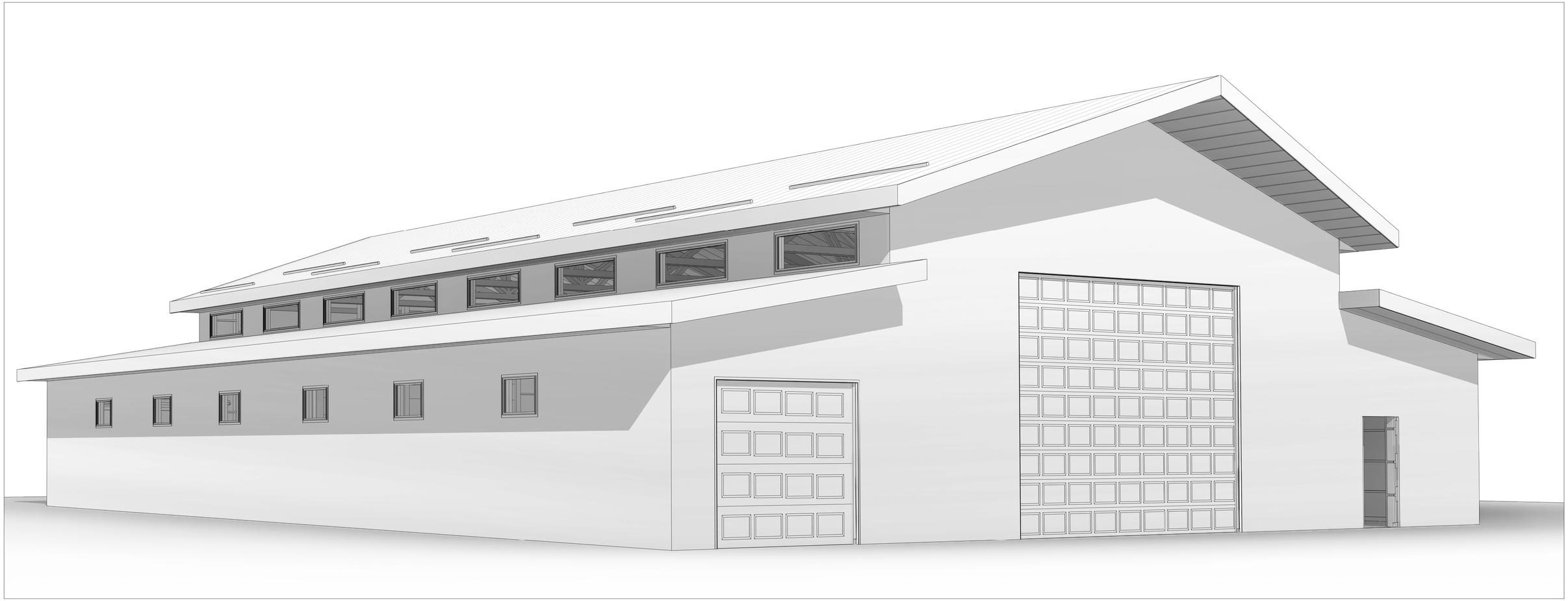
ROPER BUILDINGS

STEPHANIE & PAUL JEPPSEN BUILDING



BUILDING INFORMATION		
BUILDING INFORMATION: DIMENSIONS: 36' x 80' = 2,880 Sq. Ft. LEAN DIMENSIONS: (2)14' x 80' = 2,240 Sq. Ft. TOTAL SQUARE FOOTAGE: 5,120 Sq. Ft.	**SITE INFORMATION:** ADDRESS: 7704 East 500 North Huntsville, UT 84317	**USAGE:** Residential Accessory

ROPER BUILDINGS /
DMLP RESOURCES
PHONE: (801) 689-3630

PROFESSIONAL ENGINEER
JACKSON D SAGERS
13258211
07/29/2025
STATE OF UTAH

DRAWING INDEX	
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A1	Foundation Plan
A2	Floor Plan
A3	Framing Plan
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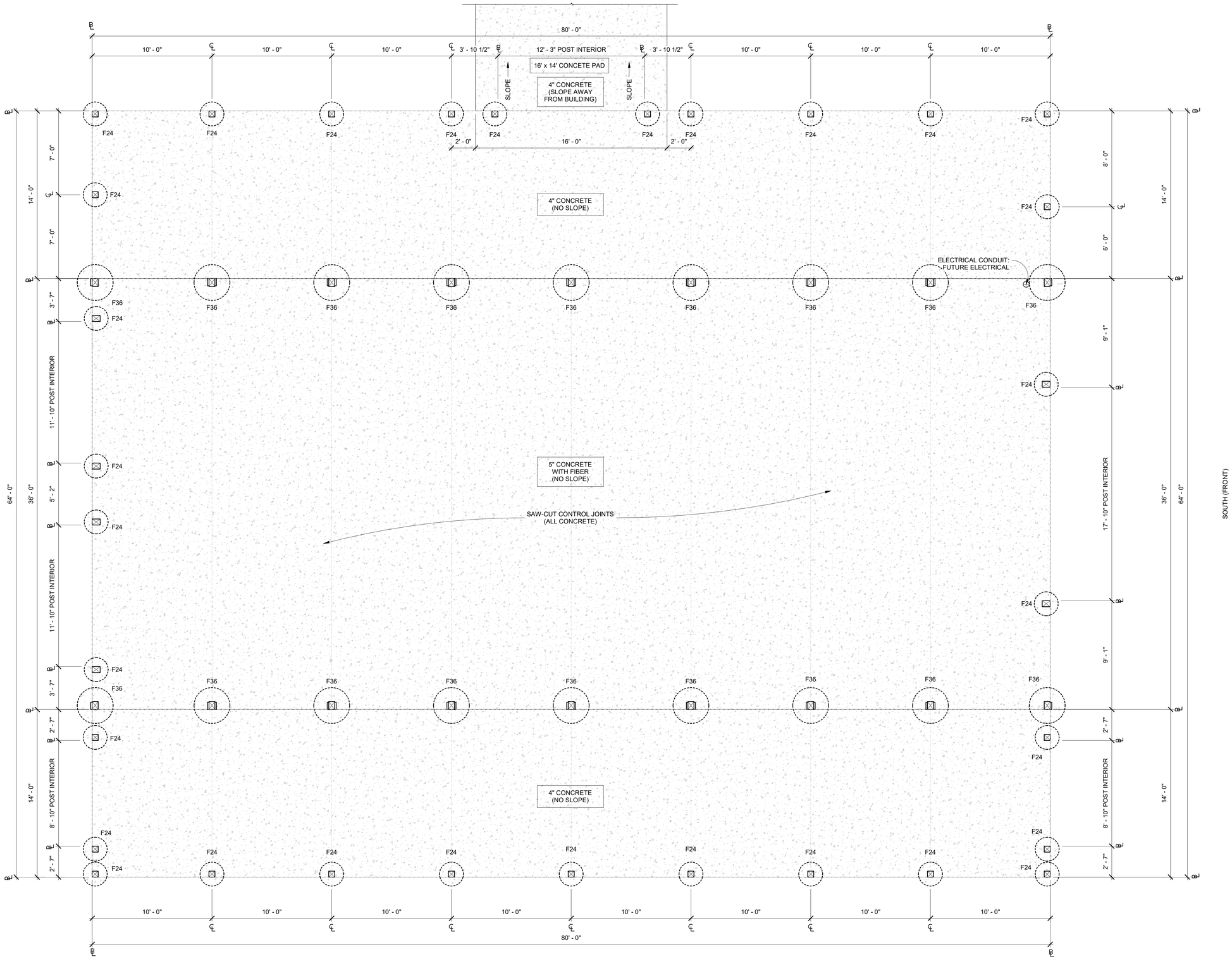
A8	Structural Details
A9	Structural Details
A10	Engineering Notes
A11	Engineering Notes

DATE:
7/29/2025

PIERS			
Count	Type	Diameter	Depth
31	F24	2' - 0"	3' - 4"
18	F36	3' - 0"	3' - 4"



FOUNDATION PLAN
1/4" = 1'-0"

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STEPHANIE & PAUL JEPPSEN BUILDING

7704 East 500 North
Huntsville, UT 84317

Date: 7/29/2025
Scale: 1/4" = 1'-0"
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Foundation Plan

A1

PROFESSIONAL ENGINEER
JACKSON D. SAGERS
132582111
07/29/2025
STATE OF UTAH

EAST

80' - 0"

10' - 0" | 10' - 0" | 10' - 0" | 3' - 10 1/2" | 12' - 3" POST INTERIOR | 3' - 10 1/2" | 10' - 0" | 10' - 0" | 10' - 0"

1' - 10 1/2" | 6' - 0" | 2' - 1 1/2"

144" x 80" MD FRAME-OUT (CLIENT TO SUPPLY DOOR) ROUGH OPENING SIZE IS 80.5" x 144.5" REFER TO MANUFACTURERS INSTALLATION SPECIFICS FOR ROUGH OPENING SIZE.

NOTE: TYPE B WINDOWS IN WALL ABOVE LEANS (TYP.)

14' x 80' ENCLOSED LEAN WITH OPEN COMMON WALL

3068 M.D. FRAME-OUT (CLIENT TO SUPPLY DOOR)

2x8 TRUSS SUPPORT TRIMMER UNDER EVERY TRUSS ALONG BEARING POSTS

102' - 5 3/8"

4 — 12' x 14'

3 — 18' x 14'

2 — 9' x 8'

4 — 12' x 14'

NOTE: TYPE B WINDOWS IN WALL ABOVE LEANS (TYP.)

2x8 TRUSS SUPPORT TRIMMER UNDER EVERY TRUSS ALONG BEARING POSTS

14' x 80' ENCLOSED LEAN WITH OPEN COMMON WALL

NORTH

SOUTH (FRONT)

WEST

FLOOR PLAN
1/4" = 1'-0"

DOORS					
Mark	Count	Description	Width	Height	Comments
2	2	INSULATED STEEL BACK OHD W/ OPENER	9' - 0"	8' - 0"	SHORT PANEL
3	1	INSULATED STEEL BACK OHD W/ OPENER	18' - 0"	14' - 0"	SHORT PANEL
4	2	INSULATED STEEL BACK OHD W/ OPENER	12' - 0"	14' - 0"	SHORT PANEL

WINDOWS					
Mark	Count	Type	Width	Height	Sill
A	6	PICTURE	3' - 0"	2' - 0"	6' - 6"
B	16	PICTURE	6' - 0"	2' - 0"	14' - 3"
D	2	SLIDER	6' - 0"	4' - 0"	3' - 4"

INSULATION		
ROOF	WALLS	ROLLS
SPRAY FOAM	R19	-
SPRAY FOAM IS 3" CLOSED CELL		

NOTE:
1. ROOF INSULATION IS IN MAIN & LEAN ROOFS (NOT PORCH)
2. WALL INSULATION IS IN ALL ENCLOSED AREAS.

POSTS			
COUNT	SAWN	LAMINATED TRIAD OPTION	TITAN TIMBER OPTION
25	6x6 HF #1	4-PLY 2x6	3-PLY 2x6
24	6x8 HF #1	4-PLY 2x8	3-PLY 2x8

NOTES:
1. EMBED POST INTO CONCRETE PIER. SEE DETAIL SHEET.
2. **WALLS:** COMMERCIAL GIRTS, 7/16" OSB WITH MOISTURE BARRIER. CLIENT TO INSTALL HARDI BOARD. COMMON WALLS ARE OPEN.
3. ALL POSTS ARE TREATED HF #1 UNLESS OTHERWISE SPECIFIED.

