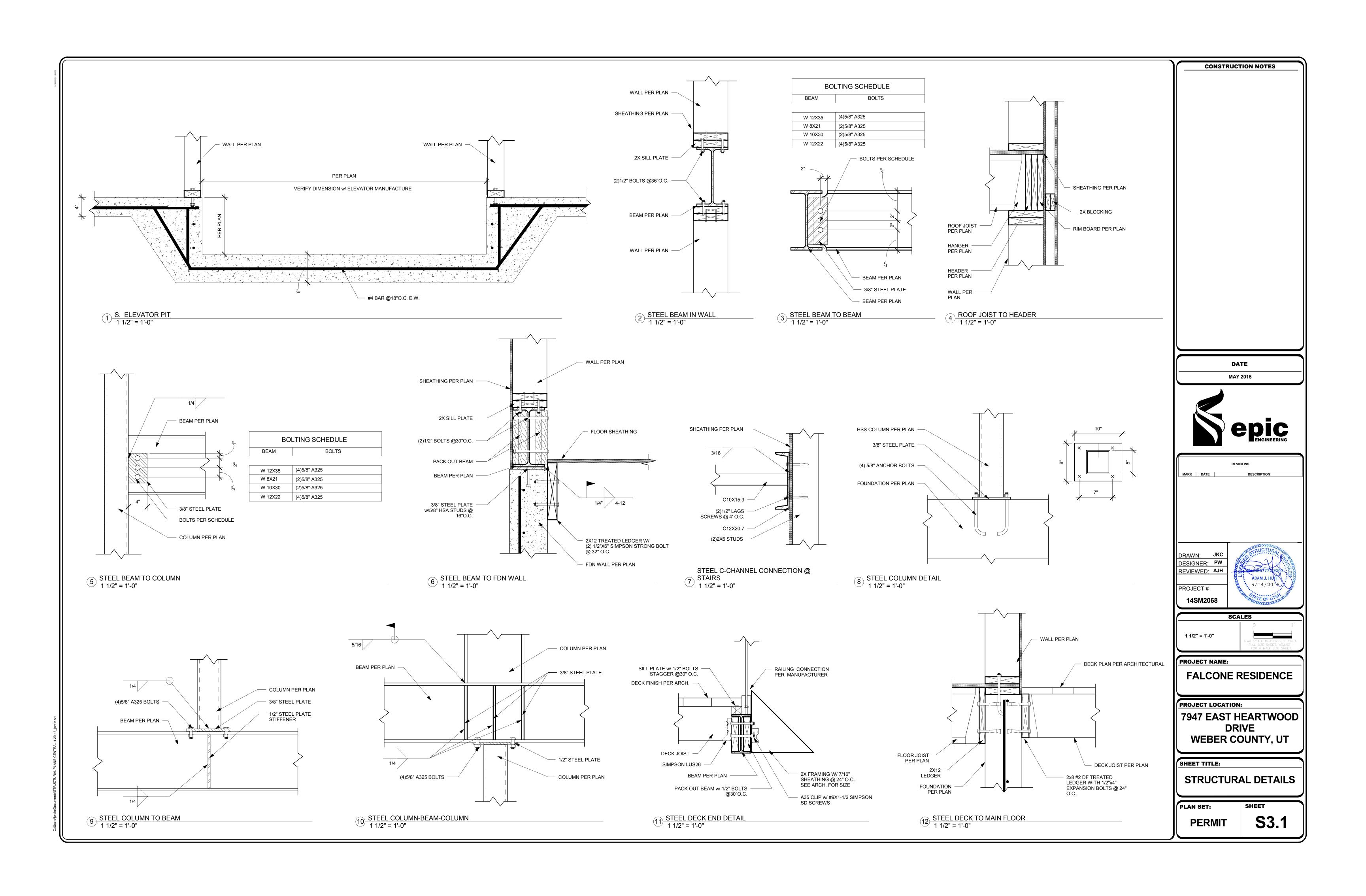
ROOF FRAMING PLAN

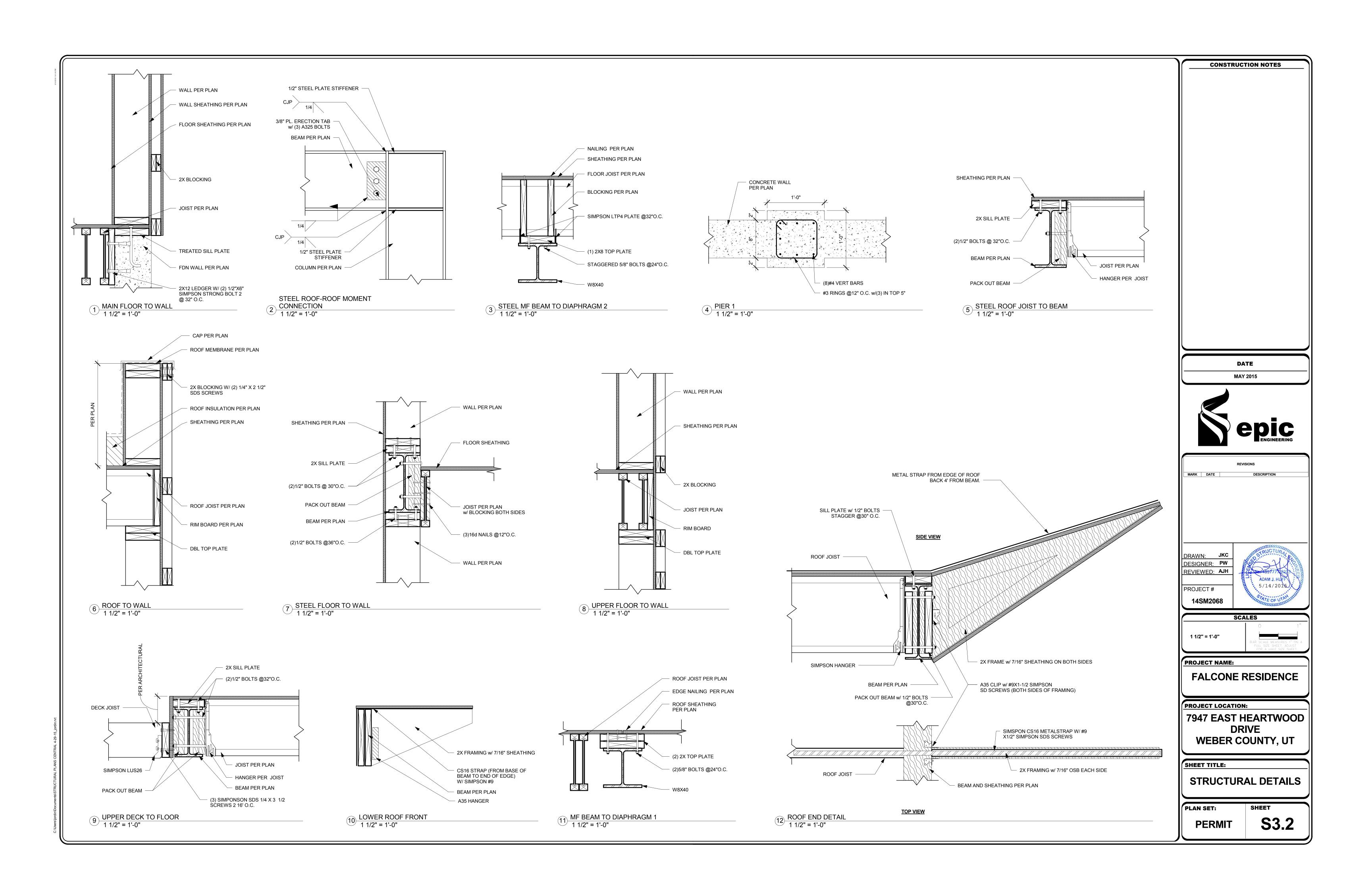
PLAN SET: SHEET