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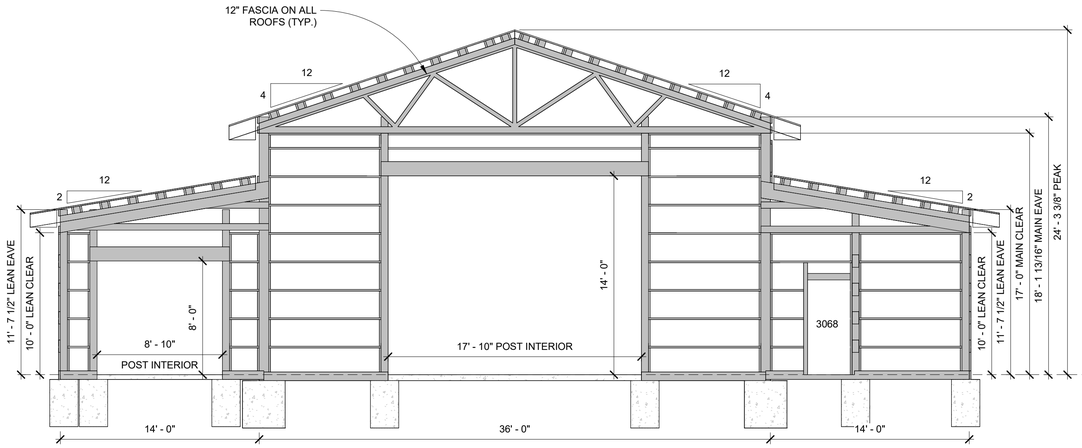
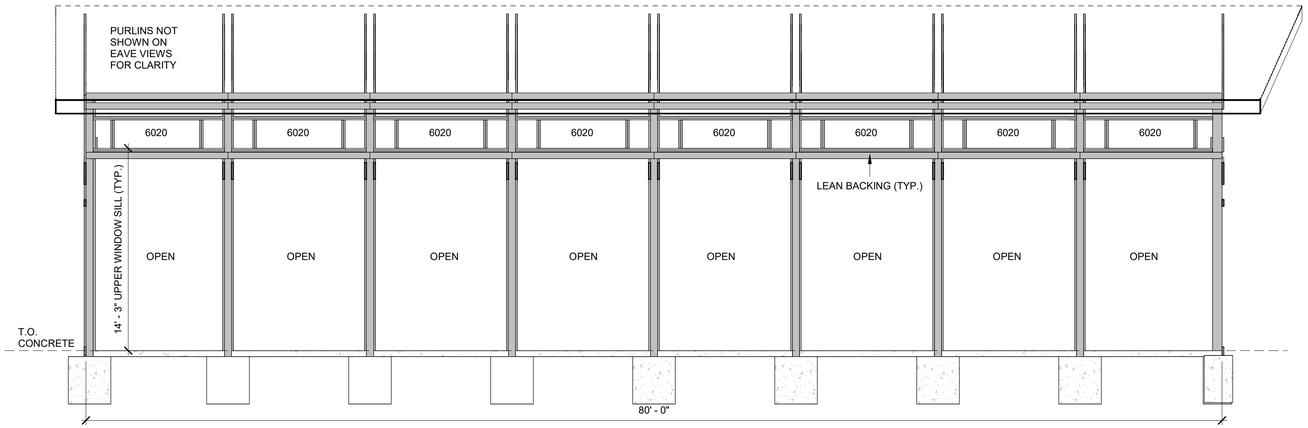
Floor Plan

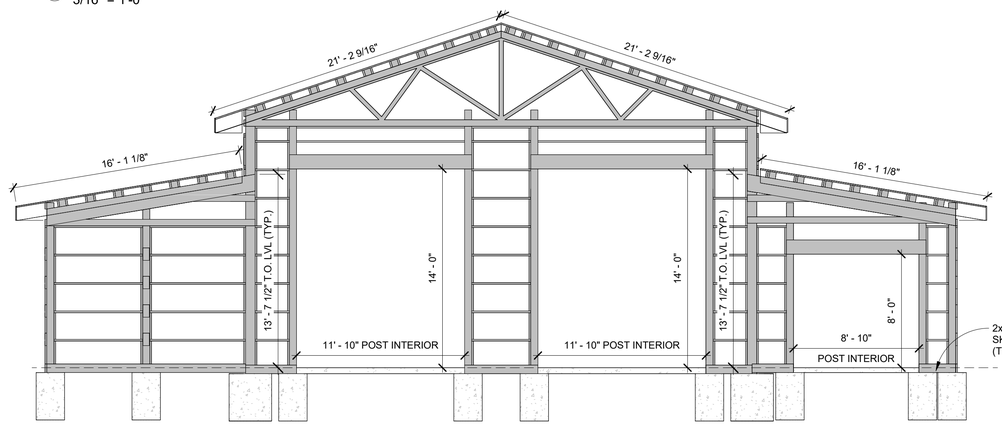
A2



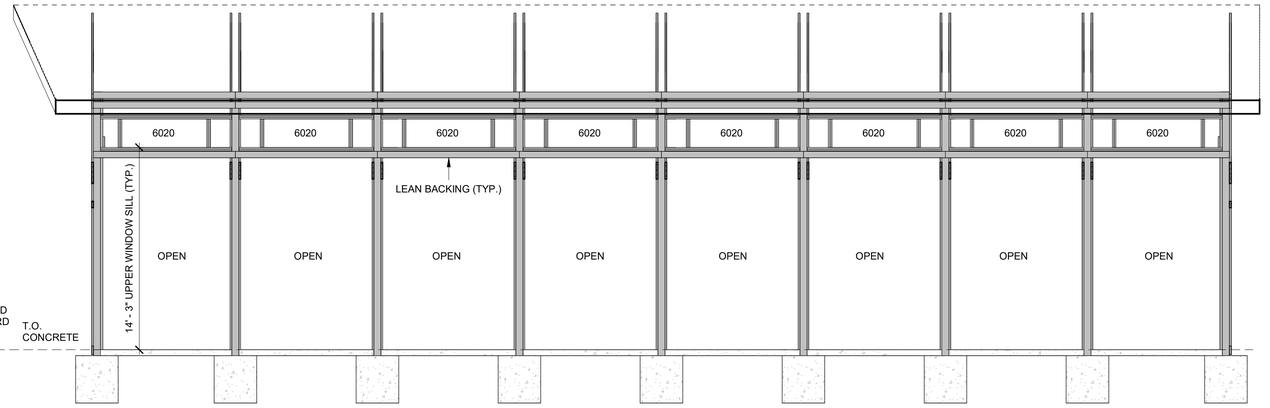
1 SOUTH GIRT
3/16" = 1'-0"



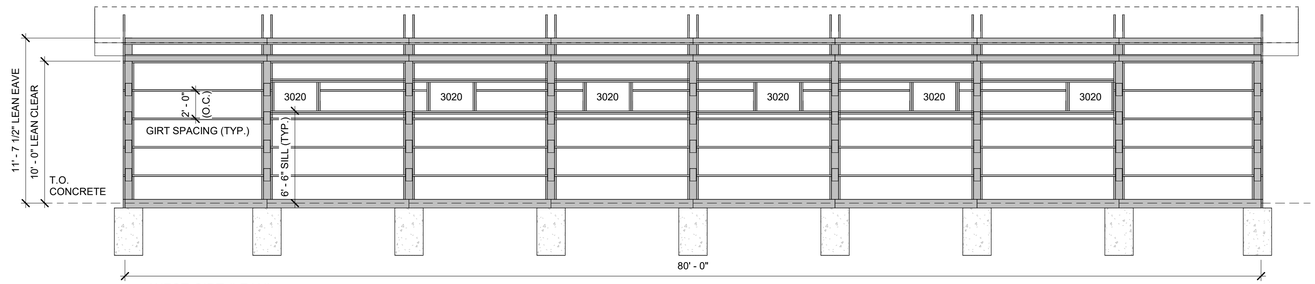
2 WEST GIRT
3/16" = 1'-0"



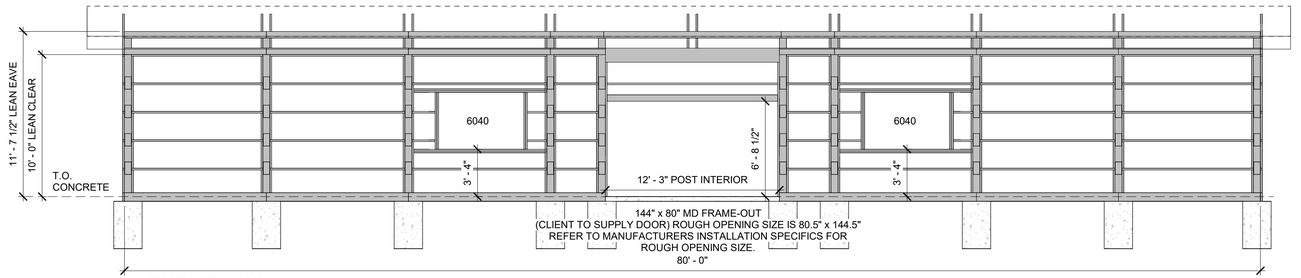
3 NORTH GIRT
3/16" = 1'-0"



4 EAST GIRT
3/16" = 1'-0"



5 WEST GIRT (LEAN)
3/16" = 1'-0"



6 EAST GIRT (LEAN)
3/16" = 1'-0"

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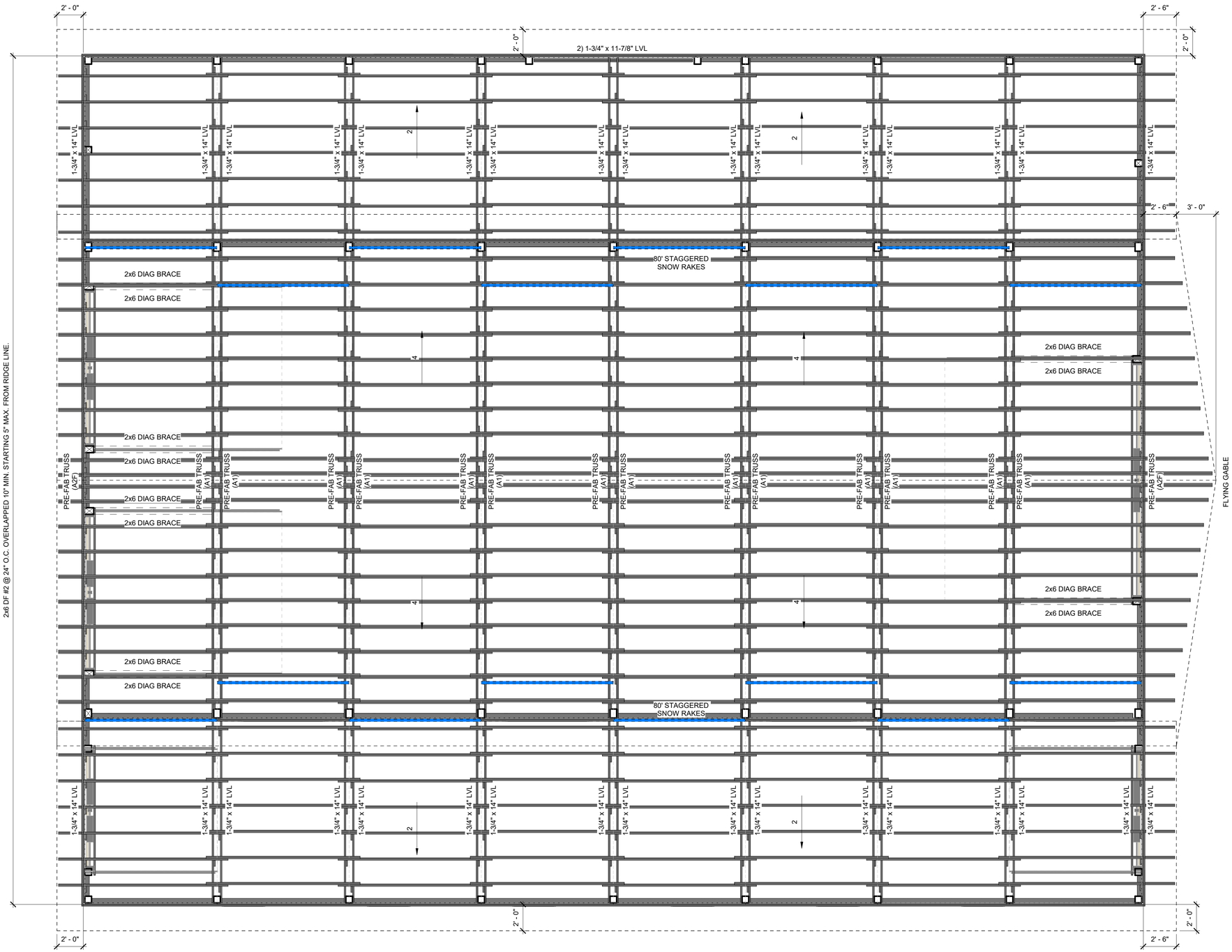
Scale: 3/16" = 1'-0"
Drawn by: CBC
Job: JEPPSEN.S
Sheet Size: D
22" x 34"