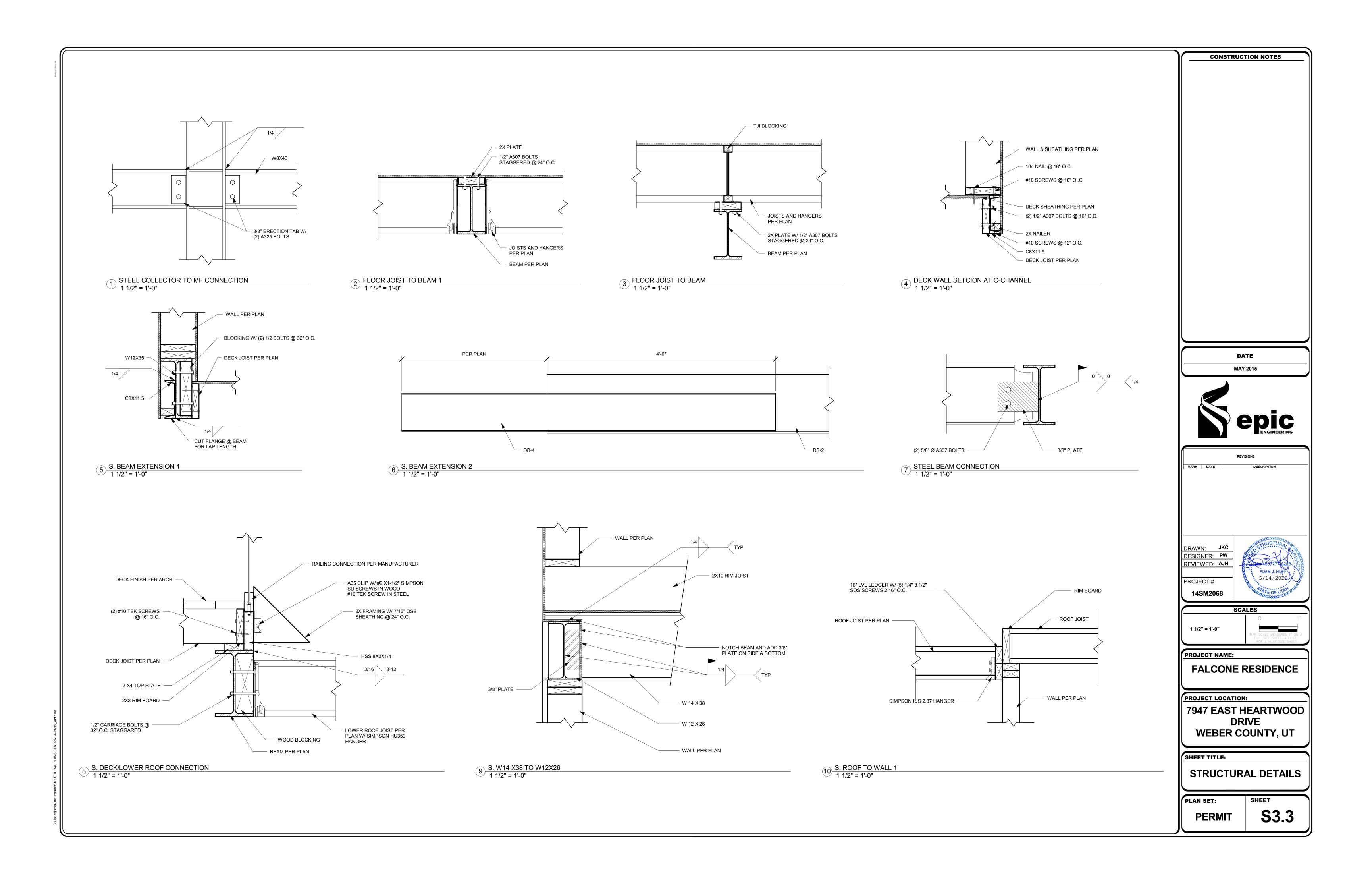
PERMIT \$2.3

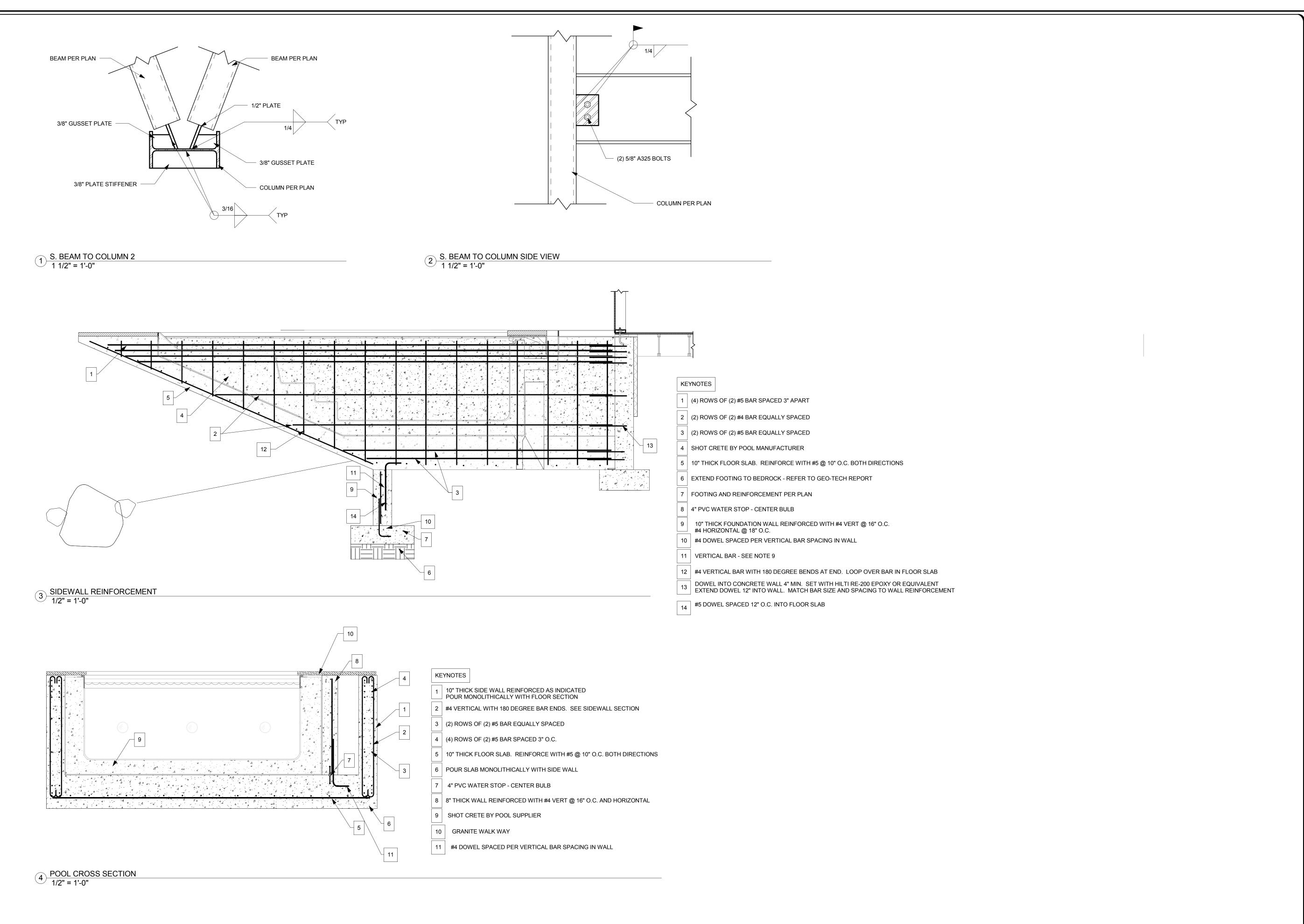




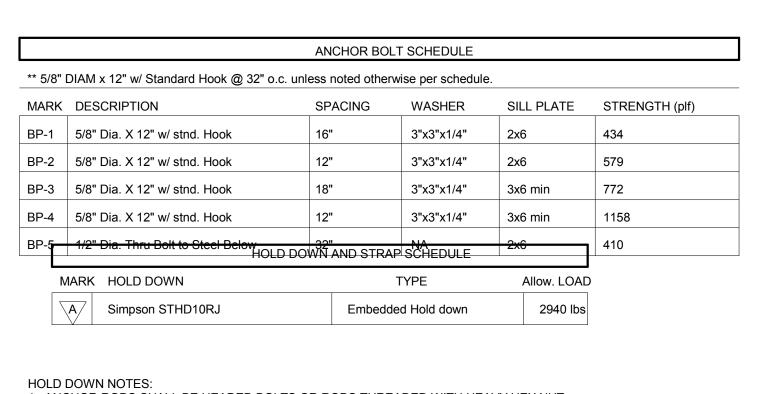