Framing Plan

A3

EAST



NORTH

2x6 DF #2 @ 24" O.C. OVERLAPPED 10" MIN. STARTING 5" MAX. FROM RIDGE LINE.

SOUTH (FRONT)

WEST

Roof Plan
1/4" = 1'-0"

ROOF FRAMING NOTES:

ROOF PITCHES:
MAIN: 4/12 ROOF
(2)LEANS: 2/12 ROOF (RAISED)

PURLINS:
2x6 DF #2 AT 24" ON CENTER OVERLAPPED 10" MIN. STARTING 5" MAX. FROM RIDGE LINE.

PURLINS ARE SINGLE PLY UNLESS OTHERWISE NOTED. ADDITIONAL PURLINS ATTACHED TO TOP PURLIN FOR ROOF RIDGE SUPPORT

OVERHANGS:
30" ON SOUTH FRONT WITH 36" OFFSET FOR FLYING GABLE. 24" ON EAST AND WEST EAVES. 24" ON NORTH (BACK).

FASCIA:
12" FASCIA ON ALL ROOF PERIMETERS

ROOF INSULATION:
3" CLOSED CELL SPRAY FOAM

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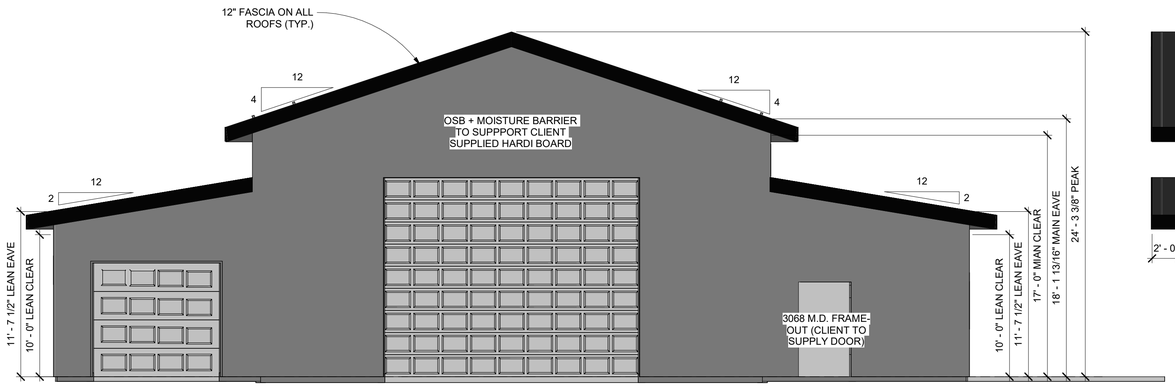
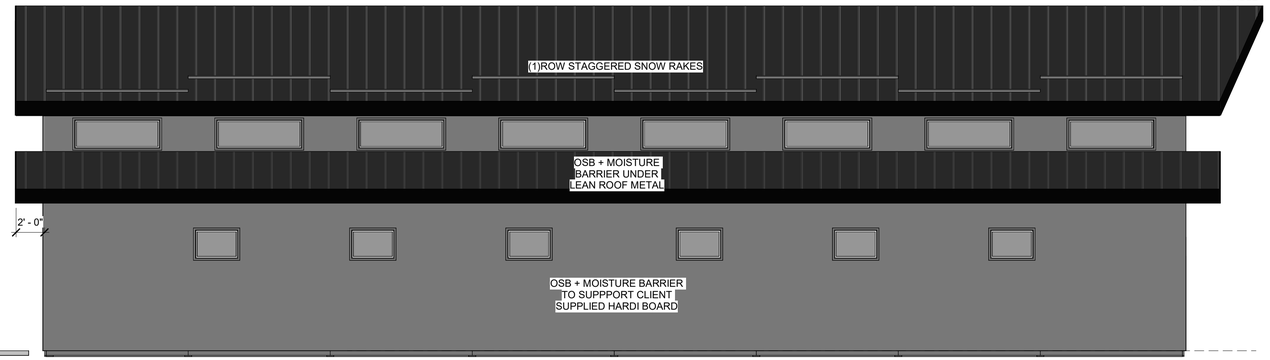
Roof Plan

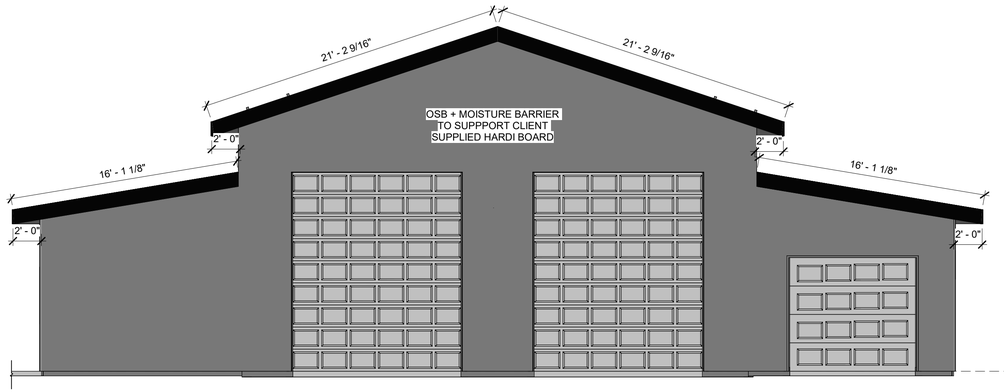
A4



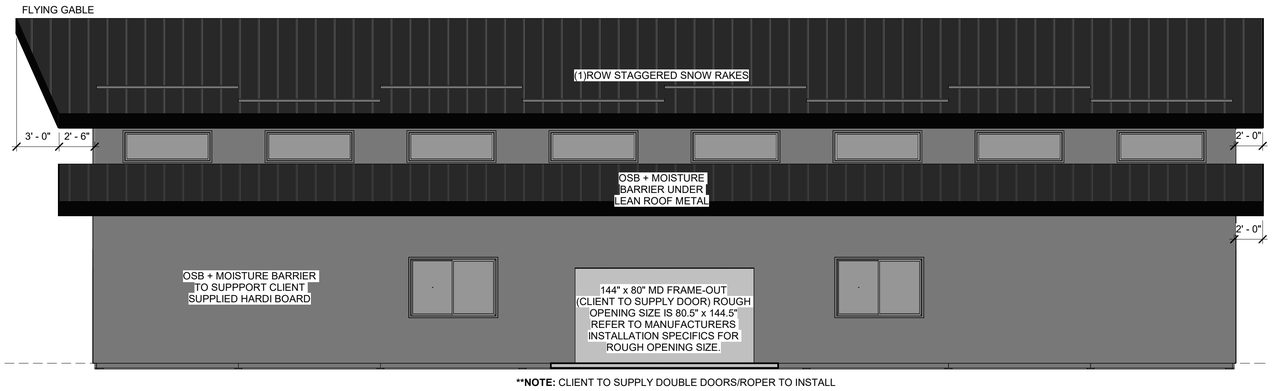
1 SOUTH (FRONT) ELEVATION
3/16" = 1'-0"



2 WEST ELEVATION
3/16" = 1'-0"



3 NORTH ELEVATION
3/16" = 1'-0"



NOTE: CLIENT TO SUPPLY DOUBLE DOORS/ROPER TO INSTALL

4 EAST ELEVATION
3/16" = 1'-0"

NOTES:
1. EXTERIOR WALLS ARE 7/16" OSB WITH MOISTURE BARRIER. CLIENT OT INSTALL HARDI BOARD.
2. ROOF FASCIA IS 12" ON ALL ROOF PERIMETERS.

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Scale: 3/16" = 1'-0"
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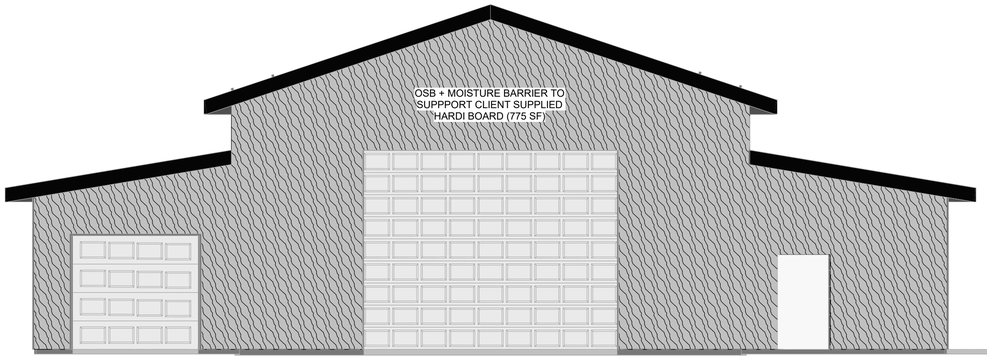
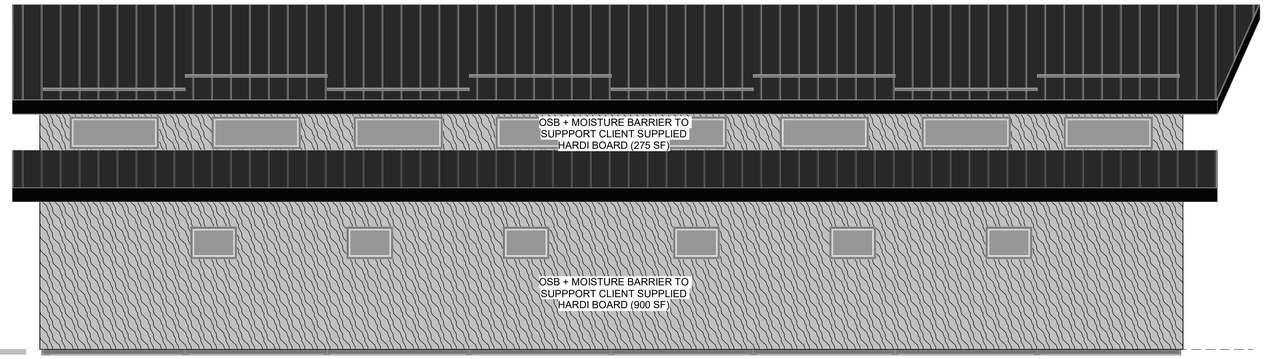
22" x 34"

Elevations

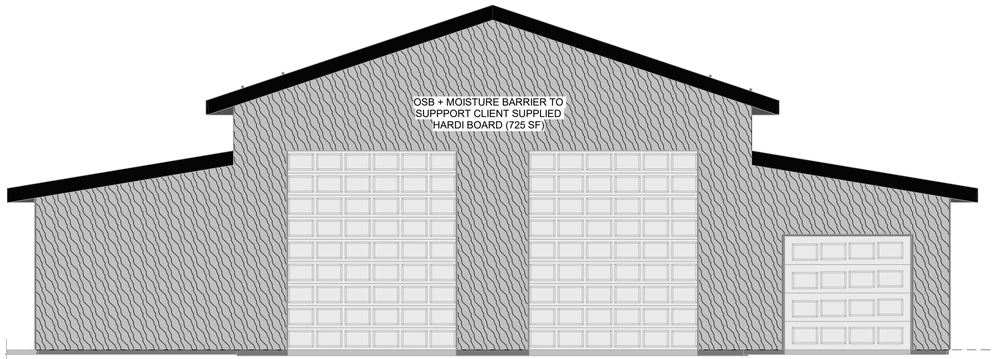
A5



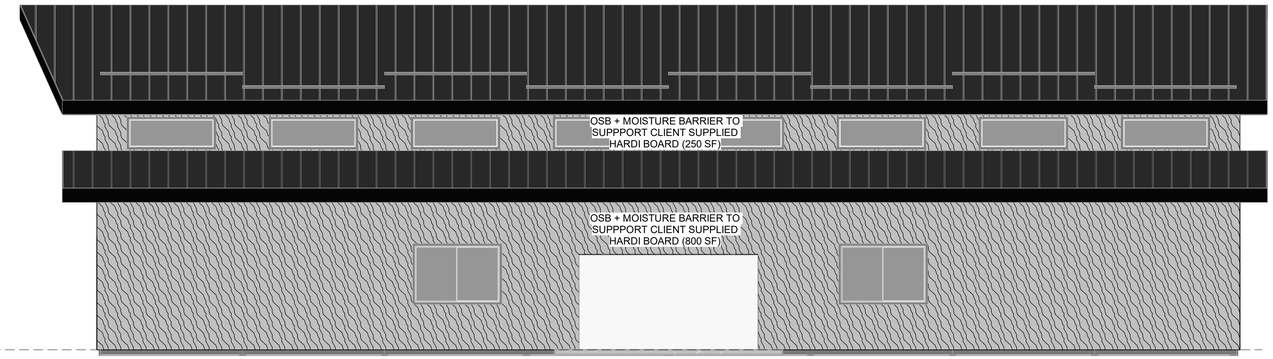
1 SOUTH (FRONT) OSB
3/16" = 1'-0"



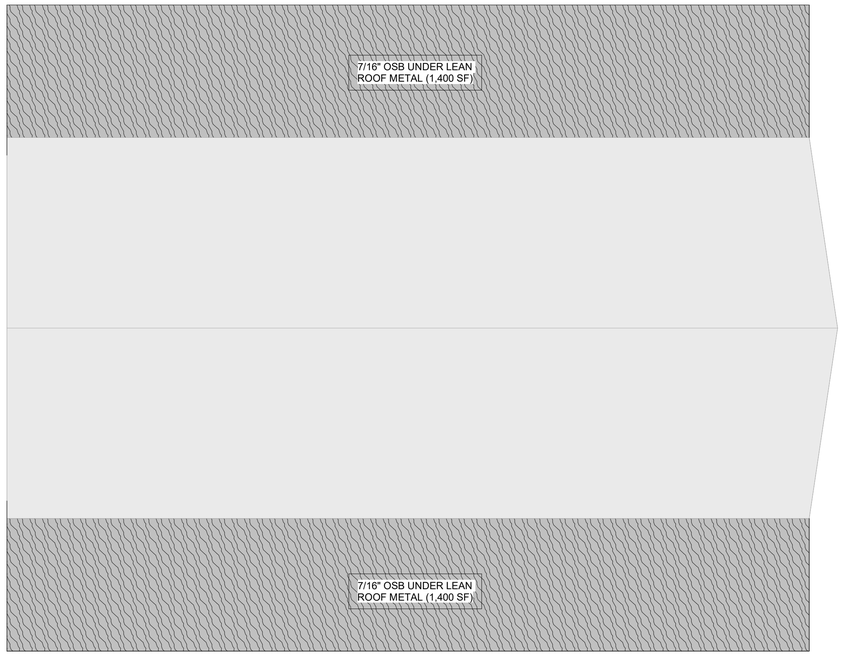
2 WEST OSB
3/16" = 1'-0"



3 NORTH OSB
3/16" = 1'-0"



4 EAST OSB
3/16" = 1'-0"



5 Roof OSB
1/8" = 1'-0"

OSB TABLE

WALL OSB: 3,725 SF
ROOF OSB: 2,800 SF

TOTAL OSB: 6,525 SF

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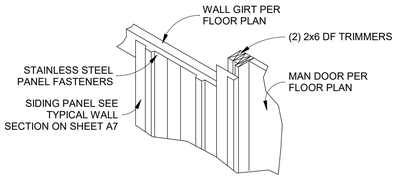
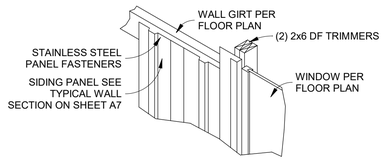
OSB Layouts

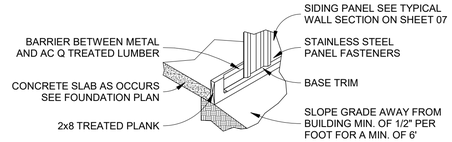
A6



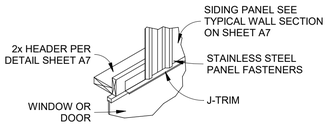
1 MAN DOOR JAMB FLASHING
N.T.S.



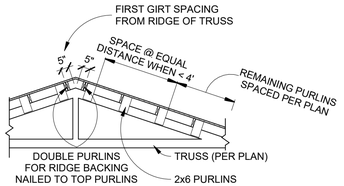
2 WINDOW JAMB FLASHING
N.T.S.



3 BASE GUARD FLASHING
N.T.S.



4 WINDOW/DOOR HEADER FLASHING
N.T.S.



5 T.O. ROOF DETAIL
1/4" = 1'-0"

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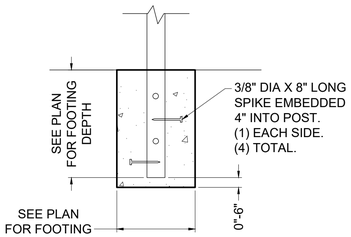
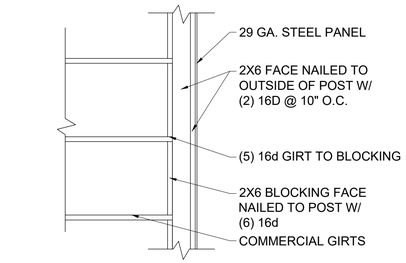
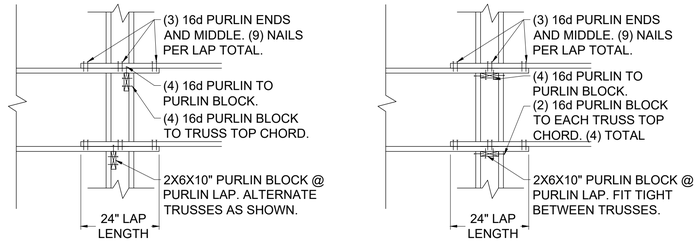
A7