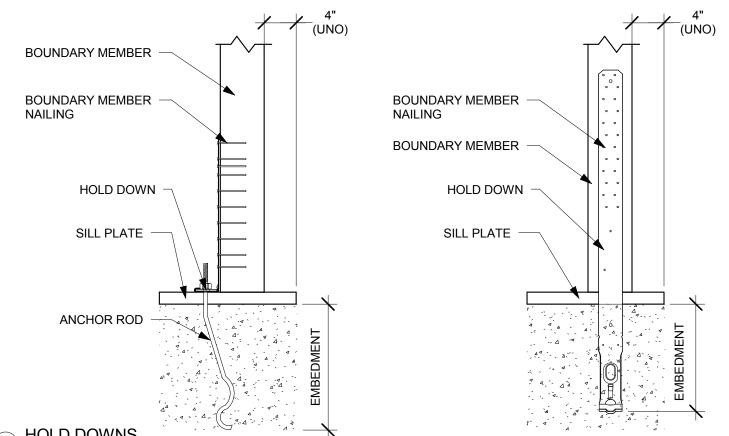


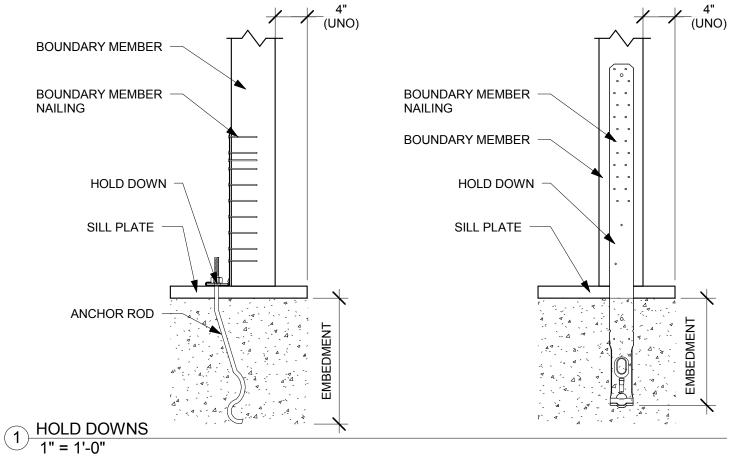


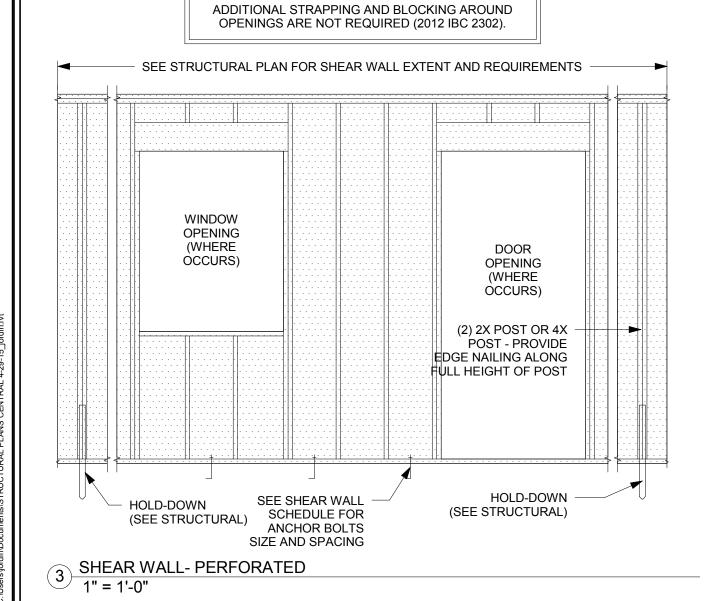
CONSTRUCTION NOTES MAY 2015 DESCRIPTION MARK DATE DESIGNER: PW REVIEWED: AJH 5/14/2015 PROJECT# 14SM2068 SCALES As indicated PROJECT NAME: FALCONE RESIDENCE PROJECT LOCATION: 7947 EAST HEARTWOOD **DRIVE** WEBER COUNTY, UT SHEET TITLE: STRUCTURAL DETAILS PLAN SET: SHEET **S3.4 PERMIT**

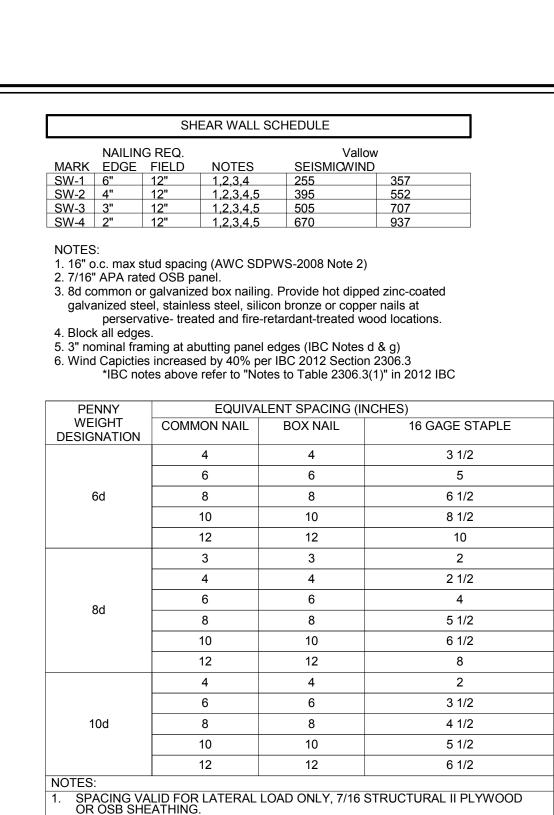


- 1. ANCHOR RODS SHALL BE HEADED BOLTS OR RODS THREADED WITH HEAVY HEX NUT.
- 2. INCREASE FOOTING DEPTH WHERE EMBEDMENT LENGTH PLUS 3" IS GREATER THAN FOOTING DEPTH SPECIFIED. 3. ALL HOLD DOWNS SPECIFIED ARE "SIMPSON-STRONG TIE", SEE GENERAL STRUCTURAL NOTES FOR SUBSTITUTIONS
- 4. LAG SCREWS SHALL NOT BE USED. 5. DO NOT OVER TORQUE NUTS; SEE MANUFACTURES TORQUE REQUIREMENTS.
- 6 SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.









WOOD SHEATHING SHEAR WALL NOTES: 1. PROVIDE 1/4" X 3" X 0'-3" WASHER PLATES AT BOLTS. CONTRACTOR HAS OPTION TO PROVIDE A DIAGONAL SLOTTED HOLE WITH A WIDTH OF T UP TO 3/16" LARGER THAN THE BOLT DIAMETER AND A SLOT LENGTH OF UP TO 1.3/4". PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND THE NUT. 2. USE COMMON NAILS. AT SILL PLATE USE HOT DIPPED OR TUMBLED GALVANIZED. 3. ANCHOR BOLTS SHALL HAVE A 7" MINIMUM EMBEDMENT INTO CONCRETE AND TERMINATE WITH A 3" STANDARD 90 DEG. HOOK AND BE HOT-DIPPED GALVANIZED STAINLESS STEEL IN ACCORDANCE WITH IBC 2304.95 4. WHERE STUDS ARE CUT FOR PLACEMENT OF ANCHOR BOLTS OR OTHER ELEMENTS, AN ADJACENT STUD SHALL BE ADDED. 5. WHERE WOOD SHEATHING IS APPLIED TO BOTH SIDES OF A WALL AND NAIL SPACING IS LESS THAN 6" O.C. ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, OR FRAMING MEMBER SHALL BE 3" OR THICKER AND NAILS ON EITHER SIDE SHALL BE STAGGERED. 6. PRE-DRILLED HOLE ARE REQUIRED AT 20d NAILS. 7. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