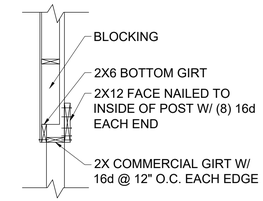
1 24"-30" DIA FOOTING
NTS



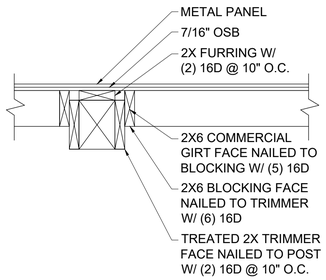
2 CORNER BLOCKING
NTS



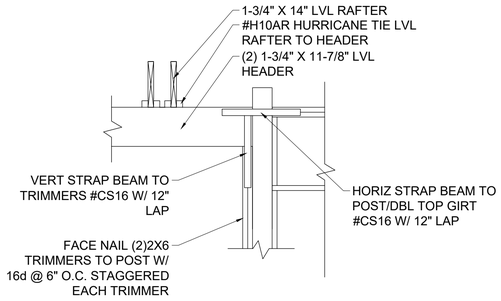
3 PURLIN LAP -OR- PURLIN LAP
NTS



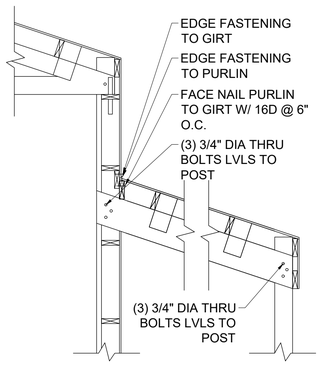
4 NON BEARING GARAGE HDR
NTS



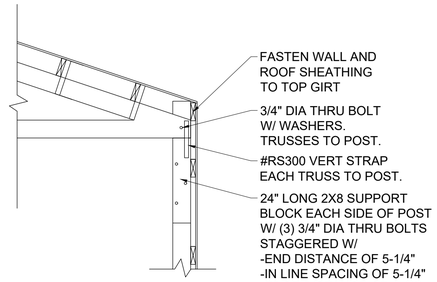
5 SECTION AT POST
NTS



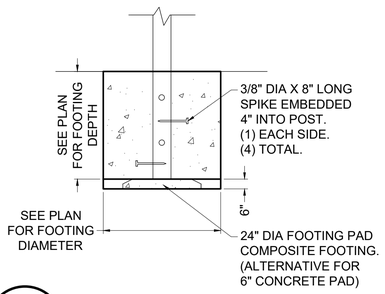
6 BEARING GARAGE HDR
NTS



7 OFFSET LEAN SECTION
NTS



8 3/4" THRU BOLT TRUSS SUPPORT ALTERNATIVE
NTS



9 36"-42" DIA FOOTING
NTS

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Structural Details
A8

PROFESSIONAL ENGINEER
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13258211
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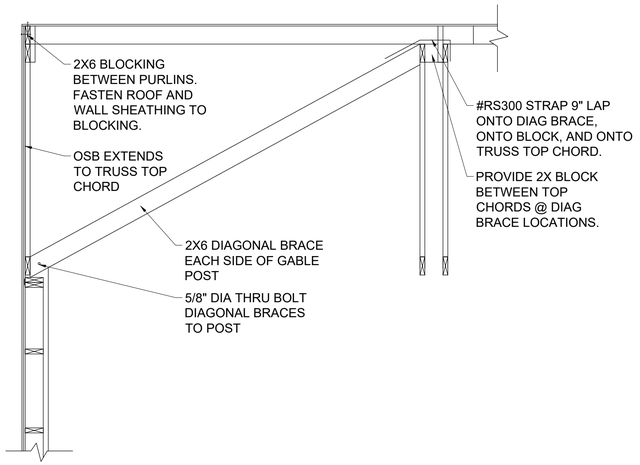
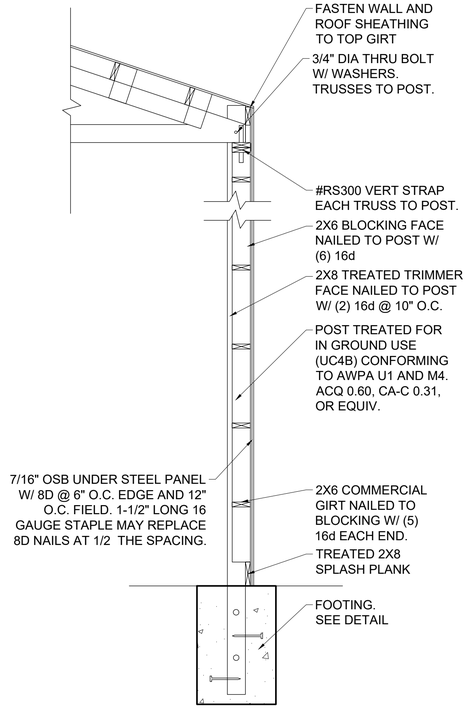
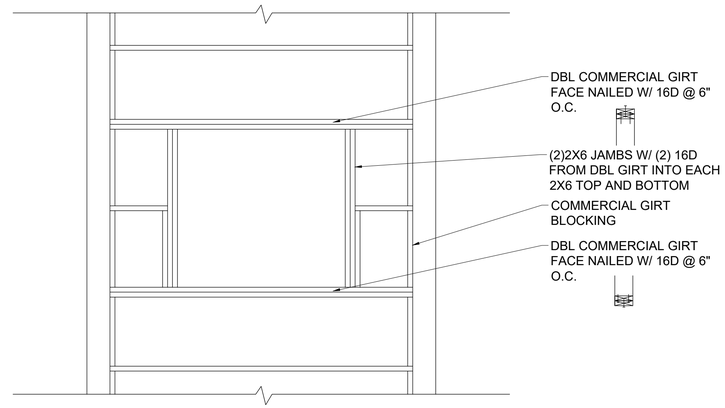


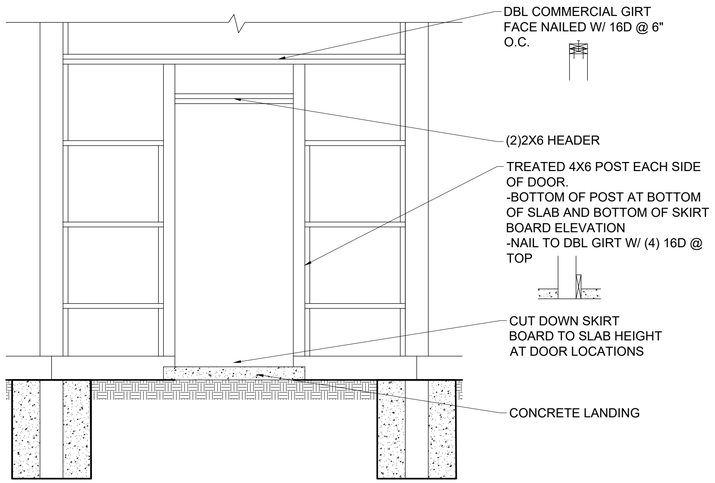
10 DIAGONAL BRACE
NTS



11 WALL SECTION
NTS



12 WINDOW FRAMING
NTS



13 MAN DOOR FRAMING
NTS

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13258211
07/29/2025
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22" x 34"
Structural Details
A9

DESIGN CRITERIA:

RISK CATEGORY
DESIGN RISK CATEGORY: I
SNOW LOAD IMPORTANCE FACTOR (I_S): 0.8
SEISMIC IMPORTANCE FACTOR (I_E): 1.0

SEISMIC LOADS
S_1: 0.26
S_{M1}: 0.68
S_{MS}: 1.07
T_L: 8
R: 6.5 (SHEAR WALLS) OR 1.5 (CANTILEVER SYSTEM)
SITE CLASS: D (ASSUMED)
SDC: D

WIND LOADS
V (3 SEC GUST): 105 MPH
EXPOSURE CATEGORY: C
ELEVATION: 4938 FT

SNOW LOADS
P_G: 52 PSF
W_S: 7 PSF
C_E: 1.0
C_T: 1.2
C_S: 1.0
P_F: 35 PSF
P_S: 35 PSF

DEAD LOADS
ROOF: 5 PSF (TRUSSES, PURLINS, METAL, OSB)
FLOOR: 10 PSF (JOISTS, OSB, FLOORING)
WALLS: 6 PSF (POSTS, GIRTS, OSB, HARDI BOARD)
CONCRETE: 145 PSF

LIVE LOADS
ROOF: 20 PSF
FLOOR: 40 PSF
DECK: 60 PSF

SOIL LOADS AND VALUES (ASSUMED):
SOIL BEARING PRESSURE: 1500 PSF
ACTIVE PRESSURE: 35 PCF
PASSIVE PRESSURE: 250 PCF
AT-REST PRESSURE: 60 PCF
LATERAL BEARING PRESSURE: 400 PSF/FT (IBC 1806.1, 1806.2, & 1806.3.4)
*SEE NOTES FOR SOIL ASSUMPTIONS AS FOUND IN THE CONCRETE FOOTINGS & FOUNDATIONS SECTION, THIS PAGE
**ENGINEER ASSUMES STABLE SOIL CONDITIONS. IF THERE ARE ANY GLOBAL STABILITY CONCERNS, A GEOTECHNICAL REPORT IS REQUIRED.
***VALUES ASSUMED ARE FOR FOOTINGS AND FOUNDATIONS PLACED IN NATIVE SOIL CONDITIONS.

GENERAL CONSTRUCTION NOTES:

1. ALL CONSTRUCTION WORK SHALL FOLLOW THE STANDARDS FOUND IN THE INTERNATIONAL BUILDING CODE (IBC 2021), INTERNATIONAL RESIDENTIAL CODE (IRC 2021), AND THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (NDS 2018).
2. THE ENGINEERED DRAWINGS PROVIDED HEREIN WERE COMPLETED USING THE PROFESSIONAL STANDARD OF CARE REQUIRED BY THE GOVERNING MUNICIPALITY AND/OR STATE. THESE DRAWINGS, HOWEVER, DO NOT AND CAN NOT PROVIDE EVERY EXPLICIT ELEMENT OR CONDITION OF THE STRUCTURE. AS SUCH, CONTRACTOR SHALL PROVIDE INDUSTRY STANDARD GOOD CARE AND PRACTICE FOR MISCELLANEOUS ELEMENTS NOT SHOWN ON PLANS AND SHALL CONTACT ENGINEER FOR FURTHER INFORMATION IF REQUIRED.
3. STRUCTURAL ENGINEERING PROVIDED HEREIN ASSUMES FINAL CONSTRUCTED CONDITION. CONTRACTOR SHALL BE RESPONSIBLE FOR STRUCTURAL INTEGRITY OF UNFINISHED STRUCTURE DURING ALL STAGES OF CONSTRUCTION. CONSTRUCTION LOADS SHALL NOT EXCEED DESIGN LIVE LOADS SHOWN IN DESIGN CRITERIA. CONTRACTOR SHALL PROVIDE BRACING OR SHORING AS NECESSARY TO SUPPORT UNFINISHED STRUCTURE.
4. WHITE PINE ENGINEERING (WPE) ASSUMES NO LIABILITY FOR THE MEANS AND METHODS OF CONSTRUCTION PRACTICES. CONTRACTOR IS RESPONSIBLE FOR ALL MEANS OF CONSTRUCTION AND FOR JOBSITE SAFETY PER OSHA REGULATIONS
3. CONTRACTOR SHALL BE RESPONSIBLE FOR RECEIVING APPROVAL AND ANY NECESSARY PERMITS FROM THE GOVERNING MUNICIPALITY PRIOR TO BEGINNING OF CONSTRUCTION.
4. ANY OBSERVATIONS PROVIDED BY WPE DURING CONSTRUCTION SHALL BE CONSIDERED OBSERVATIONS LIMITED TO THE SCOPE REQUESTED, AND NOT FULL INSPECTIONS OR APPROVAL. THE GOVERNING MUNICIPALITY SHALL BE SOLELY RESPONSIBLE FOR INSPECTIONS AND APPROVAL OF FINAL CONSTRUCTION.
5. CONTRACTOR SHALL INFORM ENGINEER OF ANY DISCREPANCIES BETWEEN ACTUAL CONDITIONS AND CONDITIONS ASSUMED ON THESE PLANS (I.E. DIMENSIONS, MATERIALS, ASSUMED LOADS, ETC).
6. SHOP DRAWINGS FOR ANY PREMANUFACTURED STRUCTURAL ELEMENTS SHALL BE STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF CONSTRUCTION AND SHALL BE SUBMITTED TO THE ENGINEER OF RECORD TO REVIEW IF DIMENSIONS OR OTHER ASPECTS OF SHOP DRAWINGS DIFFER FROM THOSE ON THESE PLANS.

CONCRETE FOOTINGS, FOUNDATIONS, AND SLABS

NOTES:
1. CONCRETE SHALL MEET REQUIREMENTS OUTLINED IN THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI318-19)
2. LOTS SHALL BE GRADED TO DRAIN SURFACE WATER AWAY FROM ALL FOUNDATION WALLS. THE GRADE SHALL FALL A MINIMUM OF 6 IN WITHIN THE FIRST 10 FT DISTANCE FROM THE BUILDING.
3. IMPERVIOUS SURFACES (I.E. CONCRETE, ASPHALT) SHALL BE SLOPED A MINIMUM OF 2% AWAY FROM THE BUILDING
4. CONTRACTOR SHALL DAMP PROOF ALL CONCRETE WALLS THAT RETAIN EARTH OR HAVE ENCLOSED USABLE SPACES BY PLACING A BITUMINOUS COATING ON BELOW GRADE EXTERIOR SECTIONS OF FOUNDATION WALL OR WITH ANOTHER APPROVED DAMP PROOFING METHOD AS OUTLINED IN R406 OF THE IRC.
5. FOUNDATION DRAINAGE SYSTEM IS NOT REQUIRED IF BACKFILLED WITH WELL-DRAINING BACKFILL OR SAND-GRAVEL MIXTURE SOILS OF GROUP 1 SOILS OF THE UNIFIED SOILS CLASSIFICATION SYSTEM AS SHOWN IN TABLE R405.1 OF THE IRC.
6. ALL CONCRETE WALLS THAT RETAIN EARTH AND ANY FOUNDATION WALLS THAT ENCLOSE USABLE AREAS WITH GROUP II-GROUP IV SOILS OF THE UNIFIED SOILS CLASSIFICATIONS SYSTEM AS SHOWN IN TABLE R405.1 OF THE IRC SHALL HAVE A FOUNDATION DRAIN INSTALLED CONSISTING OF A PERFORATED DRAIN PIPE NOT LESS THAN 1' BEYOND THE OUTSIDE EDGE OF THE FOOTINGS AND 6" ABOVE THE TOP OF FOOTING WITH A GRAVEL DRAIN WRAPPED IN AN APPROVED FILTER FABRIC.
7. ANY FOUNDATION WALLS THAT ENCLOSE USABLE SPACE WITH A HIGH WATER TABLE OR OTHER SEVERE GROUND WATER CONDITIONS SHALL BE WATERPROOFED AS REQUIRED IN SECTION R406.2 OF THE IRC.
8. WPE ASSUMES STABLE SOIL CONDITIONS. CONTACT ENGINEER IF SOIL CONDITIONS DIFFER FROM THOSE ASSUMED. SEE DESIGN CRITERIA FOR ASSUMED VALUES. THESE VALUES SHALL BE REPLACED BY DATA INCLUDED IN A SITE SPECIFIC GEOTECHNICAL REPORT IF AVAILABLE.
9. CRUSHED ROCK OR GRAVEL FILL WITH RELATIVELY LOW AMOUNTS OF FINES MAY BE USED TO BRING BOTTOM OF FOOTINGS OR SLABS UP TO GRADE WITH A MAXIMUM GRAVEL FILL DEPTH OF 2'. GRAVEL OR CRUSHED ROCK FILLS SHALL BE COMPACTED USING A VIBRATORY PLATE COMPACTOR.
10. ANY FILL SOILS OR GRAVEL FILL DEEPER THAN 2' THAT SUPPORT FOOTINGS AND FOUNDATIONS SHALL BE DESIGNED BY A LICENSED GEOTECHNICAL ENGINEER. FILL SOILS SHALL BE INSTALLED, AND TESTED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICE.
11. WHERE QUANTIFIABLE DATA OR OBSERVATIONS INDICATE EXPANSIVE, COMPRESSIBLE, SHIFTING, OR OTHER QUESTIONABLE SOIL CHARACTERISTICS OR GEOTECHNICAL HAZARDS ARE LIKELY TO BE PRESENT, THE BUILDING OFFICIAL SHALL DETERMINE WHETHER TO REQUIRE A SITE SPECIFIC GEOTECHNICAL STUDY TO DETERMINE SOIL CHARACTERISTICS OF THE SITE.
12. FROST DEPTH OF FOOTINGS AND MINIMUM FREEBOARD OF FOUNDATION WALLS SHALL BE MET AS SPECIFIED ON SHEET S1.0. NO FOOTINGS IN UNCONDITIONED SPACE SHALL BE CONSTRUCTED SHALLOWER THAN THE MINIMUM FROST DEPTH.
13. CONCRETE SHALL BE BE MECHANICALLY VIBRATED WHEN PLACED TO AVOID HONEYCOMBING ALONG FORMS AND TO ALLOW CONCRETE TO FREELY FLOW AROUND REINFORCEMENT.
14. CONCRETE SHALL NOT BE DROPPED MORE THAN 5' MAXIMUM DURING PLACEMENT.
15. CONCRETE FOR FOUNDATION WALLS SHALL CURE A MINIMUM OF 7 DAYS AND HAVE INTERIOR SLAB INSTALLED PRIOR TO ANY BACKFILL. FOUNDATION WALLS 6' IN HEIGHT AND ABOVE DESIGNED FOR TOP SUPPORT. CONTRACTOR SHALL INSTALL FLOOR OR ROOF DIAPHRAGM PRIOR TO BACKFILL AND SHALL NOT COMPACT ANY BACKFILL PLACED AROUND THESE WALLS.
16. ALL FOUNDATIONS SHALL HAVE 6" MIN. EXPOSED ABOVE GRADE.
17. CONTRACTOR SHALL ROUGHEN COLD JOINT BETWEEN FOOTING AND WALL TO $\frac{1}{4}$" AMPLITUDE.
18. ALL SLABS ON GRADE SHALL HAVE CONTROL JOINTS IN SLAB AT 15' O.C. MAXIMUM SPACING.
19. ALL CONCRETE SLABS ON GRADE SHALL BE A MINIMUM OF 4" THICK ON 4" PLATE COMPACTED GRAVEL OR OTHER APPROVED FREE DRAINING MATERIAL.
20. REINFORCING STEEL IN CONCRETE SHALL MEET THE REQUIREMENTS OF THE FOLLOWING SECTION ENTITLED "REINFORCING STEEL".
21. ALL REINFORCEMENT IN CONCRETE SHALL BE PROPERTY TIED AND SECURED PRIOR TO POURING CONCRETE IN SUCH A WAY THAT REINFORCEMENT REMAINS IN DESIGNED LOCATION. VERTICAL REINFORCING STEEL FOR MASONRY CONSTRUCTION MAY BE FLOATED INTO PLACE.
22. ANY COMPONENTS THAT ARE CAST IN PLACE SHALL BE SECURELY PLACED IN THE FORMS (I.E. STRAPS, BOLTS, SLEEVES, ETC.)
23. REINFORCEMENT IN FOUNDATION WALLS SHALL BE CENTERED IN WALL U.N.O. REINFORCEMENT FOR CONCRETE WALLS RETAINING EARTH SHALL BE PLACED WHERE DESIGNED ON PLANS.
24. ENGINEER ASSUMES 2,500 PSI COMPRESSIVE STRENGTH IN STRUCTURAL CONCRETE, THEREFORE CONCRETE FOR FOOTINGS, FOUNDATION WALLS, AND SLABS ON GRADE CAN BE EXEMPT FROM SPECIAL INSPECTIONS ACCORDING TO EXEMPTIONS LISTED IN SECTION 1705.3 IN THE IBC UNLESS NOTED OTHERWISE.

CONCRETE SPECIFICATIONS

STRUCTURAL ELEMENT	MIN. COMRESSIVE STRENGTH (f'c)	AIR CONTENT	W/C RATIO
FOUNDATION WALLS AND FOOTINGS, NOT EXPOSED TO WEATHER	3000 PSI	5%-7%(1)	.55
SLABS (EXCLUDING GARAGE SLABS)	3000 PSI	5%-7%(1)	.55
WALLS, EXPOSED TO WEATHER	3000 PSI	5%-7%	.45
SLABS EXPOSED TO WEATHER (INCLUDING GARAGE AND SUSP SLABS)	3500 PSI	5%-7%	.45

NOTES TO TABLE ABOVE:
1. AIR ENTRAINMENT ONLY REQUIRED IN CONCRETE NOT EXPOSED TO WEATHER IF EXPOSED TO FREEZE/THAW DURING CONSTRUCTION PROCESS.
2. f'c IS CONCRETE COMPRESSIVE STRENGTH AT 28-DAYS
3. PORTLAND CEMEMT TYPE I/II
4. W/C RATIO BASED ON ALL CEMENTITIOUS AND SUPPLEMENTARY CEMENTITIOUS MATERIAL IN CONCRETE MIXTURE.
5. CONCRETE SHALL NOT INCLUDE CALCIUM CHLORIDE ADMIXTURE
6. NOMINAL MAXIMUM AGGREGATE SIZE OF 3/4" AND SHALL CONFORM TO ASTM C33
7. CONCRETE MIXTURE MAY HAVE A MAXIMUM OF 25% BY MASS OF FLY ASH OR OTHER NATURAL POZZOLANS CONFORMING TO ASTM C618
8. SEE SECTION R404.1.3.3.4 (IRC 2021) FOR SLUMP REQUIREMENTS. SLUMP OF CONCRETE SHALL BE ADEQUATE TO PROVIDE WORKABILITY AROUND REINFORCEMENT.

ANCHOR BOLTS

NOTES:
1. ANCHORING OF SILL PLATES TO FOUNDATION SHALL FOLLOW SECTION 2308.3 (IBC 2021)
2. ANCHOR BOLTS SHALL BE A MINIMUM OF $\frac{1}{2}$" DIAMETER X 10" LONG "J" BOLTS @ 32" O.C. ANCHOR BOLTS SHALL BE GRADE A307 AND SHALL HAVE 7" MINIMUM EMBEDMENT IN CONCRETE. SEE SHEAR WALL SCHEDULE FOR SHEAR WALL SPECIFIC ANCHOR BOLT SPACING.
3. ANCHOR BOLTS SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF THE SILL PLATE.
4. EACH SECTION OF FOUNDATION WALL SHALL HAVE A MINIMUM OF (2) ANCHOR BOLTS (ONE ON EACH END). PROVIDE EDGE ANCHOR BOLTS WITHIN FIRST FOOT OF FOUNDATION SECTION WHEN STEPPED.
5. PROVIDE 3" X 3" X $\frac{1}{2}$" PLATE WASHER FOR EACH ANCHOR BOLT. DIAGONALLY SLOTTED HOLE IN PLATE WASHER ALLOWED TO BE $\frac{3}{16}$" WIDER THAN ANCHOR BOLT DIAMETER NOT TO EXCEED 1 $\frac{3}{4}$" IF A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND NUT.
6. $\frac{1}{2}$" DIA. X 7" LONG SIMPSON TITAN HD BOLTS @ SPACING MATCHING SHEAR WALL SCHEDULE MAY BE SUBSTITUTED FOR ANCHOR BOLT SPECIFICATION ABOVE

REINFORCING STEEL:

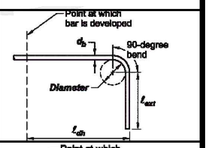
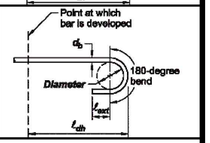
NOTES:
1. PLACEMENT OF REINFORCING STEEL SHALL MEET REQUIREMENTS OF CHAPTER 25 OF ACI 318-19.
2. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615 GRADE 60 (Fy = 60 KSI).
3. REINFORCING STEEL SHALL NOT BE WELDED UNLESS SPECIFIED ON THE PLANS. REINFORCING STEEL THAT IS WELDED SHALL BE ASTM A706 GRADE 60.
4. REINFORCING WELDED WIRE FABRIC SHALL CONFORM TO ASTM STANDARD A-185.
5. ALL REINFORCING STEEL DIMENSIONS SHALL BE TO CENTER OF BAR UNLESS DIMENSION IS SPECIFIED AS "CLEAR" OR "CLR" IN WHICH CASE THE DIMENSION IS TO EDGE OF REBAR.
6. EDGE DISTANCE, HOOK AND BEND RADII, AND REBAR LAP LENGTHS SHALL MEET THE FOLLOWING REQUIREMENTS ON THIS SHEET AND THE REQUIREMENTS IN THE ACI 318.
7. REINFORCING IN FOUNDATION WALLS AND AROUND OPENINGS SHALL MEET REQUIREMENTS OF THE FOUNDATION SCHEDULE SHOWN ON S1.0 AND THE UTAH AMENDMENT TO THE CODE 15A-3-108.
8. ON CENTER (O.C.) SPACING SHOWN ON PLANS SHALL BE MAXIMUM SPACING ALLOWABLE.
9. BARS SHALL BE LAPPED A MINIMUM OF 40 BAR DIAMETERS (d_b) TO MEET DEVELOPMENT LENGTH. DEVELOPMENT LENGTH ASSUMES UNEPOXIED REBAR AND NORMAL WEIGHT CONCRETE. CONTACT ENGINEER FOR OTHER CONDITIONS IF NEEDED.