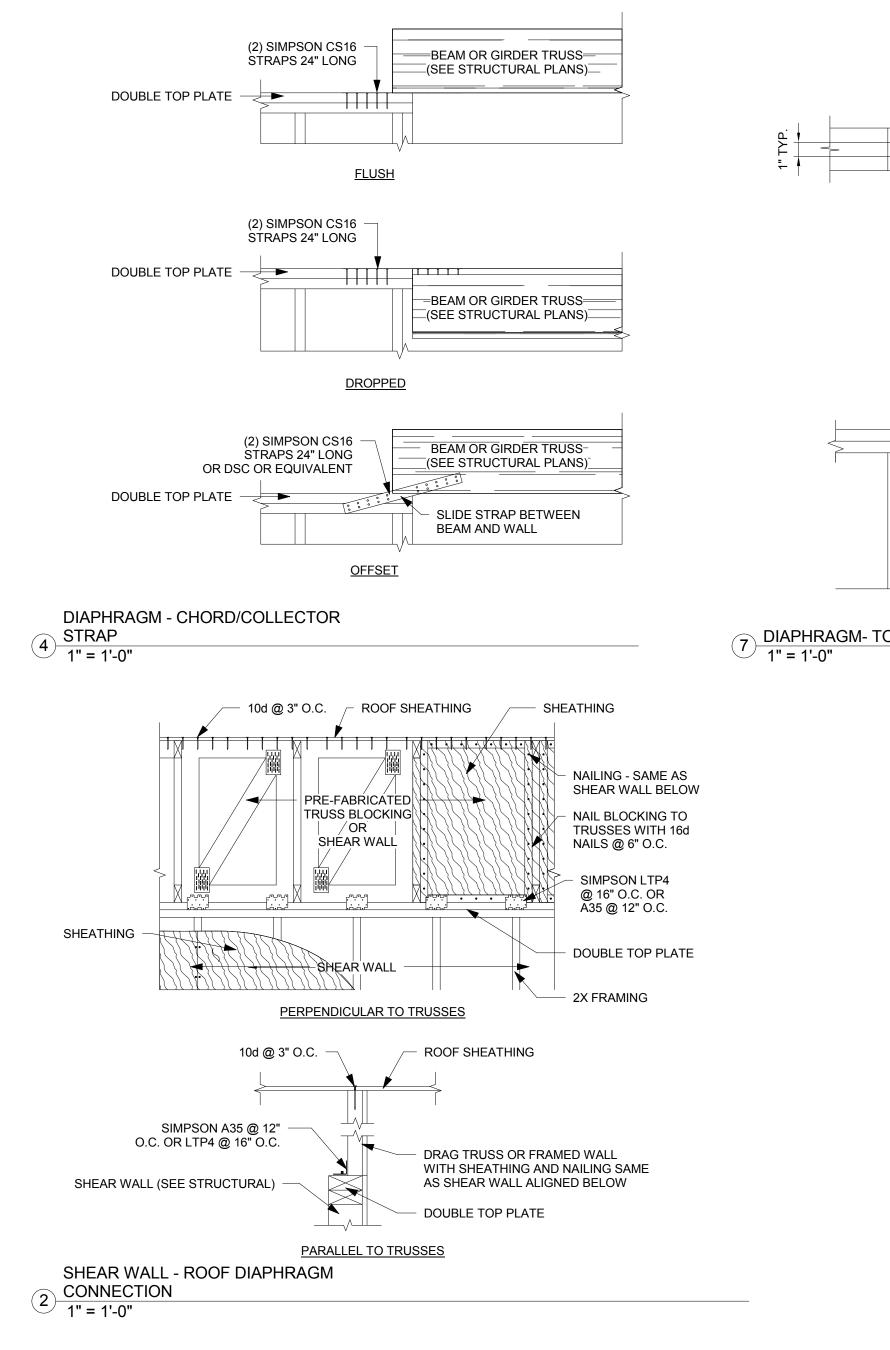
2. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16 INCH.

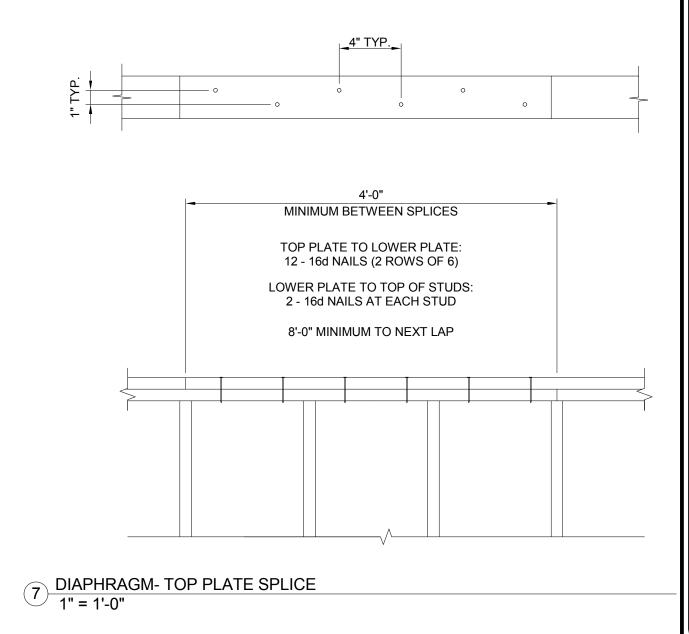
5. LOS ANGLES DEPARTMENT OF BUILDING AND SAFETY RESEARCH REPORT NO. 23633, TABLE 1.

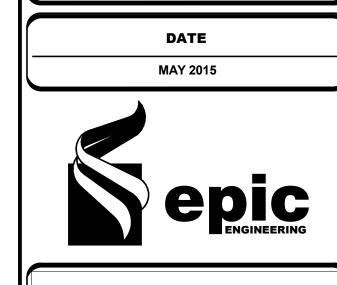
6. FASTENERS IN PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOOD SHALL BE OF HOT DIPPED ZINC-COATED GALVANIZED STEEL, SILICON BRONZE OR COPPER (2012 IBC 2304.9.5).

3. INTERNATIONAL BUILDING CODE (IBC) TABLE 2306.4.1.

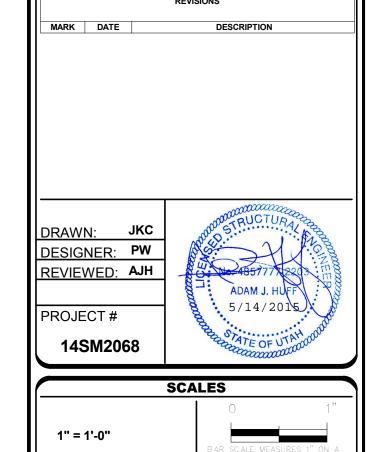
4. ICC EVALUATION SERVICE REPORT NO. 1539, TABLE 14.







CONSTRUCTION NOTES



| EAL CONE D | |
|--------------|---------------------------------------------------|
| ROJECT NAME: | |
| | |
| | FULL SIZE SHEET. ADJUST FOR A HALF SIZE SHEET. |

FALCONE RESIDENCE

PROJECT LOCATION: **7947 EAST HEARTWOOD DRIVE WEBER COUNTY, UT**

SHEET TITLE: **SCHEDULES**

| PLAN SEI: | SHEET |
|-----------|-------------|
| PERMIT | S4.1 |