REBAR CONCRETE COVERAGE DISTANCES

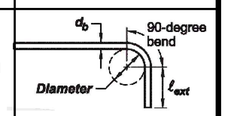
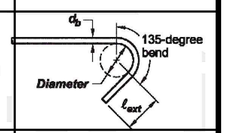
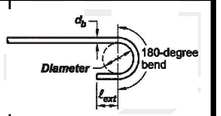
EXPOSURE CONDITION	BAR SIZE OR MEMBER	REBAR CLR DISTANCE (MIN.)
CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	ANY	3"
EXPOSED TO WEATHER	#5 AND SMALLER	1-1/2"
	#6 AND LARGER	2"
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	#14 AND # 18; SLABS, JOISTS, AND WALLS	1-1/2"
	#11 AND SMALLER; SLABS, JOISTS, AND WALLS	3/4"
	ALL SIZE BARS; BEAMS, COLUMNS, AND TENSION TIES	1-1/2"

NOTES TO TABLE ABOVE:
1. SEE TABLE 20.5.1.3.1 ACI 318-19
2. ALL SLAB ON GRADE REINFORCEMENT SHALL BE CENTERED IN SLAB UNLESS NOTED OTHERWISE

STANDARD HOOK GEOMETRY OF DEFORMED BARS IN TENSION (SEE ACI 318-19 TABLE 25.3.1 FOR ALL BAR SIZES AND REQUIREMENTS)

TYPE OF HOOK	BAR SIZE	MIN. INSIDE DIAMETER	STRAIGHT EXTENSION MIN. (l_{ext})	TYPE OF HOOK
90° HOOK	#3-#8	$6d_b$	$12d_b$	
180° HOOK	#3-#8	$6d_b$	GREATER OF $4d_b$ and 2.5"	

STANDARD HOOK GEOMETRY FOR STIRRUPS, TIES AND HOOPS (SEE ACI 318-19 TABLE 25.3.2 FOR ALL BAR SIZES AND REQUIREMENTS)

TYPE OF HOOK	BAR SIZE	MIN. INSIDE DIAMETER	STRAIGHT EXTENSION MIN. (l_{ext})	TYPE OF HOOK
90° HOOK	#3-#5	$4d_b$	GREATER OF $6d_b$ and 3"	
135° HOOK	#3-#5	$4d_b$	GREATER OF $6d_b$ and 3"	
180° HOOK	#3-#5	$4d_b$	GREATER OF $6d_b$ and 3"	

NOTES TO TABLE ABOVE:
1. A LONGER STRAIGHT EXTENSION IS ALLOWABLE BUT DOES NOT INCREASE ANCHORAGE CAPACITY
2. MINIMUM DEVELOPMENT LENGTH SHALL BE 40 BAR DIAMETERS (d_b)

PROFESSIONAL ENGINEER
JACKSON D. SAGERS
13258211
07/29/2025
STATE OF UTAH

ROPER BUILDINGS / DMLP RESOURCES
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STEPHANIE & PAUL JEPPSEN BUILDING
7704 East 500 North
Huntsville, UT 84317

Date: 7/29/2025
Scale:
Drawn by: CBC
Job: JEPPSEN.S
Sheet Size: D 22" x 34"
Engineering Notes

A10

MINIMUM NAILING SCHEDULE (IBC 2304.10.2)

No.	BUILDING ELEMENT	No.	SIZE	SPACING	LOCATION
1.1	BLOCKING BETWEEN CEILING JOISTS, RAFTERS, OR TRUSSES TO TOP PLATE OR OTHER FRAMING BELOW	(4)	8d	-	EACH END, TOENAIL
1.2	BLOCKING BETWEEN RAFTERS OR TRUSS NOT AT THE WALL TOP PLATE, TO RAFTER OR TRUSS	(2)	16d	-	END NAIL
8	STUD TO STUD (NOT AT BRACED WALL PANELS)	-	16d	24" O.C.	FACE NAIL
9	STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS)	-	16d	12" O.C.	FACE NAIL
10	BUILT-UP HEADER (2" TO 2" HEADER)	-	16d	12" O.C.	EACH EDGE, FACE NAIL
11	CONTINUOUS HEADER TO STUD	(5)	8d	-	TOENAIL
12	TOP PLATE TO TOP PLATE	-	16d	16" O.C.	FACE NAIL
13	TOP PLATE TO TOP PLATE, AT END JOINTS (MINIMUM 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT)	(12)	16d	-	EACH SIDE OF END JOINT, FACE NAIL
14	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING (NOT AT BRACE WALL PANELS)	-	16d	12" O.C.	FACE NAIL
15	BOTTOM PLATE TO JOIST, RIM JOIST, BANDS JOIST, OR BLOCKING AT BRACE WALL PANELS	(3)	16d	16" O.C.	FACE NAIL
16	STUD TO TOP OR BOTTOM PLATE	(2)	16d	-	END NAIL
17	TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	(2)	16d	-	FACE NAIL
21	JOIST TO SILL, TOP PLATE, OR GIRDER	(4)	8d	-	TOENAIL
22	RIM JOIST, BAND JOIST, OR BLOCKING TO TOP PLATE, SILL, OR OTHER FRAMING BELOW	-	8d	4" O.C.	TOENAIL
26	BUILT-UP GIRDERS AND BEAMS, 2" LUMBER LAYERS	(2)	16d	12" O.C.	FACE NAIL
28	JOIST TO BAND JOIST OR RIM JOIST	(3)	16d	-	END NAIL
29	BRIDGING OR BLOCKING TO JOIST, RAFTER, OR TRUSS	(2)	8d	-	EACH END, TOENAIL

OSB SHEAR WALL SCHEDULE

MARK	SHEATHING	EDGE NAILING	EDGE STAPLING	EDGE MEMBER	FIELD NAILING	FIELD STAPLING	FIELD MEMBER	PLF
SW-6"	7/16" OSB, 1 SIDE	8d @ 6" O.C.	1-1/2" LONG 16 GAUGE @ 3" O.C.	(1) 2X	8d @ 6" O.C.	1-1/2" LONG 16 GAUGE @ 6" O.C.	(1) 2X	240
SW-4"	7/16" OSB, 1 SIDE	8d @ 4" O.C.	1-1/2" LONG 16 GAUGE @ 2" O.C.	(1) 3X OR (2) 2X W/ 16d @ 4" O.C.	8d @ 6" O.C.	1-1/2" LONG 16 GAUGE @ 6" O.C.	(1) 2X	350
SW-3"	7/16" OSB, 1 SIDE	8d @ 3" O.C.	N/A	(1) 3X OR (2) 2X W/ 16d @ 4" O.C.	8d @ 6" O.C.	N/A	(1) 2X	450
SW-2"	7/16" OSB, 1 SIDE	8d @ 2" O.C.	N/A	(1) 3X OR (2) 2X W/ 16d @ 3" O.C.	8d @ 6" O.C.	N/A	(1) 2X	585
(2)SW-4"	7/16" OSB, BOTH SIDES	8d @ 4" O.C.	1-1/2" LONG 16 GAUGE @ 2" O.C.	(1) 3X OR (2) 2X W/ 16d @ 4" O.C.	8d @ 6" O.C.	1-1/2" LONG 16 GAUGE @ 6" O.C.	(1) 2X	700
(2)SW-3"	7/16" OSB, BOTH SIDES	8d @ 3" O.C.	N/A	(1) 3X OR (2) 2X W/ 16d @ 4" O.C.	8d @ 6" O.C.	N/A	(1) 2X	900
(2)SW-2"	7/16" OSB, BOTH SIDES	8d @ 2" O.C.	N/A	(1) 3X OR (2) 2X W/ 16d @ 3" O.C.	8d @ 6" O.C.	N/A	(1) 2X	1170

NOTES TO TABLE ABOVE:
1. GIRTS SHALL BE DF-L @ 24" O.C.
2. NAILS SHALL BE CARBON STEEL SMOOTH SHANK 8d COMMON OR 8d GALVANIZED BOX. GALVANIZED NAILS SHALL BE HOT-DIPPED OR MECHANICALLY DEPOSITED.
3. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16" AND SHALL BE INSTALLED WITH THEIR CROWNS PARALLEL TO THE LONG DIMENSION OF THE FRAMING MEMBERS.
4. NAILS/STAPLES SHALL BE DRIVEN WITH THE HEAD/CROWN OF THE NAIL/STAPLE FLUSH WITH THE SURFACE OF THE SHEATHING.
5. DOUBLE SIDED SHEAR WALLS SHALL HAVE PANEL JOINTS OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS. NAILS AT PANEL EDGES SHALL BE STAGGERED.
6. BLOCK ALL PANEL EDGES. FLAT BLOCKING/GIRTS IS ACCEPTABLE ON SINGLE SIDED SHEAR WALLS.
7. ALL WALLS SHALL FOLLOW SW-6" U.N.O.
8. STRUCTURAL PANELS SHALL BE APA APPROVED, EXPOSURE 1, AND MEET THE REQUIREMENTS OF USDOC PS 2.
9. NAILS SHALL BE LOCATED AT LEAST 3/8" FROM THE PANEL EDGES.
10. PANELS SHALL NOT BE LESS THAN 4'X8' EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING. ALL EDGES OF ALL PANELS SHALL BE SUPPORTED BY AND FASTENED TO FRAMING MEMBERS OR BLOCKING.

METAL CLAD WOOD FRAMED SHEAR WALL SCHEDULE

MARK	GIRT SPACING	FIELD FASTENERS	END FASTENERS	STITCH FASTNERS	NFBA ASSEMBLY #	PLF
MSW-1	24"	#10 X 1.0" LONG SCREW @ 9" O.C.	#10 X 1.0" LONG SCREW @ 4.5" O.C.	NONE	#2	145
MSW-2	32"	#10 X 1.0" LONG SCREW @ 9" O.C.	#10 X 1.0" LONG SCREW @ 4.5" O.C.	NONE	N/A	108
MSW-3	36"	#10 X 1.0" LONG SCREW @ 7.2" O.C.	#10 X 1.0" LONG SCREW @ 4.5" O.C.	NONE	#1	100
MSW-4	24"	#10 X 1.0" LONG SCREW @ 9" O.C.	#10 X 1.0" LONG SCREW @ 4.5" O.C.	#12 X 0.75" LONG SCREW @ 8" O.C.	#3	240

NOTES TO TABLE ABOVE:
1. ALL WALLS SHALL FOLLOW FASTENING FOR MSW-1/MSW-2 U.N.O.
2. GIRTS SHALL BE 2X6 MIN.
3. SPLASH PLANK SHALL BE TREATED 2X8
4. STRUCTURAL PANELS SHALL BE 29 GAUGE STEEL (FABRAL GRANDRIB 3 OR EQUIVALENT)
5. PANELS SHALL EXTEND FROM SPLASH PLANK TO TOP GIRT

FRAMING

NOTES:
1. FULL HEIGHT SOLID BLOCKING REQUIRED AT ALL BEARING POINTS OF TRUSSES, RAFTERS, AND JOISTS.
2. WALLS SHALL BE BALLOON FRAMED (CONTINUOUS STUDS FROM FOUNDATION TO ROOF) EXCEPT WHERE LATERALLY SUPPORTED BY A FLOOR OR ROOF DIAPHRAGM.
3. ALL LUMBER SHALL MEET PLANS SPECIFICATIONS AND BE GRADED AND STAMPED BY AN APPROVED AGENCY (I.E. APA, WWPA, ETC.)
4. BEAMS ON PLAN ARE SIZED AS A MINIMUM. LARGER SIZES AND HIGHER GRADES MAY REPLACE MEMBERS ON PLAN.
5. FLOOR BEAMS MAY BE FLUSH MOUNTED OR DROP BEAMS.
6. BEAMS CONSISTING OF (4) OR MORE PLYS SHALL BE FASTENED W/ (2) ROWS OF 1/2" DIA THRU BOLTS @ 12" O.C. 2" FROM TOP AND 2" FROM BOTTOM OF BEAM.
7. HOLES FOR BOLTS SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER. A STANDARD CUT WASHER SHALL BE PROVIDED BETWEEN THE WOOD AND THE BOLT HEAD AND BETWEEN THE WOOD AND THE NUT.
8. LEAD HOLES FOR LAG SCREWS SHALL BE BORED AS FOLLOWS

A) THE CLEARANCE HOLE FOR THE THE SHANK SHALL HAVE THE SAME DIAMETER AS THE SHANK AND THE SAME DEPTH OF PENETRATION AS THE LENGTH OF UNTHREADED SHANK.

B) THE LEAD HOLE FOR THE THREADED PORTION SHALL HAVE A DIAMETER EQUAL TO 40%-70% OF THE SHANK DIAMETER AND A LENGTH EQUAL TO AT LEAST THE LENGTH OF THE THREADED PORTION. THE LARGER PERCENTILE SHALL APPLY TO LAG SCREWS OF GREATER DIAMETERS.

9. POSTS AND WALLS SHALL BE CENTERED ON CONTINUOUS AND SPOT FOOTINGS U.N.O.
10. SQUASH BLOCKING AND LOWER COLUMNS SHALL BE PROVIDED TO CARRY ALL COLUMN LOADS TO FOOTINGS AND FOUNDATIONS. SQUASH BLOCKING AND LOWER COLUMNS SHALL BE ALIGNED W/ AND MATCH UPPER COLUMN SIZE U.N.O.
11. BUILT UP 2X COLUMNS SHALL BE FACE NAILED TOGETHER W/ 16d @ 4" O.C STAGGERED EACH PLY.
12. COLUMNS SHALL SUPPORT FULL WIDTH OF BEAM ENDS.
13. ENGINEERED WOOD RIM BOARDS SHALL CONFORM TO ANSI/APA PRR 410 OR SHALL BE EVALUATED IN ACCORDANCE W/ ASTM D7672.
14. WHERE THE DBL TOP PLATE LAP AND NAILING REQUIREMENTS PER IBC2304.10.2 No. 13 ARE NOT MET, PROVIDE #CS16 W/ 12" LAP MIN.
15. WOOD CONSTRUCTION CONNECTORS SHALL BE INSTALLED PER THE MANUFACTURER'S SPECIFICATIONS.

TRUSS AND JOIST

NOTES:
1. PRE-MANUFACTURED TRUSSES SHALL FOLLOW LAYOUT SHOWN ON PLANS. ANY CHANGES IN TRUSS LAYOUT SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER.
2. CONTRACTOR AND MANUFACTURER SHALL VERIFY CEILING HEIGHTS, TRAYS, VAULTS, AND STEPS PRIOR TO CONSTRUCTION.
3. MULTI PLY TRUSSES OR JOISTS SHALL BE FASTENED PER MANUFACTURER'S SPECIFICATIONS.
4. TRUSSES AND JOISTS SHALL BE BRACED PER MANUFACTURER.
5. PROVIDE WEB STIFFENERS ON JOISTS WHEN REQUIRED BY MANUFACTURER.
6. NO ALTERATIONS OF TRUSSES OR JOISTS ARE ALLOWED WITHOUT APPROVAL FROM MANUFACTURER.
7. PROVIDE HURRICANE TIE EACH END OF EVERY ROOF TRUSS/RAFTER TO TOP PLATE.

- #H2.5A OR #H1 FOR STANDARD TRUSSES AND BEVELED RAFTERS
- #H1 OR #H10A FOR ATTIC AND TALL HEEL TRUSSES
- #VPA_ FOR NON BEVELED RAFTERS

STEEL

NOTES:
1. W SECTIONS SHALL COMPLY W/ ASTM A992 AND HAVE Fy=50 KSI MIN. AND Fu=65 KSI MIN.
2. HSS RECTANGULAR SECTIONS SHALL COMPLY W/ ASTM A500 GRADE C AND HAVE Fy=50 KSI MIN. AND Fu=62 KSI MIN.
3. C AND L SECTION SHALL COMPLY W/ ASTM A36 AND HAVE Fy=36 KSI MIN. AND Fu=58 KSI MIN.
4. ALL WELDS ARE 1/4" FILLET CONFORMING TO THE REQUIREMENTS IN AWS D1.1 W/ AN ELECTRODE Fexx=70 KSI U.N.O.
5. ALL STRUCTURAL STEEL BOLTS SHALL COMPLY W/ A307 GRADE A AND HAVE Fnt=45 KSI AND Fnv=27 KSI U.N.O.
6. ALL WIDE FLANGE BEAMS SHALL HAVE 3/8" WEB STIFFENER EACH SIDE AT BEARING LOCATIONS U.N.O.
7. WHERE LEVELING NUTS ARE USED UNDER BASE PLATES, NON SHRINK GROUT SHALL FILL THE VOID BETWEEN CONCRETE AND STEEL. GROUT SHALL BE RATED FOR 5000 PSI MIN.
8. WOOD NAILERS ON BEAMS SHALL BE 2X OR 3X W/ 1/2" DIA BOLTS SPACED AT 32" O.C. MIN.

EPOXY

NOTES:
1. EPOXY SHALL BE SIMPSON SET-3G U.N.O. -OR- EQUIVALENT
2. DRILL AND CLEAN HOLE PER MANUFACTURER SPECIFICATIONS.
3. HOLE SHALL BE FREE OF DUST, DEBRIS, AND STANDING WATER PRIOR TO EPOXY INSTALLATION.

OSB DIAPHRAGM SHEATHING SCHEDULE

LOCATION	NOMINAL THICKNESS	SPAN RATING	EDGE NAILING	FIELD NAILING	PLF
ROOF W/ DESIGN SNOW LOAD UP TO 50 PSF (FRAMING MEMBERS @ 24" O.C.)	7/16"	24/16	8d @ 6" O.C.	8d @ 12" O.C.	170
ROOF W/ DESIGN SNOW LOAD UP TO 180 PSF (FRAMING MEMBERS @ 16" O.C.)	7/16"	24/16	8d @ 6" O.C.	8d @ 12" O.C.	170
ROOF W/ DESIGN SNOW LOAD UP TO 120 PSF	19/32"	40/20	10d @ 6" O.C.	10d @ 12" O.C.	214
FLOOR	3/4" TONGUE AND GROOVE	48/24	10d @ 6" O.C.	10d @ 12" O.C.	214

NOTES TO TABLE ABOVE:
1. ROOF AND FLOOR FRAMING MEMBERS SHALL BE PLACED NO FURTHER THAN 24" O.C.
2. NAILS SHALL BE CARBON STEEL SMOOTH SHANK COMMON OR GALVANIZED BOX. GALVANIZED NAILS SHALL BE HOT-DIPPED OR MECHANICALLY DEPOSITED.
3. NAILS SHALL BE DRIVEN WITH THE HEAD OF THE NAIL FLUSH WITH THE SURFACE OF THE SHEATHING.
4. STRUCTURAL PANELS SHALL BE APA APPROVED, EXPOSURE 1, AND MEET THE REQUIREMENTS OF USDOC PS 2.
5. FLOOR SHEATHING SHALL BE GLUED TO FRAMING MEMBERS PRIOR TO NAILING W/ AN ADHESIVE CONFORMING TO APA SPECIFICATIONS.
6. STRENGTH AXIS (LONG DIRECTION) OF PANELS SHALL BE ORIENTED PERPENDICULAR TO FRAMING MEMBERS AND PANEL END JOINTS SHALL BE STAGGERED.
7. NAILS SHALL BE LOCATED AT LEAST 3/8" FROM THE EDGES OF PANELS.
8. 1-1/2" LONG 16 GAGE STAPLES W/ 7/16" CROWN MAY BE SUBSTITUTED FOR 8d NAILS AT HALF THE SPACING FOR 7/16" PANELS ONLY. CROWNS MUST BE INSTALLED PARALLEL TO FRAMING MEMBERS.
9. PANELS SHALL NOT BE LESS THAN 4'X8' EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING WHERE MINIMUM PANEL DIMENSION SHALL BE 24" UNLESS ALL EDGES OF THE UNDERSIZED PANELS ARE SUPPORTED BY AND FASTENED TO FRAMING MEMBERS OR BLOCKING.

METAL CLAD WOOD FRAMED DIAPHRAGM SCHEDULE

LOCATION	SHEATHING	FIELD FASTENERS	END FASTENERS	STITCH FASTNERS	NFBA ASSEMBLY	PLF
ROOF W/ DESIGN SNOW LOAD UP TO 112 PSF (FRAMING MEMBERS @ 24" O.C.)	29 GAUGE PANELS	#10 X 1.5" LONG SCREW @ 9" O.C.	#10 X 1.5" LONG SCREW @ 9" O.C.	NONE	#9	116
ROOF W/ DESIGN SNOW LOAD UP TO 200 PSF (FRAMING MEMBERS @ 16" O.C.)	29 GAUGE PANELS	#10 X 1.5" LONG SCREW @ 9" O.C.	#10 X 1.5" LONG SCREW @ 9" O.C.	NONE	#9	116
ROOF W/ DESIGN SNOW LOAD UP TO 112 PSF (FRAMING MEMBERS @ 24" O.C.)	29 GAUGE PANELS	#10 X 1.0" LONG SCREW @ 9" O.C.	#10 X 1.0" LONG SCREW @ 4.5" O.C.	#12 X 0.75" LONG SCREW @ 8" O.C.	#17	215

NOTES TO TABLE ABOVE:
1. ALL ROOFS SHALL FOLLOW FASTENING FOR ASSEMBLY#9 U.N.O.
2. PURLINS SHALL BE 2X6 TOP RUNNING AND LAPPED @ 24" O.C.
3. FIELD AND END FASTENERS SHALL BE PLACED IN THE FLAT
4. STRUCTURAL PANELS SHALL BE 29 GAUGE STEEL (FABRAL GRANDRIB 3 OR EQUIVALENT)

LOCATIONS REQUIRING NATURALLY DURABLE OR PRESERVATIVE-TREATED WOOD

NOTES:
1. WOOD JOISTS/FLOORS CLOSER THAN 18" AND WOOD GIRDERS CLOSER THAN 12" TO EXPOSED GROUND IN CRAWL SPACES OR UNEXCAVATED AREAS WITHIN THE PERIMETER OF THE BUILDING FOUNDATION.
2. WOOD FRAMING MEMBERS IN CONTACT W/ THE EXTERIOR FOUNDATION WALLS AND ARE LESS THAN 8" FROM EXPOSED EARTH.
3. WOOD FRAMING MEMBERS AND FURRING STRIPS IN DIRECT CONTACT W/ MASONRY OR CONCRETE.

*WOOD USED IN THE LOCATIONS SPECIFIED SHALL BE NATURALLY DURABLE OR PRESERVATIVE-TREATED IN ACCORDANCE W/ AWPA U1.

**FASTENERS INCLUDING NUTS AND WASHERS, IN CONTACT WITH PRESERVATIVE-TREATED WOOD SHALL BE OF HOT-DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER. STAPLES SHALL BE OF STAINLESS STEEL. FASTENERS OTHER THAN NAILS, STAPLES, TIMBER RIVETS, WOOD SCREWS AND LAG SCREWS SHALL BE PERMITTED TO BE OF MECHANICALLY DEPOSITED ZINC-COATED STEEL.

***IN GROUND POSTS SHALL BE TREATED TO THE UC4B CLASSIFICATION, CONFORMING TO AWPA U1 AND M4. (ACQ 0.60, CA-C 0.31, OR EQUIVALENT)

PROFESSIONAL ENGINEER
JACKSON D SAGERS
13258211
07/29/2025
STATE OF UTAH

ROPER BUILDINGS / DMLP RESOURCES
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7704 East 500 North
Huntsville, UT 84317

Date: 7/29/2025
Scale:
Drawn by: CBC
Job: JEPPSEN.S
Sheet Size: D
22" x 34"
Engineering Notes

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